

1D/2D porosity-like model for urban flood modeling

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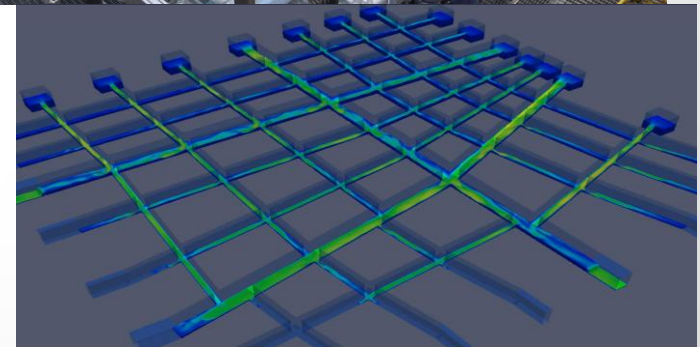
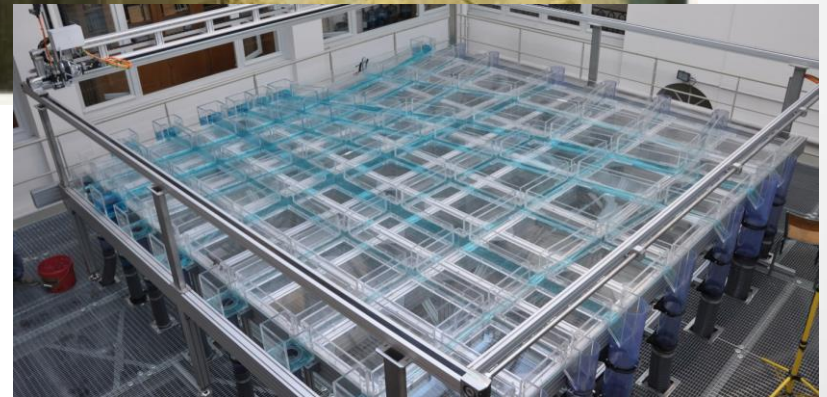
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Introduction

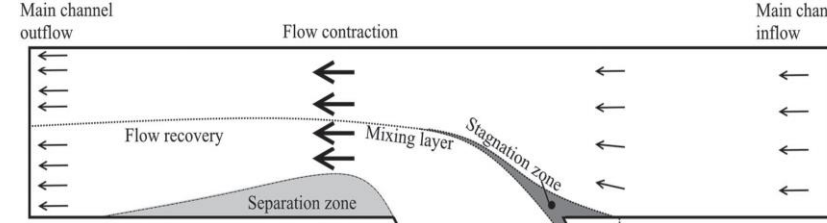
- Urbanization of floodplains - increase urban areas vulnerability
- Numerical modeling needed for flood impact forecasting
- Common modeling approaches:
 - SWE with classical friction laws
 - SWE with macroscopic porosity
 - 3D Navier Stokes equations
 => Trade off between the modeling goals, computational cost, data requirements

=> *Flood1D2D*: a 1D/2D hydraulic model taking into account the effect of 3D flow features using a parsimonious parametrization

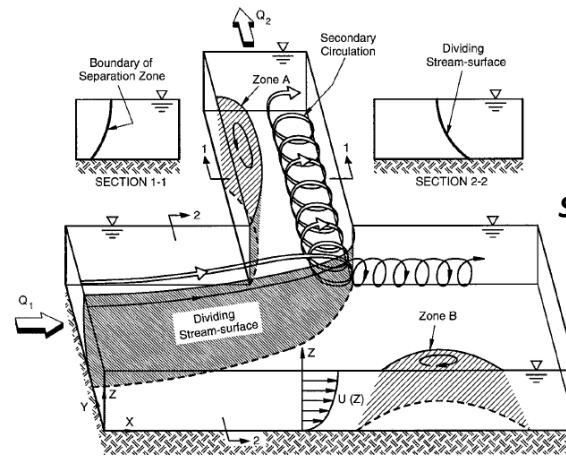


Introduction

- Urban flood flows: combination of hydraulic intersections (junctions, bifurcations, crossroads)
- Experimental and numerical studies highlight the presence of recirculation areas downstream of intersections [Finaud-Guyot et al. 2018]



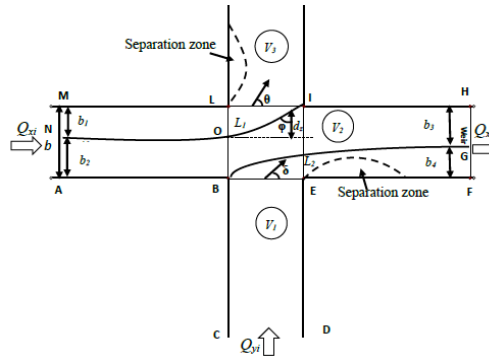
Source: [Creelle et al. 2016]



