A Kirchhoff Plate Formulation in a State-Based Peridynamic Framework

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Abstract

In recent years, there has been rapid progress on peridynamics. It has been applied to many different material systems, used for coupled field analysis and it is suitable for multi-scale analysis. This study mainly focuses on peridynamic analysis for plate-type structures. For this purpose, a new peridynamic Kirchhoff plate is developed. The new formulation is computationally efficient by having only one degree of freedom for each material point. Moreover, it is based on state-based peridynamic formulation which doesn't impose any limitation on material constants. After presenting how to impose simply supported and clamped boundary conditions in this new formulation, several numerical studies are considered to demonstrate the accuracy and capability of the proposed formulation.

1. Introduction

Peridynamic (PD) theory was introduced by Silling (2000) as an alternative formulation to Classical Continuum Mechanics (CCM). Instead of expressing equations of motion in partial differential equation form as in CCM, peridynamic equations of motion are expressed in integro-differential equation form. Moreover, peridynamic equations do not contain any spatial derivatives which offer certain advantages especially for the solution of problems including displacement discontinuities due to the existence of cracks. Besides, in peridynamics, the state of a material point is influenced by material points which are located at a finite distance within a domain of influence called as horizon. This feature positions peridynamics within non-local continuum mechanics formulations. As highlighted in dell'Isola et al. (2015), the origins of peridynamics go back to Piola. Since its introduction, there has been rapid progress on peridynamics. The PD formulation has been applied to many different material systems including linear elastic materials, metals and composite materials (Oterkus and Madenci, 2012). The PD theory is not limited to macroscopic analysis which allows researchers to use it for analyzing problems at mesoscale (De Meo et al., 2016) and nano-scale (Ebrahimi et. al., 2015). Moreover, it is currently possible to perform multi-physics analysis in a single peridynamic framework by coupling mechanical field to thermal field (Oterkus et. al., 2014a), moisture diffusion (Oterkus et. al., 2014b), electric current (Gerstle et. al. 2008), porous flow (Oterkus et al., 2017), etc. PD theory has also been effectively used for impact analysis (Amani et al., 2016). An in-depth review of PD research is given in Madenci and Oterkus (2014) and Javili et al. (2018).

This study mainly focuses on the application of PD theory for the analysis of plate-type structures. Several examples of such type of analysis currently exist in the literature. Amongst these, Taylor and Steigmann (2015) developed a PD formulation suitable for thin plates. O'Grady and Foster (2014) introduced a non-ordinary state-based Kirchhoff-Love plate

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formulation suitable for modelling thin plates using rotational springs between peridynamic bonds. Diyaroglu et al. (2015) developed a bond-based PD Mindlin plate formulation which takes into account the effect of shear deformation and is suitable for the analysis of thick plates.

In this study, a new PD Kirchhoff plate formulation is presented. This new formulation has certain advantages. First of all, each material point has only one degree of freedom, i.e. transverse deflection, as opposed to three-degree of freedom used in Mindlin plate formulation. This yields significant computational time and memory reduction in computations. Moreover, the formulation is based on PD state-based concept. Therefore, it doesn't impose any limitation on material constants. The paper starts with a derivation of the equation of motion for the new PD Kirchhoff plate formulation. It then presents how to apply simply supported and clamped boundary conditions in the current formulation. Finally, several numerical cases are demonstrated to show the accuracy and capability of the proposed formulation.

