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# Design optimization and application of bolt-shotcrete support for East Tianshan tunnel project in China

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### **1.1 Brief introduction of East Tianshan Tunnel**





East Tianshan Tunnel is located in Hami City, Xinjiang, connecting Hami City with Balikun County and crossing the Tianshan Mountains. The tunnel is two-way four-lane, and the driving speed is 80km/h. In order to meet the engineering requirements, there are two inclined shaft.



### **1.1 Brief introduction of East Tianshan Tunnel**

#### **D** Engineering characteristics

- Three "long": the tunnel length is 11775 m; the total length of inclined shafts is more than 4 km; the section requiring auxiliary construction of inclined shaft is 4.2 km.
- Two "high" : high construction risk; high environmental protection requirements.
- One "cold" : the altitude of tunnel site is more than 2000 m, and the annual minimum temperature is 32°C.





SECTION 1. Engineering background of East Tianshan Tunnel and 2# inclined shaft

### **1.2 Engineering background of the 2# inclined shaft**

- No. 2 inclined shaft is connected to the left line tunnel at ZK17+000, during the construction period, the inclined shaft needs to be assisted in the construction of the main tunnel; during the operation period, the inclined shaft is used as the ventilation channel.
- No. 2 inclined shaft is a tunnel with vertical side walls. As shown on the right, its span is 10 m, height is 8.5 m and section area is 74.25 m<sup>2</sup>.



Import situation of No.2 inclined shaft





SECTION 1. Engineering background of East Tianshan Tunnel and 2# inclined shaft

### **1.2 Engineering background of the 2# inclined shaft**



The total length of No. 2 inclined shaft is 1340 m and the maximum depth is 585 m. The grade of surrounding rock that inclined shaft passes through includes grade III, grade IV<sub>a</sub> and grade IV<sub>b</sub> (the quality of surrounding rock is generally good).



### **1.2 Engineering background of the 2# inclined shaft**





Summary of surrounding rock condition of excavation face

Surrounding rock condition of excavation face at X2K0+240

2# inclined shaft is surrounded by tuffaceous sandstone, whose color is bluegrey to grey-green, with relatively complete rock mass, relatively developed joints and fissures, and good self-stabilization ability of surrounding rock.



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### 2.1 Original support scheme of the 2# inclined shaft

□ The original supporting scheme of No. 2 inclined shaft is composite lining support. Taking the grade IV<sub>b</sub> surrounding rock section as a example, its supporting structure is shown in the right figure.

Although the composite lining support has high structural safety, the overall cost of the project is high and the construction period is long in the section with good rock quality.





### 2.2 Brief introduction of bolt-shotcrete support

- Bolt-shotcrete support is a form of support with low cost, convenient for construction, uniform structural stress, which is widely used in international tunnel engineering.
- □ The characteristics of **bolt-shotcrete support** are very suitable for **No. 2 inclined shaft project**, and it is advisable to optimize the original design with this support method.



Highway tunnel Norway Subway station Finland Railway tunnel China



SECTION 2. Support design scheme and optimization of the 2# inclined shaft

### **2.3 Optimizing support scheme of the 2# inclined shaft**

Based on the principle of **bolt-shotcrete support**, the support parameters of the grade IV<sub>b</sub> surrounding rock section are optimized, as shown in the right figure.

□ Compared with the **composite lining**, the **bolt-shotcrete support** cancels the **cast-in-situ lining structure** and changes the **contact characteristics** between the initial structure and the secondary structure.



#### Bolt-Shotcrete Support Scheme



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□ FLAC3D (Fast Lagrangian Analysis of Continua) software is used for simulation analysis, and simulation analysis relied on X2K0+430 section with a depth of 150 m.





- Model boundary size: 5 times of tunnel diameter in horizontal direction, 5 times of tunnel diameter below vertical direction, and actual buried depth above tunnel.
- Boundary conditions: horizontal displacement constraints are imposed on left and right boundary surfaces, and hinged displacement constraints are imposed on the bottom of the model.





- □ Due to the existence of **geotextile and waterproof plates** in composite lining, only **compressive stress** is transmitted between initial support and secondary lining, but **shear and tensile capacity** cannot be provided.
- □ However, there is no separation layer in the **bolt-shotcrete support structure**, and the initial shotcrete and the secondary shotcrete can bond well and deform together.



bearing mode of side wall under different structural forms



 Considering the difference of stress conduction between composite lining and boltshotcrete lining, the contact characteristics between layers of different supporting methods are set up separately.