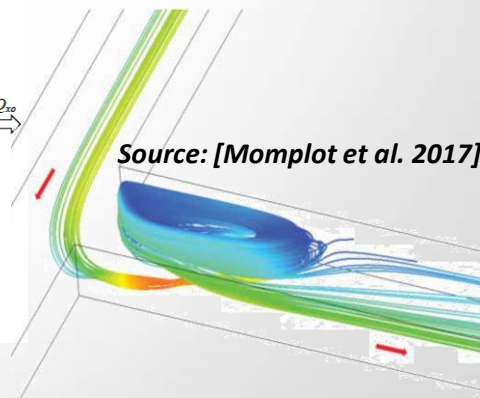
Source: [Neary et al. 1999]



Typical street network geometry (Google map)



Source: [Rivière et al. 2011]



Source: [Momplot et al. 2017]

Conceptual model

Operational requirements:

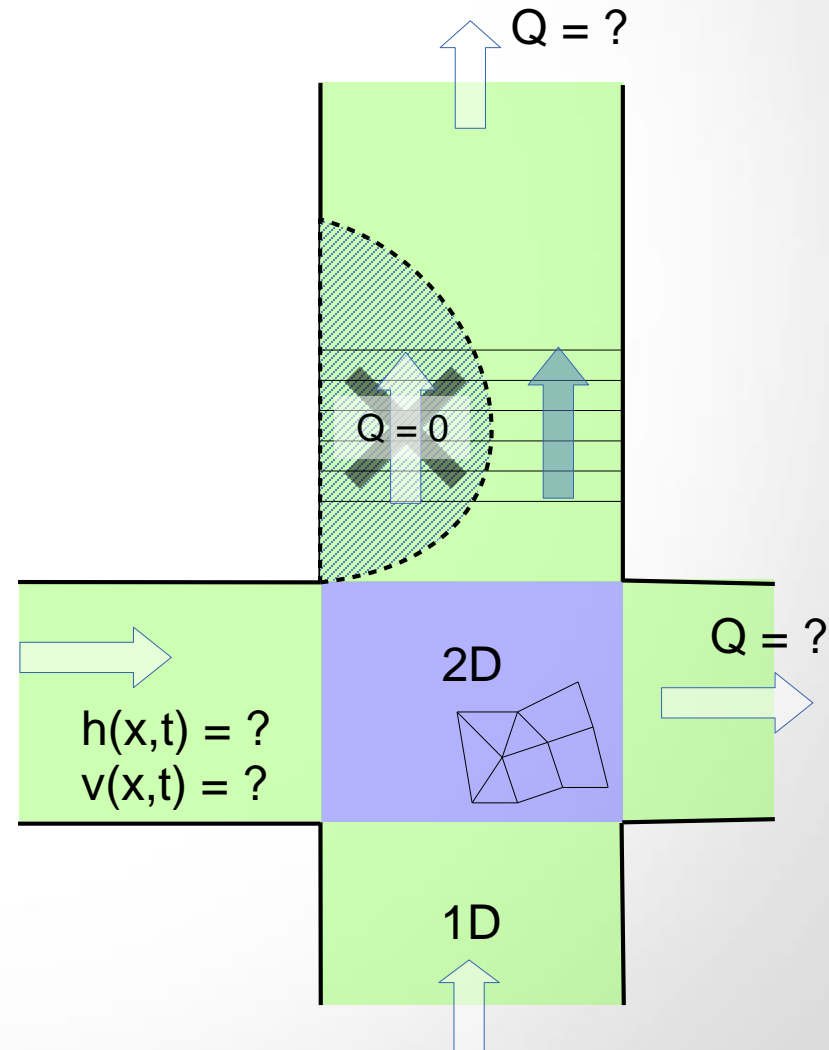
- Water depth and velocity profile in streets
=> coherent with a 1D modeling approach of the streets
 - Street discharge repartition
- Crossroad flow complexity are at least 2D

Proposed model

- 1D in street => cell width = street width
- 2D in crossroad = classical 2D unstructured mesh

Recirculation effect in 1D cell not accounted by the classical shallow water equations

- => Introduction of a subcell model in the 1D cells:
- ineffective flow area => no velocity
 - effective flow vein



Numerical approach

Classical 2D SWE in all cells

- Account for direction change
- Simplified coupling at 1D/2D edge

Finite Volume Method

$$\mathbf{U}_i^{n+1} = \mathbf{U}_i^n - \Delta t \left[\frac{1}{A_i} \sum_{j \in N(i)} \mathbf{P}_{i,j} L_j \mathbf{F}_j^n + \mathbf{S}(\mathbf{U}_i^n) \right]$$