2. Kinematics of Kirchhoff plate

Unlike the local continuum theory, in PD theory, each state of a material point is not only influenced by the material points located in its immediate vicinity but also influenced by material points which are located within a region of finite radius named as "horizon", *H*. Therefore, the general PD equations of motion for material point $\mathbf{x}_{(k)}$ can be expressed in summation form as (Madenci and Oterkus, 2014)

$$\rho_{(k)} \frac{\partial^2 \mathbf{u}_{(k)}}{\partial t^2} = \sum_{j=1}^{N_j} \mathbf{f}_{(k)(j)} V_{(j)} + \mathbf{b}_{(k)}$$
(1)

where the summation takes over the family member material points of k and N_k indicates the total number of family members, ρ , **u**, t, V and **b** represent the density, displacement, time, material point volume and the body load vector, respectively. The interaction force vector, $\mathbf{f}_{(k)(j)}$, between material points k and j has a unit of "force per unit volume squared" defined as (Madenci and Oterkus, 2014)

$$\mathbf{f}_{(k)(j)} = \mathbf{t}_{(k)(j)} - \mathbf{t}_{(j)(k)}$$
(2)

As shown in Fig. (1), the PD force density vector $\mathbf{t}_{(k)(j)}$ represents the force acting on the main material point k by its family member material point j, and, on the contrary, $\mathbf{t}_{(j)(k)}$ represents the force acting on material point j by its family member material point, k. They are also related to strain energy density function, $W_{(k)}$, as (Madenci and Oterkus, 2014)

$$\mathbf{t}_{(k)(j)} = \frac{1}{V_{(j)}} \frac{\partial W_{(k)}}{\partial \left(\mathbf{u}_{(j)} - \mathbf{u}_{(k)}\right)}$$
(3a)

and



Figure 1. Peridynamic interaction force vectors.

Kirchhoff plate theory is based on the assumption that "normals to the mid-surface of the undeformed plate remain straight and normal to the mid-surface, and unstretched in length, during deformation." To derive PD force densities of Kirchhoff plate theory (Fig. 2), the strain energy densities of material points k and j are expressed as (Leissa and Qatu, 2011)

$$W_{(k)} = \frac{1}{2} \frac{D}{h} \left[\left(\kappa_x^{(k)} + \kappa_y^{(k)} \right)^2 + 2 \left(1 - \nu \right) \left(\left(\kappa_{xy}^{(k)} \right)^2 - \kappa_x^{(k)} \kappa_y^{(k)} \right) \right]$$
(4a)

and

$$W_{(j)} = \frac{1}{2} \frac{D}{h} \left[\left(\kappa_x^{(j)} + \kappa_y^{(j)} \right)^2 + 2 \left(1 - \nu \right) \left(\left(\kappa_{xy}^{(j)} \right)^2 - \kappa_x^{(j)} \kappa_y^{(j)} \right) \right]$$
(4b)

where $D = Eh^3/12(1-v^2)$ is the flexural rigidity, v is the Poisson's ratio and h is the thickness of the plate. The linearized curvatures and twist of the plate mid-surface, κ_x , κ_y and κ_{xy} , are expressed as

$$\kappa_x^{(k,j)} = \frac{\partial^2 w_{(k,j)}}{\partial x^2}, \quad \kappa_y^{(k,j)} = \frac{\partial^2 w_{(k,j)}}{\partial y^2} \quad \text{and} \quad \kappa_{xy}^{(k,j)} = \frac{\partial^2 w_{(k,j)}}{\partial x \partial y}$$
(5)

in which w is the vertical direction displacement component to the plate surface. Substituting Eqs. (4a,b) into Eq. (3), the force densities, $\mathbf{t}_{(k)(j)}$ and $\mathbf{t}_{(j)(k)}$, can be expressed in terms of curvatures and twist as

$$\mathbf{t}_{(k)(j)} = \frac{D}{2hV_{(j)}} \left[\frac{\partial \left(\kappa_x^{(k)} + \kappa_y^{(k)} \right)^2}{\partial \left(w_{(k)} - w_{(j)} \right)} + 2(1-\nu) \frac{\partial \left(\left(\kappa_{xy}^{(k)} \right)^2 - \kappa_x^{(k)} \kappa_y^{(k)} \right)}{\partial \left(w_{(k)} - w_{(j)} \right)} \right]$$
(6a)

and

$$\mathbf{t}_{(j)(k)} = \frac{D}{2hV_{(k)}} \left[\frac{\partial \left(\kappa_x^{(j)} + \kappa_y^{(j)} \right)^2}{\partial \left(w_{(j)} - w_{(k)} \right)} + 2\left(1 - \nu\right) \frac{\partial \left(\left(\kappa_{xy}^{(j)} \right)^2 - \kappa_x^{(j)} \kappa_y^{(j)} \right)}{\partial \left(w_{(j)} - w_{(k)} \right)} \right]$$
(6b)

