Engineering Conferences International ECI Digital Archives

Shotcrete for Underground Support XIV

Proceedings

11-19-2019

Untersammelsdorf Tunnel – Challenges, Special Measures and use of Special Shotcrete for Tunneling in Lacustrine Clay

Matthias Beisler ILF Consulting Engineers (Asia) Ltd.

Johannes Benedikt ILF Consulting engineers Austria

Sebastian Höser ILF Consulting engineers Austria

Hanns Wagner DHW Pte.Ltd.

Klaus Berger

Follow this and additional works at: https://dc.engconfintl.org/shotcrete_xiv

Part of the Engineering Commons

Recommended Citation

Matthias Beisler, Johannes Benedikt, Sebastian Höser, Hanns Wagner, and Klaus Berger, "Untersammelsdorf Tunnel – Challenges, Special Measures and use of Special Shotcrete for Tunneling in Lacustrine Clay" in "Shotcrete for Underground Support XIV", Matthias Beisler, ILF Consulting Engineers Asia, Ltd., Thailand Preedee Ngamsantikul, Thailand Underground and Tunneling Group (TUTG), Thailand Herbert Klapperich, TU Freiberg, Germany Eds, ECI Symposium Series, (2019). https://dc.engconfintl.org/ shotcrete_xiv/11

This Abstract and Presentation is brought to you for free and open access by the Proceedings at ECI Digital Archives. It has been accepted for inclusion in Shotcrete for Underground Support XIV by an authorized administrator of ECI Digital Archives. For more information, please contact franco@bepress.com.

UNTERSAMMELSDORF TUNNEL – CHALLENGES, STATE-OF-THE ART MEASURES AND USE OF SPECIAL SHOTCRETE FOR TUNNELING IN LACUSTRINE CLAY

JOHANNES BENEDIKT (ILF) MATTHIAS BEISLER (ILF) SEBASTIAN HÖSER (ILF) HANNS WAGNER (ÖBB)

LAUS BERGER (ÖBB)