1D1D and 1D2D edge :

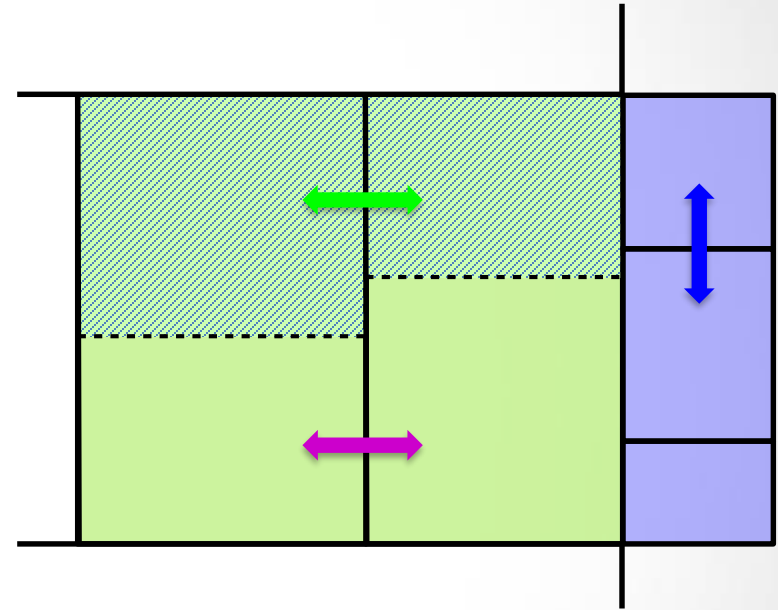
$$\mathbf{F}_j^n = \frac{L_{j,u} \mathbf{F}_{j,u}^n + L_{j,v} \mathbf{F}_{j,v}^n}{L_j} = (1 - \phi_j) \mathbf{F}_{j,u}^n + \phi_j \mathbf{F}_{j,v}^n$$

$$\phi_j = \frac{\phi_L + \phi_R}{2}$$

Riemann flux computation : PorAS solver $\mathcal{P}(\dots)$ [Finaud Guyot et al. 2010]

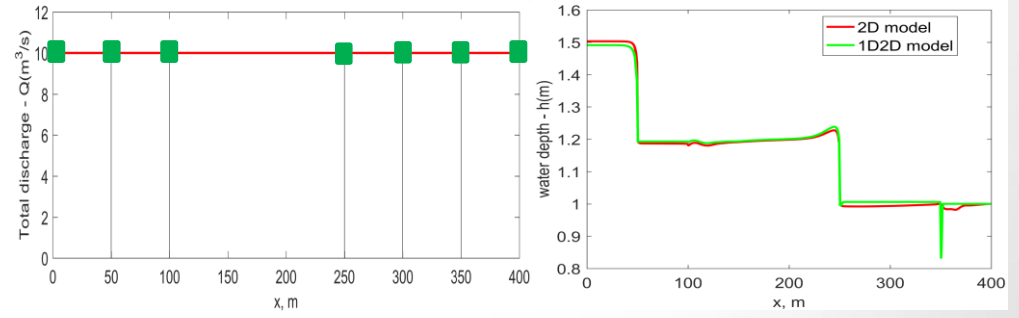
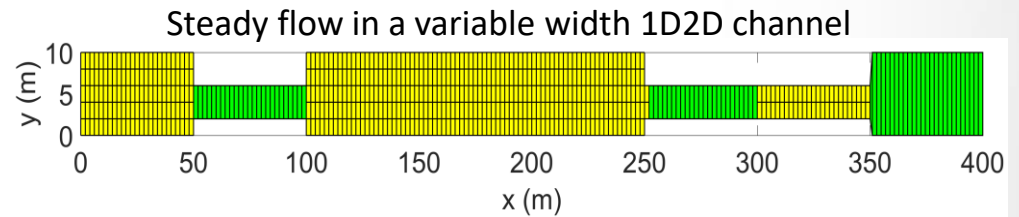
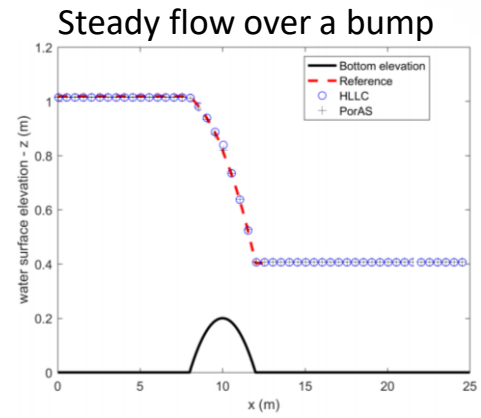
- $\mathbf{F}_j^n = \mathcal{P}([h_L \ q_L \ r_L]^T, [h_R \ q_R \ r_R]^T)$
- $\mathbf{F}_{j,u}^n = \mathcal{P}([h_L \ 0 \ 0]^T, [h_R \ 0 \ 0]^T)$
- $\mathbf{F}_{j,v}^n = \mathcal{P}([h_L \ q_L/\phi_L \ r_L/\phi_L]^T, [h_R \ q_R/\phi_R \ r_R/\phi_R]^T)$

