

Validation of the NG-18 Equations for Thick Walled Pipelines

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ABSTRACT

The applicability of the flow stress dependent NG-18 equations to thick wall pipelines such as those used to transport dense phase carbon dioxide (CO₂) is demonstrated. A comparison between the components of the NG-18 equations and BS 7910 shows that the factor M_T for through-wall defects and M_P for part-wall defects in the NG-18 equations are very close to the reference stress solutions in BS 7910 Annex P, which are applicable to thick wall pipe. Thus, by inference, the flow stress dependent form of the NG-18 equations is also applicable to thick wall pipe. A further comparison with experimental failure data for thick wall pipes shows that the flow stress dependent NG-18 equations are applicable to wall thicknesses of up to 47.2 mm when the full-size equivalent upper shelf Charpy V-notch impact energy is at least 50 J. The results suggest that in principle, the flow stress dependent NG-18 equations may be used as limit state functions in models to calculate the failure frequency due to third party external interference, for high toughness, thick wall pipelines such as those required for dense phase CO₂ pipelines.



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Keywords

Fracture, Plastic collapse, Toughness, NG-18 equations, Thick wall pipe, CO₂ pipelines

1. INTRODUCTION

Carbon Capture and Storage (CCS) has been identified as an enabling technology to reduce carbon dioxide (CO₂) emissions into the atmosphere and meet international climate change

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31 commitments (Department for Business, Energy & Industrial Strategy, 2017). CCS involves the
32 capture of CO₂ from large scale industrial emitters, and its transportation to geological sites
33 for permanent storage. The most efficient method of transporting captured CO₂ is by pipeline
34 in the dense phase (Downie, Race and Seevam, 2007). The aim of the research programme,
35 COOLTRANS (CO₂Liquid pipeline TRANSportation), led by National Grid, was to develop the
36 knowledge base necessary to allow the safe design, construction and operation of an onshore
37 dense phase CO₂ pipeline in the United Kingdom (UK) (Cooper and Barnett, 2014).

38 CO₂ is a hazardous substance which, in the unlikely event of an accidental release, could cause
39 people harm. Therefore, an important consideration for the operation of an onshore CO₂
40 pipeline is to control the risk that the pipeline presents to the public. Quantitative risk
41 assessment (QRA) involves the analysis of the frequency and consequences of a pipeline
42 failure. It provides an assessment of the individual and societal risk to the nearby populations,
43 and determines whether mitigating action is required to reduce the risk (IGEM, 2013). The
44 method required for the QRA of dense phase CO₂ pipelines was developed as part of the
45 COOLTRANS research programme.

46 Calculation of the frequency of pipeline failure from each credible failure mechanism is a
47 fundamental part of pipeline QRA. Historically, mechanical damage through third party
48 external interference is the largest individual cause of pipeline failure in the UK and therefore
49 represents the greatest risk to the surrounding population (UKOPA, 2019). Buried
50 transportation pipelines are installed in third party land, and therefore may be subject to
51 interference damage as a result of activities carried out by third parties in the vicinity of the
52 pipeline. The frequency of pipeline failure due to third party mechanical damage is calculated
53 using probabilistic structural reliability methods, combining operational data on pipeline
54 damage with semi-empirical fracture mechanics failure equations. This approach has been
55 employed for over 25 years, is tried and tested, and is well understood (Corder et al.,
56 1992)(Corder, 1995).

57 To calculate the pipeline failure frequency due to third party external interference for dense
58 phase CO₂ pipelines, it would be desirable to extend the use of the established failure
59 frequency method.

60 Existing third-party external interference failure frequency models, for oil and gas pipelines,
61 were reviewed in a recent study (Lyons et al., 2019) in order to assess whether they could be
62 applied to dense phase CO₂ pipelines. The review covered FFREQ (Corder et al., 1992), PIPIN
63 (HSE, 2001), PIE (Lyons, 2008) and Cosham (Cosham, 2008) models, and an update to the
64 pipeline damage probability distributions (Goodfellow, Turner, Haswell and Espiner, 2012).

65 It was noted by the review (Lyons et al., 2019), that operational pipeline damage data used
66 within the existing failure frequency models is restricted to existing UK pipelines with
67 standard wall thicknesses, i.e. up to 19.1 mm for the line pipe used at road/rail crossings.

68 Furthermore, the fracture mechanics failure equations used in the failure frequency models,
69 namely the NG-18 equations, used to calculate gouge failure and leak/rupture; and the British
70 Gas Dent Gouge Model, used to calculate gouged dent failure, are semi-empirical. The
71 experimental test data used in the derivation of these equations also predominantly concerns
72 standard pipeline wall thicknesses with upper limits of 21.9 mm and 15.6 mm for through-
73 wall and part-wall gouge defects respectively, and 16.4 mm for gouged dent defects (Lyons
74 et al., 2019).

75 Dense phase CO₂ pipelines have a high design pressure requirement (Noothout et al., 2014),
76 this typically necessitates the use of line pipe with a wall thickness greater than the values
77 indicated above. It was therefore concluded by the review (Lyons et al., 2019) that the wall
78 thickness of typical dense phase CO₂ pipelines would be beyond the known range of
79 applicability of the operational data and failure equations used for the established failure
80 frequency method.

81 This paper presents a detailed validation study to assess whether the fracture mechanics
82 failure equations used to calculate gouge failure for the established failure frequency method,
83 the NG-18 equations, may also be applied to thick wall pipelines such as those used for dense
84 phase CO₂ transportation. Note that throughout this paper the term “thick wall” is used to
85 denote a wall thickness in excess the upper limits used in the experimental tests performed
86 as part of the formulation of the NG-18 equations, specifically 21.9 mm for through-wall
87 defects and 15.6 mm for part-wall defects. The study consists of two parts, firstly, a
88 comparison is made between the component parts of the NG-18 equations and the BS 7910
89 defect assessment method, which is applicable to thick wall pipe, in order to show the
90 similarities and differences between them in terms of their basic structure. The component
91 parts are illustrated graphically in order to show the effect of increased wall thickness.
92 Secondly, a comparison between experimental failure data for thick wall cylindrical pressure
93 vessels and failure predictions calculated using the NG-18 equations is made to show the
94 accuracy of the equations. The NG-18 equations and the relevant sections of BS 7910 are
95 described in Section 2, the comparison between NG-18 and BS 7910 is presented in Section 3
96 and the comparison between prediction and experimental data is presented in Section 4.

97 **2. THE NG-18 EQUATIONS AND BS 7910 ASSESSMENT METHOD**

98 External interference failure frequency models use limit state functions to define the
99 conditions for pipeline failure in terms of defect size, pipeline geometry, and the material
100 properties of the line pipe steel. Existing failure frequency models, using the established
101 failure frequency method, employ the through-wall and part-wall NG-18 equations (Kiefner,
102 Maxey, Eiber and Duffy, 1973) as the limit state functions (Lyons et al, 2019) for leak/rupture
103 and gouge failure respectively.

104 The NG-18 equations are semi-empirical and pipeline specific. They were derived and
105 calibrated using the experimental results of burst and material tests on pipe specimens over

106 a range of geometries and properties which would be expected for normal pipeline operation
107 at the time, i.e. on through-wall defects in pipe up to 21.6 mm thick and on part-wall defects
108 in pipe up to 15.6 mm (Lyons et al., 2019). Validation of the use of the NG-18 equations for
109 thick wall pipelines forms the basis of this study.

110 The British Standard BS 7910 (BSI, 2013) includes a more general assessment method for
111 defects in metallic structures, which can be applied to both thin wall and thick wall pipe. This
112 section describes the NG-18 equations and the parts of BS 7910 relevant to a comparison
113 between the two.

114 **2.1 THE NG-18 EQUATIONS**

115 The NG-18 equations were developed through the 1960s and early 70s by the Battelle
116 Memorial Institute (Hahn, Sarrate and Rosenfield, 1969) (Kiefner, 1969) (Maxey, Kiefner,
117 Eiber and Duffy, 1972) (Kiefner, Maxey, Eiber and Duffy, 1973) to describe a longitudinally
118 orientated defect in a cylinder subject to internal pressure loading. Based upon the operating
119 conditions of a pipeline, the through-wall NG-18 equations are used to determine whether an
120 axially oriented through-wall defect will lead to a full-bore rupture or remain as a leak, while
121 the part-wall NG-18 equations are used to determine whether an axially oriented part-wall
122 defect (i.e. a gouge) will progress into a through-wall defect.

123 Both the through-wall and part-wall NG-18 equations exist in two forms: toughness
124 dependent and flow stress dependent. They are listed as follows for through-wall defects:

125 ***Toughness dependent form***

$$\frac{K_C^2 \pi}{8c\bar{\sigma}^2} = \ln \sec\left(\frac{\pi M_T \sigma_H}{2\bar{\sigma}}\right) \quad (1)$$

Flow stress dependent form

$$\sigma_H = M_T^{-1} \bar{\sigma} \quad (2)$$

126 where K_C is the material's fracture toughness, c is half of the axial defect length, σ_H is the
127 circumferential hoop stress at failure, $\bar{\sigma}$ is the flow stress (which is a measure of the stress at
128 which unconstrained plastic flow occurs) and, M_T is the Folias factor, which accounts for the
129 bulging which occurs when a defect is present in a pressurised pipeline.

130 The toughness dependent and flow stress dependent forms of the part-wall NG-18 equation
131 use a correction to the Folias factor, M_P , in place of M_T :

$$M_p = \left[\frac{1 - \frac{d}{t} \left(\frac{1}{M_T} \right)}{1 - \frac{d}{t}} \right] \quad (3)$$

132 where d is the depth of the part-wall defect and t is the pipe wall thickness.