The PD form of linearized curvatures and twist can be derived for the material point **x** from Taylor series expansion by ignoring the higher-order terms, $O(x^3)$ as

$$w(\mathbf{x} + \boldsymbol{\xi}) = w(\mathbf{x}) + \xi_x \frac{\partial w(\mathbf{x})}{\partial x} + \xi_y \frac{\partial w(\mathbf{x})}{\partial y} + \frac{1}{2} \xi_x^2 \frac{\partial^2 w(\mathbf{x})}{\partial x^2} + \frac{1}{2} \xi_y^2 \frac{\partial^2 w(\mathbf{x})}{\partial y^2} + \xi_x \xi_y \frac{\partial^2 w(\mathbf{x})}{\partial x \partial y} + O(x^3)$$
(7)

where ξ_x and ξ_y are the projections of reference length $\xi = |\mathbf{x}_{(j)} - \mathbf{x}_{(k)}|$ between material points *k* and *j* along *x*- and *y*- axes, respectively, and they are defined as

$$\xi_x = \xi \cos(\varphi) \tag{8a}$$

and

$$\xi_{y} = \xi \sin(\varphi) \tag{8b}$$

Note that φ represents the angle between the peridynamic interaction and x-axis as shown in Fig. 2. Substituting Eq. (8) into Eq. (7) and performing some algebraic manipulations result in

$$\frac{w(\mathbf{x}+\xi)-w(\mathbf{x})}{\xi^{2}} = \frac{\partial w(\mathbf{x})}{\partial x}\frac{\cos(\varphi)}{\xi} + \frac{\partial w(\mathbf{x})}{\partial y}\frac{\sin(\varphi)}{\xi} + \frac{1}{2}\frac{\partial^{2}w(\mathbf{x})}{\partial x^{2}}\cos^{2}(\varphi) + \frac{1}{2}\frac{\partial^{2}w(\mathbf{x})}{\partial y^{2}}\sin^{2}(\varphi) + \frac{\partial^{2}w(\mathbf{x})}{\partial x\partial y}\cos(\varphi)\sin(\varphi)$$
(9)

As explained in Appendix A, if each term in Eq. (9) is multiplied with trigonometric functions of $g_i(\varphi)$ while integrating over the circular horizon of δ the differentiations in Eq. (9) can be expressed in terms of integrations as

$$\frac{\partial^2 w(\mathbf{x})}{\partial x^2} + \frac{\partial^2 w(\mathbf{x})}{\partial y^2} = \frac{4}{\pi \delta^2} \int_0^{2\pi} \int_0^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} \xi d\xi d\varphi$$
(10a)

$$\frac{\partial^2 w(\mathbf{x})}{\partial x \partial y} = \frac{4}{\pi \delta^2} \int_0^{2\pi} \int_0^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} 2\cos(\varphi) \sin(\varphi) \xi d\xi d\varphi$$
(10b)

$$\frac{\partial^2 w(\mathbf{x})}{\partial x^2} = \frac{2}{\pi \delta^2} \left[\int_{0}^{2\pi} \int_{0}^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} \xi d\xi d\varphi + \int_{0}^{2\pi} \int_{0}^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} 2\cos(2\varphi) \xi d\xi d\varphi \right]$$
(10c)

$$\frac{\partial^2 w(\mathbf{x})}{\partial y^2} = \frac{2}{\pi \delta^2} \left[\int_0^{2\pi} \int_0^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} \xi d\xi d\varphi - \int_0^{2\pi} \int_0^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} 2\cos(2\varphi) \xi d\xi d\varphi \right]$$
(10d)