ECI

TUTG

stuv



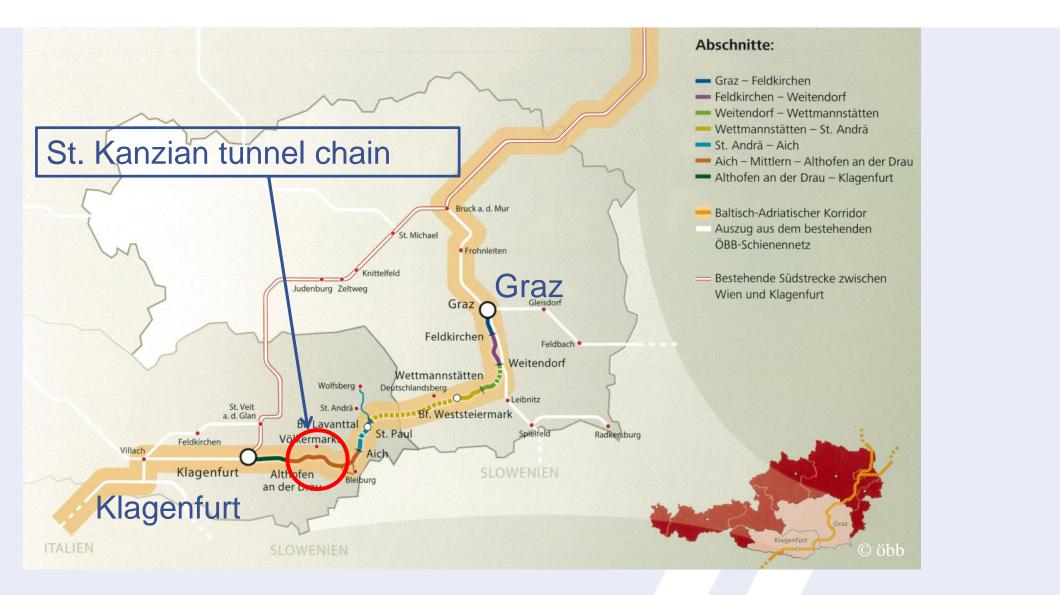
INTRODUCTION – PROJECT OVERVIEW



NOVEMBER 2019, PATTAYA



INTRODUCTION – PROJECT OVERVIEW





INTRODUCTION – PROJECT OVERVIEW

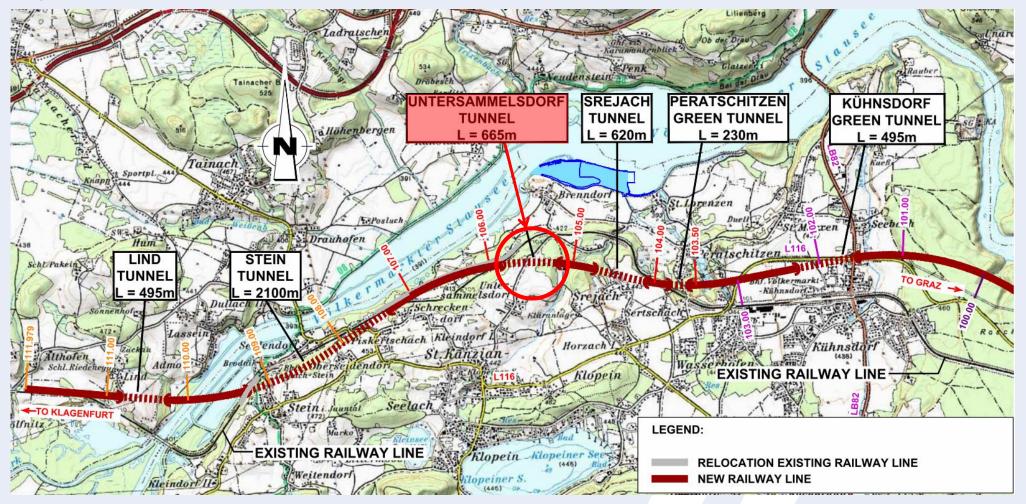
Visualization St. Kanzian tunnel chain:





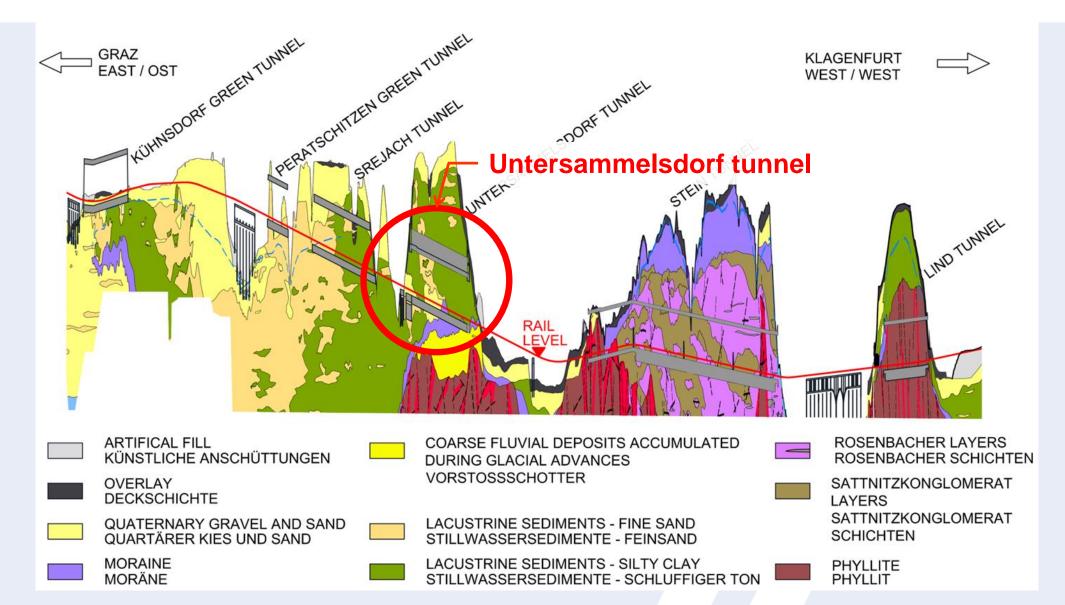
INTRODUCTION – PROJECT OVERVIEW

Layout St. Kanzian tunnel chain:



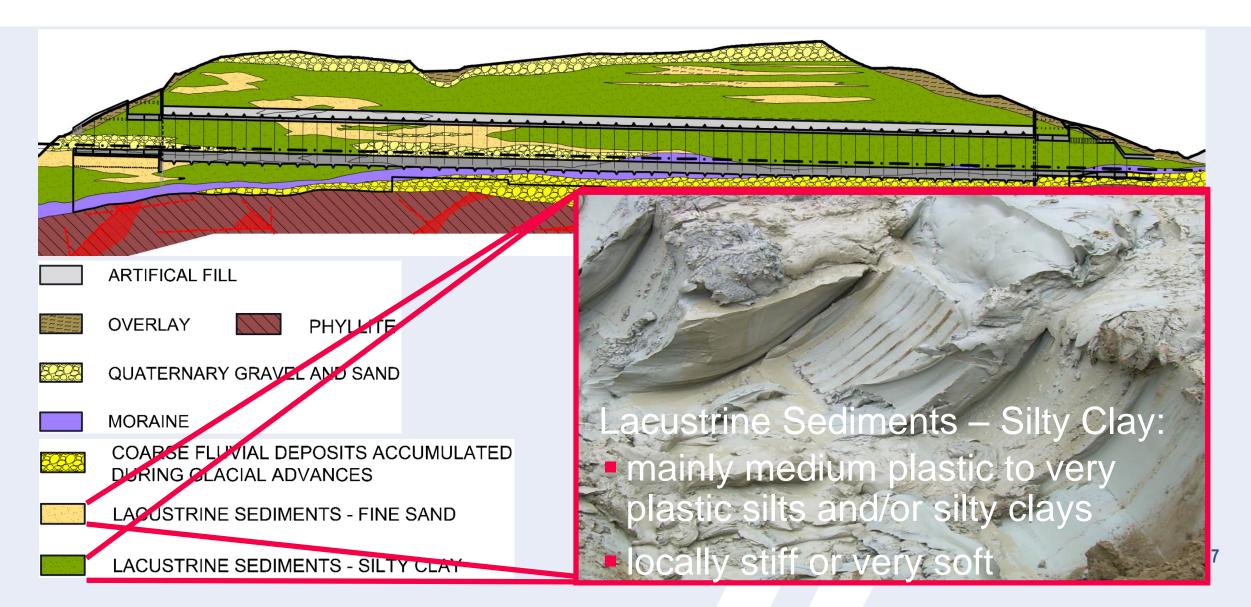


GEOLOGICAL OVERVIEW



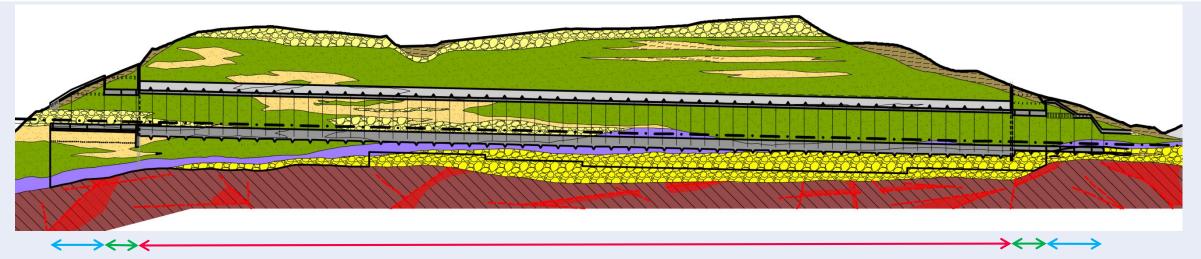


GEOLOGICAL LONGITUDINAL SECTION





GEOLOGICAL LONGITUDINAL SECTION



Trough Top – East down East

Mined tunnel L = 665 m

Top- Trough down West West

- Heading permanent in lacustrine sediments
- Bench / invert in western tunnel section in quaternary gravel and sand as well as moraine
- Overburden up to 33 m over rail level

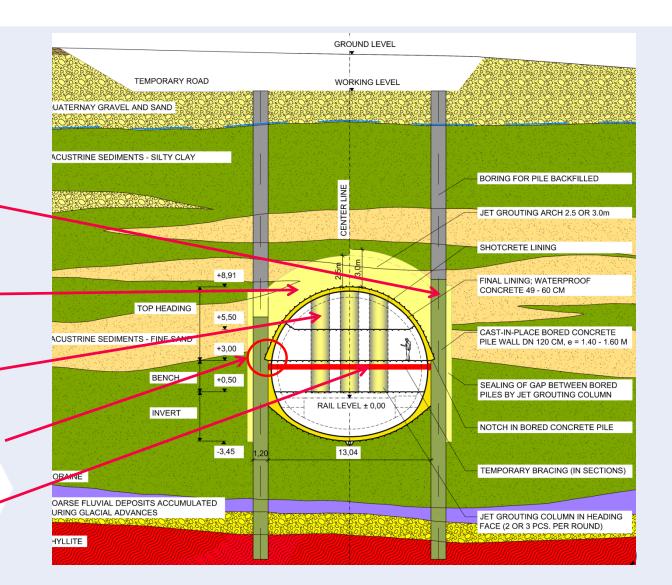




CONCEPT TUNNEL EXCAVATION – CROSS SECTION



- caste in place bored concrete _ pile wall, DN 120 cm, e = 1.40 m – 1.60 m
- jet grouting arch 2.5 m or 3.0 m thick interfingerd with pile wall
- jet grouting columns in heading face (2 or 3 pcs per round)
- Shotcrete lining with connection to bored concrete pile
- temporary bracing (in sections)



INCIDENTS AND POSSIBLE FAILURES DURING CONSTRUCTION

Cast - in - place bored concrete pile wall, DN 120 cm, e = 1.40 m - 1.60 m







INCIDENTS AND POSSIBLE FAILURES DURING CONSTRUCTION

Defective areas in tunnel crown (due to reduced diameter of jet grouting columns).

