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# OPSET Program for Computerized Selection of Watershed Parameter Values for the Stanford Watershed Model

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Research Report No. 34

OPSET  
PROGRAM FOR COMPUTERIZED SELECTION OF  
WATERSHED PARAMETER VALUES FOR  
THE STANFORD WATERSHED MODEL

Earnest Yuan-Shang Liou

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Part 1 of a completion report describing work supported in part by the Office of Water Resources Research, Department of the Interior, under provisions of Public Law 88-379, as Project Number C-1282 under Title II Research Grant No. 14-01-0001-1964.

Dr. L. Douglas James, Principal Investigator

University of Kentucky Water Resources Institute  
Lexington, Kentucky  
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## INTRODUCTION

Land use control in the tributary watershed as well as in the flood plain has been receiving increased attention as a method for reducing flood damage. One of the most complex technical questions which has to be resolved in structuring the appropriate use of this alternative is how downstream flood hazard varies with tributary watershed conditions. The approach to this problem in this research sponsored through the University of Kentucky Research Foundation and supported in part by funds provided by the United States Department of the Interior as authorized under Title II of the Water Resources Research Act of 1964, Public Law 88-379, revolved around using the Stanford Watershed Model as a tool for correlating runoff patterns with land use through model parameters as intermediate variables. The completion report for the project is in three parts.

1. Liou, Earnest Y. OPSET: Program for Computerized Selection of Watershed Parameter Values for the Stanford Watershed Model. Lexington: University of Kentucky Water Resources Institute, Research Report No. 34, 1970.

2. Ross, Glendon A. The Stanford Watershed Model: The Correlation of Parameter Values Selected by a Computerized Procedure with Measurable Physical Characteristics of the Watershed. Lexington: University of Kentucky Water Resources Institute, Research Report No. 35, 1970.

3. James, L. Douglas. An Evaluation of Relationships Between Streamflow Patterns and Watershed Characteristics Through Use of OPSET: A Self-Calibrating Version of the Stanford Watershed Model Lexington: University of Kentucky Water Resources Institute, Research Report No. 36, 1970.

The first of the reports describes the development of OPSET, a version of the Stanford Watershed Model programmed to estimate best-fit values of watershed parameters directly from climatological and streamflow data, and contains a program listing. The second report describes the application of OPSET to 17 rural watersheds and correlations derived between model parameters and watershed characteristics. It also describes and examines the significance of changes noted in parameter values with urbanization in three other watersheds. The third report applies the findings of the first two to flood control management problems. The results on all three levels have been highly encouraging. The three reports need to be read together for a complete understanding of the research approach.

The study is indebted to many besides the sponsors. Considerable use was made of the facilities of the Water Resources Institute and of the Computing Center at the University of Kentucky. Much of the data was obtained through A. B. Elam, Jr., Kentucky State Climatologist and the Louisville Office of the U.S. Geological Survey. Miss Nancy Crewe and Miss Patricia Miller prepared the reports.

## ABSTRACT

The advent of high-speed electronic computer made it possible to model complex hydrologic processes by mathematical expressions and thereby simulate streamflows from climatological data. The most widely used program is the Stanford Watershed Model, a digital parametric model of the land phase of the hydrologic cycle based on moisture accounting processes. It can be used to simulate annual or longer flow sequences at hourly time intervals. Due to its capability of simulating historical streamflows from recorded climatological data, it has a great potential in the planning and design of water resources systems. However, widespread use of the Stanford Watershed Model has been deterred by difficulties in understanding and finding a computer sufficiently large to run the bulky program. More important, the estimation of values for key parameters was both time-consuming and subjective as it had to be done by trial and error.

The objective of this study is to develop a computerized parameter optimization procedure, a self-calibrating watershed model, based on the FORTRAN version of the Stanford Watershed Model known as the Kentucky Watershed Model. This computerized procedure is named OPSET because its objective is to determine an optimum set of parameter values. The basic approach of OPSET is to match synthesized flows with recorded flows. The first step is by sensitivity studies to determine which key watershed parameters are sensitive in the simulation of flows and are difficult to measure or estimate directly. The second step is to devise a scheme for adjusting numerical estimates of the selected key parameters systematically improving flow simulation until the best possible matching is achieved

and to program this scheme into a streamlined Kentucky Watershed Model. Independent adjustment schemes are used for parameters associated with simulating runoff volumes, recession flows and flood hydrograph. The third step is to empirically test and improve this self-calibrating watershed model by applying it to a number of watersheds in Kentucky. OPSET estimates selected watershed parameters on a one water year basis, and the values of parameters best describing the watershed characteristics should be averaged from several OPSET-selected one-year-based values.

In applying OPSET to over 20 Kentucky watersheds which represent quite a wide range of topographic and soil conditions, this model was found to be rather successful. It is able to simulate streamflows and find more consistently estimated parameter values than the trial-and-error approach. The time spent on calibrating the watershed parameters is greatly reduced. The user does not have to spend so much time familiarizing himself with the program before he can properly use the Model. The program uses standardized criteria which reduce the subjectivity of estimating parameter values.

The recommendation is made that OPSET should be applied to areas where the climatological setting and geographical conditions differ from Kentucky in order to refine and modify it for a wider range of applicability. Also, the Model itself needs periodic updating in order to take advantage of subsequent empirical relationships or moisture accounting procedures.

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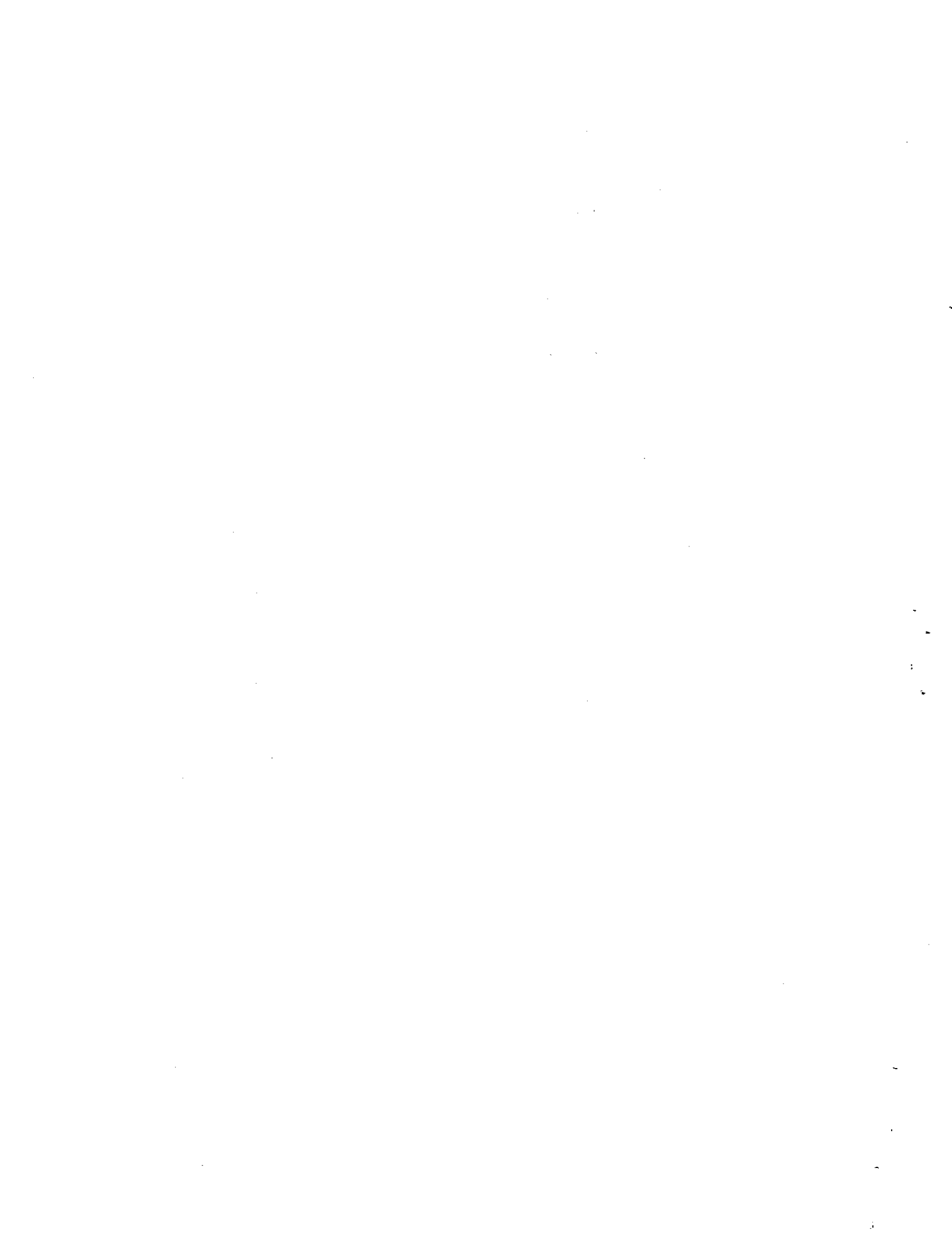


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## CHAPTER I

### HYDROLOGIC MODELING

#### THE MODELING CONCEPT

The high-speed electronic computer is revolutionizing hydrology. The capacity of the computer to store large volumes of data and to perform repetitious calculations has made hydrologic modeling a viable tool for the quantitative estimation of runoff. Chow (5\*, pp. 29-1 to 29-2) points out

"that hydrologic analysis and design necessitate processing a large amount of quantitative data which has been accumulating at a rapid rate, and that theoretical approaches have been gainfully introduced into modern quantitative hydrology and such approaches involve complicated mathematical procedures and models which can be solved practically only by high-speed computers."

A hydrologic model uses some analogous system to estimate the outcome of hydrologic processes. A model may represent only an individual hydrologic phenomenon (e. g., ground-water movement), or it may attempt to capture all the interacting processes determining catchment behavior. Some model builders have used a reduced scale laboratory replica of the natural system or an arrangement of analog components. Others have used mathematical expressions in digital computer programs. Each type of model seeks to simulate the physical response of a natural system to stimulation by climatological events and to study variation in the response as man changes the system.

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\*This number refers to the list of references at the end of this report.

In 1964, Amorocho and Hart (1) classified the methodologies used in hydrologic research into two broad categories. One is devoted to scientific research into physical hydrology in an attempt to better understand the mechanisms and interactions within each component of the hydrologic cycle. The other has been motivated by the pressing need to solve practical problems and has therefore concentrated on establishing quantitative relationships between precipitation and streamflow from data describing the functioning of the runoff system as a whole.

The distinction is found in the two aspects of modeling: the overall model and the component model. The overall model lumps the combined effects of component hydrologic processes occurring at rates varying with time and over an area into a single analogy. If the entire runoff cycle is lumped together, the generation of streamflow from runoff is modeled by one transform which makes no pretense of representing physical processes. Overall models are largely empirically derived from observed patterns of inputs and outputs from the lumped system.

On the other hand, the component model attempts to portray the overall reaction of a system by summing the functioning of its component physical processes. The modeling is shifted from the system as a whole to a series of individual processes and becomes less empirical and more theoretical. As fewer processes are lumped in a given equation, the representation becomes more complex and computationally more time-consuming. The component processes used in any model can conceptually be further subdivided into a group of lesser processes, but experience with the overall results is needed to judge which lines of additional refinement are most productive.

## THE STANFORD WATERSHED MODEL

The Stanford Watershed Model developed by Crawford and Linsley (10) is the pioneering effort modeling the runoff cycle from precipitation to streamflow by dividing overall watershed response into individual components each representing a known hydrologic process by an empirical expression. Each transform lumps the outcome of a process occurring at varying rates over a watershed segment and over a 15-minute period into a single estimate. Without lumping, even the most rapid computer could not simulate streamflow. With process-oriented lumping, a model makes the best use of information on process functioning.

The digital computer program models the whole of the land phase of the hydrologic cycle. The model utilizes a moisture accounting process to synthesize continuous streamflow from climatological data, measurable physical watershed characteristics, and a set of numerical values estimated for selected critical watershed parameters which govern key components of the runoff cycle (25, p. 15).

The Stanford Watershed Model has made a marked contribution to hydrologic research and water resources planning. The major obstacle to even wider use is the difficulty new users experience in estimating the numerous watershed parameter values required as input data. Some can be measured or approximated from raw data sources (25, pp. 40-47). A trial-and-error process has been used to estimate the other parameter values by adjusting them until achieving an acceptable matching between synthesized and recorded streamflows (10, 14). The calibration process is time-consuming, and each adjustment is subjective. Different investigators may end up with substantially different sets of parameter values for the same data.

In addition to these practical problems, use of a large number of parameters creates a conceptual difficulty. While a large number of

parameters are required to index the large number of component hydrologic processes active between precipitation and runoff, one has to be cautious because any increase in the number of parameters increases the power of a model to match recorded flows irrespective of whether or not the parameters have any physical significance. Even though the model is designed on the basis of a conceptual representation of hydrologic processes, once one decides to estimate hydrologic-process oriented parameters by an essentially numerical test of best matching between recorded and simulated flows, he cannot be sure that the value resulting for a specific parameter retains its meaning in terms of the process as originally conceived. Carried to an extreme, one has no concrete assurance that the model actually represents real hydrologic processes; perhaps any complicated series of equations with a large number of variable parameters could do as well.

These two difficulties suggest two needs. The practical difficulty suggests the need for a computerized process for estimating parameter values in a consistent and an objective manner. The conceptual difficulty suggests the need to examine relationships between paired parameter estimates and the measured watershed characteristics known to relate to the relevant hydrologic process.

#### THE PHYSICAL SIGNIFICANCE OF PARAMETERS

The potential contribution of the Stanford Watershed Model to hydrologic teaching and research and to water resources engineering design, planning, and management can only be fully realized as the practical and theoretical difficulties associated with having a large number of parameters are overcome. Let us look more closely at what these parameters represent. Hydrologic processes are continually going on at rates varying in time and by location over a real watershed. The movement of moisture at any point is in response to

acting forces (principally gravity) counteracted by the resistance to moisture movement along a given flow path and the resultant moisture queueing in low resistance zones because of high resistance areas ahead. Moisture movement would ideally be estimated from the physical factors known to govern saturated and unsaturated flow processes (including the size and shape of the particles; the porosity, orientation, and moisture content of the media; and the viscosity and surface tension of the water) and information on how these factors vary over the surface of the watershed and how the spatial patterns change with time.