Utilizing Eqs. (10a-d) for material points k and j and discretizing the integral equations, the curvatures and twist can be defined as

$$\kappa_x^{(k)} + \kappa_y^{(k)} = \frac{4}{\pi \delta^2 h} \sum_{i^k=1}^{N_{i^k}} \frac{w_{(i^k)} - w_{(k)}}{\xi_{(i^k)^{(k)}}^2} V_{(i^k)}$$
(11a)

$$\kappa_{xy}^{(k)} = \frac{4}{\pi \delta^2 h} \sum_{i^k=1}^{N_{i^k}} \frac{w_{(i^k)} - w_{(k)}}{\xi_{(i^k)(k)}^2} 2\cos\left(\varphi_{(i^k)(k)}\right) \sin\left(\varphi_{(i^k)(k)}\right) V_{(i^k)}$$
(11b)

$$\kappa_{x}^{(k)} = \frac{2}{\pi\delta^{2}h} \left[\sum_{i^{k}=1}^{N_{i^{k}}} \frac{w_{(i^{k})} - w_{(k)}}{\xi_{(i^{k})(k)}^{2}} V_{(i^{k})} + 2\sum_{i^{k}=1}^{N_{i^{k}}} \frac{w_{(i^{k})} - w_{(k)}}{\xi_{(i^{k})(k)}^{2}} \cos\left(2\varphi_{(i^{k})(k)}\right) V_{(i^{k})} \right]$$
(11c)

$$\kappa_{y}^{(k)} = \frac{2}{\pi\delta^{2}h} \left[\sum_{i^{k}=1}^{N_{i^{k}}} \frac{w_{(i^{k})} - w_{(k)}}{\xi_{(i^{k})(k)}^{2}} V_{(i^{k})} - 2\sum_{i^{k}=1}^{N_{i^{k}}} \frac{w_{(i^{k})} - w_{(k)}}{\xi_{(i^{k})(k)}^{2}} \cos\left(2\varphi_{(i^{k})(k)}\right) V_{(i^{k})} \right]$$
(11d)

and they are for material point j

$$\kappa_x^{(j)} + \kappa_y^{(j)} = \frac{4}{\pi \delta^2 h} \sum_{i^j=1}^{N_{i^j}} \frac{w_{(i^j)} - w_{(j)}}{\xi_{(i^j)(j)}^2} V_{(i^j)}$$
(12a)

$$\kappa_{xy}^{(j)} = \frac{4}{\pi\delta^2 h} \sum_{i^j=1}^{N_{i^j}} \frac{w_{(i^j)} - w_{(j)}}{\xi_{(i^j)(j)}^2} 2\cos\left(\varphi_{(i^j)(j)}\right) \sin\left(\varphi_{(i^j)(j)}\right) V_{(i^j)}$$
(12b)

$$\kappa_{x}^{(j)} = \frac{2}{\pi\delta^{2}h} \left[\sum_{i^{j}=1}^{N_{i^{j}}} \frac{w_{(i^{j})} - w_{(j)}}{\xi_{(i^{j})(j)}^{2}} V_{(i^{j})} + 2\sum_{i^{j}=1}^{N_{i^{j}}} \frac{w_{(i^{j})} - w_{(j)}}{\xi_{(i^{j})(j)}^{2}} \cos\left(2\varphi_{(i^{j})(j)}\right) V_{(i^{j})} \right]$$
(12c)

$$\kappa_{y}^{(j)} = \frac{2}{\pi\delta^{2}h} \left[\sum_{i^{j}=1}^{N_{ij}} \frac{w_{(i^{j})} - w_{(j)}}{\xi_{(i^{j})(j)}^{2}} V_{(i^{j})} - 2\sum_{i^{j}=1}^{N_{ij}} \frac{w_{(i^{j})} - w_{(j)}}{\xi_{(i^{j})(j)}^{2}} \cos\left(2\varphi_{(i^{j})(j)}\right) V_{(i^{j})} \right]$$
(12d)