SWE variables
h: water depth
q: x unit-discharge
r: y unit-discharge

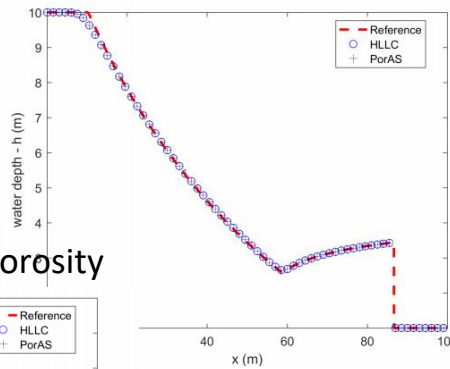


Flood1D2D

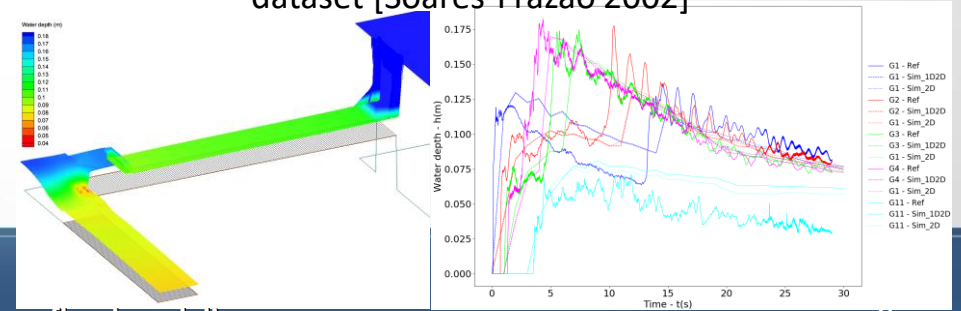
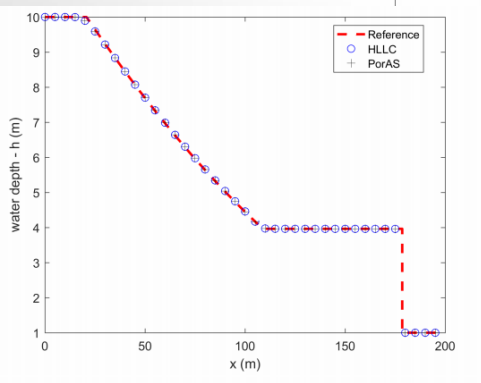
- C/C++ software
- Validated on various test cases [Chen 2018 PhD]
 - Academic
 - Experimental



Dambreak with porosity

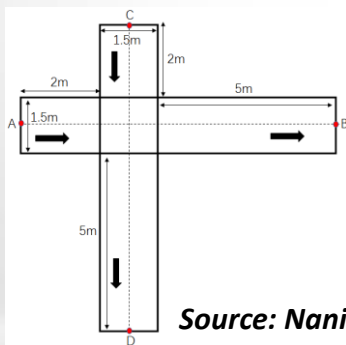
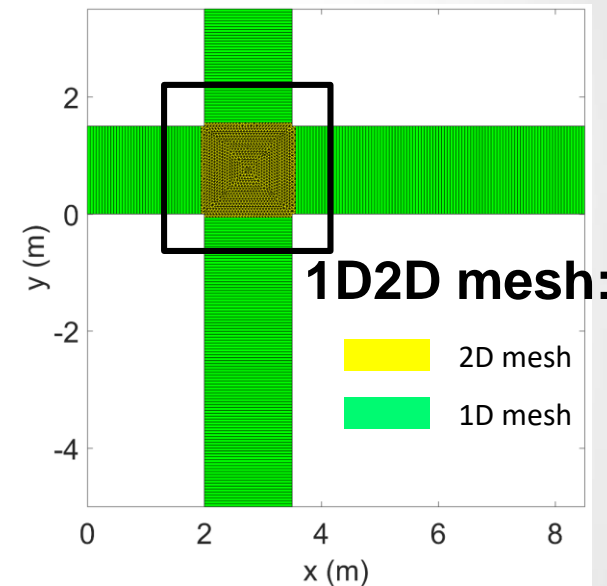