Practically, the model builder must use lumped estimation. The parameters indexing the physical factors must reflect the attributes of a large area of watershed surface. In the Stanford Watershed Model (SWM), the user has the option of using the whole watershed or subdividing it into a limited number of segments. Area lumping does not require the factors to be taken as uniform over the area. The SWM uses assumed distributions between extreme values without specifying relative locations of different values on the watershed. The estimates must also be lumped over a finite interval of time (15 minutes in the SWM). They also often can be lumped to represent a group of related hydrologic processes (interception storage is added to depression storage in the SWM). Base flow recession constants represent the cumulative effect of moisture movement along a large number of routes.

Lumped estimates imply that watershed parameters somehow aggregate the effects over space, time, and process of spot parameters. Pragmatically, the best values are those that best model the flow. However, the set of parameter values best matching simulated to recorded flows will vary for a watershed among data covering different time periods with data measurement errors, spatial patterns of major

storm sequences over the watershed, size of the area and length of time represented, etc. Furthermore, the best estimate of a parameter for use in modeling with 15-minute time increments is not necessarily the best estimate for use with 60-minute increments. The art of hydrologic modeling comes in being able to choose combinations for lumping which closely approximate known events with an acceptable computational effort. Two tactics come to mind for making the SWM parameters more closely match measurable watershed characteristics. One is to use a finer grid in space and time and a more thorough separation of hydrologic processes. Such an approach is limited by the cost of program execution and by our understanding of the processes. The other tactic is to make estimation of values for the lumped parameters as objective as possible in the hope that the resulting values will correlate with measurable watershed parameters and hypothesized hydrologic process divisions in a meaningful way. This is the goal of the study.

#### COMPUTERIZED OPTIMIZATION OF PARAMETER VALUES

Given the Stanford Watershed Model as a computational scheme operating on precipitation and evaporation data under the control of the values assigned for a set of lumped parameters, the estimation problem is to find the best set of values for these parameters. Mathematically, the problem can be visualized as a search in an n-dimensional vector space for the best set of values for the n parameters. The search requires an explicit definition of "best" and an ordered procedure for considering various points.

The mathematics of the search is complicated by the impossibility of analytically taking the partial derivative of any objective function with respect to each parameter and by physical limitations to acceptable parameter values. Dawdy and O'Donnell (12) adapted a computer-

ized technique developed by Rosenbrock (24) to find best fit values for a set of nine parameters in a hydrologic simulation model using a coarser process grid than the SWM. They tested their optimization technique by simulating a sequence of flows, changing the parameter values to something else, and determining whether their optimization approach would return to the original set of values. Their major conclusion was that "the greater the sensitivity of the model response to a parameter, the closer and sooner will that parameter be optimized" (12, p. 133). They did not at that time report how their model or the optimization worked in the context of measured data for real watersheds. No other self-calibrating model could be found in the literature.

Thus the effort to program a computerized procedure for selecting the optimum set of parameter values for the Stanford Watershed Model began from a minimum contribution from the experience of others. The research turned into a long process of gradually expanding the program to work in a wider variety of situations.

### STUDY OUTLINE

The Stanford Watershed Model was originally written in a digital computer language (BALGOL) used by the Stanford Computing Center. James translated it into FORTRAN IV and called his translated, revised, and expanded version the Kentucky Watershed Model (KWM). The objective of this study is to develop a self-calibrating watershed model based on the KWM. This model is named as OPSET because its objective is to determine the optimum set of parameter values in the watershed model.

The processes used to develop the self-calibrating model are given in Chapter II. Chapter III describes the structure of OPSET and the principles contained in its subroutine programs. Recommended procedures for use in applying the program are presented in Chapter IV.



Chapter V reviews the results and recommends further research.

Listings of the most recent version of the Kentucky Watershed Model, OPSET, and a dictionary defining and giving units for all parameters and all the other variables used in either program are provided in Appendices A, B, and C respectively. The reader should consult Appendix C for information defining all mnemonics used in the subsequent text.

Two companion reports supplement this study and provide background information which this report will reference rather than repeat. Ross (25) describes the details of collecting input data and discusses how OPSET-estimated parameter values vary with watershed characteristics such as soil depth and permeability and with urban change. James (16) reviews the accomplishments of the overall research project and makes specific applications to flood control hydrology.

## CHAPTER II

### THE DEVELOPMENT OF OPSET

#### THE BASIC MODEL

The Stanford Watershed Model has not been and in fact no regularly used large computer program can be a single fixed entity. It is continually changing. At Stanford, Crawford and Linsley designated five versions by number, and Crawford has continued updating the Model in his work at Hydrocomp International (14). Other users have made their own changes (6, 19). A number of different people are simultaneously using a number of different versions, each adapted to meet their own modeling requirements (2, 4, 15).

Some choice had to be made on a specific version to use in the development of OPSET. The version selected was the Fortran version of the Kentucky Watershed Model (KWM) which had previously been used to model runoff from several Kentucky watersheds (8, 13, 22). Each parameter was taken as defined by the programming in this version. Any change in the simulation programming which changes the flows simulated from a given deck of input data changes the relationship between the simulation parameters and physical watershed characteristics. A program estimating optimum parameter values would make compensating adjustments in the estimates to match a fixed set of measured flows.

Minor reprogramming to increase computational efficiency, to introduce some new options, and to revise output format were made to the KWM during the development of OPSET. The final KWM version (dated June 6, 1970) is listed in Appendix A. Ross (25) lists

typical input data for the KWM in his Appendix A and outlines the moisture accounting process used in the Model and the role of each parameter in that accounting. The reader unfamiliar with the Model and the parameters should read his Chapter II before continuing through this report.

### STARTING ISSUES

The use of the above version of the KWM as a basis for developing OPSET required certain decisions which need to be brought out for discussion. Three major differences exist between the Kentucky Watershed Model and the current Hydrologic Simulation Program (14) used by Crawford. Each is discussed in the following paragraphs.

Seasonal Parameters: Two parameters have been retained from Stanford Watershed Model III to control seasonal variation in infiltration (SIAC) and in upper zone storage capacity (SUZC). Physical factors supporting seasonal variation in infiltration include differences in soil temperature and hence viscosity of infiltrating water, changes in vegetation and tillage and organic content of the soil surface, and a tendency for fine grained soils to shrink and crack during warm dry periods (23). All of these factors favor higher infiltration rates in the summer than in the winters. Upper zone storage capacity would logically increase with the growth of vegetation and summer cultivation.

The decision to keep these two parameters turned out to be wise in light of the results obtained when OPSET was developed and applied. OPSET was free to pick zero values for these parameters if it could not use positive values to better match recorded flows. In fact, it selected positive values in virtually every case (25, Table 11). For two watersheds (Cave Creek near Lexington, Kentucky, and the Clemson University Experimental Watershed near Clemson, South Carolina), OPSET was also run to determine the best set of values

for all the other parameters with the two seasonal parameters arbitrarily forced to zero. The modeling, as demonstrated by the summary of the results on Table 1, was significantly worse (more so for Cave Creek than the Clemson Watershed). Even though it is dangerous to generalize from one comparison, the tabulated figures suggest a physically reasonable trend toward greater seasonal variation as one goes from South Carolina to the more northerly and continental climate of Kentucky.

Table 1 also illustrates how the values of other parameters change in an attempt to take up the slack as certain parameters are removed from the modeling. Strictly speaking, such variables as LZC and BMIR have become different entities. It is not correct to take a value for LZC as estimated by OPSET and use it literally in any other version of the Stanford Watershed Model other than that listed in Appendix A. Those wishing to use another version should adjust OPSET accordingly.

Channel Routing: The modified Muskingum routing approach used in Stanford Watershed Model IV (10) was retained instead of being replaced by the kinematic wave approach used in the HSP (14). Several factors influenced this decision. The programming from Model IV is less complex and hence consumes less computer time in execution. In the cycling which was anticipated to be necessary for estimating parameter values in OPSET, time would be an essential factor determining computational feasibility. More important, the greatest need for OPSET is to better estimate and interpret values of the parameter associated with the land phase of the runoff cycle. A program which used much time in routing computations would not be appropriate.

In practice, parameters controlling the fraction of precipitation becoming runoff are relatively independent of parameters controlling

TABLE 1  
EFFECTS OF ELIMINATING SEASONAL VARIATION IN  
MODELING UPPER ZONE STORAGE CAPACITY  
AND INFILTRATION

Selected Values for Parameter	Cave Creek 1964		Clemson Watershed 1966			
	Seasonal Variation		Seasonal Variation			
	Yes	No	Yes	No		
LZC*	1.26	1.65	10.12	5.55		
BMIR	10.77	3.71	7.40	4.64		
SUZC	1.21	0.00	0.46	0.00		
ETLF	0.04	0.50	0.14	0.19		
BUZC	0.59	3.38	0.81	0.35		
SIAC	0.66	0.00	0.74	0.00		
Minimum SSQM	0.186	1.047	0.099	0.329		
Comparison of Monthly Flow Totals						
	Recorded	Simulated		Recorded	Simulated	
October	0.9	0.4	0.3	37.1	49.9	48.7
November	2.6	0.2	0.0	24.8	22.9	22.6
December	1.8	2.8	0.0	18.1	15.9	13.4
January	59.5	70.0	43.9	34.9	26.8	23.2
February	117.4	88.3	75.9	82.3	77.2	67.6
March	536.0	634.9	606.4	67.4	79.0	80.8
April	30.4	34.5	59.8	25.8	29.1	43.5
May	14.1	13.7	21.0	28.8	27.3	34.5
June	7.6	6.4	0.8	17.4	19.1	21.2
July	5.5	0.7	0.1	12.1	12.4	14.9
August	1.2	0.4	0.0	16.0	10.2	15.3
September	10.6	10.8	24.1	10.5	10.0	10.2

\*All mnemonics with units are defined in Appendix C. Parameters are defined on Table 2.

routing. The land phase parameters can be estimated first and applied with either routing scheme. OPSET continues by fitting the parameters associated with the Muskingum approach to guide those wishing to use them and to make the modeling complete.

The emphasis of OPSET is on land surface rather than channel routing processes because this is where the greater modeling problem lies. Routing parameters based on the kinematic wave approach can already be directly estimated from measurable channel characteristics without resorting to trial and error (14).

Watershed Segmentation: The KWM does not provide the option of subdividing a watershed into segments and modeling runoff as the sum of segment totals. Even the older SWM versions provide this option, but it is not included in the KWM principally because none of the watersheds used were of such nature that modeling by segment was appropriate and hence no data were collected that were suitable for debugging a translation into Fortran of this feature. The two main advantages of segmenting are the greater ability to handle spatial variation in precipitation and the greater ability to handle spatial variation in watershed characteristics.

Spatial variation in precipitation can be either associated with consistent, usually orographical, rainfall patterns or with random differences from storm to storm. Orographic segments could be assigned individual precipitation multipliers (RGPMB) to precipitation at a single gage. Segmenting to better cope with random precipitation patterns requires several rain gages. For the small Kentucky watersheds to which OPSET was applied (25, Table 9) rain gage spacing was too large with respect to basin size to permit segmenting to reflect random precipitation patterns and orographic patterns were minor.

Segmenting to handle spatial variation in watershed characteristics

is not practical for a self-calibrating model seeking parameters giving the best fit. One must work with recorded flows undifferentiated by watershed source area. The parameters cannot be subdivided on a finer grid than the data. Ross minimized this problem by working with normally more homogeneous small watersheds. If they revealed a pattern between parameter values and watershed characteristics, derived correlations could be used to estimate parameter values from the characteristics of segments of larger watersheds; and the KWM could be revised to simulate flows by segment and accumulate the results. Some revision in the structure of OPSET may be necessary to improve results for watersheds for highly orographic rainfall patterns.

#### THE BASIC STRATEGY

The Kentucky Watershed Model simulates a sequence of streamflows from input climatological data through a defined computational procedure based on equations containing parameters. The goal is to estimate for the parameters the set of values which simulates the streamflows most closely matching gaged values. The general strategy is to define the difference between simulated and recorded flows as a scalar quantity and vary parameter values to minimize this objective. Direct analysis making use of the partial differential of the objective function with respect to each parameter is infeasible because of the complexity of the simulation algorithm and the large number of conditional routes through it. Some systematic pattern of selecting trial sets of values and making appropriate adjustments must be substituted.

One issue which had to be resolved early was which parameters to estimate by use of OPSET. Parameters were ruled out if they were amenable to direct measurement (drainage area) or if the simulated streamflows proved to be insensitive to large fluctuations

in parameter magnitude. Estimating through matching disintegrates when the simulated flow variation associated with parameter value changes is less than that associated with model inadequacies and data difficulties. As Dawdy and O'Donnell discovered (12, p. 133), the process is already becoming imprecise and time consuming when the sensitivity range is slightly greater than the difficulty range. Parameters passing both tests are listed on Table 2. Ross (25) recommends means for estimating the other parameters.

The next issue was whether the parameter values should be estimated individually, simultaneously, or simultaneously by groups. Individual estimation requires independent parameters. In OPSET, some defined characteristic of the total flow pattern must correlate closely with that parameter and be relatively independent of all the others. Interdependent parameter groups must be estimated simultaneously because a change in the value of any one changes the values of the others. Cyclic single parameter optimization sometimes converges, but more than one cycle is always needed.

The 13 parameters listed on Table 2 fall into three groups. The first six land phase parameters form a subgroup called runoff volume parameters because they control the volume and distribution over the year of simulated runoff. The recession constants can be estimated directly from recorded flows. The parameters within each of the other two groups were found through sensitivity studies to be so interdependent that simultaneous parameter adjustment by group would be required. The two groups as a whole were sufficiently independent for adjusting the first after estimating the second to be unnecessary. Each cycle requires (1) a simulation unit to synthesize flows for comparison with recorded flows for a given set of parameter values, (2) establishment of adjustment rules for selecting a new set of



TABLE 2  
PARAMETERS ESTIMATED BY OPSET

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Recession Constants

- IFRC - interflow recession constant  
BFRC - base flow recession constant

Land Phase Parameters

Runoff Volume Parameters

- LZC - lower zone storage capacity  
BMIR - basic maximum infiltration rate within watershed  
SUZC - seasonal upper zone storage capacity factor  
ETLF - evapotranspiration loss factor  
BUZC - basic upper zone storage capacity factor  
SIAC - seasonal infiltration adjustment constant

Interflow Volume Parameter

- BIVF - basic interflow volume factor

Channel Routing Parameters

- NCTRI - number of current time routing increments  
CSRX - channel storage routing index  
FSRX - flood plain storage routing index  
CHCAP - channel capacity - indexed to basin outlet
-

parameter values, and (3) a scalar objective function for judging goodness of matching.