where the summation indices i^k and i^j represent the material points inside the horizon of the main material point k and its family member point j, respectively. Substituting Eqs. (11) and (12) into Eqs. (6a,b), and performing some algebraic manipulations, the PD force densities can be expressed in terms of displacements, w as

$$\mathbf{t}_{(k)(j)} = \frac{2D}{h} \left(\frac{2}{\delta^2 \pi h}\right)^2 \frac{1}{\xi_{(j)(k)}^2} \sum_{i^k=1}^{N_{j^k}} \frac{w_{(i^k)} - w_{(k)}}{\xi_{(i^k)(k)}^2} \left[2 + (1 - \nu) \left(8 \cos^2\left(\varphi_{(i^k)(k)} - \varphi_{(j)(k)}\right) - 5\right)\right] V_{(i^k)}$$
(13a)

and

$$\mathbf{t}_{(j)(k)} = \frac{2D}{h} \left(\frac{2}{\delta^2 \pi h}\right)^2 \frac{1}{\xi_{(j)(k)}^2} \sum_{i^j=1}^{N_{i^j}} \frac{w_{(i^j)} - w_{(j)}}{\xi_{(i^j)(j)}^2} \left[2 + (1 - \nu) \left(8 \cos^2\left(\varphi_{(i^j)(j)} - \varphi_{(j)(k)}\right) - 5\right)\right] V_{(i^j)}$$
(13b)

Substituting force density vectors $\mathbf{t}_{(k)(j)}$ and $\mathbf{t}_{(j)(k)}$ (Eq. 13) into interaction force vector, $\mathbf{f}_{(k)(j)}$ (Eq. 2), the PD equation of motion (EOM) of Kirchhoff plate theory can be derived from Eq. (1) as

$$\rho_{(k)} \frac{\partial^{2} w_{(k)}}{\partial t^{2}} = \frac{8D}{\pi^{2} \delta^{4} h^{3}} \sum_{j=1}^{N_{j}} \frac{1}{\xi_{(j)(k)}^{2}} \left[\sum_{i^{k}=1}^{N_{j^{k}}} \frac{w_{(i^{k})} - w_{(k)}}{\xi_{(i^{k})(k)}^{2}} \left[2 + (1 - \nu) \left(8 \cos^{2} \left(\varphi_{(i^{k})(k)} - \varphi_{(j)(k)} \right) - 5 \right) \right] V_{(i^{k})} \right] + \mathbf{b}_{(k)}$$

$$-\sum_{i^{j}=1}^{N_{j^{j}}} \frac{w_{(i^{j})} - w_{(j)}}{\xi_{(i^{j})(j)}^{2}} \left[2 + (1 - \nu) \left(8 \cos^{2} \left(\varphi_{(i^{j})(j)} - \varphi_{(j)(k)} \right) - 5 \right) \right] V_{(i^{j})} \right] + \mathbf{b}_{(k)}$$
Original Configuration



Figure 2. PD interactions in Kirchhoff plate theory.

3. Boundary conditions

The prerequisite of the derivation process of PD EOM is that the material points' influence domain must be completely embedded in a material domain. For this reason, the PD EOM derived in Eq. (14) is valid only if the main material point, k and its family member, j have intact horizons which are fully embedded in an actual material domain, R, as shown in Fig. 3. However, near boundary points, k and j, can have incomplete influence domains. Therefore, the supplemental equations valid for the fictitious boundary layer, R_c , outside the boundary of the actual material domain, are necessary as explained in Madenci and Oterkus (2014). The width of this layer can be chosen as the double size of the horizon. The possible types of boundary conditions and their corrections are explained below for Kirchhoff plate theory from PD point of view.



Figure 3. The truncated horizons of k and j and the introduction of fictitious boundary layer

3.1 The clamped boundary condition

To implement clamped boundary condition (Fig. 4), a fictitious boundary layer is created outside the actual material domain. The horizon size can be chosen as $\delta = 3\Delta x$ in which the discretization size is Δx . This horizon size is sufficient enough to represent macro-scale displacements in plate problems (Diyaroglu et al, 2014). Hence, the width of the fictitious region is chosen as 2δ .