**The interface element** in FLAC software is used to simulate contact characteristics.





### **3.2 Material parameters of simulation model**

□ According to the relevant data (geologic examination, relevant literature), the parameters of the simulation model are as follows:

#### Mechanical parameters of solid element materials

Material 🖉	Constitutive model @	Elastic modulus /GPa @	Poisson's ratio @	Internal friction angle / °	Cohesion /kPa «	Density / kg/m <sup>3</sup> ~
Tuffaceous sandstone @	Mohr Coulomb 🖓	4.25 ↔	0.32	33 🖓	0.45 @	2100 +2
C25 shotcrete @	Elastic 🖉	25 🕫	0.2 🕫	- +2	- +3	2300*
C30 shotcrete 🖉	Elastic 🖉	27 🕫	0.2 🕫	<b>-</b> ↓ <b>-</b>	- +3	2300*
C30 modeled concrete @	Elastic 🖉	30 🕫	0.2 🕫	- ↔ -	= + <sup>2</sup>	2500 +2

#### Mechanical parameters of cable element

Material «	Elastic modulus /Pa &	Cross-section-are-/m <sup>2</sup> , $^{\circ}$	Stiffness of cement · slurry ·/Pa ↔	Cohesion of cement slurry ∕N ↔	tensile strength /N o
$\Phi$ 22 cable $_{e^2}$	2×10 <sup>10</sup> ,	3.8×10 <sup>-4</sup>	2×10 <sup>7</sup> «	1×10 <sup>6</sup> <sup>43</sup>	3.1×10 <sup>5</sup> 43

#### Mechanical parameters of interface element

Type 🕫	Shear strength / Pa 🕫	Tensile strength ∕Pa ₀	Tangential stiffness / N/m @	Normal-stiffness-/N/m @
Composite lining interface **	9.2×10 <sup>3</sup> «	7×10 <sup>3</sup> «	1×10 <sup>3</sup> «	4×10 <sup>9</sup> «
Bolt-shotcrete support interface @	9.2×10 <sup>5</sup> + <sup>3</sup>	7×10 <sup>5</sup> "	4×10 <sup>9</sup>	4×10 <sup>9</sup> <sup>"</sup>



### **3.3 Internal force analysis of support structure**

- □ The maximum axial force of bolt-shotcrete support is 78.5% of that of composite lining, but the bending moment is one order of magnitude smaller. This reflects the structural stress characteristics of **bolt-shotcrete support** which mainly bears the surrounding rock load in the way of "**small bending moment, large axial force**".
- This bearing mode can give full play to the good compressive performance of concrete, and has a good optimization effect on the stress of concrete structure.





### **3.4 Internal stress analysis of support structure**

■ The section stress distribution of the side wall and arch foot is extracted, as shown in the following figure. It should be noted that the cross-section stresses are all compressive stresses.

□ Due to different contact effects, there is a **sudden change** in **internal stress** of **composite lining** Arch foot **structure**, but no change in boltshotcrete support structure.

□ The maximum stress of boltshotcrete support is 8.09 MPa, while that of composite lining is 11.51 MPa. The structure stress of bolt-shotcrete support with smaller thickness is even smaller.



Contrast of internal stress (unit: MPa)



### **3.5 Displacement analysis of support structure**

- □ The maximum values of **vault settlement** and **horizontal convergence** of boltshotcrete support are 5.82 mm and 2.58 mm, while that of composite lining are 3.46 mm and 1.81 mm.
- The displacement of bolt-shotcrete support is slightly larger than that of composite lining, but the above displacement values are still within a reasonable range.



Contrast of structural deformation (unit: mm)



### **3.6 Stability analysis of surrounding rock**

- □ YAI(Yield Approach Index) method is an evaluation method of surrounding rock stability. Based on YAI method of Drucker-Prager criterion, YAI value distribution map of tunnel support is obtained. In short, the smaller YAI value, the more unstable.
- □ In composite lining tunnel, the stability of surrounding rock outside the side wall and at the bottom of the tunnel is not good, while unstable surrounding rock is located at arch waist, arch foot and side wall in bolt-shotcrete support tunnel.



(a) Composite lining



(b) Bolt-shotcrete support

Distribution of YAI contour after support



### **3.7 Main conclusions of simulation analysis**

- The internal force distribution of **bolt-shotcrete support** presents the characteristics of "small bending moment and large axial force", which makes the structural stress mainly compressive stress and can give full play to the compressive performance of concrete materials.
- □ Compared with composite lining, **bolt-shotcrete support** can achieve **the same or even better mechanical performance** with **less concrete consumption**, and has **higher economy.**
- □ Due to the small support stiffness, the ability of **bolt-shotcrete support** to limit the deformation of surrounding rock is relatively weak, but **the displacement is still in a reasonable range**.
- □ In **bolt-shotcrete support tunnel**, the stability of surrounding **rock outside arch waist, arch foot and sidewall** is poor, which should be paid attention to.