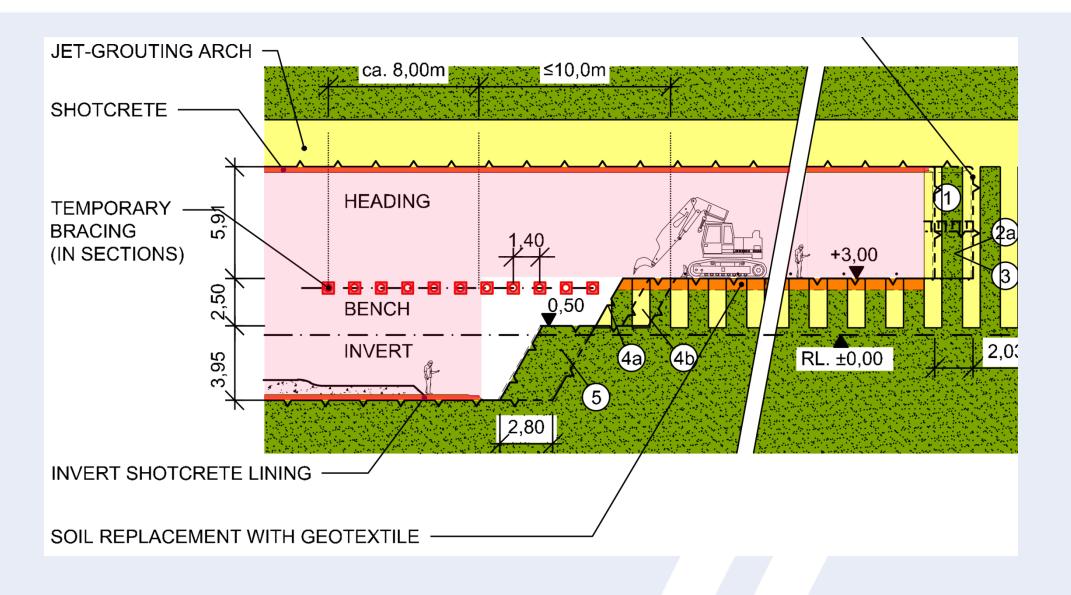








CONCEPT TUNNEL EXCAVATION – LONGITUDINAL SECTION



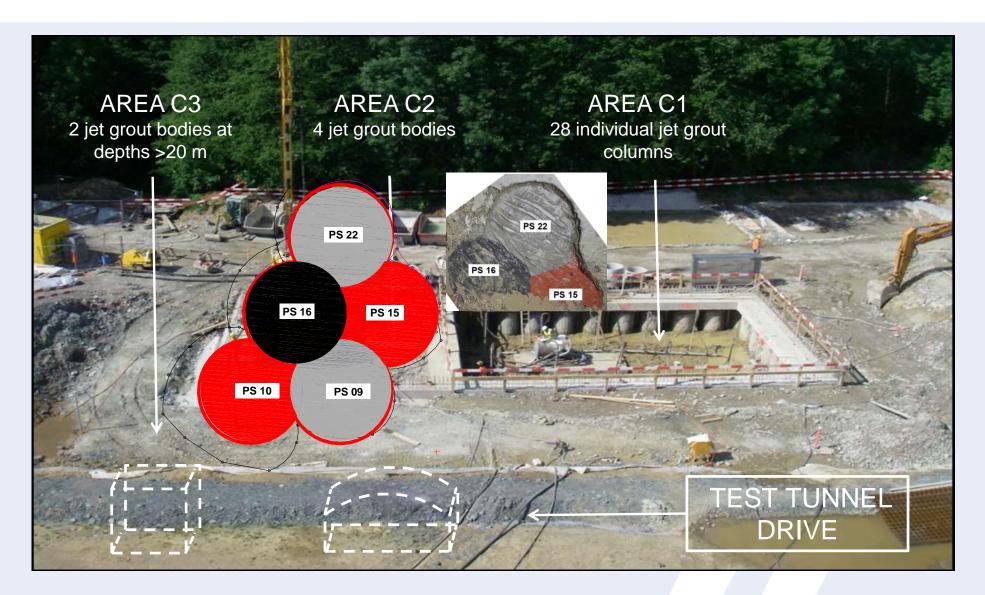
TEST PIT FOR SOIL STABILIZATION TESTS







TEST FIELD – AREAL VIEW