The clamped boundary condition for the vertical edges along the *y*-axis is obtained by imposing zero displacements and zero rotation for the material points adjacent to the clamped end as

$$w(x_0) = 0$$
 and $\frac{\partial w(x_0)}{\partial x} = 0$ (15)

in which x_0 is on the boundary line. Therefore, as shown in Fig. 4, the displacements of the material points close to the boundary line can be assumed as zero:

$$w_{(k)(1)} = w_{(k)(-1)} \approx 0$$
 for $k = 1, 2, ... n$ (16)

where the first subscript represents the number of rows along the *y*-direction and the second subscript is for the number of columns along the *x*-direction. The displacement field of material points near the boundary region are necessarily imposed to be symmetric to the clamped boundary as

$$w_{(k)(-i)} = w_{(k)(i)}$$
 for $i = 2, 3, ...6$ (17)



Figure 4. Clamped boundary condition.

Similarly, the horizontal direction edges parallel to the x-axis has constraint conditions of

$$w(y_0) = 0$$
 and $\frac{\partial w(y_0)}{\partial x} = 0$ (18)

in which y_0 is on the horizontal boundary line. The corresponding PD boundary conditions can be obtained as in Eqs. (16) and (17).

3.2. The simply supported boundary condition

To implement simply supported boundary condition, the fictitious boundary layer is again chosen to be equal to 2δ . The vertical direction boundaries (along the y-axis) are imposed to have zero displacements and curvatures as

$$w(x_0) = 0$$
 and $\frac{\partial^2 w(x_0)}{\partial x^2} = 0$ (19)

In the discretized form of PD formulation, these conditions can be achieved by enforcing antisymmetrical displacement fields to the material points in the fictitious region as opposed to the actual displacement fields as shown in Fig. 5. Thus, it is defined as

$$w_{(k)(-i)} = -w_{(k)(i)}$$
 for $i = 1, 2, ...6$ and $k = 1, 2, ...n$ (20)



Figure 5. Simply supported boundary condition. Similarly, the horizontal direction edges have the constraints of

$$w(y_0) = 0$$
 and $\frac{\partial^2 w(y_0)}{\partial y^2} = 0$ (21)

and in discretized form these conditions take a similar form as in Eq. (20).

4. Numerical Validation

To verify the validity of new PD formulation for a Kirchhoff plate, the PD solutions are compared with the corresponding finite element (FE) analysis results.

4.1. Clamped Plate

A clamped plate with a length and width of L = W = 1 m and a thickness of h = 0.01 m is considered as shown in Fig. 6. The Young's modulus and Poisson's ratio of the plate are E = 200 GPa and v = 0.3, respectively. The model is discretized into one single row of material points along with the thickness and the distance between material points is $\Delta x = 0.01$ m. A fictitious region is introduced outside the edges as the external boundaries with a width of 2δ . The plate is subjected to a distributed transverse load of p = 100 N/m through

the y-center line. The line load is converted to a body load of $b = \frac{pW}{2(W / \Delta x)\Delta V} = 5 \times 10^5 \text{ N/m}^3$

and it is distributed to two columns of material volumes through the center line as shown in Fig. 7.



Figure 6. The geometry of a clamped Kirchhoff plate.



Figure 7. The PD discretization of a clamped plate.



Figure 8. The comparison of the vertical displacement components of (a) FE and (b) PD results (unit: m).



Figure 9. The comparison of the vertical displacements along a) x- and b) y- central axes

The FE model of the plate is created by using SHELL181 element in ANSYS. The PD and FE transverse displacement contours are compared in Fig. 8. They yield similar displacement variations. The maximum difference between PD and FE results is less than 0.5%. Moreover, the vertical displacement components along central *x*- and *y*- axes are compared in Fig. 9. These

results verify the accurateness of the current PD formulation for a Kirchhoff plate theory under clamped boundary conditions.