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### 4.1 Experimental scheme

□ According to bolt-shotcrete support scheme, C30 shotcrete is needed for tunnel construction. In order to obtain the concrete material proportion suitable for the site, the experiment of three kinds of concrete material proportion was carried out with the water cement ratio as a variable. The material proportion parameters are as follows:

number .	cement .	water 🖓	sand .	gravel .	fly ash -	ore fines .	water-reducing agent ہ	accelerator .	steel fiber .
A e	459 .	195 .	750 .	<b>749</b> ₀	<b>92</b>	23 💩	1.00% ~	5% *	32 🖓
B 🕫	433 .	195 .	<b>796</b> ₽	735 💩	87 .	22 🖓	1.00% ~	5% ~	32 🕫
C 🕫	410	195 .	842 .	718 💩	82 💩	21 .	1.00% ~	5% *	32 +

#### Material proportion parameters

□ The test items include **compressive strength** (12h, 1d, 3d, 7d, 14d and 28d), **slumps** and **rebound degree** of sprayed concrete.



### 4.2 Experimental process



Configuration process of concrete specimens



Laboratory testing



Field effect verification of material proportion



Strength testing of shotcrete on-site

□ Through **laboratory test** and **field effect test**, the actual effect of material proportion of each group was evaluated.



### **4.3 Experimental results**

- The early compressive strength of each test group increased rapidly, and except for 12h compressive strength, the compressive strength of other age groups showed "A > B > C". At the same time, the strength of each age can meet the requirements of C30 shotcrete in Chinese standards.
- □ The ratio B's rebound degree is lower and more economical.
- Considering strength and cost comprehensively, the ratio B is finally adopted in the field.

number a		compre	essive st	rebound degree .				
	12h	1d .	3d .	7d 🛛	14d .ª	28d @	Sidewall (%) .	Vault (%)
A .º	6 .	10.3	27.3 .	34.4 .	40.6 *	42.1 .	8 🕫	21 .
B .	5.6 .	9.4 .	25.1 .	33 +	38.8 .	40.5 .	9 🕫	17 .
C	6	8.6 .	20.3 .	31 .	37.8 .	38.9	12 .	23 .

Results of field testing and laboratory testing



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### **5.1 Field test scheme**

- □ In order to verify the performance of **bolt-shotcrete support** structure, three typical sections (X2K0+378, X2K0+420 and X2K0+450) were selected to monitor the structural deformation and internal stress of bolt-shotcrete support structure.
- □ The test points of the field test scheme are arranged as shown in the following figure.



Layout of displacement measuring points

Layout of concrete measuring points



### 5.2 Field test analysis

- □ The maximum compressive stress is 9.39 MPa at the left arch foot. And the maximum tensile stress appears at the right wall, with a value of 0.45 MPa.
- The structural stress at each position can meet the requirements of compressive and tensile properties of C30 concrete structure.





### 5.2 Field test analysis

- □ Vault settlement is 1.92 mm, and peripheral convergence is 1.46 mm. The deformation of bolt-shotcrete support structure is generally small, which **can meet the requirements of Chinese standards.**
- Meanwhile, The difference between the measured displacement and the simulated result is relatively small.





### **5.3 Application effect evaluation**

- The bolt-shotcrete support structure is stable and there is no cracking.
  Structural deformation is small, and structural stress is within the range of material strength. It can be considered that bolt-shotcrete support can meet the support needs.
- However, the field situation shows that there are some shortcomings in the structure waterproofing of boltshotcrete support.



Drill hole



Grid arch arrangement



Dripping water



**Blasting effect** 



Concrete spraying



Overall situation of support



### **5.4 Cost evaluation of engineering**

- □ Without considering the influence of over-excavation, the construction cost per meter is reduced by 25.24% and the excavation speed is increased by 38.92% after bolt-shotcrete support is adopted.
- □ Therefore, on the premise of meeting the support safety, bolt-shotcrete support is an economical and efficient construction method.





### **5.5 Engineering progress**

- □ With **bolt-shotcrete support method**, **No. 2 inclined shaft** was completed **67 days ahead of schedule**.
- □ On the other hand, the completion of No.2 inclined shaft greatly improves the construction efficiency of the main tunnel.



Completion of No.2 inclined shaft on June 27, 2018



Completion of the exit of left line tunnel on June 13, 2019



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### 6 Conclusion

- □ The internal force distribution of **bolt-shotcrete** support presents the characteristics of "**small bending moment and large axial force**", which makes the structural stress mainly compressive stress and can give full play to the compressive performance of concrete materials.
- Compared with composite lining, **bolt-shotcrete support** can achieve the same or even better mechanical performance with less concrete consumption, and has higher economy.
- the field monitoring results show that the deformation of bolt-shotcrete support structure is small, the structural stress meets the material performance requirements. In short, the bolt-shotcrete support structure is stable.
- During the implementation of bolt-shotcrete support, the cost of support per meter is reduced by 36.78%, and the average excavation efficiency is increased by 38.9%, which verifies the applicability and advantages of the optimization scheme.





# THANK YOU FOR LISTENING

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