Dambreak without porosity

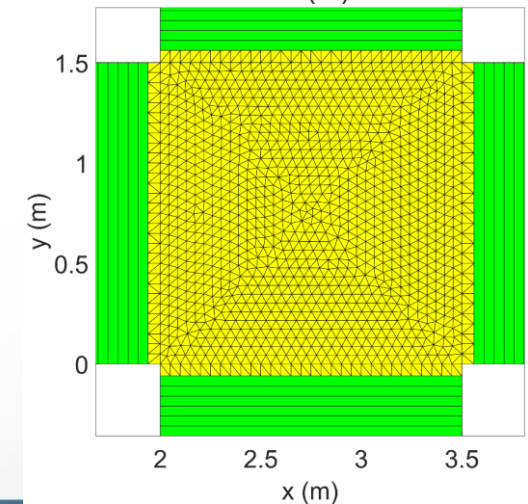
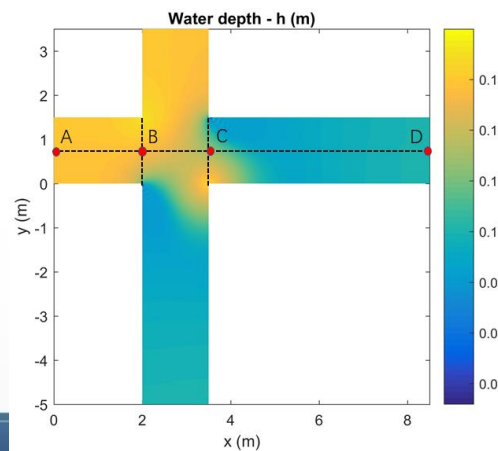


Flood1D2D - Local scale validation

- Four branch junction flow [*Nania et al., 2011*]
 - $Q_{up,x} = Q_{up,y} = 75L/s$
 - Downstream water depth $h = 0,1m$
 - Friction : $K = 100 m^{1/3}/s$
- Simulations:
 - Reference = Telemac 2D
 - Flood1D2D parameterized with a supposed porosity profile

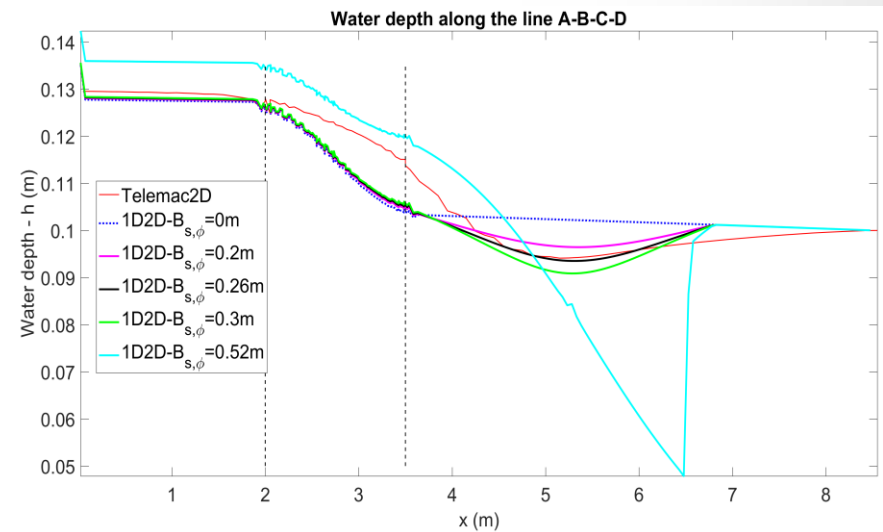


Source: Nania et al. 2011



Flood1D2D - Local scale validation

- Water depth profile inflexion in flow throat (due to recirculation) not predicted by friction parameterized SWE
- Adding a porosity parameterization allows to reproduce the water depth profile in the downstream street
- Porosity estimated from 2D model (recirculation width) leads to incorrect 1D/2D flowlines
 - Different modeled physics (friction effect, turbulence, ...)
 - Potentially equifinality problem between friction and porosity



New 1D2D shallow water model with porosity like parameterization able to reproduce complex 2D flow features.

