## QUALITY CONTROL OF OFFSHORE POSITIONING SURVEYS

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#### ABSTRACT

This paper discusses the need for a quality control method for offshore positioning surveys and then its actual use. It limits the discussion to radio locationing systems only, but this restriction does not imply that Satellite Navigation or Underwater Acoustic Positioning Systems do not require such quality control.

The subject is divided into two parts. Firstly the calibration methods necessary to determine the zero/delay settings for the survey equipment and to establish the propagation velocity required in the survey. Secondly, the positioning of the survey vessel with the necessary, but often missing, "simultaneous quality control" of this positioning.

The methods described in this paper are compared with field results, from which can be seen the often limited value of the most commonly used, and sometimes time-consuming and expensive, calibration methods.

It will be shown here that the generally accepted principle of the location offshore of vessels and structures using only two position lines not only lacks reliability but can often result in costly resurveys.

The lack of data redundancy in offshore surveys stands in sharp contrast to the practice in land surveying where data redundancy in observation and control surveys is common practice.

With the increasing practice of having extra positioning equipment on stand-by—especially in remote areas—and with the increase in cost of offshore surveys, Shell's Topographic Survey Department has developed a method based on redundancy of data. This method provides a continuous quality control of the offshore positioning while surveying, and avoids unpleasant surprises after the survey vessel has left the area.

#### INTRODUCTION

Positioning in marine surveys for the oil industry makes use of a wide variety of techniques ranging from sextant and theodolite measurements for inshore work to satellite navigation, integrated with long-range navigational aids and sonar doppler for the remote areas beyond 300 km from the coast. The in-between zone is covered by a wide range of radiolocation (survey) systems, whereas for some special projects underwater acoustic systems are employed. The applications for these techniques vary from the positioning of seismic shotpoints, the placing of mobile drilling rigs on previously shot seismic lines, the horizontal control of airborne or seaborne gravity/magnetometry surveys, the positioning control of pipeline route surveys and the laying of these pipes thereafter, the shallow geophysical surveys and subsequent accurate siting of large production structures as well as the control of large-scale engineering and hydrographic surveys.

Since the majority of these activities still take place in the zone between the inshore visual-control area and the more remote sea areas an effort is being made to identify the present problems in radiopositioning and to try to improve upon performance.

This paper deals with the need for a reliable standard method of calibration prior to offshore surveys and a real-time quality control during such surveys, with the aim of reducing errors and ambiguities and at the same time giving confidence to the survey party on board and providing subsequent scrutiny and correction possibilities to the remote onshore processing department.

The radio systems are categorized by the type of geometric pattern they generate, and this determines the calibration procedures to be used.

There are three distinct types of pattern in radiopositioning : hyperbolic, circular and linear.

#### Hyperbolic patterns

These patterns are generated by two pairs of beacons. The equipment on the survey vessel records a signal that is the result of either a measured phase difference or a measured time difference between the signals transmitted by each of the two beacons in a pair (Hi-Fix, Syledis systems).

The formula for the calculation of the hyperbolic pattern is :

 $P = (Dist (MS) + Dist (MV) - Dist (SV)) \cdot F/V + Z$ 

in which :

 $\mathbf{P} = \text{pattern value}$ 

 $\mathbf{F} = \text{transmission frequency}$ 

 $\mathbf{Z} = \mathbf{z}\mathbf{e}\mathbf{r}\mathbf{o}/\mathbf{d}\mathbf{e}\mathbf{l}\mathbf{a}\mathbf{y}$  correction

- V = propagation velocity of the radiowave
- Dist (MS) = distance between the shore beacons (Master and Slave)
- Dist (MV) = distance Master beacon to vessel

Dist(SV) = distance Slave beacon to vessel

### **Circular** patterns

These patterns are each generated by one shore station which is triggered by the transmitter on board the survey vessel. The observed signals are either time intervals measured between the vessel's outgoing signal and the returning one retransmitted by the shore beacon, or the phase difference between the two signals (Maxiran, Argo systems).