The optimization procedure used in each cycle is shown schematically in Figure 1. The simulation unit for the first cycle (TRIP 1) is a streamlined version of the portions of the inner loop of the KWM which simulate surface runoff. The simulation unit for TRIP 2 is a streamlined version of the portions of the inner loop of pertaining to channel routing. Whenever a new set of flows is simulated, the parameters are saved if the matching is better than any found previously but the adjustment continues until hope of finding a still better matching is lost. Various sensitivity studies were made and were used to guide the adjustment rules.

#### REARRANGEMENT OF THE MODEL

The first step in the development of OPSET was to rearrange the programming from the order efficient for proceeding directly from given input data to a single simulated flow sequence as found in the KWM to an order efficient for executing the optimization strategy of Figure 1. Because of the large number of times it would have to be called, all programming performing computations identical for all simulation runs was pulled out of the simulation unit and placed earlier in the program. All climatological data were placed in fixed arrays so they could be held constant for each parameter set. Constants representing combinations of parameters not included in the 13 were developed so they would not have to be formed within the inner loop. Programming to provide supplementary output was dropped.

The KWM contains a hierarchy of daily, hourly, and period loops. The inner-most or 15-minute loop simulates rapid hydrologic processes such as interception, infiltration, upper and lower zone moisture storages, overland flow, interflow, etc. Slower processes are

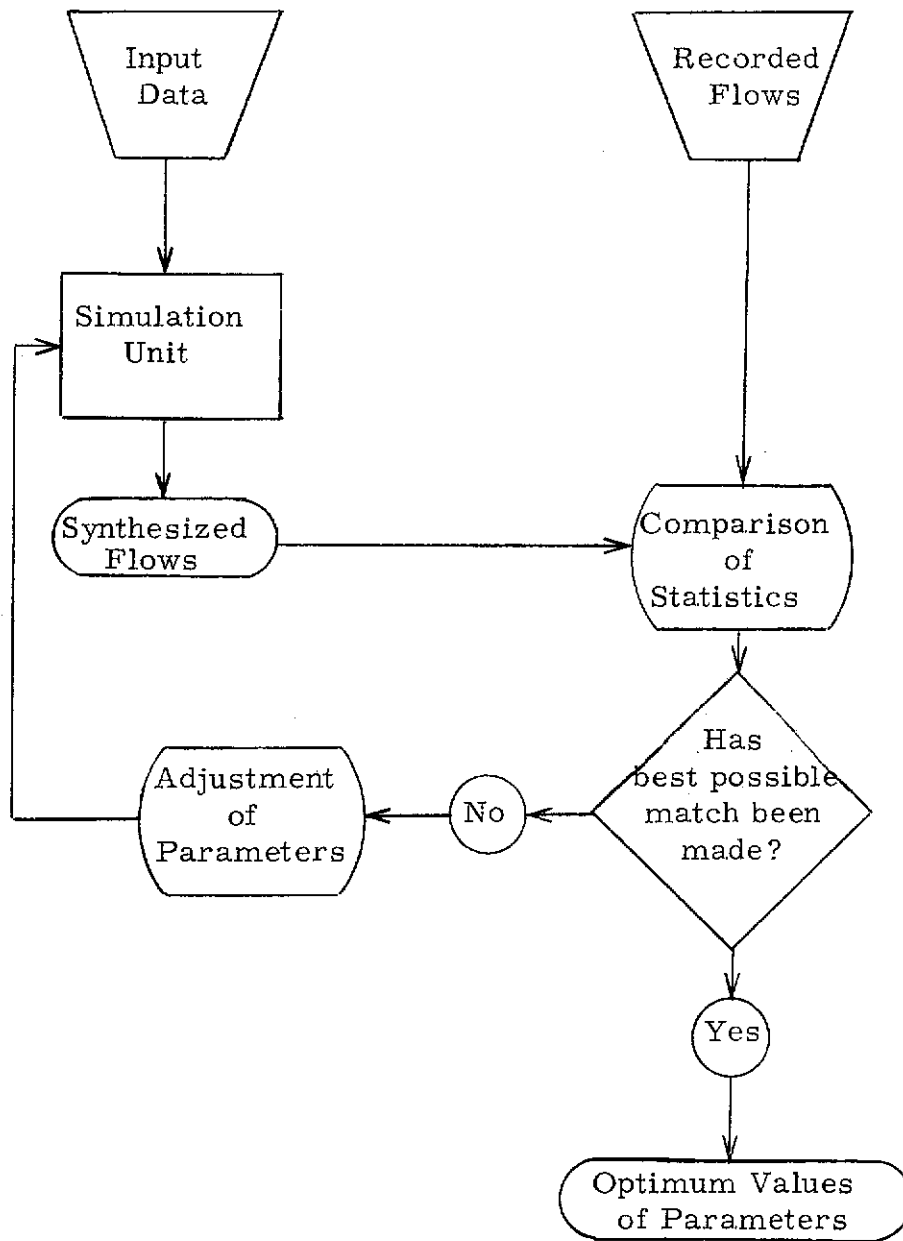


FIG. 1. Schematic Diagram of Parameter Optimization Procedure.

simulated hourly (for example, stream evaporation and base flow). The outer or day loop orders the calculations and sums the flows in daily totals.

### THE OBJECTIVE FUNCTION

The true flow in a stream varies continuously throughout the year. Flows are simulated by the KWM by discrete 15-minute periods. Recorded flows can be estimated from recorder charts or punched tape on a comparable time base, but published flows represent calendar day average values. The logical objective function would minimize some measure of the difference between recorded and simulated flows taken at some appropriate time interval and summed over the year. With parameters placed in three groups, an appropriate objective function had to be found for each one.

The objective used to estimate the two recession constants was to minimize the sum of the squares of the differences between recorded and simulated average daily flows during selected recession sequences. The objective used to estimate the first six land phase parameters was to minimize the sum of the squares of normalized ratios relating simulated to recorded monthly flow totals. The objective used to estimate BIVF was to minimize the difference between total simulated and total recorded interflow summed over selected three-day periods. The objective used to estimate the channel routing parameters minimized the difference between recorded and simulated flow peaks within specified timing constraints.

The monthly time grid used for the objective function for estimating the runoff volume parameters reduced interdependence between the land phase and channel routing parameter groups. The channel routing parameters, for small watersheds, may significantly change the distribution of flows among days. Months are long enough for the

channel routing of direct runoff to have minimal effect on the month it reaches the gage and short enough to reflect seasonal runoff patterns. Monthly totals are published and are units with which most hydrologists are familiar (see Subroutine DAYSUM). The use of time periods divided at low flows between storms was considered but rejected because of added programming complexity and problems associated with different seasons being represented by different numbers of periods.

More precise definitions of each objective are given in the descriptions of Subroutines SET2RC, SETFDI, SETFVP, and SETHRP.\*

#### EVALUATION OF TWO RECESSION CONSTANTS

Both a base flow recession constant and an interflow recession constant are needed in the Model. Traditional graphical techniques for estimating appropriate values take too much time, often require flows subdivided by a finer time grid than a day, and are somewhat subjective; therefore, a computer procedure was sought to make data preparation easier for the user and minimize any effect caused by subjectively estimated values on subsequent parameter estimation.

The first approach tried to evaluate the base flow recession constant (BFRC) by averaging flow ratios for successive days on which neither direct runoff nor interflow were synthesized, and to estimate the interflow recession constant (IFRC) from the ratio of the sum of interflows (total recorded flow minus simulated base flow) of the second to the first day after the major recorded streamflow rises. The approach was later abandoned in favor of the least squares method to estimate the two recession constants developed by James and Thompson (18). This approach uses selected recorded daily

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\*Each Subroutine is presented individually in Chapter III.

streamflow data and assumes that each sequence contains two linear recession flow components. Much more consistent and reasonable results were obtained. The approach as adopted in OPSET is presented in greater detail in the discussions of Subroutines RECESS, SET2RC, and SET1RC.

### EVALUATION OF SIX RUNOFF VOLUME PARAMETERS

The task most critical to the success of OPSET was development of a workable algorithm for estimating the six runoff volume parameters (LZC, BMIR, SUZC, ETLF, BUZC, and SIAC).<sup>1</sup> A least squares approach to simultaneously estimate these six parameter values was tried first. The approach postulates a model

$$\begin{pmatrix} q_1 \\ q_2 \\ \vdots \\ q_{12} \end{pmatrix} = \frac{q}{12 \times 1} = \Gamma^* \begin{pmatrix} 1 \\ x_1 \\ x_2 \\ \vdots \\ x_6 \end{pmatrix} \quad (1)$$

$12 \times 7$                        $7 \times 1$

where  $q_1, q_2, \dots, q_{12}$  are the simulated monthly flows with six parameter values  $x_1, x_2, \dots, x_6$ , and  $\Gamma^*$  is a  $12 \times 7$  matrix of constants. Assuming the model is correct and there are no data or modeling difficulties, a unique set of parameter values, say  $\tilde{x}_1$ , would synthesize a set of monthly flows which exactly match the recorded monthly flows  $\eta_1, \eta_2, \dots, \eta_{12}$ . If the matrix  $\Gamma^*$  can be estimated, then the approach would obtain the best set of  $x$ 's by some sort of inverse relationship by substituting  $\underline{\eta}$  for  $\underline{q}$ .

However,  $\Gamma^*$  is unknown and furthermore it is not square. To formulate the model so that an inverse of the matrix of constants can be obtained, seven functions of the simulated annual hydrograph, say

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<sup>1</sup>For discussion on the grouping of parameters see 16, pp. 27-32.

$f_1, f_2, \dots, f_7$ , were chosen. The model is then

$$\begin{pmatrix} f_1(\underline{q}) \\ f_2(\underline{q}) \\ \vdots \\ f_7(\underline{q}) \end{pmatrix} = \begin{matrix} f \\ 7 \times 1 \end{matrix} = \begin{matrix} \Gamma \\ 7 \times 7 \end{matrix} \begin{pmatrix} 1 \\ x_1 \\ x_2 \\ \vdots \\ x_6 \end{pmatrix} \quad (2)$$

$7 \times 1$

By redefining the model as Equation 2,  $\Gamma$  is square; and if it is non-singular, its inverse can be used to estimate the parameter values  $\tilde{x}_1$ .

The seven functions of the  $q$ 's chosen for this purpose were the annual runoff volume, the peak monthly volume, the minimum monthly volume, the total volume in selected summer months, the total volumes in selected winter months, the total volume in selected low flow months, and the sum of squares of monthly flow deviations. After the seven functions  $\{f_i(\underline{q})\}$  were defined, eight sets of parameter values  $\{\underline{x}_i\}$  which were systematically arranged by high and low combinations were used to establish the relationships in the following equations:

$$\begin{aligned} \underline{f}_1 &= \Gamma \underline{x}_1 & \underline{f}_1' &= \underline{x}_1' \Gamma' \\ \underline{f}_2 &= \Gamma \underline{x}_2 & \Rightarrow \underline{f}_2' &= \underline{x}_2' \Gamma' & \Rightarrow F &= X \Gamma' \\ & \vdots & & \vdots & & \\ \underline{f}_8 &= \Gamma \underline{x}_8 & \underline{f}_8' &= \underline{x}_8' \Gamma' \end{aligned} \quad (3)$$

where  $\underline{f}_i$  and  $\underline{x}_i$  were  $7 \times 1$  vectors as defined in Equation 2. The least squares estimate of  $\Gamma$  is

$$\hat{\Gamma} = (X'X)^{-1} X'F \quad (4)$$

With the estimated  $\Gamma$ , Equation 2 was used to obtain the estimated parameter values  $\{\tilde{x}_i\}$  by substituting the recorded monthly flows  $\underline{\eta}$  for  $\underline{q}$ ; that is

$$\begin{pmatrix} 1 \\ \hat{x}_1 \\ \vdots \\ \hat{x}_6 \end{pmatrix} = \hat{\Gamma}^{-1} \begin{pmatrix} f_1(\underline{\eta}) \\ f_2(\underline{\eta}) \\ \vdots \\ f_7(\underline{\eta}) \end{pmatrix} \quad (5)$$

This approach did not prove successful. The parameter values did not converge well and were inconsistent from run to run. This failure may have been caused by the variance of  $\hat{\Gamma}$  being too large or the linear approximation being too gross. The approach was finally abandoned and was later replaced by another approach which produced better results.

The second and more successful method adjusted the estimate of each parameter according to rules based on the deviations between recorded and simulated flows which were known to be most sensitive to that parameter. The sensitivity was determined by varying each of the six parameters one at a time with a selected set of climatological data (see Subroutine SETFVP) as checked by what seemed reasonable from qualitative knowledge of the hydrologic cycle.

The method was refined as it was applied to 69 station-years on 20 Kentucky watersheds. Through detailed analysis of synthesized flows and comparison of parameter estimates among runs for different years, the method was continually modified to improve the correlation between synthesized and recorded flows closer and closer, to estimate computer selected parameter values within the physically reasonable range, and to improve the consistency of the parameter values from year to year. Safeguards were built in to keep the program from becoming severely upset by faulty precipitation data. Highlights of the adjustment process include

1. Some adjustment rules not only examine monthly total flows, but also component flows. Some parameters affect one component



flow, say overland flow, in a quite different pattern than other components of the total flow.

2. Alternative adjustment rules are set up for use when the initial adjustment rules do not work well.

3. Upper and lower limits on parameter values were established by reviewing the experience others have had with the model, and attempts to adjust values outside this range are taken to suggest that an alternate adjustment procedure should be tried.

4. Limitations on the size of a given adjustment are made to prevent estimates for one parameter from being kept from convergence by rapid fluctuation in the values of other parameters.

The estimation of the runoff volume parameters starts with an initial set of trial values which are judged to be near the median encountered in modeling watersheds. A year of flows are simulated, and the parameters are adjusted according to the established rules. The value, SSQM, of the objective function for these parameter values is estimated. Using the adjusted parameter values, a new year of flows is simulated, and SSQM is again calculated. The set of parameter values with the smaller SSQM is judged as the better set, and both the value of SSQM and the parameter value are saved to compare with the next run. The process is continued until the number of trials since the last improvement suggests that the best possible set has been found.