133 For the purpose of this study, the Folias factor (Folias, 1975) used in the NG-18 equations is
 134 the 2-term expression applicable to long defects:

$$M_T = \sqrt{1 + 0.26 \left(\frac{2c}{\sqrt{Rt}} \right)^2} \quad (4)$$

135 where R is the external radius of the pipeline.

136 The empirical flow stress (Corder et al., 1992)(Corder, 1995) used is given by:

$$\bar{\sigma} = 1.15\sigma_Y \quad (5)$$

137 where σ_Y is the yield strength of the structure. Although they are not the original terms used
 138 for the NG-18 equations, the use of the above expressions in this study results from their
 139 historical use in the established failure frequency method (Lyons, 2008).

140 For the toughness dependent NG-18 equations, the material fracture toughness value K_C is
 141 determined using a correlation with the Charpy V-notch energy. This correlation was
 142 empirically derived using experimental data and is as follows:

$$K_C^2 = C_v \frac{E}{A} \quad (6)$$

143 where C_v is the 2/3 Charpy V-notch upper shelf impact energy, A is the fracture area of the
 144 Charpy specimen, and E is the Young's Modulus of steel.

145 **2.2 BS 7910**

146 BS 7910 defect assessments are performed using a Failure Assessment Diagram (FAD)
 147 approach. This method is derived from fracture mechanics theory and considers that failure
 148 of the structure can occur due to either fracture or plastic collapse. Based on the geometry of
 149 the structure, its operating conditions and the dimensions of the defect, two separate
 150 quantities are calculated, one representing fracture and one representing plastic collapse.

151 The quantity representing fracture is known as the fracture ratio, K_r :

$$K_r = \frac{K_I}{K_{mat}} \quad (7)$$

152 where K_{mat} is the material fracture toughness and K_I is the stress intensity factor. The form of
 153 the stress intensity factor is dependent on the type and the dimensions of the defect and the
 154 geometry and operating conditions of the structure being assessed.

155 The general form of the stress intensity factor is:

$$K_I = (Y\sigma)\sqrt{\pi a} \quad (8)$$

156 where, a , is half the defect length in the case of a through-wall defect, or the defect depth in
 157 the case of a part-wall defect; Y is a function which depends on the geometry; and σ is the
 158 applied stress.

159 The stress is linearised into membrane and bending components, i.e. P_m and P_b , using the
 160 expressions in Figure 2 of BS 7910 (BSI, 2013).

161 $Y\sigma$ in Equation (8) is given by equations (M.3), (M.4) (M.5) in Annex M of BS 7910 (BSI, 2013).
 162 Note that for the purposes of the comparison, secondary stresses are assumed to be zero.
 163 The factors due to regions of local stress concentration, structural discontinuities and
 164 misalignment (M_{km} , M_{kb} , k_t , k_{tm} , k_{tb} , k_m) are taken to be equal to 1. The finite width correction
 165 factor, f_w , is calculated using the expressions given in Section M.2 of BS 7910. The typical
 166 length of a section of line pipe of 12 m is assumed in this study.

167 For the case of a through-wall defect, Equation (M.21) from Section M.6.1 of BS 7910 (BSI,
 168 2013) for curved shells under internal pressure, is used to calculate the bulging correction
 169 factor. The membrane and bending load factors, M_m and M_b respectively, are taken from
 170 Section M.3.1 of BS 7910.

171 For the case of a part-wall defect, Equation (M.22) from Section M.6.1 of BS 7910 (BSI, 2013)
 172 is used to calculate the bulging correction factor and the membrane and bending load factors
 173 from Section M.6.2 of BS 7910 (BSI, 2013) are used. These include the flat-plate solutions from
 174 BS 7910 Section M.4.1.2 for the membrane term M_m and BS 7910 Section M.4.1.3 for the
 175 bending term M_b .

176 It should be noted that different expressions for the flow stress and Folias factor are used in
 177 the BS 7910 assessment method. Note also, that BS 7910 uses the term “flow strength” in
 178 place of “flow stress”.

179 The flow strength in BS 7910 is defined as the average of the yield strength and the tensile
 180 strength:

$$\bar{\sigma} = \frac{\sigma_Y + \sigma_U}{2} \quad (9)$$

181 where σ_U is the tensile strength of the structure.

182 The Folias factor is defined in BS 7910 using the following two-term expression:

$$M_T = \sqrt{1 + 0.40 \left(\frac{2c}{\sqrt{Rt}} \right)^2} \quad (10)$$

183 For comparison purpose, the external radius of the pipeline is used to calculate the Folias
184 factor for stress intensity factors and reference stresses using BS 7910 method.

185 In BS 7910, the quantity representing plastic collapse is known as the load ratio, L_r :

$$L_r = \frac{\sigma_{ref}}{\sigma_Y} \quad (11)$$

186 where σ_{ref} is the reference stress which is dependent on the type and dimensions of the defect
187 and the geometry and operating conditions of the structure being assessed.

188 For the reference stress term in the load ratio, Equation (P.17) from Section P.9.1 in Annex P
189 of BS 7910 is used for the through-wall defect case and Equation (P.18) from BS 7910 Section
190 P.9.2 is used in accordance with Section P.9.4 for the external part-wall defect case. In
191 Equations (P.17) and (P.18), the factor of 1.2 which occurs in the first term of both equations
192 has not been applied. This factor was originally introduced in order to increase the level of
193 conservatism in the reference stress solutions for curved shells and therefore a more accurate
194 comparison with the NG-18 equations can be achieved by ignoring it.

195 Once the specific fracture ratio (K_r) and load ratio (L_r) for the defect and structure have been
196 calculated, they can be plotted as a point on a FAD. If the calculated values of K_r and L_r lie
197 within the failure assessment line, then the defect is considered acceptable.

198 **3 FAILURE MODEL COMPARISON**

199 **3.1 COMPONENT ANALYSIS**

200 The defect assessment code PD 6493:1991 (BSI, 1991) was a precursor to BS 7910. If the
201 equation for the FAD in PD 6493 Level 2 is considered:

$$K_r = S_r \left\{ \frac{8}{\pi^2} \ln \sec \left(\frac{\pi}{2} S_r \right) \right\}^{-0.5} \quad (12)$$

202 It can be seen that this is very similar to the toughness dependent NG-18 equations (Equation
203 (1)). This is because the NG-18 equations and the Level 2 FAD in PD 6493:1991 (BSI, 1991) are
204 both based on the Dugdale strip yield model. Note that in Equation (12), the load ratio S_r is
205 similar to L_r but instead uses the flow stress in place of the yield strength.

206 The toughness dependent NG-18 equations may therefore be expressed in the form of a FAD
 207 with equivalent fracture and plastic collapse terms. If we consider the toughness dependent
 208 form of the through-wall NG-18 equation (Equation (1)), this can be rearranged as follows
 209 (Cosham, Hopkins and Leis, 2012):

$$K_c^2 = c\bar{\sigma}^2 \frac{8}{\pi} \ln \left[\sec \left(\frac{\pi}{2} \left\{ \frac{M_T \sigma_H}{\bar{\sigma}} \right\} \right) \right] \quad (13a)$$

$$\left\{ \frac{M_T \sigma_H \sqrt{\pi c}}{K_c} \right\} = \left\{ \frac{M_T \sigma_H}{\bar{\sigma}} \right\} \left\{ \frac{8}{\pi^2} \ln \left[\sec \left(\frac{\pi}{2} \left\{ \frac{M_T \sigma_H}{\bar{\sigma}} \right\} \right) \right] \right\}^{-\frac{1}{2}} \quad (13b)$$

210 If the left hand side of Equation (13b) is defined as the fracture ratio, K_r , and the expression
 211 $\left\{ \frac{M_T \sigma_H}{\bar{\sigma}} \right\}$ on the right hand side is defined as the load ratio, S_r , then the equation becomes:

$$K_r = S_r \left\{ \frac{8}{\pi^2} \ln \sec \left(\frac{\pi}{2} S_r \right) \right\}^{-\frac{1}{2}} \quad (13c)$$

212 The failure assessment line implied within the toughness dependent NG-18 equations is
 213 therefore identical to that of PD 6493 (Equation (12)).

214 A comparison of the stress intensity factor and reference stress solutions in BS 7910 with the
 215 equivalent terms in Equation (13b) is shown in Table 1.

BS 7910	NG-18
Stress Intensity Factor	
$K_I = (Y\sigma)\sqrt{\pi a}$ (($Y\sigma$) from Equations (M.3), (M.4), (M.5), (M.21) and Sections M.6.1 and M.3.1 from Annex M of BS 7910)	$K_I = (M_T \sigma_H)\sqrt{\pi c}$ (Implied by Equation (13b))
Reference Stress	
$\sigma_{ref} = M_T P_m + \frac{2P_b}{3 \left(1 - \frac{2a}{W}\right)}$ (Equation P.17 in Section P.9.1 from Annex P of BS 7910)	$\sigma_{ref} = M_T \sigma_H$ (implied by Equation (13b) and $S_r - L_r$ relationship)

216 **Table 1:** Fracture and Plastic Collapse Components from BS 7910 and the Toughness
 217 Dependent Through-Wall NG-18 Equation

218 In Table 1, W is the structure width in the defect plane. It can be seen from this table that
 219 firstly, the Folias factor, M_T , in the NG-18 equation is equivalent to the function Y in BS 7910.
 220 Secondly, the product of the Folias factor and hoop stress in the NG-18 equation, $M_T \sigma_H$, is
 221 equivalent to the reference stress, σ_{ref} , in BS 7910.

222 For through-wall defects, both methods have similar expressions for the stress intensity factor,
 223 K_I , and the reference stress, σ_{ref} . However, different formulae are used to calculate the Folias
 224 factor M_T , as described in Sections 2.1 and 2.2. For the stress intensity factor, the difference

225 between section M.6.1 in BS 7910 and the NG-18 equation is the finite width correlation
 226 factor, f_w . For the reference stress, the difference between Equation P.17 in BS 7910 and the
 227 NG-18 equation is the inclusion of the bending stress, P_b .