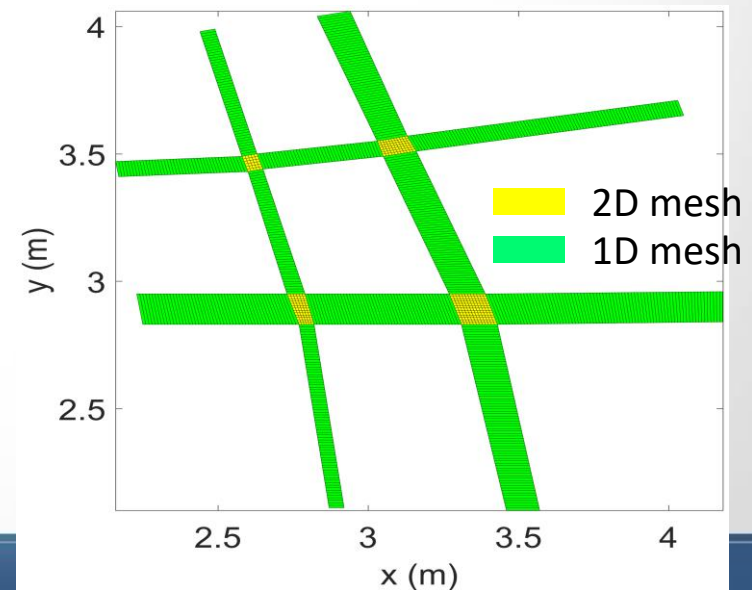
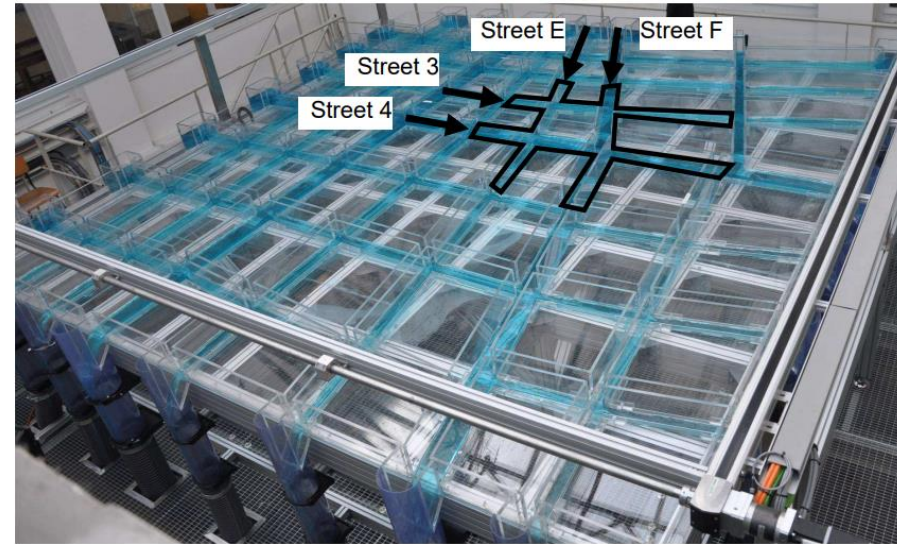
Flood1D2D - Global scale validation

Icube urban flood experimental rig

- Water depth profile
- Street discharge repartition

Simulation with Flood1D2D

- Boundary conditions
 - Upstream : Q_{exp}
 - Downstream: $h(Q)$
- Run 1 : K_{cross} and K_{street}
 - $K_{cross} \in [25, 50, 75, 100, 125, 150]$
 - $K_{street} \in [25, 50, 75, 100, 125, 150]$
- Run 2 : K , ϕ_{large} and ϕ_{narrow}
 - $K = 75m^{1/3}/s$
 - $\phi_{large} \in [0.25, 0.50, 0.75, 1.00]$
 - $\phi_{narrow} \in [0.25, 0.50, 0.75, 1.00]$