The formula for the calculation of the circular pattern is :

 $\mathbf{P} = \mathbf{Dist} \left( \mathbf{MV} \right) \cdot \mathbf{2F} / \mathbf{V} + \mathbf{Z}$ 

### Linear patterns

These patterns are generated by shore beacons transmitting the angular value of horizontal rotation of their aerials relative to a Reference Object (R.O.). (Artemis system). Theodolite stations generate similar linear patterns.

The formula for the calculation of the linear pattern is :

in which :

 $\mathbf{P} = \mathbf{R} + \mathbf{A} + \mathbf{Z}$ 

R = reference bearing to R.O. A = angular value of aerial rotation.

Each of the above three types of pattern gives one geometric position line (LOP). The position of the vessel is the intersection of two of such lines of position. This statement, however, is the classical misconception of the positioning problem, as will be seen later.

There is no mariner or surveyor in the world who in conventional navigation does not use a check bearing just to make sure. Why then does one, with radiopositioning systems, use only two LOPs ?

#### CALIBRATIONS

## A. PRESENT METHOD OF CALIBRATION

The hyperbolic and circular types both have the propagation velocity of the radiowaves (at the comparison frequency), the transmission frequency and the zero/delay constants in the equipment directly related to their type. The main parameter governing the propagation velocity of systems in the LF and MF is the electromagnetic conductivity of the surface over which the radiowaves are transmitted, while for the systems working in the HF and UHF band, the propagation velocity is governed by the tropospheric refractive index.

Calibration methods hitherto varied, depending largely upon the equipment, skill, experience and improvisation talents of the survey personnel. Unfortunately, however, too often insufficient time has been allocated for this all-important exercise. Many operators nowadays still consider this part of the survey as not necessary—partly due to ignorance or overselling by the equipment manufacturers, but more often due to the demand for high production figures since calibration is not considered to be productive. All types of pattern, however, have a basic element in common : the coordinates of the shore-based radiobeacons and the consistency of the geodetic network in the area. Without these, the actual position of the LOPs cannot be correctly computed.

Frequency calibration was, and still is, not normally carried out in the field, due to the need for delicate specialized equipment. This calibration is, hopefully, done in the manufacturer's workshop. It is, however, possible to correct for a change in frequency during the computation as part of the velocity term.

Recent experience shows an urgent requirement for the ability to measure the absolute or relative signal strength in order to ensure accuracy of signals above a rapidly deteriorating minimum. Signal-to-noise ratio is also an important factor for accurate radio positioning. These aspects have been neglected by the majority of positioning survey contract firms.

The method of calibration is dependent upon pattern type.

#### **Hyperbolic** patterns

Until recently only two pairs of LOPs were generated. Baseline extensions (maximum and minimum values) were observed during the lanecount. Pattern values were checked at known points. The observed differences were either entered into the calculation as C - O (calculated minus observed) values or else minimised by the more experienced surveyor by the adoption of a new propagation velocity bearing in mind the physical properties of the various types of soil/water/air at the transmitted frequency. In some cases, however, a value supplied by the manufacturer was used.

It is obvious that a check on the consistency of the station coordinates from this calibration method is very difficult indeed. It takes a great deal of skill and patience to (a) determine one or more stations to be "out", and (b) pinpoint the culprit.

### **Circular patterns**

These patterns were normally calibrated over a long baseline, the length of which depended on local circumstances—preferably being equivalent to the average distance from base station to survey area. The observed difference on this baseline was either entered into the calculation as a zero/delay correction or mechanically fed into the equipment. As for the case of hyperbolic calibrations, the best known velocity value was assumed or else the value supplied by the manufacturer was used (particularly in cases where the equipment readout is in units of length).