In order to save computer time, channel routing is by-passed while estimating runoff volume parameters. This means that synthesized land phase runoff is taken as the simultaneous outflow at the mouth of the watershed. Unless the watershed is large or an unusually large storm occurs on the last day of the month, the above assumption will not materially affect the distribution of monthly flow volumes. A second strategy to save computer time is to increase

the time period represented by the inner loop. Table 3 shows the effect of changing the length of this time period on synthesized monthly flows. A 15-minute loop (i. e., the number of loops equals 4) is used as a base, and the entries of Table 3 are the volume ratios. Inspection of Table 3 reveals that hourly looping gives only a coarse under-estimation of the 15-minute looping flows while 20-minute looping provides a much closer approximation. Table 4 shows the effect of the number of loops within an hour on the number of adjustments required to converge on a best set of parameter values, and the monthly flow deviations. Use of a shorter inner loop period increases the computer time required to simulate a year of flows and increases the number of times a year flows must be simulated to estimate a set of parameters.

Significant computer time can be saved by first adjusting runoff volume parameter values by hourly looping to bring the estimates in range. Then, 20-minute looping can be used to refine the best estimates obtained in hour looping. Use of 20 rather than 15 minutes saves lots of time at a small sacrifice in accuracy. The first stage of adjustment which uses hour looping to simulate streamflows is called the "Rough" adjustment cycle, and the 20-minute looping is called the "Fine" adjustment cycle. The best set of parameters obtained by the fine adjustment cycle is taken as the optimal set of parameter values. Detailed description of the two adjustment cycles is presented in Chapter III.

#### EVALUATION OF ONE INTERFLOW VOLUME PARAMETER

Crawford and Linsley recommend that the interflow volume parameter be adjusted by matching the simulated hydrograph shape to the recorded shape (10, p.69). If recorded flows were readily available on hourly intervals, a possible objective would be to minimize hourly

TABLE 3  
 EFFECT OF INTERVAL USED IN INNER  
 LOOP ON SIMULATED MONTHLY FLOW VOLUMES:  
 ELKHORN CREEK NEAR FRANKFORT, KENTUCKY  
 1964 CLIMATOLOGICAL DATA

	Number of Loops within an Hour					
	1	2	3	4	5	6
Oct.	0.990	0.995	1.000	1.000	1.000	1.005
Nov.	0.976	0.976	1.000	1.000	0.976	0.976
Dec.	0.628	0.868	0.953	1.000	1.010	1.028
Jan.	0.931	0.977	0.992	1.000	0.997	0.999
Feb.	0.902	0.972	0.992	1.000	0.989	0.988
Mar.	0.997	1.000	1.000	1.000	1.001	1.000
Apr.	1.021	0.991	0.996	1.000	1.002	0.997
May	0.930	0.963	0.988	1.000	1.012	1.022
June	0.642	0.845	0.942	1.000	1.045	1.078
July	0.795	0.934	0.979	1.000	1.024	1.047
Aug.	0.839	0.950	0.994	1.000	1.025	1.037
Sep.	0.915	0.969	0.992	1.000	1.004	1.007

Note: All volumes are expressed as ratios to volumes with 4 loops. See right hand column of Table 11 for absolute units.

TABLE 4  
 EFFECT OF INTERVAL USED IN INNER LOOP ON  
 NUMBER OF ADJUSTMENTS TO CONVERGENCE,  
 OPTIMAL PARAMETER VALUES  
 AND THE MONTHLY FLOW DEVIATIONS:  
 ELKHORN CREEK NEAR FRANKFORT, KENTUCKY  
 1964 DATA

No. of loops within an hour	1	2	3
Cycles to convergence	8	13	15*
Optimal parameter values			
LZC	4.89	4.74	4.72
BMIR	4.00	4.00	3.47
SUZC	1.96	1.92	1.81
ETLF	0.10	0.10	0.12
BUZC	1.16	1.23	1.10
SIAC	0.75	1.51	2.53
Monthly flow deviations			
Oct.	-1.847	-1.046	-1.065
Nov.	-1.097	-0.828	-0.804
Dec.	-0.146	0.022	-0.314
Jan.	-0.375	-0.489	-1.060
Feb.	-0.109	-0.127	-0.255
Mar.	-0.049	-0.059	-0.073
Apr.	-0.018	0.040	0.063
May	-1.923	-1.314	-1.040
June	-0.338	-0.043	0.005
July	-0.600	-0.282	0.300
Aug.	0.071	0.244	0.207
Sep.	0.738	0.713	0.549
SSQM**	9.513	4.416	4.594

\*Optimization procedure still improving flows.

\*\*Index defined in presentation of Subroutine SETFDI (pp. 40-43).

differences. The required data, however, is not published by the USGS and may be difficult to obtain. Rather than let this become an obstacle discouraging use of OPSET, a criterion based on daily flow totals was substituted. The daily flow totals most sensitive to interflow are those immediately following major peaks. The first method was tried to adjust BIVF by multiplying the parameter value by the ratio of the sum of the estimated recorded interflows to the sum of the synthesized interflow volumes. The estimated recorded interflow is obtained by subtracting the simulated base flow from the recorded daily total flow. The results were not stable.

Later, the method was improved by using interflow in the first three days after a peak flow. The recorded interflows were estimated by the method used in estimating the recession constants (see Subroutine SETRBF), and instead of using a ratio of sums, the average of the daily ratios was used for adjusting the parameter value. The new method proved to be more consistent, and the values were more reasonable. A more detailed description is given under Subroutine SETBIV. The adjustment of BIVF occurs in the Fine adjustment cycle for the six runoff volume parameters.

#### EVALUATION OF FOUR CHANNEL ROUTING PARAMETERS

The estimation process for the channel routing parameters like that for the runoff volume parameters follows the optimization scheme shown in Figure 1. The simulation unit in optimizing runoff volume parameters is the inner loop. In optimizing hydrograph parameters, it is the channel routing unit. By using different sets of trial parameter values, the routed hydrographs are compared with the recorded hydrographs in the timing and magnitudes of the peaks. The set of parameter values which gives the best match is accepted.

The channel routing parameters are estimated after values for

the other nine parameters have been determined. The nine estimated values are used in simulating an annual hydrograph with 15-minute looping. Synthesized land phase runoffs during the periods contributing to read recorded hydrograph peaks (the beginning and ending hours of the runoff periods estimated as contributing to the recorded hydrograph are estimated by Subroutine STRHRS) are saved. In order to overcome the bias caused by using different volumes in estimating hydrograph routing parameters, the synthesized runoff volumes are adjusted to match the recorded volumes (See Subroutine ADJHYD). The adjusted land phase hydrograph is used as the input data shown in Figure 1. Two channel routing subroutines (Subroutine TIMERT performs channel time routing, and Subroutine STORRT performs channel storage routine) serve as a simulation unit. Trial parameter values are used to route the adjusted inflow hydrographs, and the routed outflow hydrographs are compared with the recorded hydrographs in flow peaks and in times. The parameters are adjusted until the best set of parameter values are found.

Early in the development of the optimization procedure for the hydrograph parameters, the number of time routing increments (NCTRI) was found to be the primary parameter governing the flood peak timing, and the storage routing index (SRX) was found relatively more influential in controlling the magnitude of hydrograph peak. First, they were adjusted separately, but a later and better procedure combined the two adjustment procedures into Subroutine SETHRP.

#### ADJUSTMENTS OF CLIMATOLOGICAL DATA

The simulation takes input precipitation and evaporation data, operates on it in a manner dictated by a set of parameter values, and produces a synthesized annual hydrograph. OPSET estimates the set of parameter values which operate on given climatological data to

most closely match given streamflow data. In order for the estimates to truly represent the watershed and correlate well with physical watershed characteristics, the climatological data must truly reflect watershed experience and the streamflow data must truly measure actual flows. Otherwise, the estimated parameters will contain components which are nothing more than mathematical attempts to compensate for faulty data.

Precipitation falls in a pattern which varies continuously with time and at a given time varies in rate over the area of the watershed. A precipitation gage network samples this variation in time and space by measuring rainfalls at selected spots. Some error is associated with incorrect measurement of spot rainfall. A larger problem is subdividing measured totals over time. Storage gages are read daily at times which vary from gage to gage. Recording gages provide a record of accumulated totals from which clock hour totals are published. A great deal of work is involved in going to the original records to obtain totals on a finer time grid. A still larger problem is estimating precipitation over larger areas from spot values. Where orographic influences are minimal, one normally assumes that spot precipitation represents areal precipitation on a probability if not on an historical basis.

Streamflow varies continuously with time. A stream gage measures this flow by indicating a water surface elevation and converting it to flow through a stage-discharge curve. Some error is caused by inadequacies in the stage-discharge curve, particularly where the relationship changes with calendar time or flows must be estimated for stages significantly higher than that for the largest measured discharge. The largest errors are normally associated with very high or very low flows. Continuous stage measurements

are integrated to daily flow totals in the published record. Very large flood peaks are also given. A great deal of work is required to obtain flows on a finer time grid.

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OPSET had to be designed to handle constant errors, random errors, and grid errors. Constant errors can be handled by a rainfall multiplier (RGPMB) and an additive streamflow (DIV). Random errors can only be handled by using a long enough record for positive and negative effects on parameter estimates to cancel. The larger these errors are the longer the record has to be. Therefore, an effort to filter records for obvious precipitation-streamflow anomalies can significantly reduce required computer time.

The selected strategy for dealing with hydrologic data was to design OPSET to use published data sources rather than require the user to analyze original records to subdivide totals over a finer time grid. A finer time grid has some merit for better estimating the channel routing parameters and better matching recorded flood peaks but it is not much help with respect to the primary research goal of estimating the land phase parameters. Better estimation of runoff volume from precipitation is lagging research on better channel routing.

The input data thus consists of hourly precipitation totals, daily evaporation totals, and daily streamflow totals.\* Procedures used by the Stanford Watershed Model for doing such things as allocating daily evaporation totals by hour, taking watershed rainfall as a weighted average of amounts gaged at two points, and providing for streamflow diversions are retained and are described by Ross. Several new features were added for OPSET:

1. The adjustment of precipitation data where there are precipitation-streamflow anomalies. A subroutine was made to check

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\*For more detailed information see Ross (25, pp. 25-35, 56).



for anomalies and to adjust the recorded precipitation where necessary (see Subroutine PRECHK).

2. Provision for non-uniform distribution of rainfall within the hour for small watersheds where the time of concentration is comparatively short to better match flood peaks. A subroutine was developed to divide the hourly rainfall totals among 15-minute periods using an average distribution (see Subroutine PREPRD). Hydrocomp International (14) has an option for directly reading 15-minute precipitation, and the same option could be readily added to OPSET.

3. Provision for handling situations where changes in precipitation gage location or storage gage reading time occurs during the year.

4. Addition of provision for an approximate approach to estimate the daily evaporation data from estimated total annual evapotranspiration. A subroutine was developed to distribute an annual total among the days of the year (see Subroutine EVPDAY).

#### INITIAL PARAMETER VALUES

The parameter estimation scheme shown on Figure 1 begins with some initial set of parameter values. The initial set of values can only be chosen arbitrarily if the procedure will converge on the same final set from any beginning. Because the cost of computer time is an important factor in trial-and-error estimation, the initial trial values should be reasonably close to the final ones most often encountered.

A study was made to see how much difference variation in the initial set made on the final estimated set of parameter values. The approach used low, middle, and high starting values and three years of data for Cave Creek near Lexington, Kentucky. Table 5 shows the results and reveals that the parameter values have a tendency to converge toward common values but exhibit certain degree of variation.

TABLE 5  
STUDIES ON SENSITIVITY TO STARTING PARAMETER VALUES:  
CAVE CREEK NEAR LEXINGTON, KENTUCKY

Starting Values				OPSET Estimates <sup>1</sup>								
				1955			1958			1965		
Low	Middle	High	Low	Middle	High	Low	Middle	High	Low	Middle	High	
Parameter												
LZC	2.0	12.0	30.0	8.00	9.22	10.23	2.62	2.00	2.00	1.86	1.66	1.56
BMIR	0.2	1.20	4.0	2.02	3.59	9.79	8.24	12.46	16.60	3.95	19.27	25.44
SUZC	0.3	1.30	4.0	0.72	1.10	0.80	0.30	0.30	0.30	0.60	0.39	1.46
ETLF	0.05	0.25	0.6	0.13	0.36	0.31	0.21	0.22	0.15	0.09	0.12	0.29
BUZC	0.2	1.50	5.0	0.72	1.62	5.41	0.50	1.13	4.55	0.27	0.70	1.35
SIAC	0.3	0.90	4.0	0.03	0.64	1.29	0.03	0.49	0.03	0.15	0.77	0.87
Statistics												
Number of Rough Cycles				3	2	3	9	7	10	2	3	4
Number of Fine Cycles				2	3	3	2	2	2	5	9	4
Final SSQM				0.685	0.671	0.338	1.310	1.549	1.781	0.293	0.313	0.328
Annual Total												
Recorded					1051			1442			1032	
Simulated				1079	988	984	1398	1415	1419	953	962	916
Max. Peak												
Recorded					117			87			40	
Simulated				163	160	105	90	83	94	79	83	101

<sup>1</sup>A preliminary version of OPSET was used for this study.

In later versions of OPSET this amount was reduced by increasing adjustment jumps for such sluggish parameters as BUZC. Comparing the results from the three sets of starting values shows the speed of convergence (numbers of Rough and Fine cycles) and the matching of synthesized to recorded flows (SSQM, annual totals, and maximum flood peaks) to be quite close to each other. For the consistent estimate of reasonable parameter values the medium starting values work best because they have the best chance of being close to the final estimate and less chance to produce out-of-range adjusted values which may cause the program to stop before the optimum point is reached. The value to be gained by rerunning the computations for Table 5 with the final version of OPSET did not seem worthwhile and thus the results shown are only a tool for program improvement and are not indicative of the sensitivity of the final program to initial values.

#### TIME PERIOD COVERED BY CALIBRATION RUN

The ordering of computations in the KWM is to read and store values which remain fixed for a given watershed, read values of initial moisture storage by storage category, read and store climatological data for a water year, simulate a year of flows, take the ending moisture storages as the initial moisture storages for the next year, read and store climatological data for the next year, and continue this cycle for as many years of simulated flows as are desired. In designing OPSET, one question was how long a period of flows should be used in a run to estimate parameter values. Selection of a long enough period to dampen out parameter estimating errors caused by random data measurement problems did not prove practical because too many years would be required. Storage requirements to simultaneously hold all the data for such a long period are excessive.