228 A similar analysis can be performed using the part-wall NG-18 equation. The respective stress
 229 intensity and reference stress components from BS 7910 and NG-18 for part-wall defects are
 230 shown in Table 2.

BS 7910	NG-18
Stress Intensity Factor	
$K_I = (Y\sigma)\sqrt{\pi a}$ (($Y\sigma$) from Equations (M.3), (M.4), (M.5), (M.22), Sections M.4.1.2 and M.4.1.3 from Annex M of BS 7910)	$K_I = (M_P\sigma_H)\sqrt{\pi d}$ (implied by Equation (1) and analysis from Equations (13a) to (13b))
Reference Stress	
$\sigma_{ref} = M_S P_m + \frac{2P_b}{3(1 - \alpha'')^2}$ (Equation P.18 from Annex P of BS 7910)	$\sigma_{ref} = M_P \sigma_H$ (implied by Equation (1), analysis from Equations (13a) to (13b) and $S_r - L_r$ relationship)

231 **Table 2:** Fracture and Plastic Collapse Components from BS 7910 and the Toughness
 232 Dependent Part-Wall NG-18 Equation

233 In Table 2, the parameter α'' is defined in Appendix P9 of BS 7910 (BSI, 2013). The comparison
 234 for part-wall defects shows that the stress intensity factors are significantly different for the
 235 two methods. M_P in the NG-18 equation is an empirical term. Its derivation indicates that it is
 236 not appropriate to use to calculate the stress intensity factor. However, for the reference
 237 stress, M_S in BS 7910 is very similar to M_P in the NG-18 equation. The difference between
 238 Equation P.18 and the NG-18 equation is the inclusion of the bending stress, P_b .

239 While the structure of the toughness dependent form of the through-wall NG-18 equation is
 240 similar to that of BS 7910, the toughness dependent part-wall equation is not recommended
 241 for the calculation of part-wall defect failure because the stress intensity factor is not
 242 appropriate.

243 Comparison of the reference stresses show that M_T and M_P in the NG-18 equations are very
 244 similar to the equivalent terms in the reference stress solutions in BS 7910 Annex P, which
 245 are applicable to thick wall pipe. The main difference between them is that BS 7910 includes
 246 both the membrane stress, P_m , and the bending stress, P_b , whilst the NG-18 equations only
 247 include the membrane stress. By inference, M_T and M_P are therefore applicable to thick wall
 248 pipe and, provided the effect of the bending stress remain small, the flow stress dependent
 249 form of the NG-18 equations are also applicable to thick wall pipe. The effect of the bending
 250 stress is considered in Section 3.2.

251 It should be noted that the correlations between the material fracture toughness and the
252 Charpy V-notch impact energy in BS 7910 differ significantly from the correlation used in the
253 NG-18 equations. The correlations in BS 7910 are very conservative because they are
254 intended to be used when no measured fracture toughness data are available and are
255 intended to give a lower bound estimate of the fracture toughness. They also cover materials
256 other than line pipe. By contrast, the correlation in the NG-18 equations is empirical. The
257 fracture toughness was calculated using the results of tests on through-wall defects rather
258 than using high-constraint fracture mechanics test specimens. Therefore, the correlation
259 might not be applicable to thick wall pipe. As toughness increases, the burst strength tends
260 to a toughness independent, or flow stress dependent, form representing the plastic collapse
261 limit state. If defect failure in the line pipe used in pipeline construction is controlled by plastic
262 collapse rather than fracture, then the flow stress dependent form of the NG-18 equations
263 would be the most appropriate for application to high toughness steel.

264 For the flow stress dependent NG-18 equations to be applicable, the steel must have a high
265 enough toughness such that the failure is controlled by plastic collapse. According to both
266 IGEM/TD/1 Edition 5 (2016) and PD 8010 (BSI, 2015), drop weight tear testing (DWTT)
267 requirements should be applied to pipe with a diameter greater than or equal to 323.9 mm
268 in order to assess resistance to prevent propagating brittle fracture. The fracture propagation
269 transition temperature is defined by the 85% shear appearance transition temperature (SATT)
270 from the DWTT. The fracture propagation transition temperature is higher than the fracture
271 initiation transition temperature. The DWTT specimen is a full-thickness test specimen. The
272 requirement for the DWTT testing ensures that line pipe is on the upper shelf. The failure of
273 a part or through-wall defect will be ductile. That is not, however, necessarily sufficient for
274 failure to be controlled by plastic collapse. That depends on the upper shelf Charpy V-notch
275 impact energy. The toughness necessary for failure to be controlled by plastic collapse
276 remains undefined, which will be subject to further study.

277 It is therefore concluded that in principle, the flow stress dependent NG-18 equations would
278 be suitable for application to thick wall CO₂ pipelines, provided the pipe material was of a high
279 toughness.

280 **3.2 GRAPHICAL ILLUSTRATION WITH WALL THICKNESS**

281 In order to illustrate the similarities and differences between the BS 7910 and NG-18
282 approaches, comparisons have been made between the stress intensity factor, K_I and
283 reference stress, σ_{ref} for each assessment method, considering both through-wall and part-
284 wall defects. The comparisons show the variation in each component with increasing wall
285 thickness for a range of defect dimensions.

286 The pipeline parameters used in the comparison, are shown in Table 3. Two different external
287 diameters, i.e. 610mm and 914mm, with the same material properties and design factor are
288 considered. The pressure was calculated using Lamé's equation for the hoop stress in a thick

289 wall cylinder (Lamé and Clapeyron, 1833) such that the stress at the internal pipe wall always
 290 remains at a value of 72 percent of the specified minimum yield strength.

Input	Value
External Diameter (mm)	610, 914
Material Grade	L450
Specified Minimum Yield Strength (Nmm ⁻²)	450
Specified Minimum Tensile Strength (Nmm ⁻²)	535
full-size equivalent Charpy V-Notch Impact Energy (J)	100
Design Factor	0.72

291 **Table 3:** Pipeline Parameters for NG-18 and BS 7910 Comparison

292 To select a range of defect dimensions to be used in each comparison case, the United
 293 Kingdom Onshore Pipeline Operators’ Association (UKOPA) Pipeline Fault Database (UKOPA,
 294 2019) was consulted. This database contains records of third party damage incidents affecting
 295 gas and liquid pipelines in the UK and gives information on specific defect dimensions. The
 296 dimensions of three through-wall defects and three part-wall defects were chosen directly
 297 from the database. Three variations are included, a shallow long defect, a mid length and mid
 298 depth defect and a short deep defect. Note that the depths of the part-wall defects are
 299 represented in the comparisons as a percentage of wall thickness. As wall thickness increases,
 300 defect dimensions may not be representative of realistic defects which could occur as a result
 301 of third-party external interference. The purpose of the comparisons is to observe the effect
 302 of the differences between NG-18 and BS 7910 with increasing wall thickness. The dimensions
 303 of the through-wall and part-wall defects are shown in Table 4 and Table 5:

Defect No.	Length (mm)
1	203
2	89
3	5

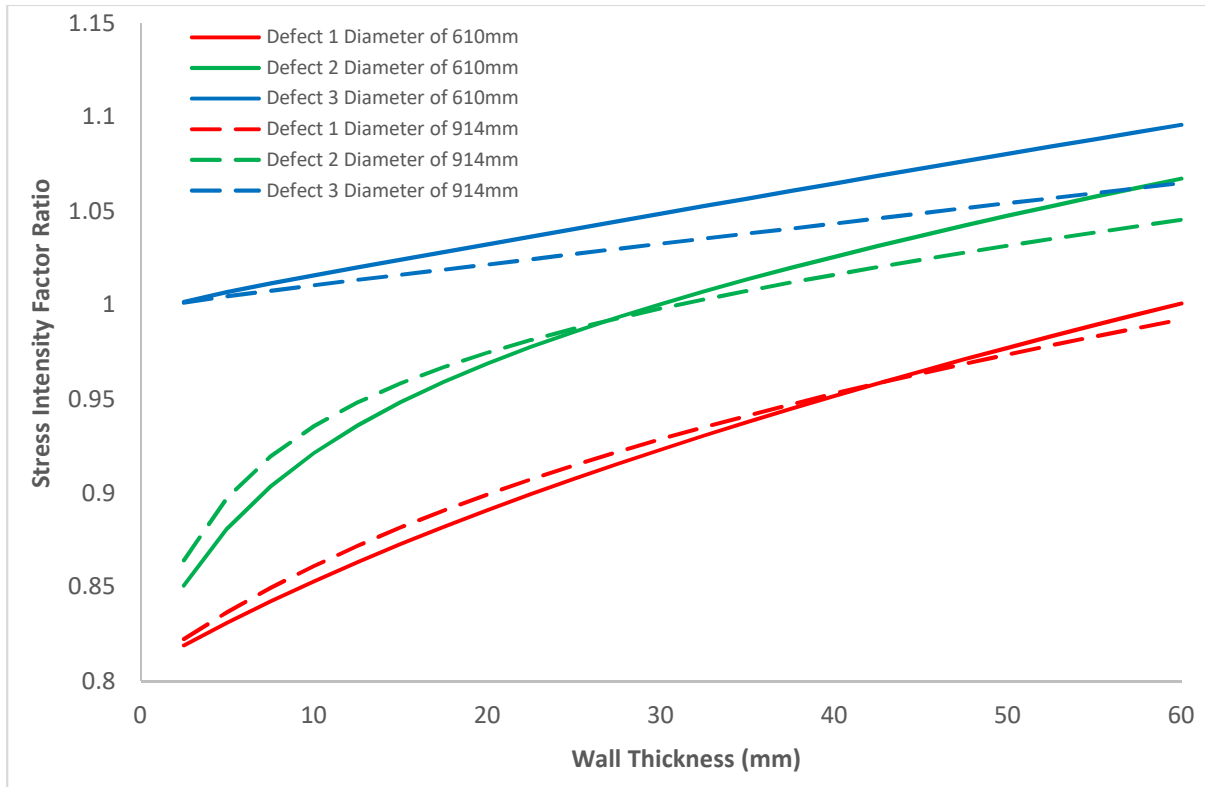
304 **Table 4:** Through-Wall Defect Dimensions for NG-18 and BS 7910 Comparison