The more experienced man however insisted upon presetting a known or calculated delay into the equipment and had it checked by internal station calibration. Thereafter a minimum of two known baselines were measured. This facilitated a check on the consistency of the coordinates, if (as was normally done) the equipment was erected on the base stations to be used for the survey, and provided a better assumption of the propagation velocity. An additional check on zero/delay errors was carried out, circumstances permitting, by crossing the baselines both internally and externally. From the difference between the value obtained internally (an addition) and the external one (a subtraction) the zero/delay error can be directly calculated for the beacon on whose side the extension was crossed. The value obtained for the other beacon consists of that beacon's delay error and the velocity error over double the baseline distance, as can be seen from the following formulae :

 $\begin{array}{l} Pm1 + Zm + Ps1 + Zs = b \cdot V/2F \\ Pm2 + Zm - Ps2 - Zs = b \cdot V/2F \\ Pm1 + Pm2 + 2Zm + Ps1 - Ps2 = 2b \cdot V/2F \ (addition) \\ 2Zm = 2b \cdot V/2F - (Pm1 + Pm2 + Ps1 - Ps2) \\ Pm1 - Pm2 + Ps1 + Ps2 + 2Zs = 0 \ (subtraction) \\ 2Zs = Pm2 - (Pm1 + Ps1 + Ps2) \end{array}$ 

It must be stated that, for various reasons, this method was unfortunately not always used.

#### Linear patterns

The problem of propagation velocity is irrelevant to this type of pattern. However, station coordinates and R.O. are as important as in the other types. Hardly any calibration for this type of pattern has been carried out in the past. One relied completely on the information supplied by the manufacturer and on the "wisdom" of the engineer setting up the equipment.

### **B. PROPOSED METHOD OF CALIBRATION**

Many of the surveys for the oil industry are in remote areas of the world. Positioning equipment should arrive in the area well before the survey vessel, and in this period the equipment can be tested and calibrated.

A new method of calibrating circular pattern type equipment has been developed which avoids costly logistic support and at the same time offers a maximum of checking possibilities.

- In this procedure we must establish the following :
- a) check that the equipment is working properly;
- b) determine the zero/delay values for each set and each exchangeable part of the equipment, including all spares;
- c) define the best average propagation velocity of the radiowaves for the survey area, including changes due to local conditions.

The procedure has been established as follows.

On arrival all the equipment is checked at the operations base. The surveyor in charge must ensure that all pieces are tagged with an identification number. He must carefully note that the method of putting in zero/delay settings is not to be changed. In some cases (Maxiran system) we found it necessary to request sealing of the correction dials as they are very apt to be accidentally altered.

In the calibration procedure we insist that the determination of the zero/delay value is also carried out at the operations base. To effect this a shore baseline of 800-1 500 metres is laid out by simple tape measurement, for instance along a jetty or a quiet road. Its minimum length is governed by the manufacturer's specifications, since swamping by the beacon's transmitter can occur at very close range. Use of an attenuator is in most cases advisable.

Since the uncertainty in the propagation velocity is less than 0.1 %, the error on a 1500 m baseline is negligible for the purpose of determining the delay setting.

The procedure has several advantages : it is easily adaptable to all sets, there are no communication problems, and it allows the surveyor to follow the complete procedure and to record the results on the spot. This procedure for basically checking the zero/delay error has now been applied in many surveys (in Portugal, Brazil, Tunisia, the North Sea, and Japan) and some facts have come to light. The cables of the Syledis aerial, for instance, had been assumed to have a certain constant delay per metre, but it was found that in some cases the value was nearly double that figure. With Maxiran equipment it was generally accepted by operators that the linear amplifiers and other parts of the equipment had a constant delay factor, as determined in the workshop (this is often a very misleading statement). However, with the procedure described above, errors of up to 30 metres were found.

The other factor, the propagation velocity, cannot be determined over the short baseline. The simplest way to determine this factor is the measurement of ranges over a long baseline, preferably over sea water and close to the area to be surveyed. Often such a baseline is difficult to find, or else the absence of adequate logistic support prevents this measurement. As an error in the third factor—the station coordinates—may exist, one must be careful in accepting these values, particularly if the velocity found differs considerably from the expected one. In that case the surveyor is then compelled to observe a second baseline.

Baseline crossings with the survey vessel (both internal and external crossings) whilst en route to the "prospect"—the industry's term for survey area—serve the same purpose as the long shore-baseline calibration.

Here the additional feature of being able to check base station coordinate consistency should not be neglected.