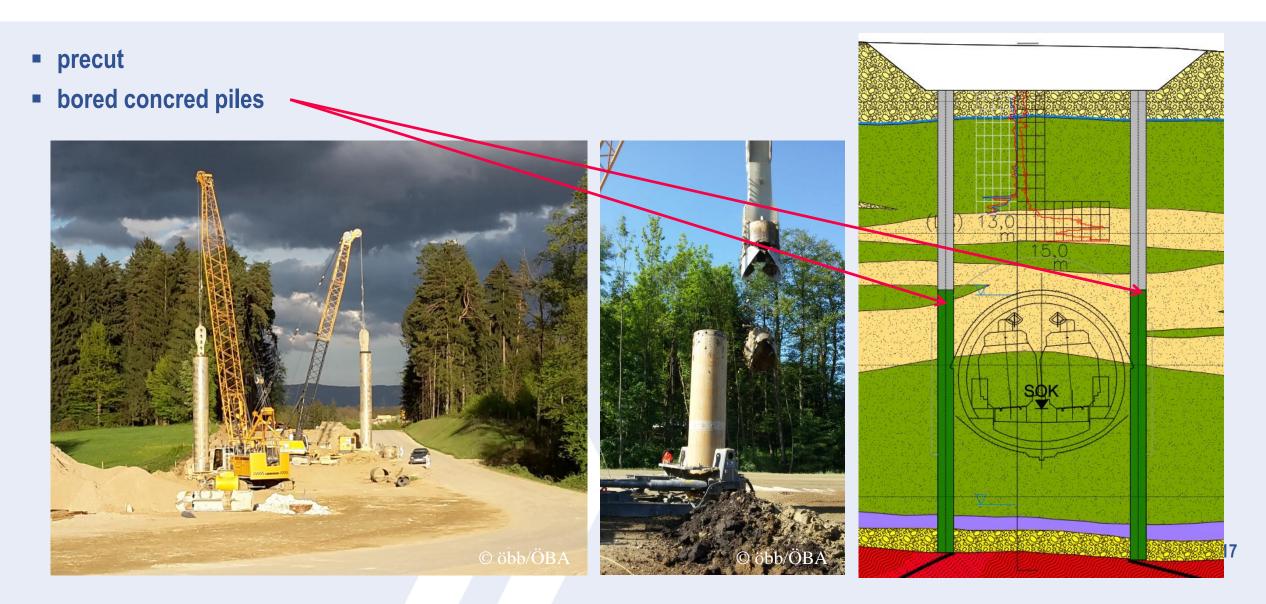










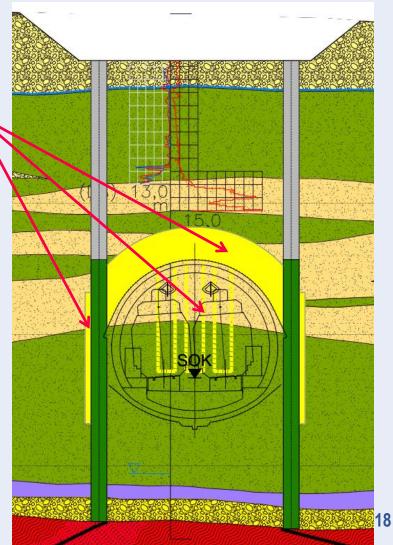




- precut
- bored concred piles
- Jet grouting arch, jet grouting face columns and jet grouting for sealing gap between bored piles