Computer time requirements to simulate so many flows per trial set of parameter values are also excessive. The decision was to design OPSET to estimate parameters from one year of record. Chapter IV discusses means for estimating appropriate values for a watershed from a group of OPSET estimates for different water years.

With only one year of flows to be used, it was necessary to estimate appropriate initial moisture storages. The best estimate of these values would come from the ending moisture storages from a simulation run for the previous year using the optimum set of parameter values. However, the optimum set of values would not be known in advance, and it was impractical in light of storage and computing time constraints to simulate blocks of flow longer than one year. The decision was to make the best possible estimate of each initial storage as of October 1, base that estimate on the context of the corresponding set of parameter estimates, but leave October matching out of the least squares criterion to minimize bias caused by initialization error (pp. 40-43, 102).

One advantage of designing OPSET to estimate parameters from one year of flows is that it permits one to select the years of record which by inspection seem to be relatively free of data problems capable of upsetting the estimating process.\* More important, it permits selection of years with diverse flow patterns so that OPSET can be tried under as diverse a range of conditions as possible.

In selecting years to use, years with certain types of initial conditions should be avoided. A year following a large storm late enough in September for surface runoff and significant upper zone storage to continue into the next water year is undesirable. So is

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\*These problems are discussed in more detail on p. 155.

a year with very low October and November rainfalls. The effects of initial conditions dampen more quickly in wet months.

### SUMMARY

OPSET is built around a streamlined inner loop of the KWM. Watershed parameters are estimated simultaneously by group and three separate trip sequences are devised. Since the two recession constants can be estimated from the recorded streamflows by a least squares method, they are evaluated before the first trip begins. In the first trip (TRIP 1), the six runoff volume parameters are approximated in Rough adjustment cycles with hourly looping, and then the six estimates are refined and the one interflow volume parameter is estimated in Fine adjustment cycles with 20-minute looping. The optimization of the six volume parameters is based on matching synthesized to recorded monthly totals, and the adjustment of the one interflow volume parameter is based on matching the interflow volumes for the first three days after the major floods. In TRIP 1, channel routing is by-passed to save computer time.

The second trip (TRIP 2) uses 15-minute looping to synthesize land phase runoff hydrographs based on the values for the first nine parameters as estimated in TRIP 1. These hydrographs are then used in trial-and-error estimation of the four channel routing parameters.

Using all 13 parameter values as estimated, a final trip (TRIP 3) is made to simulate a year of streamflows with 15-minute looping so that the effectiveness of the whole optimization process can be observed and evaluated.

## CHAPTER III

### PROGRAM DESCRIPTION

OPSET is composed of one master program (MAIN) and 22 subroutine programs. MAIN controls operations as directed by coded control options by reading the required input data, setting up the parameter estimating processes, controlling the streamflow simulation and calling subroutine programs. Subroutine programs calculate statistics used in the adjustment of parameters, adjust the parameters according to established rules, check and if necessary adjust input data, read the coded data in a free format, and print the requested output.

This chapter describes the programming in detail. Because of the length of the program, statement-by-statement analysis is not practical; however, it is hoped that the reader will find the discussion herein sufficient for him to comprehend, understand the reasoning behind, and properly use the program.

#### MAIN Program

MAIN has three major parts. The first part (MAIN0005-0238)\* initiates the computer run. The second part (MAIN0239-0746) is basically the streamlined inner loop of KWM and is used to simulate streamflows. The third part (MAIN0747-0834) sequences the calling of the subroutines used in parameter estimation.

In the first part, MAIN reads the control options and data required

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\*This designation locates the referenced statements within the program listings of Appendix B.

for a computer run, calculates other values which will remain constant no matter what parameter values are chosen later. Control options include the number of station-years included in a given computer run (NSYT), three input data control options (CONOPT), and three operational control options (MNRC, NFTR, NLTR). All will be discussed in detail in Chapter IV. The data required for a computer run include a time-area histogram, values for the parameters estimated by the user, evaporation data, recorded daily streamflows, storm hydrographs, and precipitation data. Sample data are listed in Appendix B of the report by Ross (25). He also described methods of collecting data and for estimating fixed parameters. The two recession constants (BFRC and IFRC) are estimated. Precipitation data are checked for anomalies and adjusted as necessary. Climatological data are converted from the form in which it is read into the arrays needed for flow simulation. A title read from alphanumeric input, the recorded daily streamflows, and the recorded storm hydrographs are printed out for visual comparison.

The second part contains the essence of the streamflow synthesis of KWM. It performs the moisture accounting process to synthesize flows. Depending on the particular run, it can simulate a year of continuous streamflows with 15-minute, 20-minute, or hourly looping. No detailed description of this part of the program will be given in this report. It is amply covered in the description of the Stanford Watershed Model by Crawford and Linsley (10) the HSP Manual by Hydrocomp International (14), and the report of an evaluation study of the Model based on KWM by Ligon et al. (19).

The third part with all its associated subroutine programs is the essence of OPSET. Except for the two recession constants which are evaluated in the first part of MAIN, the remaining eleven of the 13

selected critical parameters are estimated here.

### TRIP 1

The trip number denotes the conditions pertaining when the program passes through the streamflow simulation process in MAIN. During TRIP 1, streamflow routing is bypassed. The procedure starts with the initial middle set of assigned parameter values and simulates a year of continuous streamflows using hourly looping (Rough Cycle). Then Subroutine SETFVP is called to adjust LZC, SUZC, ETLF, BUZC, and SIAC and Subroutine SETBMI is called to adjust BMIR. Using adjusted parameters, another year of streamflow is simulated. SSQM is computed in Subroutine SETFDI and compared with that found previously. The set of parameter values associated with the smaller SSQM is considered as the better one, and its values are saved. The Rough adjustment cycle continues, each time using the last simulation to adjust parameter values for the next simulation cycle until the value of SSQM is less than 0.15 (MAIN0801), or the number of the Rough adjustment cycles exceeds the preassigned minimum number of Rough adjustment cycles (MNRC) and the SSQM values are found to have consecutively worsened at least twice (MAIN0788). In other words, the process continues as long as simulated flows continue to improve. The "twice" enters because sometimes the flows get worse and then start to improve again with the next adjustment. The minimum number of adjustment cycles is necessary because estimates often jump around a lot at first before they settle into a groove converging on a best value.

The optimization procedure then shifts to the Fine adjustment cycle which uses a 20-minute looping interval, and starts with the saved best set of parameter values from the Rough adjustment cycle. The optimization process of the Fine adjustment cycle is essentially



the same as that of the Rough adjustment cycle except for the termination of the adjustment procedure. In the Fine adjustment cycle, the only criterion for termination is for a new set of adjusted parameters to have a larger SSQM than the previous set. The six volume parameter values from the run in the Fine adjustment cycle with the smallest SSQM are taken as the optimal values. A numerical example is given in the description of Subroutine SETFVP (Table 9).

In the Fine adjustment phase, the interflow volume parameter (BIVF) is adjusted if the program finds that the interflow recession constant (IFRC) exceeds 0.30 (MAIN0222-0226, 0769). The adjustment of BIVF is based on matching the first three days of interflow volumes after major streamflow rises, and the best value of BIVF is taken as the adjusted value of the last run instead of the best run of the Fine adjustment cycle (See Subroutine SETBIV). If the value of IFRC is less than 0.3, the interpretation is that a division between base flow and direct runoff is sufficient for that station-year data, and the value of BIVF is set as 0.0 (MAIN0222-0226) without calling Subroutine SETBIV.

#### INITIAL MOISTURE CONDITIONS

OPSET estimates a set of values for the parameters for a particular watershed from one year of data (p. 35). The year starts with some amount of water stored on the land surface, stored in the upper zone, stored in the soil, stored below the water table, etc. (25, p. 24). An appropriate procedure had to be devised for estimating each initial moisture storage within the basin at the start of the water year because computer storage and time restraints made it impractical to simulate several months at the end of the previous water year for the sole purpose of establishing initial conditions. The drier the late summer weather, the further back it would be necessary to go, and the more

trouble this approach would cause.

Fortunately, in fact purposefully, the water year begins at a time (October 1) when moisture storages on an average annual basis are near minimum levels because of the long summer of evaporation excess. Since the user of OPSET can exercise discretion in selecting which years to use, he can avoid years known to immediately follow a large storm in late September. The overland flow unrouted storage (OFUS), the overland flow unrouted storage on impervious surfaces (OFUSIS), the unrouted direct runoff in the channel (URHF), the upper zone storage (UZS), and the interflow storage (IFS) are set to be zero (MAIN0255-0272).

The groundwater storage (GWS) is estimated by substituting estimated October 1 baseflow (OCT1BF) and the base flow recession constant into the simulation equation (MAIN0289). One twentieth of the total recorded October flow is taken as a first estimate of the October 1 base flow. If less than one twentieth of the total recorded October flow occurs on October 1, it is assumed that there was minimal direct runoff that day, and the flow recorded on October 1 is taken as the October 1 base flow. OCT1BF may still exceed the true base flow value. For example, the October 1 flow may contain appreciable direct runoff or interflow but be less than five percent of the total runoff during a wet October. Much of this moisture will have run off two days later. If the recorded October 3 daily flow is less than the calculated third day base flow, OCT1BF is taken as the October 3 recorded flow brought forward in time by twice dividing by BFRC (MAIN0286-0288). After OCT1BF is estimated, the initial groundwater storage (GWS) is estimated by solving the groundwater simulation equation (MAIN0519):

$$GWS = \frac{(OCT1BF/BFRC^{0.5})}{WCFS * BFRL} \quad (6)^*$$

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\*The asterisk (\*) is used in all equations of this report as denoting multiplication in the convention of Fortran IV.

where the numerator is the base flow at the beginning of October 1 (adjusted by one half day from the daily average value), BFRL is the logarithm of the hourly base flow recession constant, and WCFS is a factor to convert cfs into watershed inches.

The lower zone storage (LZS) at the beginning of the year is most difficult moisture storage to initialize. Its value depends on the lower zone capacity (LZC). Since in TRIP 1, LZC is adjusted from cycle to cycle, the value of LZS (unlike the value of all the other initial moisture storage values) needs to be kept in harmony with the value of LZC.

LZS is taken as 6.00 for the first adjustment cycle to be commensurate with the starting trial value for LZC of 12.0. The initial values of LZS for all subsequent adjustment cycles are adjusted by the following rules gradually evolved through trial-and-error experience.

1. LZS is estimated as the end-of-year LZS of previous cycle multiplied by the ratio of the adjusted LZC to the previous cycle multiplied by the ratio of the adjusted LZC to the previous value of LZC. LZS increases with LZC, and this ratio assumes direct proportionality. If the end-of-September value of LZS is greater than the end-of-August value, then the end-of-August value is taken as the end-of-year value of LZS (MAIN0278-0279) under the assumption that an abnormally wet September has raised the value of LZS above a best estimate.

2. In the case when LZC is adjusted by annual runoff (See SETFVP), the initial LZS is estimated by subtracting from the adjusted LZC an amount which is the product of the difference between the adjusted LZC and the previous end-of-year value of LZS and the ratio used to adjust LZC (MAIN0280 and 0781). Subtraction eliminated the possibility of LZS exceeding LZC in the case when the two were of nearly equal value.

3. If after five Rough adjustment cycles the synthesized monthly flows for November and December are way too high or too low, it is taken as a sign that the starting value of LZS may be bad. The "fall trouble index" (FTX) is used to indicate these cases and to adjust LZS as well. When the sum of the two monthly deviation indices is greater than 2.0\* (synthesized flow too high), FTX is set to be 0.90; and when the sum is less than -2.0\* (synthesized flow too low), FTX is set to be 1.10 (STFV0009-0010). The adjustment of starting value of LZS is (MAIN0282)

$$LZS = FTX * BBYLZS * \left( \frac{LZC}{BLZC} \right) \quad (7)$$

where BBYLZS = initial value of LZS used in the previous adjustment cycle which had the smallest value of SSQM (MAIN0799),

BLZC = saved value of LZC used in the previous best adjustment cycle (MAIN0792).

Equation 7 is particularly helpful when LZS is getting out of line from the first adjustment rule because the previous year ended with significantly different moisture storage than the current one.

The saved best starting value (BBYLZS) of the best Rough adjustment cycle is used (MAIN0284) when the program enters fine adjustment or later trips. When the computer run starts from TRIP 2 or TRIP 3, the starting value of LZS is needed as input data (See Chapter IV).

### TRIP 2

The purpose of TRIP 2 is to estimate the four channel routing parameters CHCAP, CSRX, FSRX, and NCTRI to best match the time and peak flow of synthesized to recorded hydrographs. CHCAP

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\*These numbers are defined in Subroutine SETFDI.

and NCTRI are first read as input data while both CSRX and FSRX are initially taken to be 0.98. Up to five recorded hydrograph peaks and times of peak are read as input data. Within TRIP 2, a year of streamflows is synthesized using the other nine parameter values, the preliminary estimates for these four values, and a 15-minute looping interval. During the periods contributing to the recorded hydrographs, synthesized land surface runoffs are saved. Then Subroutine ADJHYD adjusts the total runoff volume to match the recorded volume. The adjusted synthesized runoff provides the inflow hydrograph to be routed through the channel system and compared with the corresponding recorded hydrograph. The estimation procedure calls Subroutine SETHRP to find the best pair of NCTRI and SRX values to match each recorded hydrograph. The optimum value of NCTRI is determined in Subroutine SETHRP, while the SRX values are regressed on the magnitudes of hydrograph peaks to determine how much they really vary with flow by calling Subroutine SETSRP.

When the watershed is small (time of concentration is less than 1.5 hours) and the streamflow routing is done every 15 minutes (CONOPT 92) = 0), Subroutine PREPRD is called at the beginning of TRIP 2 to distribute the hourly rainfalls into the four periods according to a typical unequal distribution so as to better match hydrograph peak (MAIN0377). Peak intensities for periods shorter than one hour are needed to simulate flood peaks from small watersheds. When called, this same unequal distribution is also carried into TRIP 3.

### TRIP 3

Within TRIP 3, the final set of estimated parameter values and 15-minute inner looping are used to simulate a year of streamflows for comparison of "best" synthesized and recorded streamflows. The purpose is simply to provide information needed to observe the effectiveness of the optimization procedure.