If, in the worst case, neither of these velocity determinations can be made, then this factor can be computed from a series of at least four stationary (either drifting or at anchor) threeway fixes widely spread over the area of coverage. An improved estimate of the propagation velocity can be computed from the redundant observations by the least squares method. By executing this same procedure on a number of selected points gathered during the survey this initial value can be improved. This last subject will be discussed further in the section dealing with onboard quality control of positioning data.

The method of calibrating the hyperbolic type of pattern is basically the same as that described earlier. The additional compulsory third pattern line, however, offers a more realistic possibility of obtaining a mean value for the propagation velocity. The method is similar to the one described in the paragraph on circular patterns. It has been tested extensively on a Shoran-Hi-Fix/3 system in Borneo with success.

The main problem remaining to be solved lies in the as yet unaccountable anomalies occuring in different parts of the coverage area. Thought is being given to a solution whereby for each of the patterns (lines of position) a variable velocity will be used which is a function of the surface over which the wave passes, or else to a solution which treats non-uniform velocities. The same problem exists for the circular systems when working at extreme capacity limits, since a drop in signal strength gives an effect which is comparable to a change in velocity.

The principal problems in the calibration of line pattern systems are the setting up of the equipment, and the entering of the R.O. value and the coordinates of the stations, but in the context of this paper they can be considered as basic surveying procedures. A check by the surveyor is essential, in particular one on the sealing of the R.O. setting. To enable the surveyor to check this setting during the course of the survey it would be preferable that the manufacturer include this reference value, together with the rotation in angle, in the coded transmission.

In the calculation, the bearing should be calculated from the coordinates of the R.O., and the difference with the reference value should be used as a zero correction instead of using the reference value directly in the computation, as is being done now. A further discussion is given in the section on quality control.

The improved calibration procedures will have a direct impact on the results obtained with the quality control package.

#### QUALITY CONTROL

During the course of a survey normally only limited control is carried out. Phase comparison hyperbolic systems seldom have redundant LOPs. Their lane values are ambiguous and prone to lane loss due to atmospheric disturbances, radio noise, power loss, or skywave effects. Frequently this is only checked by repeating readings at previously determined points.

Circular patterns are very often used in the two-range mode, and even if three beacons are available many systems can only display two values simultaneously. The recording of the third pattern value can then only be effected either before or after the survey of a line as the equipment has to be switched over to the third beacon. If the checks are taken as so-called "running" three-way fixes (i.e. non-simultaneous), then the ship's movement can considerably affect the validity of the check. During the initial period of offshore exploration a few systems only monopolized the industry; by always using the same positioning system a high degree of repeatability was achieved, enabling a vessel or drilling rig to return to a seismic "event" for either a follow-up survey or spudding a well. No real emphasis was placed on "absolute" positions that could be reproduced with other systems. The need for the latter arose when more and better systems became available; this requirement became a "must" by reason of factors such as frequency licensing limitations and the specifications for international and concession boundary determinations.

Long after the survey had been completed and the vessel had left the area errors were sometimes discovered or suspected from "misties" in seismic interpretation. However, to prove the interpreting department right or wrong in their accusations was either very difficult or impossible, since only scanty, or no, redundant observations were available. In cases where different surveys positioned with different systems are being correlated, if the discrepancies reported prove genuine, the absence of redundant data could become serious indeed. It is difficult to estimate how many of the surveys carried out—particularly those in the early years of offshore exploration—contain large positioning errors. The cost involved in resurveying or in drilling a well in the wrong place is considerable.

This is one of the main reasons why onboard positioning quality control is vital. To prove a survey correct, redundant field data in the form of one or more extra position lines should be recorded and analysed in real time. It is only with the help of these redundant observations that real confidence in the survey positioning can be obtained. The acquisition of redundant survey data has always been a basic principle in land surveying.