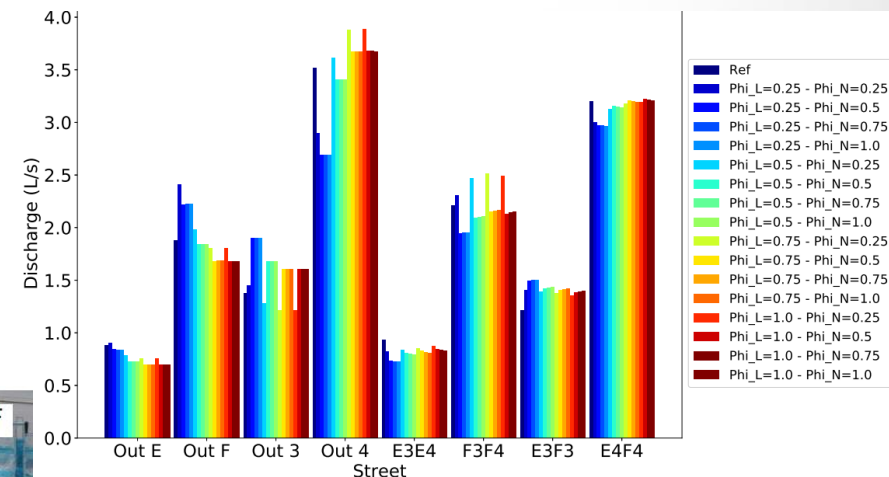
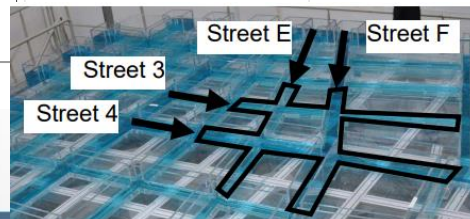
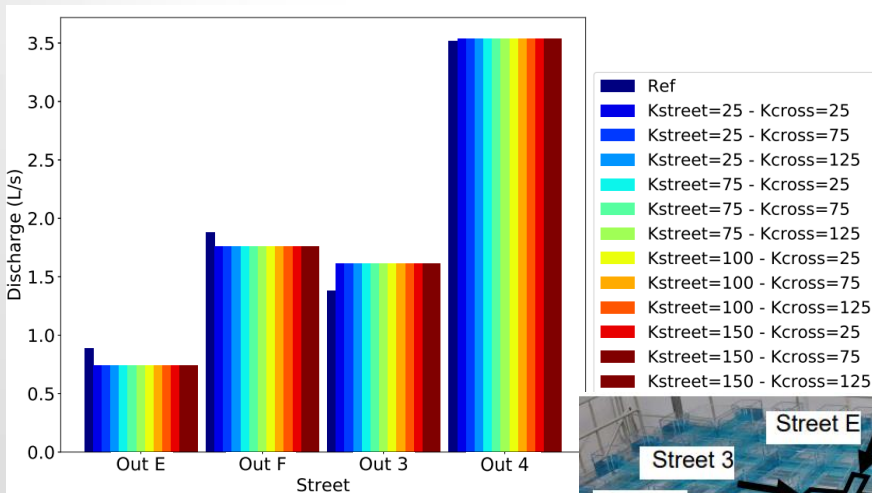
Flood1D2D - Global scale validation

Run 1 : K_{cross} and K_{street}

- Downstream discharge insensitive to friction coefficient
- Over or underestimation of the experimental discharge

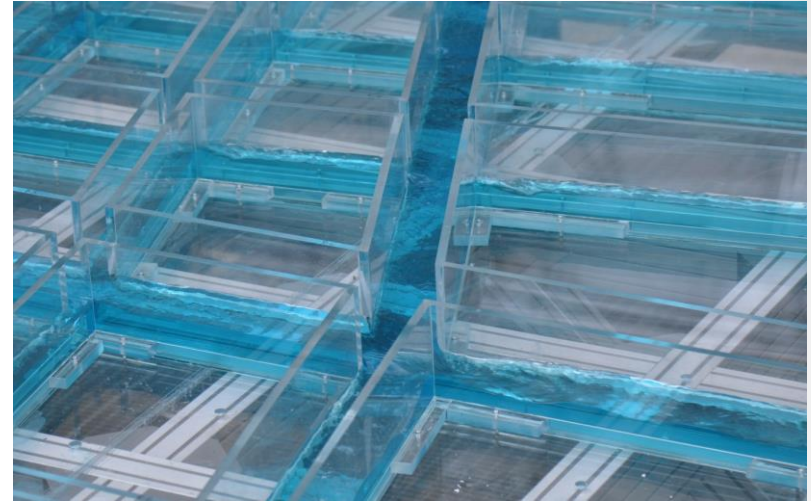
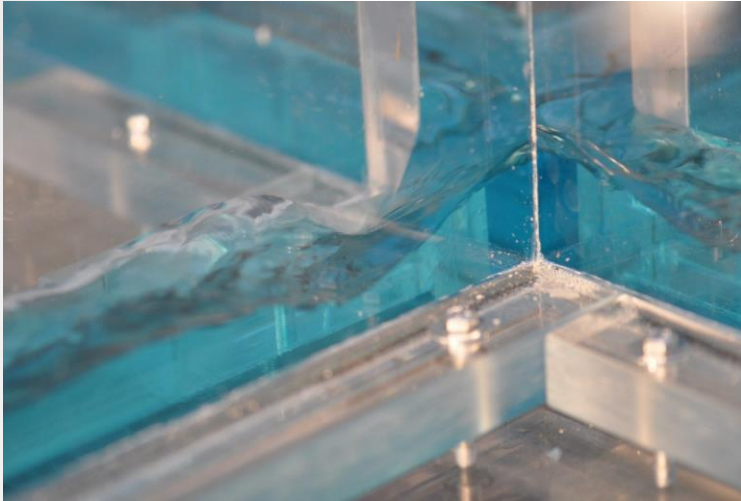
Run 2 : K , ϕ_{large} and ϕ_{narrow}