## ARRANGEMENT OF SUBROUTINE PROGRAMS

There are 22 subroutine programs used in OPSET. Each subroutine performs its own function as an independent unit; however, some subroutines are closely related to one another in program execution. Such subroutines are grouped together in the presentation in order to make the report more readable and minimize confusion. The subroutines are put into four groups and presented within the group in an order designed to help the reader follow the programmed approach. Table 6 gives the arrangement of the 22 subroutine programs in the following sections. It should be noticed, however, that the subroutines are listed alphabetically in Appendices A and B for the convenience of the reader.

### Subroutine READ

(First Accounting Subroutine)

## CONTEXT

Large amounts of climatological data are required in streamflow simulation. OPSET, as does the KWM, uses a specially developed Subroutine READ to read unformatted data and thereby make data preparation easier. With this subroutine, it is not necessary to check to make sure the data is punched in specific columns, one can use data obtained from others but punched in a different format without repunching, and one has greater freedom to punch explanatory notes directly on the cards as a means of remembering or conveying certain points to others. This subroutine is written in computer machine language and is available on the University of Kentucky Computer Center's IBM system 360/65. A listing is presented by Cline (9, pp. 249-253). The subroutine cannot read alphanumeric data; therefore all alphanumeric data is formatted as usual. An alphanumeric data card

TABLE 6

SUMMARY OF SUBROUTINE PROGRAMS SHOWING  
MNEMONIC DEFINITIONS, PROGRAM LISTING LOCATIONS,  
AND ABBREVIATED NAMES

Subroutine Name	Mnemonic Definition	Program Listing Location	Abbreviated Name
A. <u>Accounting Subroutines</u>			
READ	Reads numerical input data	Cline (9, pp. 249-253)	---
DAYNXT	Determines next day of the yr.	Appendix A	DYNX
DAYSUM	Sums daily values to get monthly and annual totals	Appendix B	DYSM
DAYOUT	Prints out daily values in tabular form	Appendix A	DYOT
EVPDAY	Determines dated pan evaporation totals	Appendix A	EVDY
PRECHK	Checks precipitation-stream- flow anomalies and adjusts precipitation where necessary	Appendix B	PRCK
PREPRD	Divides hourly precipitation totals among periods for small basins	Appendix A	PREP
B. <u>Recession Constant Subroutines</u>			
RECESS	Establishes recession sequences	Appendix B	RCSS
SET2RC	Sets 2 recession constants	Appendix B	ST2R
SET1RC	Sets 1 recession constant	Appendix B	ST1R
C. <u>Land Phase Parameter Subroutines</u>			
SETFVP	Sets new values of flow volume parameters	Appendix B	STFV
SETFDI	Sets values of flow deviation indices	Appendix B	STFD
SETBMI	Sets new value of basic maximum infiltration rate within watershed	Appendix B	STBM
SETRBF	Sets values of interflow and base flow at recession beginning	Appendix B	STRB
SETBIV	Sets new value of basic interflow volume factor	Appendix B	STBV

TABLE 6 (cont'd.)

Subroutine Name	Mnemonic Definition	Program Listing Location	Abbreviated Name
D. <u>Channel Routing Parameter Subroutines</u>			
STRHRS	<u>Sets beginning and end hours of runoff entering recorded hydrographs</u>	Appendix B	SHRS
ADJHYD	<u>Adjusts synthesized hydrograph volumes</u>	Appendix B	ADJH
SETHRP	<u>Sets best values of hydrograph routing parameters</u>	Appendix B	STHP
FIXTRI	<u>Fixes values of time routing increments</u>	Appendix B	FXTI
TIMERT	<u>Performs channel time routing</u>	Appendix B	TMRT
STORRT	<u>Performs channel storage routing</u>	Appendix B	SRRT
SETSRP	<u>Sets best values of storage routing parameters</u>	Appendix B	STSP



must follow a card containing numerical data; it cannot follow a card containing only a comment.

### PURPOSE

Subroutine READ is employed to read numerical input data (real or integer) from the punched data cards. The input data can be coded and punched onto computer cards without format and with explanatory messages.

### PROCEDURE

The following two examples illustrate the use of Subroutine READ. One may want to read the values of real variables ALPHA and BETA and integer variable IOTA from one or more punched cards without worrying about the specific locations of these values on the cards. The instruction `CALL READ (ALPHA, BETA, IOTA)` would cause the data cards to be scanned consecutively from left to right with the first value found being stored as ALPHA, the second as BETA, and the third as IOTA, regardless of the spacing of the values on the card or how many blank cards are passed before the data is found. However, it is necessary that there be at least one blank column between any two values. As a second example, the instruction `CALL READ (DATA (I), I = 1, 27)` will cause the first 27 values encountered to be stored in array DATA (27). The 27 values may be placed on a single card or may be spread over any desired number of cards.

Variable values should be punched on the cards in the calling order of the corresponding variable names, and the type of values (real or integer) should agree with the specification of the variable names. An integer number read for a real variable will usually cause floating point underflow in program execution. A floating point number read for an integer variable will usually disrupt program execution by misdirecting program control.

When an asterisk (\*) is encountered in scanning a card, READ skips on to the next card, ignoring all notes punched on columns to the right of the asterisk. This feature makes it very convenient to place identifying data labels, which may be several cards long, throughout the data list. Many examples of such labeling can be found in Appendices A and B in the report by Ross (25).

Subroutine DAYNXT  
(Second Accounting Subroutine)

CONTEXT

Frequently within the program, it is necessary to begin at the first day of the water year and loop through the subsequent days in chronological order. The numbers assigned the days of the year within the program go from 274 for October 1 to 365 for December 31, from 1 for January 1 to 59 for February 28, 366 for February 29 in leap years, and from 60 for March 1 to 273 for September 30.

PURPOSE

Because the required order of days cannot conveniently be prescribed by a Fortran DO loop, Subroutine DAYNXT is used to determine the number of the next day of the year with the number of the current day given.

PROCEDURE

The number of the next day of the year is the number of current day of the year plus 1, unless:

(1) the current day is February 28 in a leap year, then the number of the next day is set equal to 366, and the following day is set equal to 60.

(2) the current day is December 31, then the number of the next day is set equal to 1.

Subroutine DAYSUM  
(Third Accounting Subroutine)

CONTEXT

The estimation of the runoff volume parameters is based on an objective function matching synthesized to recorded flow volumes on a monthly time grid (pp. 19-20). By comparing the twelve recorded and synthesized monthly flows, one can observe patterns in seasonal variance and make adjustments in search of a better set of parameter values.

PURPOSE

Subroutine DAYSUM sums monthly and annual flow volumes from daily values. The monthly and annual totals are used in the printed tabulations and in the process estimating five runoff volume parameters.

PROCEDURE

DAYSUM first sums daily values to get cumulative flows through the end of each month in the calendar year. The annual runoff is the amount accumulated through December. Monthly flows are then calculated from the excess of the cumulative flows at the end of each month over that at the end of the previous month. Finally, the monthly flows are converted to a water year order. Both recorded and synthesized daily streamflows are summed to obtain the annual and monthly flow volumes. (MAIN0127, 0686).

DISCUSSION

The real issue associated with Subroutine DAYSUM is deeper than the need to sum daily values to get long-term totals. OPSET is seeking to estimate the set of watershed parameter values minimizing the deviation between recorded and synthesized flow totals. The issue is

how many flow periods should be used (p. 19). Too few will not properly test the distribution of synthesized flows over the year. Too many will require routing to simulate runoff at the watershed mouth during the right period. The quick simulation of flow volumes for trial sets of parameter values requires that the routing be bypassed. Therefore, the flow periods must be long enough so that the shift of runoff volumes from a later to an earlier period by ignoring channel routing delay will have a relatively small effect on total period volumes. Monthly periods provide a convenient compromise between the effects of rainfall in one period producing runoff in the next period and the need to consider the seasonal distribution of runoff. The OPSET user needs to be wary, however, of selecting years of data in which major storms occur the last day of any month, particularly when both that month and the following month otherwise have low flows because severe cases can badly upset the estimating procedure. If OPSET is to be applied to larger watersheds, it may be advisable to add an automatic delay feature moving the precipitation timing later by a number of hours indexed to watershed size to reduce the boundary problem without taking time to route.

#### Subroutine DAYOUT

(Fourth Accounting Subroutine)

#### CONTEXT

It is desirable to be able to print out a complete listing of daily values of various items in a neatly labelled table. As this requires about 40 lines of programming, it is convenient to have a single subroutine for this purpose.

#### PURPOSE

Subroutine DAYOUT prints a table of daily values given the magnitude of each desired value, the day of the year of the last day of each

month, and the number of days in the year.

### PROCEDURE

The tabulation process converts values arranged by calendar-year day (VDCY) into values arranged by month day (VDMD) and then prints the daily values arranged by month in the water-year month order-- as shown on Table 7. Provision is made for leap year and the irregular number of days per month.

### Subroutine EVPDAY (Fifth Accounting Subroutine)

### CONTEXT

One of the aspects of data collection for the Stanford Watershed Model which has been most troublesome to users has been the development of suitable evaporation data. Evaporation pans are much fewer and more scattered than either precipitation or stream gages, and their operation is often discontinued during periods of subfreezing weather.

According to the quality of the data he has available, the user of OPSET can choose from among three approaches to reading the necessary information. These in order of decreasing refinement and as specified by control option 1 (25, pp. 29-35) are:

0. Individual values for every day of the year. These should be taken directly from a nearby record and reflect the weather conditions on that day.

1. Average values over 10-day periods. Where actual recorded values are available for the same days for which flows are to be synthesized, it is both more work and less accurate to average the numbers by 10-day periods. The primary purpose of this approach is to serve other situations. A user may have to estimate evaporation

TABLE 7  
 TABLE PRINTED BY SUBROUTINE DAYOUT  
 1967-68 WATER YEAR STREAMFLOWS: POND CREEK, LOUISVILLE, KY.

Day	Recorded Flows											
	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.
1	8.5	104.0	106.0	28.0	155.0	18.0	314.0	18.0	160.0	14.0	20.0	6.2
2	8.5	72.0	499.0	28.0	684.0	18.0	145.0	17.0	370.0	19.0	15.0	7.4
3	8.5	37.0	294.0	29.0	184.0	17.0	169.0	15.0	198.0	26.0	42.0	8.8
4	8.5	56.0	94.0	28.0	119.0	17.0	2840.0	14.0	134.0	24.0	24.0	9.0
5	8.5	28.0	63.0	27.0	91.0	22.0	493.0	12.0	104.0	22.0	11.0	17.0
6	23.0	18.0	51.0	24.0	74.0	25.0	246.0	10.0	87.0	20.0	12.0	15.0
7	11.0	16.0	46.0	21.0	66.0	24.0	166.0	12.0	71.0	18.0	17.0	15.0
8	29.0	12.0	31.0	19.0	57.0	24.0	131.0	12.0	57.0	14.0	30.0	15.0
9	31.0	10.0	23.0	17.0	51.0	31.0	93.0	16.0	48.0	18.0	32.0	14.0
10	11.0	7.6	30.0	18.0	36.0	63.0	69.0	60.0	39.0	27.0	298.0	14.0
11	11.0	64.0	102.0	20.0	32.0	98.0	56.0	138.0	30.0	33.0	310.0	14.0
12	10.0	50.0	80.0	22.0	30.0	566.0	41.0	68.0	21.0	18.0	290.0	13.0
13	24.0	19.0	49.0	23.0	26.0	174.0	27.0	46.0	20.0	14.0	240.0	13.0
14	94.0	18.0	89.0	25.0	24.0	113.0	297.0	42.0	20.0	15.0	190.0	14.0
15	16.0	16.0	125.0	24.0	24.0	93.0	340.0	38.0	20.0	31.0	160.0	12.0
16	12.0	12.0	69.0	22.0	24.0	251.0	128.0	34.0	102.0	39.0	135.0	10.0
17	59.0	11.0	164.0	23.0	21.0	166.0	115.0	30.0	68.0	19.0	110.0	7.3
18	28.0	9.0	221.0	27.0	19.0	116.0	119.0	31.0	40.0	16.0	90.0	12.0
19	13.0	7.6	101.0	43.0	20.0	96.0	88.0	15.0	24.0	27.0	76.0	16.0
20	13.0	6.7	84.0	70.0	28.0	134.0	177.0	9.1	15.0	18.0	62.0	13.0
21	12.0	5.8	285.0	169.0	31.0	1180.0	88.0	8.0	14.0	12.0	50.0	13.0
22	12.0	5.8	615.0	207.0	28.0	991.0	60.0	8.0	13.0	13.0	41.0	12.0
23	12.0	7.5	120.0	204.0	21.0	488.0	65.0	101.0	15.0	20.0	34.0	6.8
24	55.0	16.0	79.0	120.0	16.0	499.0	51.0	160.0	15.0	16.0	29.0	8.1
25	120.0	13.0	68.0	77.0	15.0	497.0	36.0	85.0	20.0	32.0	44.0	13.0

TABLE 7 (cont'd.)

Day	Recorded Flows											
	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.
26	20.0	10.0	57.0	55.0	16.0	324.0	31.0	1190.0	18.0	44.0	33.0	10.0
27	16.0	8.2	42.0	50.0	16.0	191.0	29.0	674.0	20.0	74.0	25.0	11.0
28	15.0	6.4	37.0	56.0	16.0	146.0	25.0	246.0	18.0	87.0	17.0	8.2
29	13.0	30.0	32.0	64.0	17.0	126.0	22.0	160.0	15.0	24.0	13.0	6.1
30	11.0	460.0	29.0	406.0		112.0	20.0	116.0	13.0	18.0	6.7	5.9
31	12.0		28.0	207.0		207.0		81.0		22.0	5.8	

from other climatological data because no, or an incomplete, pan record is available and evaporations must be estimated from other climatological data (21, pp. 99-108). A lot of time can be saved by only going to the charts for one day out of ten. In other cases, the user may want to synthesize many years of record but have to rely on a few years of pan data. He can take 10-day period averages for the recent years as reasonable estimates for corresponding 10-day periods in earlier years.