Around 1974 the topographical department of Shell Expro London, which is directly involved in the UK's North Sea surveys, made an effort to force survey contractors to include the recording of at least one more pattern. At the same time a software package for the table-top HP 9830 calculator was developed to enable instantaneous position computation aboard ship utilising from two to eight positioning patterns of the hyperbolic, circular and/or linear type, or any combinations thereof. The calculation is based on the method of variation of coordinates (least squares adjustment), and the derived standard deviation for the single observation is the tool and criterion for the reliability of the vessel's position.

In 1978 a revised program was written for the more powerful HP 9845 computer, the revision being based on Shell Expro's experience. This quality control method is now being implemented in all Shell-controlled surveys.

#### Calculation of a position fix

The observed pattern values are first corrected for the zero/delay error and for the propagation velocity. The following formulae are applicable :

Hyperbolic :  $U_i = (P_i - Z) \cdot V/F - Dist (MS)$  (in metres)

Circular :  $U_i = (P_i - Z) \cdot V/2F$  (in metres)

Linear : U<sub>i</sub> = Azi (MS) + (P<sub>i</sub> - R - Z)  $\cdot \pi/180$  (in radians)

where :

 $\mathbf{U}_{\mathbf{i}} = \mathbf{corrected}$  pattern value

- $P_1 = pattern reading$
- $\mathbf{Z} = \text{zero/delay correction}$
- $\mathbf{V}$  = corrected propagation velocity
- $\mathbf{F} = \text{transmission frequency}$
- Dist(MS) = distance Master to Slave beacon
  - $\mathbf{R}$  = reference bearing to R.O. in degrees, as set in the equipment
- Azi(MS) = calculated bearing to R.O. in radians

The calculation of the position should be made on the spheroid since long distances can be involved. Such a computation would take 4-6 seconds per point on the HP 9845, which is considered too long in view of the time interval of data acquisition on board. Therefore, a quicker solution was found consisting of correcting the metre pattern values for the Transverse Mercator projection scale factor and carrying out the position iteration on the TM grid. This method is four times faster, and the error between the spheroidal calculation and the TM grid is less than one metre at a distance of 300 km if appropriate formulae for the scale factor are applied to the lines. The iteration is repeated until the vector of the differences is less than one metre.

The standard deviation can now be calculated from the differences between the corrected observed pattern values and the values computed from the final coordinates :

$$\delta = \sqrt{(\Sigma \mathbf{V}_1 \mathbf{V}_2)/(n-2)}$$

n being the number of patterns observed.

It is stressed that the value of the standard deviation, especially when using a small number of redundant data, is only an indication of the measure of disagreement between the pattern values.

The least squares subroutine, as used in the HP 9845 program, is given as Appendix A to this paper. The program is written in the BASIC language. Appendix B contains a three-range position calculation program for the pocket calculator HP 97. The same techniques as described above have been used.

In the method of calculation the type of pattern is irrelevant. Theoretically, a fix can be calculated from any combination of the three patterns from different systems. However, only when the coordinates of the shore stations are based on the same geodetic datum and when the accuracy of the different systems can be considered comparable—and only then—can two or more different systems be directly used for the quality control program. No weighting has yet been included.

A remark must be made with regard to the calculation of a linear pattern. Normally, it has been the practice to accept the reading setting on the master beacon as being the correct bearing of the reference line. In the case of a wrong setting it is not possible to detect the error at a later stage as this bearing does not appear in the logged observations. In the method of calculation described above, the calculated value (Azi) and the beacon value (R) are used. This reduces the possibilities of error. The program thus serves as a check on positioning equipment generating linear patterns.

#### FUTURE DEVELOPMENTS

Further investigation is needed into some elements of the above method which are still to a certain extent affecting the results.

- (1) The propagation velocity has been assumed to be uniform throughout the computation method. We know that this is not true, but the problem is firstly what computational model to accept for different velocities, and secondly what factors are contributing to the changes in velocity (i.e. types of soil, the salinity and temperature of water, the seasonal effects on ground conductivity, the signal strength to mention but a few).
- (2) Pattern geometry and pattern stability both have an effect on the standard deviation as calculated by this method, but they have so far not been taken into account.