- Sensitivity of the street/outlet discharge to the porosity
- Allows to surround the experimental value

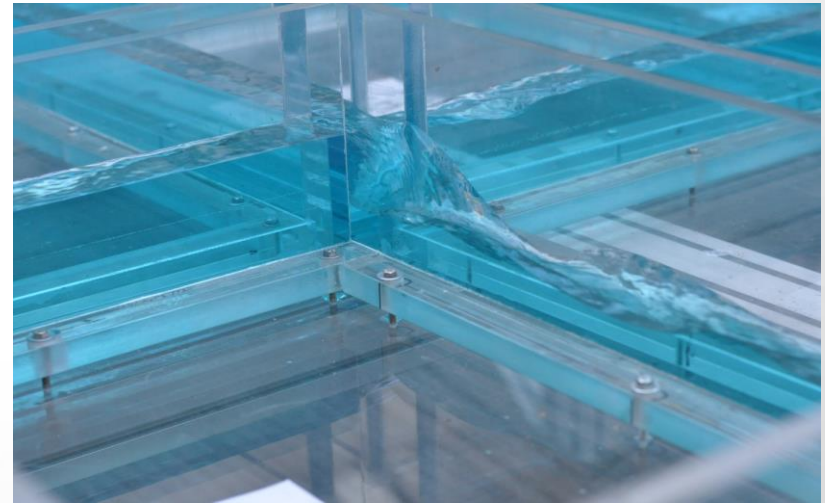
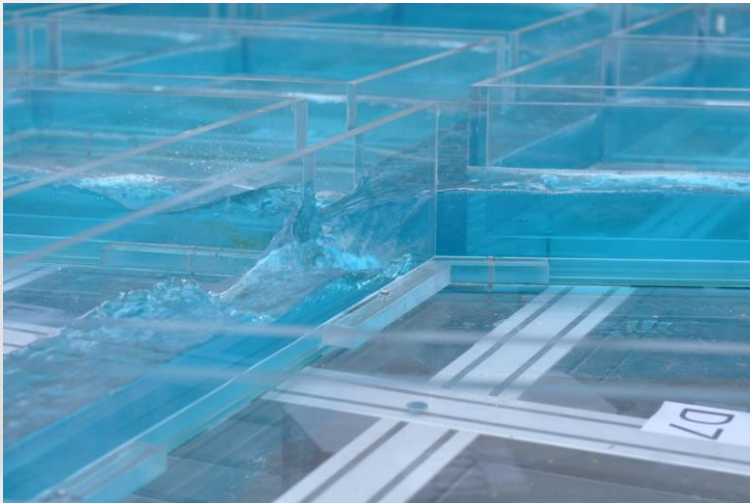


Conclusion

- Flood1D2D - New modeling paradigm using SW equations and porosity like approach proposed for urban flood flows
- Predictive capabilities and accuracy demonstrated on various steady and unsteady test cases
- Ongoing studies:
 - Application of the model to larger scales
 - Calibration method based on flow features



Thanks for your attention



Bibliography

- Chen, 2018. Modèle effectif par une approche de Saint-Venant pour les écoulements complexes lors d'inondations urbaines. PhD Thesis, Université de Strasbourg.
- Creelle et al., 2016. Modelling if the tributary momentum contribution to predict confluence head losses. JHR 1-15.
- Finaud-Guyot, 2009. Modélisation macroscopique des inondations fluviales et urbaines. PhD Thesis, Université Montpellier 2.
- Finaud-Guyot et al., 2010. An approximate-state Riemann solver for the two-dimensional shallow water equations with porosity. IJNMF 62(12), 1299-1331.
- Finaud-Guyot et al., 2018. Experimental insight for flood flow repartition in urban areas. UWJ, 1-9.
- Momplot et al., 2017. Typology of the flow structures in dividing open channel flows. JHR 55(1), 63-71.
- Nania et al., 2011. Experimental study of subcritical dividing flow in an equal-width four-branch junction. JHE 137(10), 1298-1305.
- Neary et al., 1999. Three-dimensional numerical model of lateral intake inflows. JHE 125(2), 126-140.
- Rivière et al., 2011. Subcritical open channel flows in four branch intersections. WRR 47(10).
- Soares-Frazaio et Zech, 2002. Dambreak in channels with 90° bend. JHE 128(11), 956-968.