2. Total average annual lake evapotranspiration (EPAET) and the average annual number of days of measurable rain recorded per year (MNRD). Where one desires to synthesize a large number of flow records in the same general area and has only fragmentary evaporation data, it is convenient to be able to use the model without having to work up an entire data set for each location. Subroutine EVPDAY uses regional data to distribute a total annual evaporation over the days of the year. Rainy days enter into the distribution because more evaporation usually occurs on clear than on rainy days. The distribution programmed in OPSET was developed from Kentucky data in the manner described by Ross (25, p. 35), but a check against California data showed that it did not work badly there either. The distribution seems fairly good for central United States but should be slightly more summer peaked for northern latitudes or higher elevations and slightly flatter for areas further south.

#### PURPOSE

Subroutine EVPDAY distributes the estimated annual evapotranspiration at a particular location among the days in the water year based on the Kentucky distribution of daily fractions of the average annual total.

#### PROCEDURE

The sequence of computations required to estimate daily potential evapotranspiration totals from EPAET using Subroutine EVPDAY is



relatively straightforward. First, the annual potential evapotranspiration (EMAET) which would occur during the year if no rain were recorded is estimated by using the equation (MAIN0109):

$$\text{EMAET} = \text{EPAET} \left( \frac{365 + \text{MNRD}}{404} \right) \quad (8)$$

where EPAET and MNRD are the estimated potential annual evapotranspiration and the mean annual number of rainy days respectively and are read as input data (25, pp. 31-33). If rain were equally likely to occur any day of the year, Equation 8 would have  $365 + 0.5 * \text{MNRD}$  in the denominator to offset the reduction of evaporation by one half on rainy days (MAIN0186). The value of 404 worked a little better for Kentucky where rainy days are slightly more likely to occur during times of the year when evaporation is low.

Within Subroutine EVPDAY, EMAET is multiplied by each of the 366 daily evaporation fractions derived in the method described by Ross (25, p. 35). Later each value is multiplied, by one half if rain is recorded during the day (MAIN0186, 0358).

## DISCUSSION

The concept behind Subroutine EVPDAY is very helpful for use in locations where evaporation data is sparse; however, one of the other two approaches to input data should be used when more extensive data is available. The precise numbers used in apportioning the total annual evaporation among the days should be adjusted if Subroutine EVPDAY is to be used in a climatic setting much different than that of Kentucky.

Subroutine PRECHK  
(Sixth Accounting Subroutine)

## CONTEXT

In streamflow simulation and particularly where one must rely on precipitation gages located at a distance from the watershed, one must continually cope with the problem of the gaged record inaccurately

representing average precipitation over the basin. For years containing major storms when the input data deviates significantly from precipitation actually experienced within the basin, OPSET will adjust the watershed parameters in an attempt to compensate. For example, data specifying far too little rainfall for a given runoff volume will suggest parameter values which greatly understate the ability of the watershed to store moisture. Such distorted parameter estimates can cause wildly fluctuating estimates of parameter values and greatly increase the number of years whose results must be averaged to get a good set of values (pp. 173-176).

Early in the development of OPSET, no check was made for major precipitation anomalies; and a great deal of difficulty was experienced with fluctuating parameter estimates. Therefore, it became necessary to develop a subroutine to filter out the worst storms, mostly spotty summer thunder showers. As examples, storms with runoff volumes exceeding recorded rainfall volumes or with no recorded runoff from very large rainfalls simply do not make hydrologic sense.

#### PURPOSE

Subroutine PRECHK is used to check for precipitation-streamflow anomalies and adjust precipitation where necessary.

#### PROCEDURE

In a study on the effect of rainfall variability on streamflow simulation, Dawdy and Bergmann (11) discussed errors caused in estimation of parameter values by errors in measurement of storm volume and intensity over the basin. In the application of OPSET, cases were found when recorded rainfall was simply not commensurate with recorded streamflow. Improper estimation of parameter values is inevitable if there are data errors in either the recorded flow or the recorded precipitation. The most severe problem is caused by the raingage being too far from the watershed or by trying to represent too large a watershed with too few gages.

In order to balance the effects of positive and negative errors, one may use many years of data on the same watershed. However, it was found that the same goal can be achieved by checking the data for precipitation-streamflow anomalies and adjusting the rainfall data at a significant savings in the time and cost of data compilation and computer execution. As a result, the required number of data years are much fewer.

Rainfall adjustments were made in cases where the recorded streamflow rises significantly exceed the volume of recorded rainfalls, or where very large rainfall but small or no streamflow rises are recorded. For most station-years in the OPSET trial runs of Kentucky watersheds, only two or three summer thunderstorm rainfalls are adjusted. Table 8 summarizes the number of adjustments made by watershed.

Two items were used to check for discrepancies between recorded streamflows and recorded rainfalls. One was daily recorded flow rise (RFRISE), and the other was the recorded watershed rainfall on the corresponding day (RWRAIN). RFRISE is the net increase in flow volume during the current day over the previous day in inches. RWRAIN is the rainfall recorded for that day as estimated from the accumulated recorded hourly rainfalls in inches. Inspection of the results when Subroutine PRECHK was not used showed the primary difficulty to occur during the summer months. Therefore, the checking and adjusting processes were made from the end of March through the end of September (i. e. , days of the year 90 through 273).

If on any day, RFRISE exceeds 0.1 inch and RWRAIN is less than 0.05 inch, an indication that there was a significant streamflow rise but very little (if any) rainfall was recorded, the adjustment is made by adding an amount of rainfall which was set after comparing the results of a series of runs with OPSET to make the total be 1.0

TABLE 8  
 NUMBER OF ADJUSTMENTS MADE FOR RAINFALL  
 ANAMOLIES FOR TESTED WATERSHEDS

Name	Area mi. <sup>2</sup>	Distance from Gage to Watershed Centroid		Anamolies		Smallest SSQM
		Recording Gage	Storage Gage	+ / yr	- / yr	
Bear Branch	2.21	14.86	14.00	1.67	0.67	0.654
Cane Creek	0.67	10.75	6.60	0.00	0.33	0.194
Cave Creek	2.53	2.00		0.33	1.00	0.788
Elkhorn Creek	473.0	10.00	4.00	0.00	0.00	4.003
Flat Creek	5.63	7.50	7.10	0.67	0.00	7.660
Green River	22.4	16.00	16.00	0.00	0.67	4.208
Helton Branch	0.85	10.94	6.70	0.33	0.67	0.226
McDougal Creek	5.34	3.00		1.00	0.33	1.544
McGills Creek	2.14	15.00	5.50	0.67	1.00	1.613
M. Fork Beargrass Creek	18.9	11.20		0.13	0.13	1.584
Perry Creek	1.72	19.00		0.33	0.33	0.695
Pond Creek	64.83	3.50		0.17	0.00	3.431
Rock Lick Creek	20.1	13.00		0.67	0.67	9.001
Rose Creek	2.10	9.00		0.67	2.00	0.656
S. Elkhorn Creek	24.00	0.50	5.50	0.00	1.33	10.656
S. Fork Beargrass Creek	17.2	5.00		0.33	0.17	4.062
S. Fork Little Barren River	18.3	4.00		0.33	1.33	10.246
Stillwater Creek	24.00	23.00		0.67	2.00	2.545
West Bays Fork	7.47	15.50	3.00	0.00	0.00	3.905
Wood Creek	3.89	2.00		0.33	0.67	3.630

inch plus twice the streamflow rise (PRCK0026). If RFRISE exceeds 0.1 inch, and RWRAIN is less than RFRISE yet greater than 0.05 inch, an indication that there was a significant streamflow rise which exceeded a significant recorded rainfall, the recorded rainfalls are multiplied by twice the ratio of RFRISE to RWRAIN (PRCK0031,0039).

In the situation of a large recorded rainfall but little recorded streamflow rise, the precipitation is reduced by multiplication by ten times of the ratio of RFRISE to RWRAIN, if 1) RWRAIN exceeds a preassigned maximum value of rainfall without runoff (RMWR) and RFRISE is less than two percent of RWRAIN, or 2) RWRAIN exceeds three inches and RFRISE is less than five percent of RWRAIN (PRCK 0022, 0023, 0033). The value of RMWR varies with watershed size and soil surface conditions. One would expect RMWR to be largest for a larger watershed following dry weather conditions. A value of 1.25 inches is used in late spring and early summer, while 2.00 inches is used in later summer (day of the year greater than 200) when the expected soil moisture is less. For watersheds with areas over 28 square miles, a value of 2.00 for RMWR is always used since more rainfall is required to produce a significant streamflow rise from larger watersheds.

#### DISCUSSION

As OPSET had previously been run for a number of watersheds without checking for rainfall-runoff anomalies, the process used to establish the adjustment rules for Subroutine PRECHK began by inspecting the output to find storms causing difficulty. Recorded rainfalls, runoffs, and dates for these storms were tabulated as were like statistics for the storms OPSET was able to handle without undue distortion of parameter estimates. While the decision on what is or is not undue distortion is subjective, a distinct boundary could be seen

between the two types of events as tabulated; and this boundary provided the basis for the dates and volumes programmed into PRECHK to separate out storms requiring rainfall adjustment. The goal of PRECHK is to reduce the number of years of record required to estimate a set of parameter values, and one can argue that the quality of the adjustment rules used affects the standard deviation of estimates among years more than the mean of estimates from many years of record. Furthermore, the subroutine will seldom select any storms for adjustment in a well instrumented watershed.

The multipliers or additive rainfalls used for adjusting the selected storm rainfalls were selected by trial and error for representative years. Trial results were compared to minimize SSQM and provide estimates of parameter values commensurate with those estimated from other years of record for the same watershed. Those using OPSET in climatic settings where thunderstorms or other highly localized precipitation events frequently occur outside the months from April through September or where prevailing soil moisture conditions vary radically from those in Eastern United States may be able to improve their results by adjusting some of the empirical constants used in PRECHK.

#### Subroutine PREPRD

(Seventh Accounting Subroutine)

#### CONTEXT

The Kentucky Watershed Model employs hourly precipitation data, but moisture movement is simulated by 15-minute periods. While 15-minute rainfalls could just as well be read from the point of view of the Model (this may be done optionally by the HSP Model, 14), this information may be difficult to obtain without time-consuming analysis

of the original records. Hours are the shortest time periods for which precipitation data are available on a continuous basis in published sources. Hourly totals are used in the Model, and 15-minute values are simply taken as one quarter of the hour totals.

The assumption of precipitation uniformly distributed over the hour is not correct, but it does not make much difference for watersheds having routing lags over two or three hours. Precipitation never falls at a constant rate for periods anywhere near an hour; however, the flows at the mouth of a larger watershed so blend rain falling at different times that hourly precipitation can be evenly distributed for streamflow simulation.

The problem comes with smaller watersheds. Fifteen-minute rainfall data would be desirable; but since it may not be readily available, the question is whether the results could be improved by using some pattern of unequal rather than equal distribution of hourly totals among 15-minute periods. The problem is most significant in connection with underestimation of flood peaks from short intense rainstorms on small watersheds.

#### PURPOSE

Two basic approaches to unequal distribution of hourly rainfalls among 15-minute periods are possible. One would be to employ a stochastic process randomly selecting from observed hourly rainfall patterns according to their observed frequency of occurrence. This approach was not used because it is complicated to program, expensive in terms of computer time to execute, and requires extensive data collection and analysis to construct. Furthermore, the difference in distributions which happen to be randomly selected for the various hours in the storms used to estimate the channel routing parameters, may well have an adverse influence on parameter estimation.

Randomly selected distributions are likely to differ sharply from a particular historical experience and introduce a new source of parameter estimation error.

The second approach is to utilize an average distribution. The largest 15-minute precipitation would be the portion of the hourly rainfall occurring on the average during the wettest of the first, second, third, and fourth quarter hours. The second largest would be the portion during the second wettest, etc. Subroutine PREPRD divides hourly precipitation among the 15-minute periods using the average distribution approach.

### PROCEDURE

The average distribution curve used is shown in Figure 2 and taken from a similar curve (26, p. 32) developed by U. S. Bureau of Reclamation for the area in the United States east of 105° meridian. This cumulative distribution curve of rainfall within an hour gives four incremental fractions for successive 15-minute periods: namely 0.46, 0.28, 0.16, 0.10. Because of the lack of data and likelihood of a negligible effect, the difference between peak consecutive 15 minutes and peak clock 15 minutes was neglected.

The next issue was how to arrange these fractions in an order that gives a reasonable approximation of patterns of hourly precipitation. For example, the rainfall distribution within an hour when the precipitation total exceeds both the previous and the succeeding hour may be quite different from that within an hour when the total is less than either the previous or the succeeding hour.

PREPRD uses four patterns of hourly precipitation rainfall fractions and selects from among them by comparing current hourly precipitation with that during previous and succeeding hours. They are:

1. If the current hourly precipitation exceeds both the previous



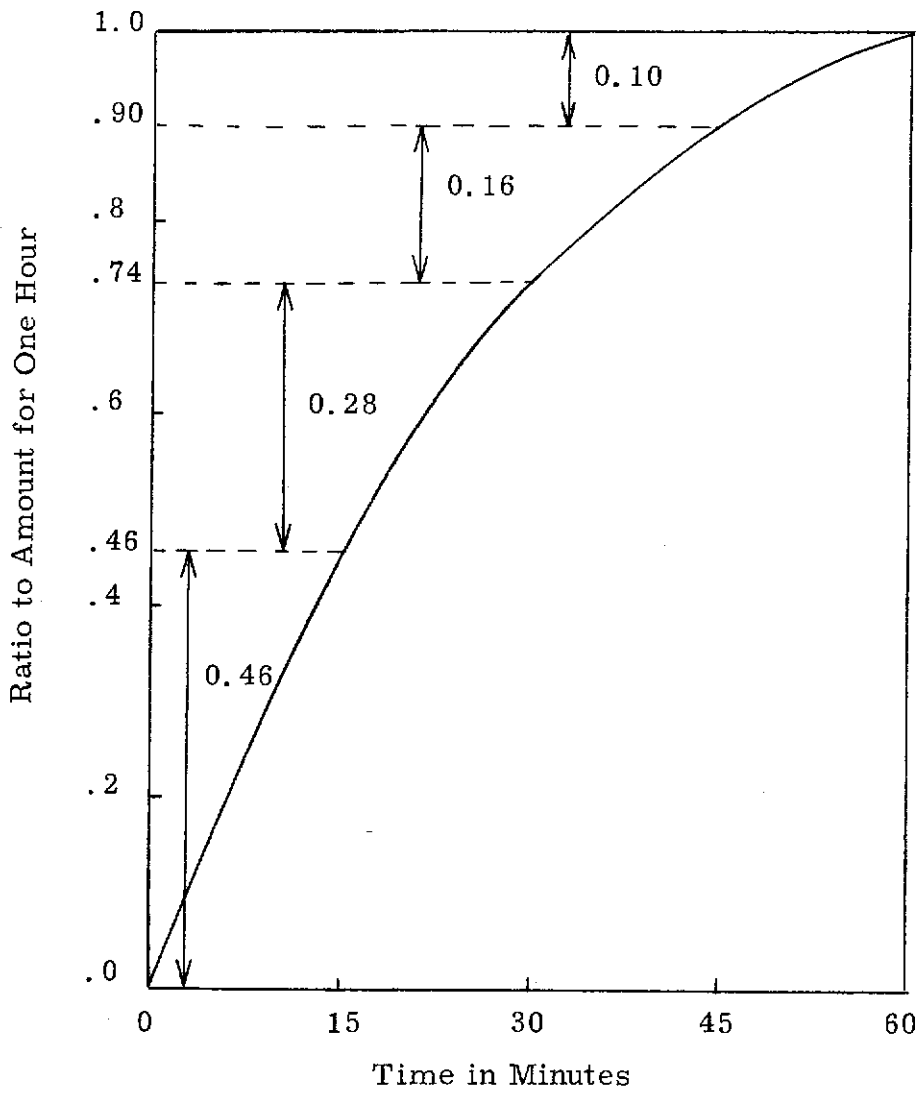


FIG. 2. Distribution of 1-hour Rainfall for Area East of 105° Meridian.

and the next hour, the four 15-minute period precipitations are taken as 0.10, 0.28, 0.46, and 0.16 of the hourly total.