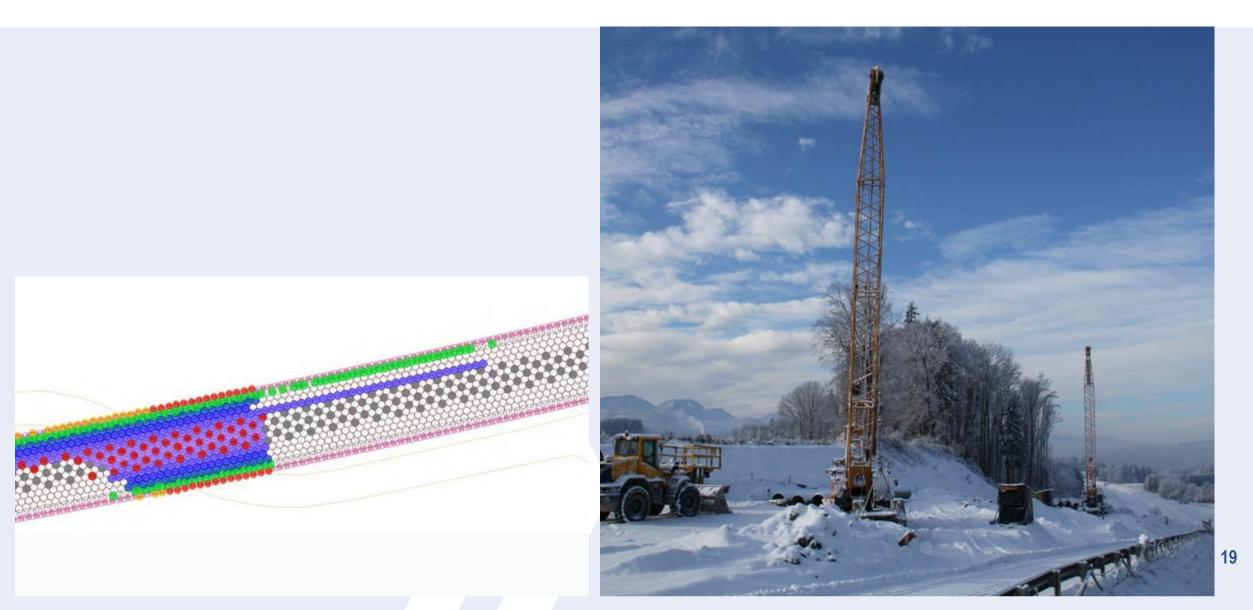








CONSTRUCTION - JET GROUTING COLUMNS

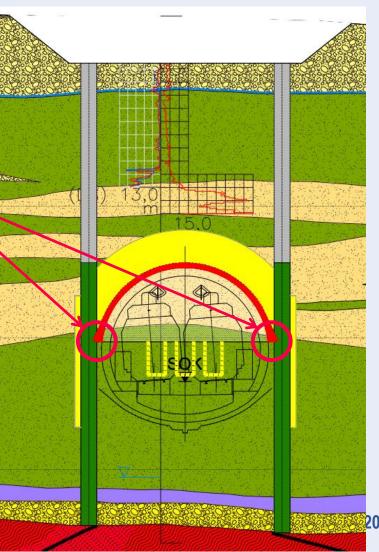




- precut
- bored concred piles
- Jet grouting arch, jet grouting face columns and jet grouting for sealing gap between bored piles
- advance of heading with connection of the shotcrete to the bored piles by a notch



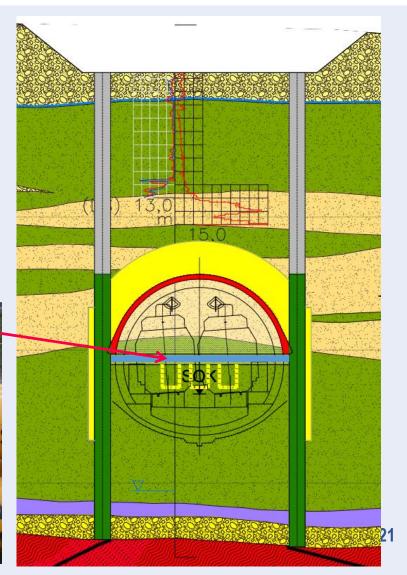




- precut
- bored concred piles
- Jet grouting arch, jet grouting face columns and jet grouting for sealing gap between bored piles
- advance of heading with connection of the shotcrete to the bored pile by a notch
- temporary bracing at bench level



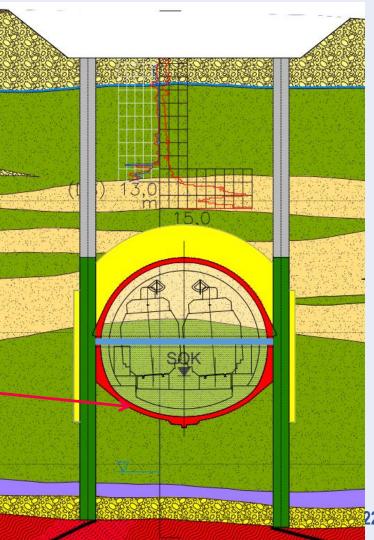






- precut
- bored concred piles
- Jet grouting arch, jet grouting face columns and jet grouting for sealing gap between bored piles
- advance of heading with connection of the shotcrete to the bored pile by a notch
- temporary bracing in bench level
- advance of bench and invert with rapid ring closure

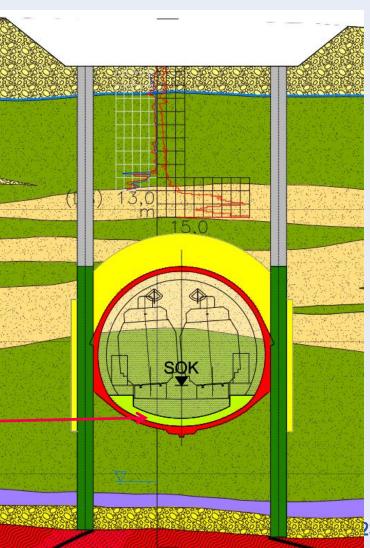






- precut
- bored concred piles
- Jet grouting arch, jet grouting face columns and jet grouting for sealing gap between bored piles
- advance of heading with connection of the shotcrete to the bored pile by a notch
- temporary bracing in bench level
- advance of bench and invert with rapid ring closure
- concrete lining invert