Defect No.	Length (mm)	Depth (% wall thickness)
1	1350	14
2	480	54
3	20	63

305 **Table 5:** Part-Wall Defect Dimensions for NG-18 and BS 7910 Comparison

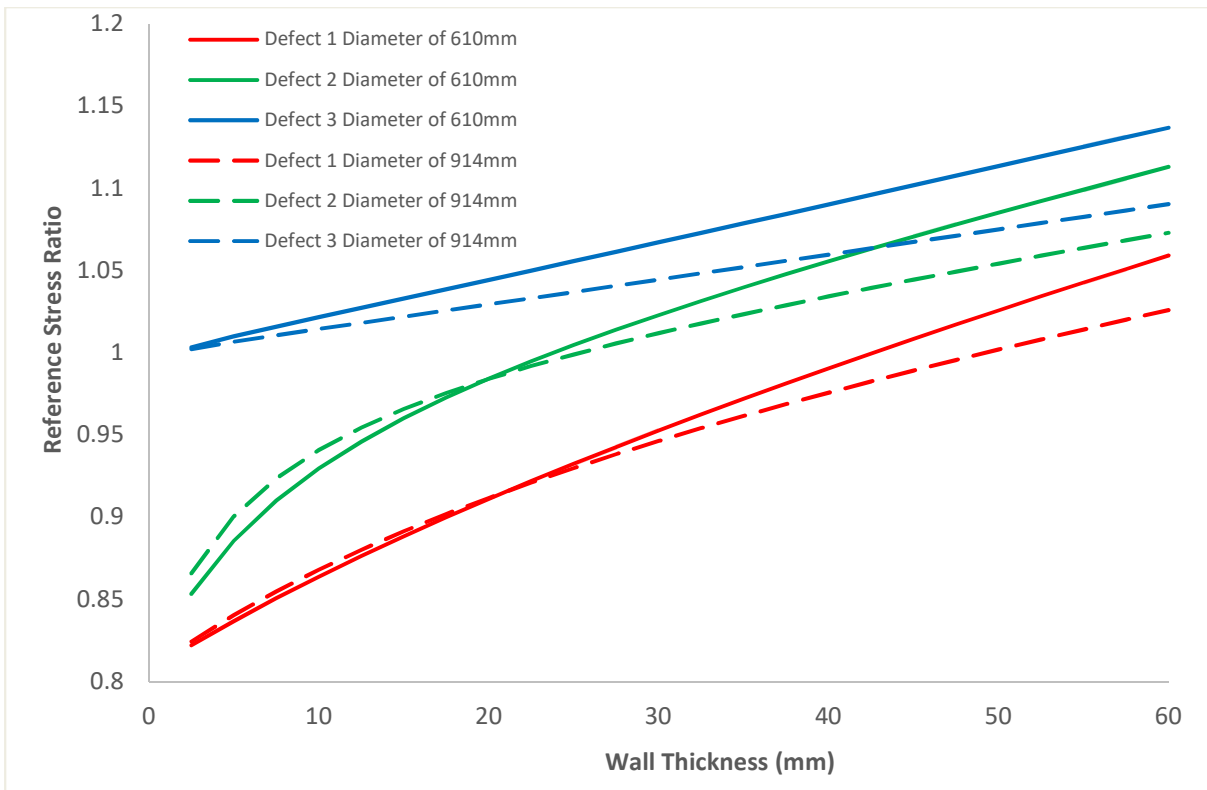
306 **Through-Wall Defects**

307 For through-wall defects, the comparison has been performed using the three defects listed
308 in Table 4 with pipeline parameters from Table 3. The results of the investigation are shown
309 in Figure 1 and Figure 2. They are presented as the ratio of the value calculated using the NG-
310 18 equations to the value calculated using BS 7910.



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312 Figure 1 Stress Intensity Factor Ratio of NG-18 and BS 7910 with Increasing Wall Thickness
313 for Three Different Axial Through-Wall Defects



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Figure 2 Reference Stress Ratio of NG-18 and BS 7910 with Increasing Wall Thickness for Three Different Axial Through-Wall Defects

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The proximity of the data to unity in Figure 1 and Figure 2 illustrates the similarity between the stress intensity factors and reference stresses used in the through-wall NG-18 equations and BS 7910. Almost the same values are produced for each of the three defects investigated at all wall thicknesses considered with the differences remaining small. In the case of the stress intensity factor, the differences between the methods result from the presence of the bending stress and the finite width correction factor, f_w , in BS 7910; and the slightly different Folias factors used in each method.

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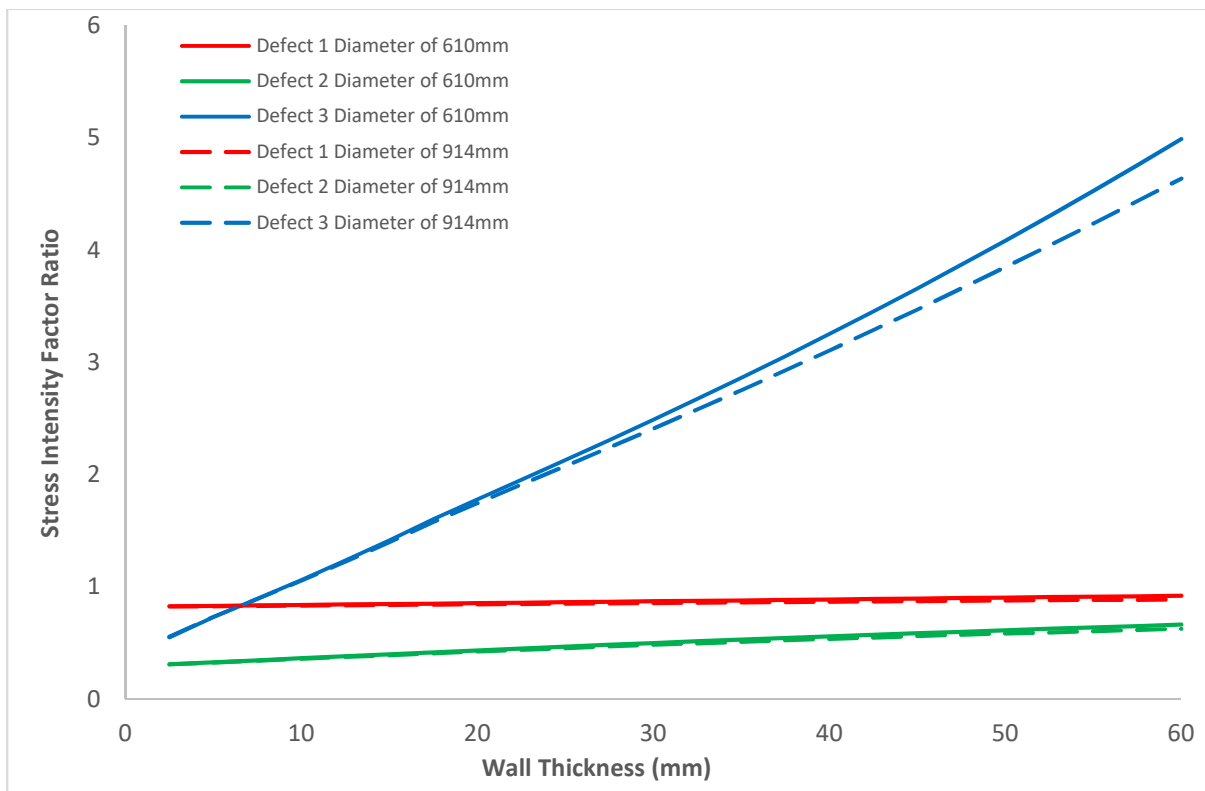
332

In the case of the reference stress, the difference between the methods result from the presence of the bending stress in BS 7910 and the slightly different Folias factors used in each method. In each case at low wall thickness, the bending stress is very small. The difference between the methods therefore primarily results from the finite width correction factor (for stress intensity only) and the different Folias factors. The effect of the Folias factor is observed by the difference between the short, mid and long defects at low wall thickness. As wall thickness is increased, the bending stress increases, the increasing influence of these quantities with increasing wall thickness causes the trends shown in each ratio. Figure 1 and Figure 2 also show the effect of pipe diameter to be minor for the thickness range considered.