The program is, however, already being used for navigation of a vessel during surveys. Its usefulness to the quality control man on board to detect errors on the spot has been proven, since any gradual or sudden change in the standard deviation value will serve to alert him. It also gives the survey party the opportunity to remedy the majority of these errors whilst still in the area, in particular errors caused by wrong base station coordinates, changes in equipment, pattern drifts observed at a monitor station but not compensated for, etc.

When post-processing, the data can be more carefully scrutinized, and velocity calculations can be repeated in order to achieve a better result since the proposed logging procedure ensures that all data are available.

A program is under development to derive the best fitting propagation velocity and zero/delay corrections from the redundant data. The solution will also use the least squares principle.

	Appendix A
2770 L	eastsquares: REDIM X(Maxpatt.2).Y(2.Maxpatt).Z(2.2).A(2).T(2).U(Maxpatt).
V(Maxp	att), W(Maxpatt)
2775	MAT U=ZER
2780	19=0
2783	
2795	M2=A FOR SIEL TO Maynett
2800	IF R(S1)*K(S1)=0 THEN 2840
2805	ON ABS(P(S1,9)) GOTO 2810,2820,2830
2810	U(\$1)=AB\$(AB\$(R(\$1))+D(\$1)+G(\$1))+(C(\$1)/F(\$1))-B(\$1)
2815	G0T0 2835
2820	U(\$1)=AB5(R(\$1)+D(\$1)+G(\$1))*C(\$1)/(2*F(\$1)) COTO 2025
2830	udiu 2033   ({\})=R({\})+(ABS(P({\}))=R({\})=C({\})*(220/C({\}))*(PT/100)
2835	19=19+1
2840	NEXT 51
2845	IF 1942 THEN 3185
2850	19=1
2855	FOR S2=1 TO 10 ! Maximum number of iterations=10
2865	NNI XIZEK Nat 0-750
2870	111
2875	N9= <a <1-e2+sin(n4="">&gt;2&gt;&gt;2&gt;&lt;1-E2&gt;</a>
2880	FOR S1=1 TO Maxpatt
2885	IF R(\$1)*K(\$1)=0 THEN 3030
2890	ON ABS(P(\$1,9)) GOTO 2895,2895,2970
2895	$W4=SUR((W2-P(SI,11))^2+(WI-P(SI,10))^2)$
2905	₩>=\W2=\d2\^2+\W2=\d2)*\F(31,11)=\d2)+\F(51,11)=\d2)^2 W5=(1+(1+W5/(36+U9))*U9)/£U0)\zva
2910	ON ABS(P(\$1,9)) GOTO 2915,2950
2915	W6=\$QR((W2-P(S1,13))^2+(W1-P(S1,12))^2)
2920	W7=(W2-X0)^2+(W2-X0)*(P(S1,13)-X0)+(P(S1,13)-X0)^2
2925	W7={1+{1+W7/(36*W9))*W7/(6*W9))*K0
2930	X(S1, 1)=(W1-P(S1, 12))/W6-(W1-P(S1, 10))/W4
2935	∧(S1,Z)=(NZ=P(S1,13))/N6=(NZ=P(S1,11))/N4 V(S1)=((S1)=N2/N=N2/N=N=N=N=N=N=N=N=N=N=N=N=N=N=N=N
2945	
2950	X(\$1,1)=(W1-P(\$1,10))/U(\$1)
2955	X(S1,2)=(W2-P(S1,11))/U(S1)
2960	V(S1)=U(S1)*H5-H4
2965	
2975	UT=L Gradrg(F(51,10),F(51,11),W1,W2,W3,W8) UT=LR+UR+CG+A 0405_5
2980	$X(S1, 1) = (P(S1, 11) - W2) \times W7$
2985	X(S1,2)=(W1-P(S1,10))/W7
2998	V(S1)=U(S1)-W5
2995	IF V(\$1)>-PI THEN 3010
3000	V(S1)=V(S1)+2*PI COTO 2005
3010	GOTU 2993 IF V(S))/PI THEN 3025
3015	V(S1)=V(S1)-2*PI
3020	GOTO 3010
3025	Y(S1)=Y(S1)+60+180/PI
3030	NEXT SI
3040	NDI YEIKN(X) NDI YEYKY
3045	MAT A=Y*V
3050	IF DET(2)=0 THEN 3165
3055	MAT Z=INV(Z)
3060	MAT T=Z*A
3065	W3=SQR(T(1)^2+T(2)^2)
3070	M14M17151) M2=M04T(0):
3080	IF W3>166 THEN 3165
3085	IF W3<1 THEN 3100
3090	NEXT S2
3095	GOTO 3165
3100	
3110	NJEW6EU FOR Stel TO Maxwaste
~ * * <b>*</b>	FUN DIEI FU NAXDALL

```
3115
       IF R($1)*K($1)=0 THEN 3130
3120
        W5=V(S1)^2+W5
        W6=W6+1
3125
3130
        NEXT S1
3135
        S=0
        IF W6=2 THEN 3150
3140
3145
        S=SQR(W5/(W6-2))
        Y=₩1
3150
3155
        X=W2
3160
        GOTO 3190
3165
        IF (X=X7) AND (Y=Y7) THEN 3185
        X≠X7
3170
3175
        Y = Y7
3180
        GOTO 2775
3185
        19 = 1
3190
        RETURN
```