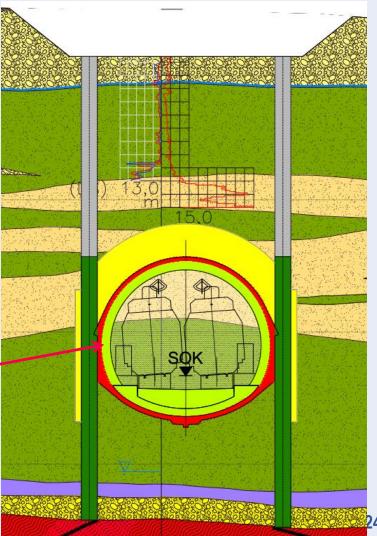






- precut
- bored concred piles
- Jet grouting arch, jet grouting face columns and jet grouting for sealing gap between bored piles
- advance of heading with connection of the shotcrete to the bored pile by a notch
- temporary bracing in bench level
- advance of bench and invert with rapid ring closure
- concrete lining invert
- concrete lining profile

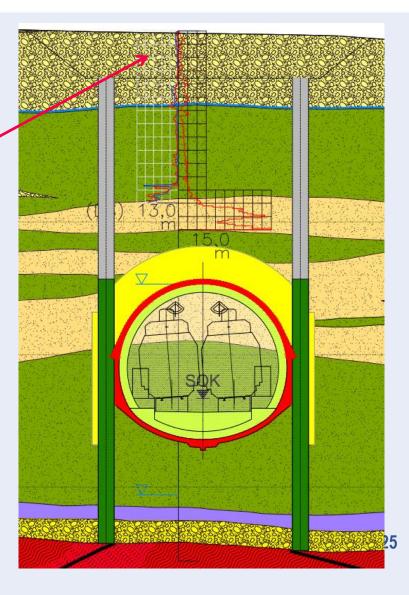








- precut
- bored concred piles
- Jet grouting arch, jet grouting face columns and jet grouting for sealing gap between bored piles
- advance of heading with connection of the shotcrete to the bored pile by a notch
- temporary bracing in bench level
- advance of bench and invert with rapid ring closure
- concrete lining invert
- concrete lining profile
- refilling of precut



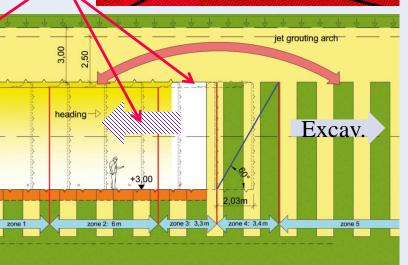
Load bearing system:

- entire ground load of up to 6,000 kN/m lies on the jet grouted arch (in area of excavation)
- due to the high stiffness of loadbearing system, small displacements and accordingly sudden system failure had to be expected after slight exceedance of the loadbearing capacity
- danger of local failure in area of excavation with a zip effect in long. direction

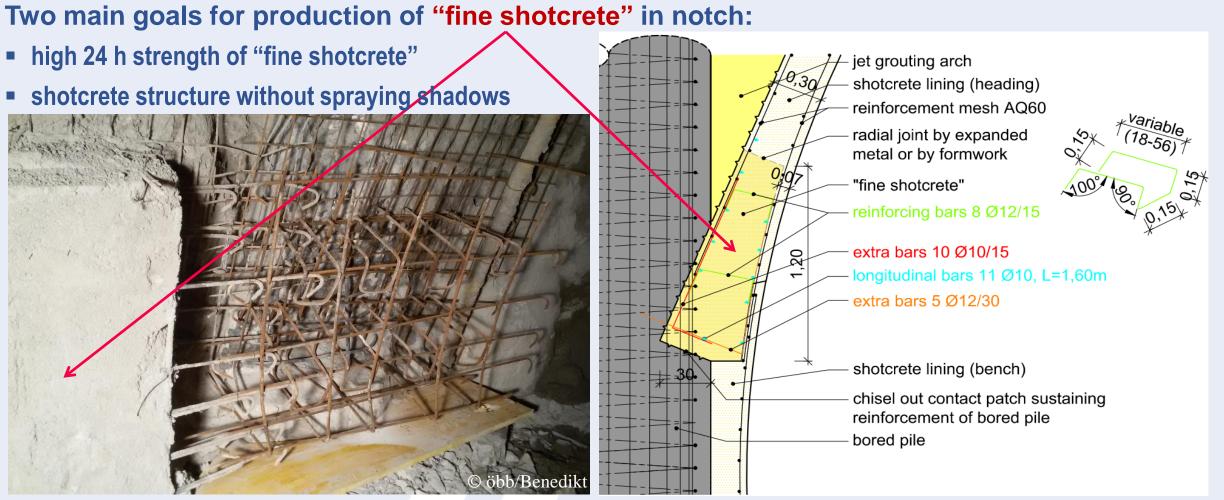
Decisive question:

- parameters for friction and cohesion in the joints between jet grouted arch and bored piles
- ⇒ lack of reliable values for these parameters
- Additional measure: load transfer from the shotcrete support layer through notches in the bored piles