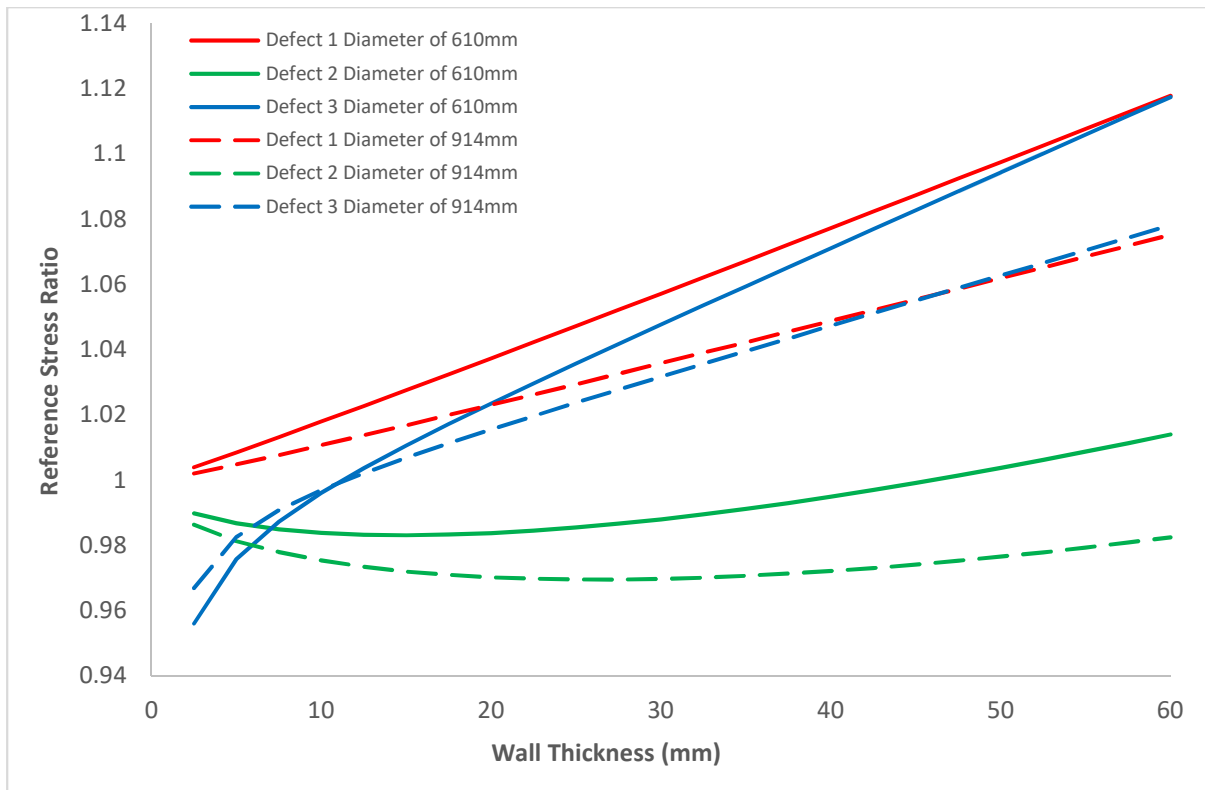
333 **Part-Wall Defects**

334 Similarly, a comparison was also made for part-wall defects using the three defects listed in
335 Table 5 with pipeline parameters from Table 3. The results are shown in Figures 3 and 4.

336 For stress intensity factor, Defect 3, which is short and deep, shows large variation in the
337 difference between the two methods with increasing wall thickness. At the lowest wall
338 thickness considered the NG-18 stress intensity factor is approximately half that of the
339 equivalent BS 7910 value. As wall thickness is increased, the relative size of the NG-18 value
340 to BS 7910 also increases, such that at a wall thickness of 60 mm, it is between 4 and 5 times
341 as large as the BS 7910 value for the diameters considered. For Defect 2, which is mid length,
342 mid depth, the NG-18 stress intensity factor is lower than the BS 7910 equivalent for all wall
343 thicknesses considered. Its value varies from approximately 0.3 times the BS 7910 equivalent
344 at the lowest wall thickness to approximately 0.6 times the BS 7910 equivalent at 60 mm.
345 Defect 1, which is long and shallow, shows the closest agreement between the two methods
346 at all wall thicknesses considered. The comparison highlights that the stress intensity factor
347 implied by the toughness dependent part-wall NG-18 equation is not appropriate.



348 Figure 3 Stress Intensity Factor Ratio of NG-18 and BS 7910 with Increasing Wall Thickness
349 for Three Different Axial Part-Wall Defects
350



351
352
353

Figure 4 Reference Stress Ratio of NG-18 and BS 7910 with Increasing Wall Thickness for Three Different Axial Part-Wall Defects

354 For reference stress, similar to Figure 1 and Figure 2, the data is very close to unity for each
 355 of the wall thicknesses considered. This highlights the similarity between the reference stress
 356 solutions for each assessment method at all wall thicknesses considered, with the differences
 357 remaining small. The difference between the methods again result from the presence of the
 358 bending stress in BS 7910 and the slightly different Folias factors used in each method. At low
 359 wall thickness, the bending stress is small. The difference between the methods therefore
 360 primarily results from the different Folias factors. The effect of the Folias factor is observed
 361 by the difference between the short, mid and long defects at low wall thickness. As wall
 362 thickness is increased, the bending stress increases, the increasing influence of these
 363 quantities with increasing wall thickness causes the trends shown in each ratio. The effect of
 364 pipe diameter is shown to be minor for the thickness range considered.

365 In summary, the comparisons show that the reference stresses derived from the NG-18
 366 equations are similar to those from BS 7910 for both through-wall and part-wall defects. The
 367 effect of the bending stress is small and remains small even when the pipeline wall thickness
 368 is increased. A similar effect is observed for the stress intensity factor for through-wall
 369 defects. However, the stress intensity factor implied by the toughness dependent part-wall
 370 NG-18 equation is not appropriate. In line with Section 3.1, it is concluded that the flow stress
 371 dependent form of the NG-18 equations are applicable to thick wall pipe.

372 **4. COMPARISON WITH REAL FAILURE DATA**

373 The above component analysis shows that the flow stress dependent form of the NG-18
374 equations would, in principle, be a suitable method to apply to thick wall CO₂ pipelines,
375 provided that the pipe material was of a high toughness. However, in order to satisfactorily
376 determine the accuracy of the flow stress dependent NG-18 equations when applied to thick
377 wall pipelines, a comparison between predicted values of failure pressure and experimental
378 test data is required. In this section, the predictions of both toughness and flow stress
379 dependent forms of the NG-18 equations are compared with through-wall and part-wall
380 defect burst tests on thick wall pipe sections and pressure vessels taken from a search of the
381 available literature.

382 As explained in Section 3.1, the correlations between the fracture toughness and Charpy V-
383 notch impact energy in BS 7910 are very conservative when compared to the correlation in
384 the NG-18 equations. Consequently, predictions using BS 7910 are not included in this section.
385 Comparisons with BS 7910 would only be informative if the fracture toughness was available.

386 **4.1. THROUGH-WALL DEFECTS**

387 **4.1.1 THROUGH-WALL FAILURE DATA**

388 The experimental failure data available for burst tests of through-wall defects on thick wall
389 vessels originates from Sturm and Stoppler in 1990 (Sturm and Stoppler, 1990) (Staat, 2004).
390 Three tests were performed on vessels constructed from 20 MnMoNi 55 grade manganese-
391 molybdenum-nickel alloy steel (150 J) and one test was performed on a vessel constructed
392 from 22 NiMoCr 37 mod nickel-molybdenum-chromium alloy steel (50 J). Details of the
393 vessels and tests are summarised in Table 6.

Input	Value
External Diameter (mm)	798
Wall Thickness (mm)	47.2
Material Grade	20 MnMoNi 55, 22 NiMoCr 37 mod
Yield Strength (Nmm ⁻²)	428, 417
Tensile Strength (Nmm ⁻²)	605, 622
Charpy V-Notch Impact Energy (J) Full-Size	150, 50
Defect Length Range (2c) (mm)	650 – 1105

394 **Table 6:** Thick Wall, Through-Wall Burst Test Vessel Details, Sturm and Stoppler, 1990

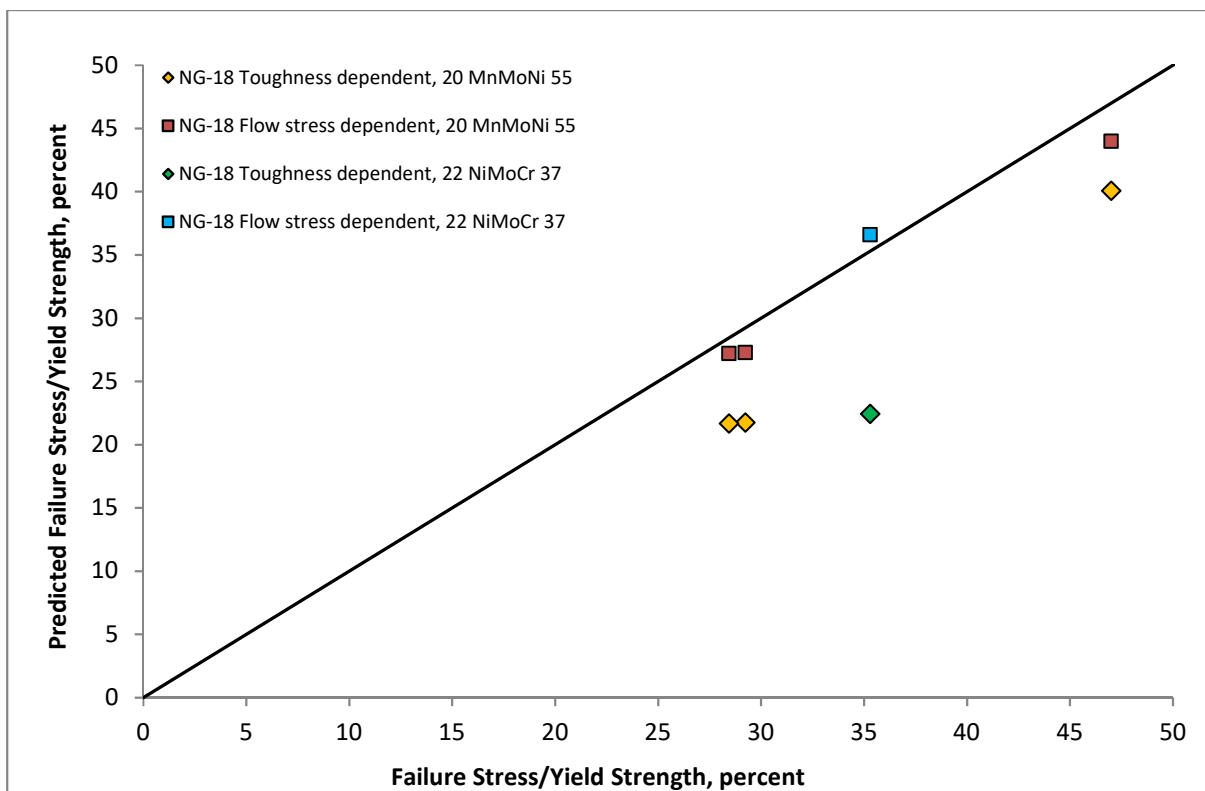
395 Data from burst tests of through-wall defects on thin wall vessels have also been included in
396 order to provide a comparison. This data originates from Battelle in 1973 (Kiefner, Maxey,
397 Eiber and Duffy, 1973) and includes data from 90 burst tests of through-wall defects on thin
398 wall pressure vessels. These tests were performed for a range of different vessels and
399 parameters. Details are summarised in Table 7.