The following parameters are passed on from the main program:

X = first approx. value for Easting Y = first approx. value for Northing R(i) = Range/Hyperbolic/Linear pattern value for pattern i K(i) = Pattern on/off array (0 or 1)D(i) = Zero or delay correction for pattern i G(i) = Fixed zero or delay value for pattern i C(i) = Propagation velocity (or factor) for pattern iF(i) = Comparison frequency for pattern iB(i) = Baseline distance pattern iP(i, 9) = type of pattern : 1 = hyperbolic, 2 = range, 3 = linearP(i, 10) = Northing of Slave beaconP(i, 11) = Easting of Master beaconP(i, 12) = Northing of Master beaconP(i, 13) = Easling of Master beaconYO = False Northing XO = False EastingQO = Latitude Origin (radians)

A = Semi-major axis of spheroid

- E2 = Excentricity to power 2
- KO = Scale factor at central meridian

User Instructions



STEP	INSTRUCTIONS	INPUT DATA/UNITS	KEYS	OUTPUT DATA/UNITS
	LOAD PROGRAM CARA			
2	STATION DATA: Noching	N;		
	Easting	_ Ei		
	Pattern number	ź	A	
3	Repeat '2' for other stations			
	· · · · · · · · · · · · · · · · · · ·			
4	Input (C-O) data: pattern 1	(50),		
	pattern 2	(0-0),		
	Mattern 3	$(c - \delta)_1$		
	<b>/</b>			
5	Sphereid / Projection date: serie regiments	a		
	inserse flat bening	f		
	soste factor al origin			
	false Easting	E		
6	Optional: propagation velocity			
	factor ( if not used 21)			
	Also conversion factor from range			
	unit to meters.			
7.	Aprox. position (mid area): Latitude	- Qn		
<u> </u>	Northing			
	Easting	Em		
	START OF CALCULATION.			
	To write dote card needed in			<u> </u>
·	program : CALC. Insert black cord.	· · · · · · · · · · · · · · · · · · ·		card.
9	to print list of input parameters		L I ≠ L € L	list
				·
		<u>├</u> ── ~-		
				┝
	·····			┣────
		1		1

Appendix B

# **Program Description**

THREE RANGE POSITIONING PREP Program Title Nomo ir. J.G. Riemersma / C. Pryce Dono Address Shell Internationale Petroleum Maatschagepy Dete 1.3.79 State Netherlands Zo Code City the Hague Program Description, Equations, Variables, etc. <u>Purpose</u> : to calculate T.M. cuordinates from 2-3 pattom ranges. <u>Input</u>: Station coordinates (in meters), (C-0) values, semi major axis spheraid, inverse flattening spheroid, scale factor at origin, false Easting, approxiatitude for centre of area, northing and easting of approx.pos. to start iteration. Propagation vehicity factor (if not used al) can also be used to convert ranges to meters. <u>Dubput:</u> The produced data cand is required in program: THREE RANGE POSIFIONING - CALC "Geodesy" by Co. Bomford. Third edition po. sty-sts <u>Eormalde</u> : b= =.(1-1/f) e2= (24-64)/24 R = Vp.v = =. V(1-e2) / (1-e2. SIN 2 pm) The values  $K_0$ ,  $E_0$  and  $6R^2K_0^2$  are transferred on the data card to the computation program, together with station data, (C-0) corrections and propagation velocity factor. to and Warnings. The program is based on the use of Transverse Mercator coordinates, but Operating Limits and Warnings could be machipied for use with other projected system coordinates.