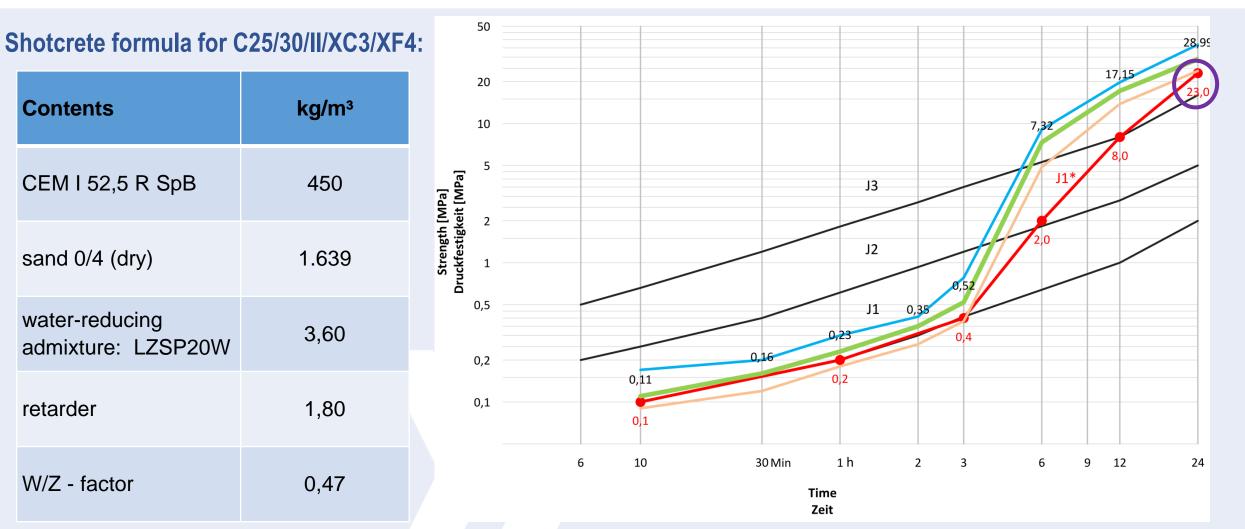








Contents	kg/m³		
CEM I 52,5 R SpB	450		
sand 0/4 (dry)	1.639		
water-reducing admixture: LZSP20W	3,60		
retarder	1,80		
W/Z - factor	0,47		





- over 50 tests
- Variation of capacity from 0,5 to 25,5 [m³/h]
- Variation of accelerator for solidification from 2% to 8% of the binder content

Shotcrete - machine	Mai2Pump Taurus	ALIVA TYP 263	SPM 500 Putzmeister
Capacity [m³/h]	0.5 - 2.0	2.5 - 4.0	4.0 - 25.5
accelerator solidification	2% - 8%	2% - 8%	2% - 8%



concrete

bored pile

compact struc

of shotcret

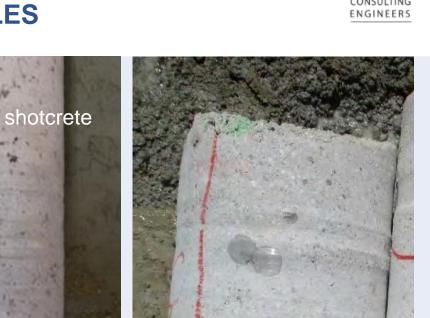
© öbb/ÖBA

DESIGN DETAIL: SHOTCRETE IN NOTCH OF BORED PILES

Findings:

- accelerator content strongly affected the development of compression strength within the first 3 hours
- after 3 hours the chemical reaction of the cement and the water / cement ratio of 0.47 were decisive for the development of compression strength
- Compression strength of 23 N/mm² after 24 h could be reached in all tests
- Best results were delivered through a combination of the highest pumping rate (25.5 m³/h) with the lowest accelerator content (2 %).
- This resulted in wet fine shotcrete with a plastic consistency.
 Trapping of rebound or spraying shadows behind the reinforcing bars could be avoided.





ØBB

SHOTCRETE FOR UNDERGROUND SUPPORT XIV, NOVEMBER 2019, PATTAYA UNTERSAMMELSDORF TUNNEL – CHALLENGES, SPECIAL MEASURES AND USE OF SPECIAL SHOTCRETE FOR TUNNELING IN LACUSTRINE CLAY



Summary of conclusions:

Untersammelsdorf tunnel represented an unique and highly challenging tunnel project, requiring state of the art design and construction methods.

Different construction methods had to be used **simultaneously** to successfully cope with specific conditions and problems.

- ⇒ careful preparation and planning (e.g. Untersammelsdorf test field),
- ⇒ inovative solutions (construction method, "fine shotcrete"),
- ⇒ further developments in heavy construction (jet grouting)

ensured the project execution on-time and within the budgeted costs.