Input	Value
External Diameter Range (mm)	168 – 1219
Wall Thickness Range (mm)	4.9 – 21.9
Yield Strength Range (Nmm ⁻²)	220 – 735
Tensile Strength Range (Nmm ⁻²)	338 – 908
Charpy V-Notch Impact Energy (J) Full-Size	20 – 136
Defect Length Range (2c) (mm)	25 – 508

400 **Table 7:** Thin Wall, Through-Wall Burst Test Vessel Details, Kiefner et al., 1973

401 **4.1.2. FAILURE DATA COMPARISON**

402 Figure 5 shows predicted and actual failure stresses as a ratio of yield strength for the set of
 403 axial through-wall defects in thick wall pressure vessels (Table 6). The predictions are made
 404 using both the toughness and flow stress dependent forms of the NG-18 equations. Data
 405 points which lie below the line of unity are conservative, with the model predicting a failure
 406 stress below that of the experimental failure stress. The closer data points are to the line of
 407 unity, the more accurate the prediction of failure stress. The failure stress is assumed to be
 408 the hoop stress at the failure pressure.

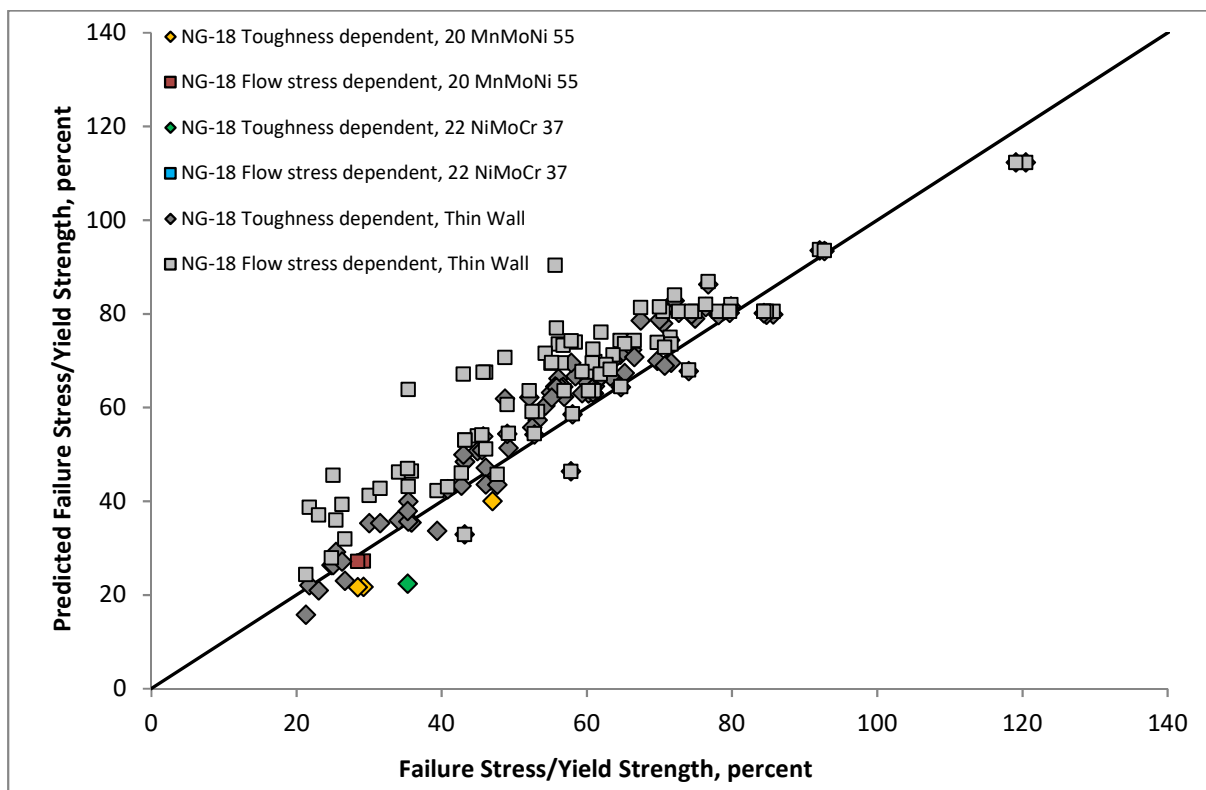


409

410 Figure 5 Predicted versus Actual Failure Stress for Axial Through-Wall Defects in Thick Wall
 411 Pipe Sections According to the Toughness and Flow Stress Dependent Forms of the NG-18
 412 Equations, Sturm and Stoppler

413 Figure 5 shows that, for the tests considered, the predictions of the toughness dependent
414 form of the NG-18 equation are more conservative than those of the flow stress dependent
415 form. The failure stresses calculated by the flow stress dependent form of the NG-18 equation
416 are between approximately 10% and 75% greater than those calculated by the toughness
417 dependent form. Figure 5 shows the flow stress dependent NG-18 equation to be the more
418 accurate method for calculating predictions of pipeline failure stress for through-wall defects
419 in thick wall pipelines.

420 Figure 6 shows the same data from Figure 5 and also includes the results of burst tests on
421 axial through-wall defects in thin wall pressure vessels from Battelle in 1973 (Table 7). This
422 figure shows the thick wall data to be contained within the scatter of the data points of the
423 thin wall data.



424
425 Figure 6 Predicted versus Actual Failure Stress for Axial Through-Wall Defects in Thick and
426 Thin Wall Pipe Sections According to the Toughness and Flow Stress Dependent Forms of
427 the NG-18 Equations, Sturm and Stoppler, Kiefner et al.

428 In summary, Figure 5 and Figure 6 suggest that the flow stress dependent NG-18 equation is
429 a valid assessment method for through-wall defects in pipelines up to at least 47.2 mm wall
430 thickness with a full-size equivalent upper shelf Charpy V-notch impact energy of at least 50 J.

431 **4.2. PART-WALL DEFECTS**

432 **4.2.1. PART-WALL FAILURE DATA**

433 The experimental failure data (Staat, 2004) available for burst tests with part-wall defects on
 434 thick wall vessels originates from Eiber et al. in 1971 (Eiber et al., 1971), Wellinger and Sturm
 435 in 1971 (Wellinger and Sturm, 1971), Sturm and Stoppler in 1990 (Sturm and Stoppler, 1990),
 436 Keller et al. in 1987 (Keller, Junker and Merker, 1987) and Mannucci et al. in 2001 (Mannucci,
 437 Demofonti, Harris, Barsanti and Hillenbrand, 2001).

438 As reported by Eiber et al. (Eiber et al., 1971), four tests were performed on vessels
 439 constructed from A 106 B grade steel, two tests were performed on vessels constructed from
 440 Type 316 stainless steel and one test was performed on a vessel constructed from A 316
 441 stainless steel. Details of the vessels and tests are summarised in Table 8.

Input	Value
External Diameter (mm)	609.6
Wall Thickness Range (mm)	38.1 – 43.7
Material Grade	A 106 B, Type 316, A 316
Yield Strength (Nmm ⁻²)	155, 241
Tensile Strength (Nmm ⁻²)	426, 570
Charpy V-Notch Impact Energy (J) Full-Size Range	81 – 200
Defect Length Range (2c) (mm)	76 – 361
Defect Depth Range (d) (% wall thickness)	47 – 88

442 **Table 8:** Thick Wall, Part-Wall Burst Test Vessel Details, Eiber et al. 1971

443 Wellinger and Sturm (1971) report 23 tests that were performed on vessels constructed from
 444 St 35 grade steel and two tests that were performed on vessels constructed from FB 70 grade
 445 steel. Details of the vessels and tests are summarised in Table 9.

Input	Value
External Diameter (mm)	88.9
Wall Thickness (mm)	22.2
Material Grade	St 35, FB 70
Yield Strength Range (Nmm ⁻²)	199 – 473
Tensile Strength Range (Nmm ⁻²)	438 – 614
Charpy V-Notch Impact Energy (J) Full-Size Range	56 – 71
Defect Length Range (2c) (mm)	40.5 – 123
Defect Depth Range (d) (% wall thickness)	18.9 – 88.7

446 **Table 9:** Thick Wall, Part-Wall Burst Test Vessel Details, Wellinger and Sturm 1971

447 As stated by Sturm and Stoppler (1990), four tests were performed on vessels constructed
 448 from 20 MnMoNi 55 grade steel and three tests were performed on vessels constructed from
 449 22 NiMoCr 37 grade steel. Details of the vessels and tests are summarised in Table 10.

Input	Value
External Diameter (mm)	797.9, 793.9
Wall Thickness (mm)	47.2
Material Grade	20 MnMoNi 55, 22 NiMoCr 37
Yield Strength (Nmm ⁻²)	428, 417
Tensile Strength (Nmm ⁻²)	622, 605
Charpy V-Notch Impact Energy (J) Full-Size	150, 50
Defect Length Range (2c) (mm)	709 – 1500
Defect Depth Range (d) (% wall thickness)	74 – 81

450 **Table 10:** Thick Wall, Part-Wall Burst Test Vessel Details, Sturm and Stoppler 1990

451 According to Keller, Junker and Merker (1987), two tests were performed on vessels
 452 constructed from 34 CrMo 4 grade steel. Details of the vessels and tests are summarised in
 453 Table 11.