4.2. Simply Supported Plate

A simply supported plate (Fig. 10) has the same geometrical and material properties as of the clamped plate case. Again, it is discretized with a single row of material points along the thickness direction and the discretization size is $\Delta x = 0.01$ m. A fictitious region is created outside the region of boundaries and its width is equal to two times the size of the horizon, δ . The plate is subjected to a distributed transverse line load of p = 100 N/m through the y-central

line. It is imposed to two columns of material points with a body load of $b = 5 \times 10^5$ N/m³ as in Fig. 7.



Figure 10. The geometry of a simply supported Kirchhoff plate.



Figure 11. The comparison of vertical displacement components of (a) FE and (b) PD results (unit: m).



Figure 12. The comparison of vertical displacements along a) x- and b) y- central axes.

The transverse displacement components of FE and PD theory show very close variations as shown in Fig. 11. The maximum difference between PD and FE results is less than 0.5%. Furthermore, the displacement variations along central *x*- and *y*- axes are on top of each other for FE and PD results as shown in Fig. 12. This is to confirm the current PD formulation of Kirchhoff plate theory under simply supported boundary conditions.

5. Final remarks

This study presents a new state-based peridynamic formulation for Kirchhoff plate theory. The constitutive equation is obtained by utilizing the strain energy density of a material point in the form of curvatures and twist. Taylor series expansion up to the order of two is utilized to obtain PD form of strain energy density function. Due to the nonlocal characteristic of peridynamic theory, the boundary condition implementation needs extra care. Thus, implementation of two different type boundary conditions, clamped and simply supported, in the current formulation is presented. Two different numerical cases incorporating such boundary conditions are considered and very good agreement is observed between peridynamic and finite element analysis results.

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Appendix

The Taylor Series expansion up to the second order for the deflection $w(\mathbf{x})$ can be written as

$$w(\mathbf{x} + \xi) = w(\mathbf{x}) + \frac{\partial w(\mathbf{x})}{\partial x} \xi \cos(\varphi) + \frac{\partial w(\mathbf{x})}{\partial y} \xi \sin(\varphi) + \frac{1}{2} \frac{\partial^2 w(\mathbf{x})}{\partial x^2} \xi^2 \cos^2(\varphi) + \frac{1}{2} \frac{\partial^2 w(\mathbf{x})}{\partial y^2} \xi^2 \sin^2(\varphi) + \frac{\partial^2 w(\mathbf{x})}{\partial x \partial y} \xi^2 \cos(\varphi) \sin(\varphi)$$
(A.1a)

and rearranging its terms yield

$$\frac{w(\mathbf{x}+\xi)-w(\mathbf{x})}{\xi^2} = \frac{\partial w(\mathbf{x})}{\partial x} \frac{\cos(\varphi)}{\xi} + \frac{\partial w(\mathbf{x})}{\partial y} \frac{\sin(\varphi)}{\xi} + \frac{1}{2} \frac{\partial^2 w(\mathbf{x})}{\partial x^2} \cos^2(\varphi) + \frac{1}{2} \frac{\partial^2 w(\mathbf{x})}{\partial y^2} \sin^2(\varphi) + \frac{\partial^2 w(\mathbf{x})}{\partial x \partial y} \cos(\varphi) \sin(\varphi)$$
(A.1b)

Considering material point **x** as fixed, multiplying each term in Eq. (A.1) with trigonometric functions $g_i(\varphi)$ with i = 1, 2, ... and integrating over a circular horizon with a radius δ results in