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# **Program Listing**

# **User Instructions**



STEP	INSTRUCTIONS	INPUT DATA/UNITS	KEYS	OUTPUT DATA/UNITS
1	LOAD DATA CARD FROM 'PRED'			
	OR			
	RUN PROGRAM PREP			
	-			
2	LOAD PROGRAM CARD: CALC			
2	INPUT: RANGE 1 or O			
	RANGE 2 OF O			
	RANGE 3 or O		$[\mathbf{A}][$	
4	OUTPUT: Residual 1			-
	Residual 2			Ľ.
	Residual 3			
	Standard Deviation			â
	Northing			No
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	or 0 if no solution.			
5	Optional: NEW Vehicky factor	D	<b>B</b>	
	Last calculation will be repeated			
	with new value			
6	Ontional: NEW amores position	N		
	to start iteration. Last cake letin	E		
	will be repeated with new value.			
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	with new value.	10-012		
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	414676.97 ***			

# **Program Description**

THREE RANGE POSITIONING - CALC Program Title Name in J.G. Riemersma / C. Pryce Date 1.3.79 Notices Shell Internationale Petroleum Maatschappy Elite on the Hague sum Netherlands zip Cose Program Description, Equations, Variables, etc. <u>Purpose</u>: to calculate TM coordinates from 2-3 ranges, together with the standard deviation in single obsv. <u>Method</u>: Least squares using variation of coordinates method. <u>Input:</u> Data card from program: PREP and 2-3 ranges. Optional: change in propagation velocity, (C-O) and new iteration start point. Normal equations: If the observation equation is a.  $\delta N + \delta$ .  $\delta E = E$ then the normal equations are:  $\Sigma = 0.JN + \Sigma = 0.SE = \Sigma = 0.E$   $\Sigma = 0.JN + \Sigma = 0.SE = \Sigma = 0.E$ from which  $\delta N$  and  $\delta E$  can be solved. Standard deviation:  $\hat{m} = V\Sigma + \Gamma - I(n-2)$ residual , ne nr of patterns. Openeng Limits and Warnings 1. Computation time 1 to 2 minutes per point. 2. Calculation is based on TM projection and can be modified for other projections. The false Easting must be a 0 or a multiple of 10000. To save storage the false Easting is stored tugether with the velocity factor. 3. Computation has been used succesfully with ranges of 300 Km. 4. If no solution, try a new approximation.

STEP	KE		KEY CODE	COMMENTS	STEP	KEY	ENTRY	KEY CODE	COMMENTS
	861	ALB_H	21 11	Input ranges:	Ι	857	6103	22 03	
	682	STOE	35 15	R3		858	RCL3	36 83	
	803 -	F.	-31			659	57+5	35-55 85	
	884	STC 4	35 11	R <sub>2</sub>		868	RCL4	36 84	
	885	P4	-31			661	S7+6	35-55 86	Update stored
1	886	P≓S	16-51			662	PCL2	36 82	Nm, Em
	687	STOE	35 Ø6	K,		863	1	01	
	885	F#5	16-51			864	24Y?	16-35	Heater >1m
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123	0511	16 25 46	2 (		- F		105	2705	-31	1	
124	6321	16 25 46					100	5200 CT04	37 67 -	- CLER	(correction)
12:	KLLD	36 66					101		21 82		
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150	RCLT	36 87		~ ~			266	4	84	f	
151	X2	53	,				207	x	-35	<i>A</i>	
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# **Program Listing**