Input	Value
External Diameter (mm)	564.8, 565.4
Wall Thickness (mm)	20.4, 21.7
Material Grade	34 CrMo 4
Yield Strength (Nmm ⁻²)	878, 866
Tensile Strength (Nmm ⁻²)	990, 979
Charpy V-Notch Impact Energy (J) Full-Size	64, 65
Defect Length (2c) (mm)	48, 32.5
Defect Depth (d) (% wall thickness)	78.9, 66.8

454 **Table 11:** Thick Wall, Part-Wall Burst Test Vessel Details, Keller et al. 1987

455 As reported by Mannucci (Mannucci, et al., 2001), two tests were performed on vessels
 456 constructed from API 5L X100 grade steel. Details of the vessels and tests are summarised in
 457 Table 12.

Input	Value
External Diameter (mm)	1422.4
Wall Thickness (mm)	19.25, 20.1
Material Grade	API 5L X100
Yield Strength (Nmm ⁻²)	740, 795
Tensile Strength (Nmm ⁻²)	774, 840
Charpy V-Notch Impact Energy (J) 2/3-Size	261, 171
Defect Length (2c) (mm)	180, 385
Defect Depth (d) (% wall thickness)	54, 18.9

458 **Table 12:** Thick Wall, Part-Wall Burst Test Vessel Details, Mannucci et al. 2001

459 Data from burst tests of part-wall defects on thin wall vessels has also been included in order
 460 to provide a comparison. This data originates from Battelle in 1973 (Kiefner, Maxey, Eiber and
 461 Duffy, 1973) and includes data from 33 burst tests of part-wall defects on thin wall pressure
 462 vessels. These tests were performed for a range of different vessels and parameters. Details
 463 are summarised in Table 13.

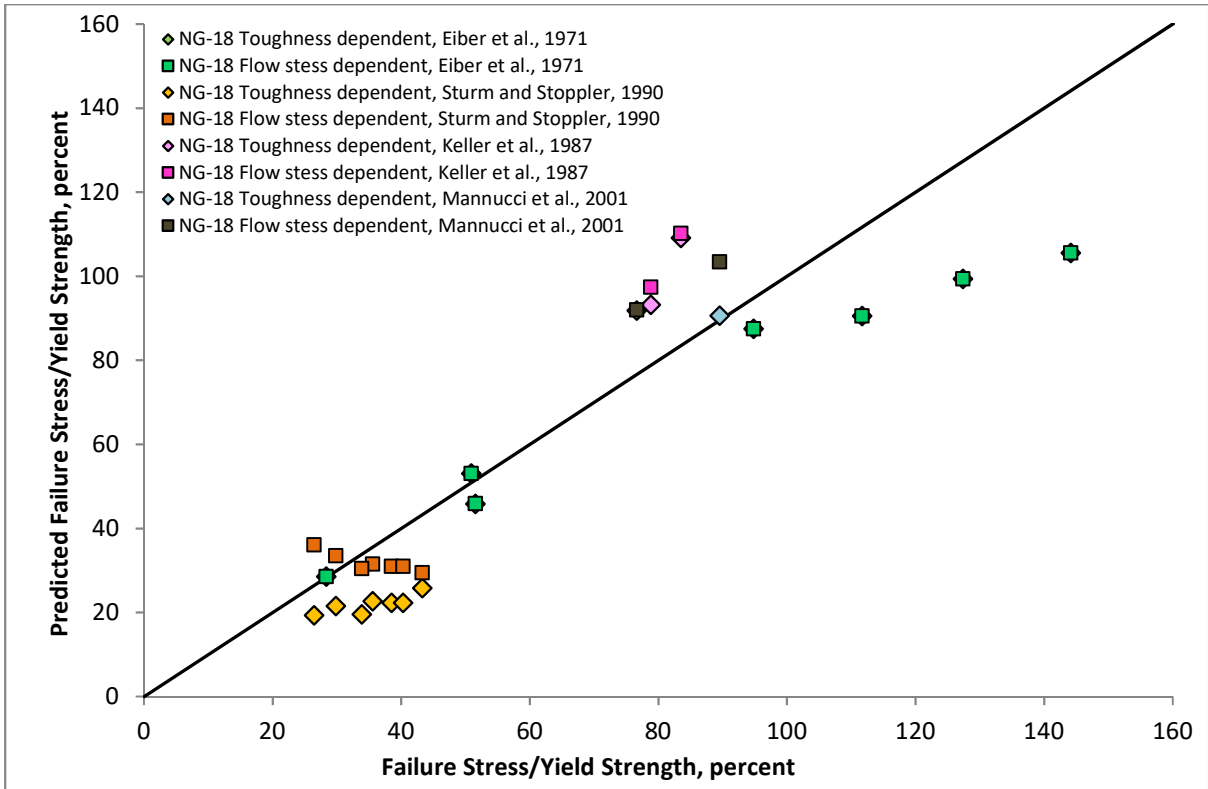
Input	Value
External Diameter Range (mm)	762 – 1067
Wall Thickness Range (mm)	9.1 – 15.6
Yield Strength Range (Nmm ⁻²)	379 – 510
Tensile Strength Range (Nmm ⁻²)	531 – 634
Charpy V-Notch Impact Energy (J) Full-Size	24 – 69
Defect Length Range (2c) (mm)	64 – 610
Defect Depth Range (d) (% wall thickness)	25 – 92

464 **Table 13:** Thin Wall, Part-Wall Burst Test Vessel Details, Kiefner et al., 1973

465 **4.2.2. FAILURE DATA COMPARISON**

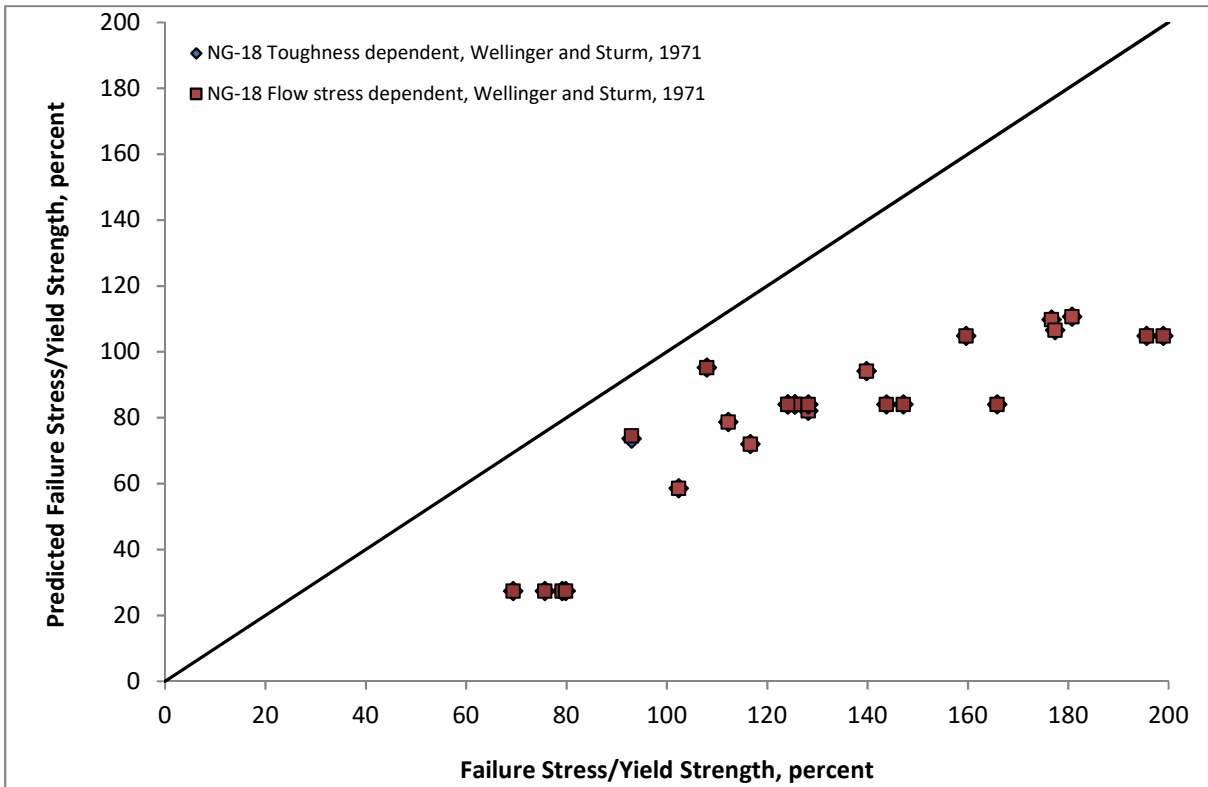
466 A comparison has been made between the actual failure stresses for the set of axial part-wall
 467 defects in thick wall pressure vessels and those predicted for the same set of defects by the
 468 part-wall toughness dependent and flow stress dependent NG-18 equations. The results are
 469 shown in Figures 7 to 10. The failure stress is assumed to be the hoop stress at the failure
 470 pressure.

471 For the experimental data considered in Figure 7, both the toughness and flow stress
 472 dependent forms of the NG-18 equations produce reasonably accurate results. Furthermore,
 473 the predictions of each form of the NG-18 equation are very similar. Figure 8 in particular,
 474 shows the predictions of the toughness and flow stress dependent forms of the NG-18
 475 equation to be almost identical for this particular set of experimental data.