$$\int_{0}^{2\pi} \int_{0}^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^{2}} g_{i}(\varphi) \xi d\xi d\varphi = \frac{\partial w(\mathbf{x})}{\partial x} \int_{0}^{2\pi} \int_{0}^{\delta} \cos(\varphi) g_{i}(\varphi) d\xi d\varphi + \frac{\partial w(\mathbf{x})}{\partial y} \int_{0}^{2\pi} \int_{0}^{\delta} \sin(\varphi) g_{i}(\varphi) d\xi d\varphi + \frac{1}{2} \frac{\partial^{2} w(\mathbf{x})}{\partial x^{2}} \int_{0}^{2\pi} \int_{0}^{\delta} \cos^{2}(\varphi) g_{i}(\varphi) \xi d\xi d\varphi + \frac{1}{2} \frac{\partial^{2} w(\mathbf{x})}{\partial x^{2}} \int_{0}^{2\pi} \int_{0}^{\delta} \cos(\varphi) g_{i}(\varphi) \xi d\xi d\varphi + \frac{1}{2} \frac{\partial^{2} w(\mathbf{x})}{\partial x \partial y} \int_{0}^{2\pi} \int_{0}^{\delta} \cos(\varphi) \sin(\varphi) g_{i}(\varphi) \xi d\xi d\varphi$$
(A.2)

where $g_i(\varphi)$ should be constructed based on orthogonality condition. By defining $g_1(\varphi) = 1$ and substituting into Eq. (A.2), the Laplacian term can be obtained as

$$\nabla^2 w(\mathbf{x}) = \frac{\partial^2 w(\mathbf{x})}{\partial x^2} + \frac{\partial^2 w(\mathbf{x})}{\partial y^2} = \frac{4}{\pi \delta^2} \int_0^{2\pi} \int_0^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} \xi d\xi d\phi$$
(A.3)

Defining $g_2(\varphi) = \cos(\varphi)\sin(\varphi)$ and substituting into Eq. (A.2) results in

$$\frac{\partial^2 w(\mathbf{x})}{\partial x \partial y} = \frac{4}{\pi \delta^2} \int_0^{2\pi} \int_0^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} 2\cos(\varphi) \sin(\varphi) \xi d\xi d\varphi$$
(A.4)

Defining $g_3(\varphi) = \cos^2(\varphi)$ and $\sin^2(\varphi)$ and substituting into Eq. (A.2), respectively, result in

$$\int_{0}^{2\pi} \int_{0}^{\delta} \frac{w(\mathbf{x}+\xi)-w(\mathbf{x})}{\xi^{2}} \cos^{2}(\varphi)\xi d\xi d\varphi = \frac{3\pi\delta^{2}}{16} \frac{\partial^{2}w(\mathbf{x})}{\partial x^{2}} + \frac{\pi\delta^{2}}{16} \frac{\partial^{2}w(\mathbf{x})}{\partial y^{2}}$$
(A.5a)

$$\int_{0}^{2\pi} \int_{0}^{\delta} \frac{w(\mathbf{x}+\xi)-w(\mathbf{x})}{\xi^{2}} \sin^{2}(\varphi)\xi d\xi d\varphi = \frac{\pi\delta^{2}}{16} \frac{\partial^{2}w(\mathbf{x})}{\partial x^{2}} + \frac{3\pi\delta^{2}}{16} \frac{\partial^{2}w(\mathbf{x})}{\partial y^{2}}$$
(A.5b)

By solving Eqs. (A.5) simultaneously and performing some algebraic manipulations, the second order partial derivatives are obtained as

$$\frac{\partial^2 w(\mathbf{x})}{\partial x^2} = \frac{2}{\pi \delta^2} \left[\frac{\int_0^{2\pi} \int_0^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} \xi d\xi d\varphi}{+ \int_0^{2\pi} \int_0^{\delta} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} 2\cos(2\varphi) \xi d\xi d\varphi} \right]$$
(A.6a)

$$\frac{\partial^2 w(\mathbf{x})}{\partial y^2} = \frac{2}{\pi \delta^2} \begin{bmatrix} \sum_{0=0}^{2\pi} \frac{\delta}{0} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} \xi d\xi d\varphi \\ -\sum_{0=0}^{2\pi} \frac{\delta}{0} \frac{w(\mathbf{x} + \xi) - w(\mathbf{x})}{\xi^2} 2\cos(2\varphi) \xi d\xi d\varphi \end{bmatrix}$$
(A.6b)