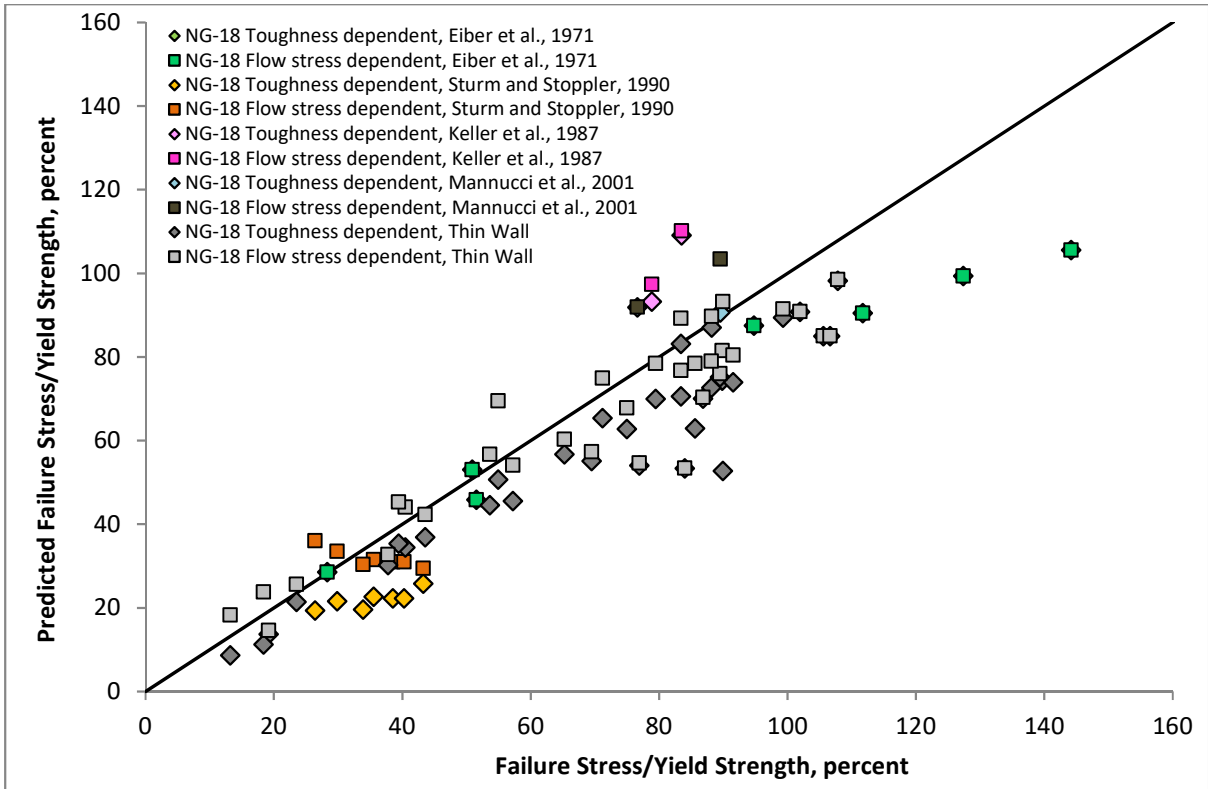
476

477 Figure 7 Predicted versus Actual Failure Stress for Axial Part-Wall Defects in Thick Wall Pipe
 478 Sections According to the Toughness and Flow Stress Dependent Forms of the NG-18
 479 Equations, Eiber et al., Sturm and Stoppler, Keller and Mannucci et al.



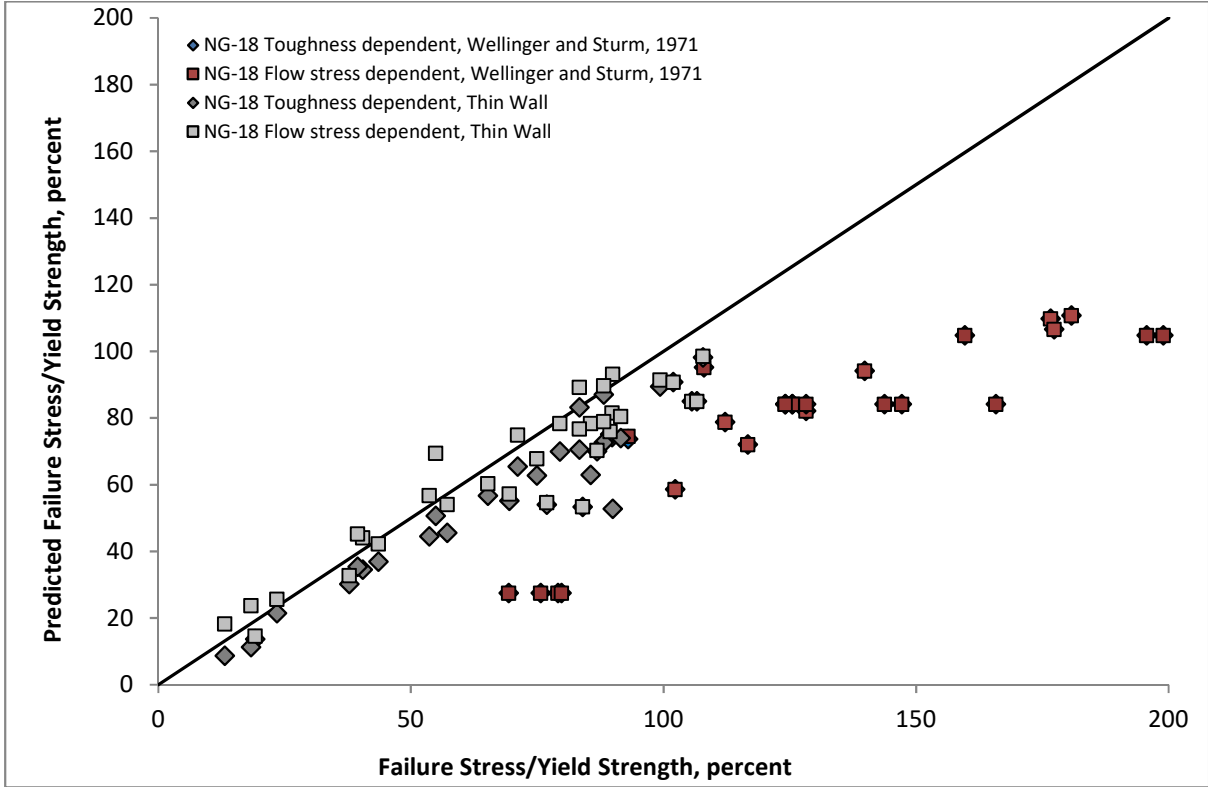
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481 Figure 8 Predicted versus Actual Failure Stress for Axial Part-Wall Defects in Thick Wall Pipe
482 Sections According to the Toughness and Flow Stress Dependent Forms of the NG-18
483 Equations, Wellinger and Sturm



484
485
486
487

Figure 9 Predicted versus Actual Failure Stress for Axial Part-Wall Defects in Thick and Thin Wall Pipe Sections According to the Toughness and Flow Stress Dependent Forms of the NG-18 Equations, Eiber et al., Sturm and Stoppler, Keller and Mannucci et al., Kiefner et al.



488

489 Figure 10 Predicted versus Actual Failure Stress for Axial Part-Wall Defects in Thick and Thin
490 Wall Pipe Sections According to the Toughness and Flow Stress Dependent Forms of the NG-
491 18 Equations, Wellinger and Sturm, Kiefner et al

492 Figure 9 and Figure 10 include thin wall failure data from Battelle in 1973 (Table 13). In Figure
493 9, the thick wall data for NG-18 is contained within the scatter of the data points of the thin
494 wall data. In Figure 10, the thick wall data for the NG-18 equation does not lie within the
495 scatter of the data points of the thin wall data and they tend to be much lower than the actual
496 failure values. The reason for this is the small pipe diameter to wall thickness ratio of the
497 vessels used in these tests, that emphasises the conservative nature of Barlow's equation for
498 thin wall pipelines when applied to thick wall pipelines.

499 In summary, Figures 7 to 10 suggest that the flow stress dependent NG-18 equation is a valid
500 assessment method for part-wall defects in pipelines up to at least 47.2 mm wall thickness
501 with a full-size equivalent upper shelf Charpy V-notch impact energy of at least 50 J.

502 **5. CONCLUSIONS AND RECOMMENDATIONS**

503 The component parts of the through-wall and part-wall NG-18 equations and BS 7910 were
504 compared and it was found that the factor M_T for through-wall defects and M_P for part-wall
505 defects in the NG-18 equations are very close to the reference stress solutions in BS 7910,
506 which are applicable to thick wall pipe. Furthermore, calculations for example defects showed
507 that the effect of the other differences between the two assessment methods, such as the
508 inclusion of the bending stress in BS 7910, are small and remain so even when the pipeline
509 wall thickness is increased. Comparisons with experimental failure data for thick wall vessels
510 also showed that the flow stress dependent NG-18 equations are valid assessment methods
511 for through-wall and part-wall defects in pipelines up to a wall thickness of at least 47.2 mm,
512 when the full-size equivalent upper shelf Charpy V-notch impact energy is at least 50 J.

513 These findings suggest that, in principle, the flow stress dependent NG-18 equations may be
514 used as limit state functions in the established failure frequency method used to calculate the
515 failure frequency due to third party external interference for high toughness, thick wall
516 pipelines such as those required for dense phase CO₂ pipelines.

517 For modern, high toughness line pipe steel, defect failure will occur as a result of plastic
518 collapse rather than fracture. In a plastic collapse failure, the fracture toughness, by definition,
519 has no effect. The flow stress dependent NG-18 equations should be suitable for application
520 to thick wall CO₂ pipelines, provided the pipe material has a high toughness. However, it is
521 unclear as to where the boundary between high (entirely plastic collapse) and low toughness
522 (fracture/plastic collapse combination) lies. Therefore, further study is required to determine
523 the minimum toughness for failure to be controlled by plastic collapse.

524 It is recommended that further work is carried out in order to determine the toughness limit
525 at which it is acceptable to use the flow stress dependent form of the NG-18 equations.

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