2. If the current hourly precipitation is less than both the previous and the next hour, the order is 0.28, 0.10, 0.16, and 0.46.

3. If the current hourly precipitation exceeds the previous hour but is less than the next hour, the order is 0.10, 0.28, 0.16, and 0.46.

4. If the current hourly precipitation is less than the previous hour but exceeds the next hour, the order is 0.46, 0.16, 0.28, and 0.10.

PREPRD is called only during TRIPS 2 and 3 and only then when stream routing is being done on a 15-minute basis and the initial estimate of the time of concentration is less than 90 minutes (MAIN 0047, 0377).

### DISCUSSION

While consistent use of the average distribution of hourly precipitation among 15-minute periods and the arbitrary selection of order for the four periods within the hour does not duplicate actual historical storm patterns, it does simulate higher peaks from small watersheds. The approach thus reduces the systematic simulating bias toward low flow peaks from small watersheds, and this is the primary justification for its use.

### Subroutine RECESS

(First Recession Constant Subroutine)

### CONTEXT

The Stanford Watershed Model required base flow and interflow recession constants as input data. Crawford and Linsley (10, p. 68) recommend the graphical approach suggested by Barnes (3) to estimate these two values. It requires plotting the logarithm of recession flow

against time and measuring the slopes of two straight lines on the plot. More recently, James and Thompson (18) developed a least squares method to estimate these two recession constants from daily flow totals in an attempt to avoid the human randomness of graphical curve fitting and the time spent in obtaining and plotting complete hydrographs.

Subroutine RECESS selects recession flow sequences from recorded daily streamflows and then calls Subroutines SET2RC and/or SET1RC to evaluate the recession constants from each selected sequence. It estimates average values of the two recession constants by weighting sequence values proportional to sequence length because longer sequences were found to give more reliable results.

For sequences with both baseflow and interflow, the first day values of these two flow components are estimated by a least squares method for use in estimating the basic maximum infiltration rate (BMIR) and basic interflow volume factor (BIVF). Subroutine SETRBF is called from RECESS (RCSS0110) to make these estimates.

### PURPOSE

Subroutine RECESS is used to examine daily flows throughout the year to pick out recession sequences for determining the two recession constants (BFRC and IFRC) and to establish volumes of base flow and interflow as a first step in estimating BMIR and BIVF.

### PROCEDURE

The sequence selection procedure described by James and Thompson (18) is programed in RECESS. For each station-year, up to 20 flow sequences are selected, and the length of each sequence has the maximum limit of 50 days.

The minimum number of days required for an estimate of a single recession constant is two. For two constants, the minimum

is four days. However, longer sequences provide better estimates from both models as the estimating procedure goes from a forced solution to an estimate minimizing random observation error by statistical methods. An analysis of the consistency of results among estimates from sequences of different lengths showed sequences at least 8 days long to be desirable. As the first or peak runoff day should not be included in the data because it usually contains a large percentage of direct runoff, the first pass through the daily flows has a minimum acceptable number of recession sequence days (MRSL) equal to 9. If three or fewer such sequences are found, MRSL is reduced to 6; and a second pass is made. Thus recession sequences as short as 5 days are used where longer sequences do not exist. If the data do not contain a single recession sequence as long as 6 days in the 365 daily values, OPSET will be halted; but this situation was never encountered in any real data.

As recessions from very small flow peaks seldom give good results (possibly because measurement errors in flow differences from day to day tend to be relatively larger with low flows) a criterion that the second-day flow should be either greater than 10 cfs or greater than  $0.4 * \text{AREA}$  (where AREA is the watershed area in square miles) is used. Here, the second day is defined as the day after the peak or the first day whose flow is actually used in estimating the recession constants. The AREA criterion is used to increase the number of accepted sequences from small watersheds. The recession sequence is terminated by a flow rise exceeding  $0.1 * \text{SQRT}(\text{AREA})$  cfs. A small positive value is used to avoid ending the sequence at very small rises stemming from channel precipitation or non-hydrologic causes. The magnitude of these effects also tend to vary with the size of the watershed (RCSS0011).

After the recession sequences are established and printed, Subroutine SET2RC and SET1RC are used to estimate IFRC and BFRC. Subroutine SET2RC is always called first to try to estimate values for both recession constants (RCSS0076) from each recession sequence unless the sequence is found to begin with a relatively low flow and flat recession. In this case, the low probability of significant interflow being present made it advisable to assume only base flow was present from the beginning and use Subroutine SET1RC. If Subroutine SET2RC cannot produce two estimates in the range preselected as being reasonable (See Subroutine SET2RC), the data sequence is next used in Subroutine SET1RC to estimate a single recession constant (BFRC). If Subroutine SET1RC does not come up with a value of BFRC between 0.6 and 1.2, the sequence is entirely discarded.

The sequence estimates for BFRC and IFRC are then used to obtain average values by weighting each sequence value proportional to the length of the sequence. In the weighting, the base flow days include the length of all accepted sequences while the interflow days count only days in those sequences with acceptable IFRC values. Also, a maximum of 20 days is used in the interflow weighting. Interflow estimation is not significantly improved from longer sequences (also 95 percent of sequences with interflow are shorter than 20 days).

The annual average values are then inspected for reasonableness. If BFRC is greater than 0.99, it is set equal to 0.99; and if BFRC is less than 0.70, it is set equal to 0.70. (RCSS0105, 0106). If IFRC is less than 0.30, it is assumed that the watershed can be satisfactorily modeled by using only direct runoff and base flow. Interflow is either minimal or so rapid it can just as well be classified with direct runoff. IFRC is nominally set equal to 0.10, and this value is subsequently used throughout the program as a test to exclude interflow from the

flow simulation (MAIN0222, 0225).

Finally, given the weighted average values of BFRC and IFRC, the sequences with interflow and base flow are used to estimate values of interflow and base flow at the beginning of each recession sequence by calling Subroutine SETRBF (RCSS0110).

### Subroutine SET2RC

(Second Recession Constant Subroutine)

#### CONTEXT

Subroutine RECESS selects from the complete water year tabulation of recorded daily flows those sequences sufficiently long to provide an adequate basis for estimating recession constants. Separation of subsurface flow into interflow and base flow requires two recession constants; however, a given recession sequence may contain only one of the two flow types. The approach was to try to estimate both constants if possible but revert to the estimation of only one where that is all the data allows.

#### PURPOSE

Subroutine SET2RC is used to estimate two recession constants (BFRC and IFRC) for each recession sequence. It returns to Subroutine RECESS either with both values or the message that data at hand does not yield two real solutions.

#### PROCEDURE

The subroutine follows the approach of James and Thompson (18) of fitting the recession sequence  $q_0, q_1, q_2 \dots q_n$  into the model

$$q_t = K_b Q_{b,t-1} + K_i Q_{i,t-1} + \epsilon_t \quad (t = 1, 2, \dots, n) \quad (9)$$

where  $q_t$  = the recorded daily flow on  $t^{\text{th}}$  day of the sequence under consideration;

$Q_{b,t-1}$  = the base flow on the  $t-1^{\text{th}}$  day;

$Q_{i,t-1}$  = the interflow on the  $t-1^{\text{th}}$  day;

$K_b$  = base flow recession constant, BFRC;

$K_i$  = interflow recession constant, IFRC;

$\epsilon_t$  = a random error on the  $t^{\text{th}}$  day.

By assuming that both base flow and interflow follow a linear model, they derived the expression:

$$q_t = (K_b + K_i) q_{t-1} - K_b K_i q_{t-2} + \eta_t \quad (10)$$

where

$$\eta_t = \epsilon_t - (K_b + K_i) \epsilon_{t-1} + K_b K_i \epsilon_{t-2} \quad (11)$$

For convenience, let

$$\alpha = K_b + K_i \quad (12)$$

$$\text{and } \beta = -K_b K_i \quad (13)$$

then the model (Equation 10) became

$$q_t = \alpha q_{t-1} + \beta q_{t-2} + \eta_t \quad (14)$$

The least squares estimates of  $\alpha$  and  $\beta$  ( $\hat{\alpha}$  and  $\hat{\beta}$ ) were found by solving the "normal equations"

$$\begin{bmatrix} \sum_{t=1}^{n-1} q_t^2 & \sum_{t=0}^{n-2} q_t q_{t+1} \\ \sum_{t=0}^{n-2} q_t q_{t+1} & \sum_{t=0}^{n-2} q_t^2 \end{bmatrix} \begin{bmatrix} \hat{\alpha} \\ \hat{\beta} \end{bmatrix} = \begin{bmatrix} \sum_{t=1}^{n-1} q_t q_{t+1} \\ \sum_{t=0}^{n-2} q_t q_{t+2} \end{bmatrix} \quad (15)$$

Combining equations 12, 13, and 14, and bearing in mind that  $K_b$  is always greater than  $K_i$  because the base flow recedes slower, the estimated  $K_b$  and  $K_i$  were shown to be

$$\hat{K}_b = \frac{\hat{\alpha} + \sqrt{\hat{\alpha}^2 + 4\hat{\beta}}}{2} \quad (16)$$

$$\hat{K}_i = \frac{\hat{\alpha} - \sqrt{\hat{\alpha}^2 + 4\hat{\beta}}}{2} \quad (17)$$

if the discriminant  $\hat{\alpha}^2 + 4\hat{\beta}$  is non-negative. If the discriminant is negative (Equation 13 shows  $\beta$  to be negative), the complex estimates of  $K_b$  and  $K_i$  have no physical meaning. There also has to be at least four  $q$ 's for the matrix of coefficients in Equation 15 to be non-singular.

### DISCUSSION

If the recession sequence data can be represented by two distinct flow categories, each with a fixed linear recession constant, Subroutine SET2RC will provide a least squares estimate of two constants and designate the larger BFRC and the smaller IFRC. However, observed recession data will deviate from the model because of non-linear recession of subsurface runoff, the presence of small quantities of direct runoff from storms too small to cause a flow rise, flow measurement errors, or other reasons (18). If Subroutine SET2RC is applied to a recession sequence which is best modeled by a single recession constant, it will estimate a value for that constant. The estimate for the other constant will be based on error term residuals and have no physical meaning. When either  $K_b$  or  $K_i$  falls outside the reasonable range or can only be estimated as a complex number, the results with Subroutine SET2RC are discarded, and Subroutine RECESS shifts to Subroutine SET1RC to try to estimate a single value. The single constant is assumed to represent base flow as



it is difficult to picture a prolonged period of time when all flow is interflow.

The reasonable ranges were taken as 0.6 to 1.2 for  $K_b$  and -0.4 for 0.8 for  $K_i$ . These ranges are broadly defined in order to include sequences producing estimates with positive and negative random estimating error. As many sequences are averaged in Subroutine RECESS to get an overall estimate, the positive and negative estimating errors tend to cancel. The range of acceptable estimates of BFRC and IFRC becomes much narrower (p. 68). Use of such a limited range at this point would bias the averaged estimate.

Subroutine SET1RC  
(Third Recession Constant Subroutine)

CONTEXT

The recession sequences selected by Subroutine RECESS potentially have two flow components. When attempts were made to estimate two constants with SET2RC from flow sequences with a small beginning flow and slow recession, the results were usually bad because base flow usually predominated. As a cutoff which gave the best results, the sequences with a first flow less than  $0.4 \cdot \text{AREA}$  and a second flow greater than 80 percent of the first flow were filtered out and were fitted into a model to evaluate a single recession constant (RCSS0073, 0074).

Some other flow sequences yielded physically unreasonable values for either one or both of the two recession constants. These also were considered to be base flow sequences. In both cases, the need was for an estimating procedure to evaluate a single recession constant (RCSS 0076-0078).

## PURPOSE

Subroutine SET1RC is used to estimate the base flow recession from sequences of daily flow totals determined to be essentially base flow.

## PROCEDURE

Given a sequence of base flows  $q_0, q_1, \dots, q_n$  following the linear model

$$q_t = K_b q_{t-1} + \epsilon_t \quad (t = 1, 2, \dots, n) \quad (18)$$

the least squares estimator for the coefficient  $K_b$  (BFRC) as developed by James and Thompson (18) can be expressed as

$$K_b = \frac{\sum_{t=0}^{n-1} q_t q_{t+1}}{\sum_{t=0}^{n-1} q_t^2} \quad (19)$$

Equation 19 was programed into Subroutine SET1RC (STIR 0006-0010) and used to estimate BFRC for each base flow sequence. The values of BFRC for all the flow sequences (including those estimated by SET2RC for sequences with interflow) were then used in RECESS to estimate an average BFRC for that station-year.

## DISCUSSION

The value of BFRC estimated by OPSET proved to be consistent among sequences and among years. Close inspection of the results, however, revealed a trend toward larger estimates of BFRC from sequences of relatively lower flows than from sequences beginning with higher flows. This trend is the very reason for use of a second base flow recession parameter (BFNLR) in the Kentucky Watershed Model to make the base flow recession relatively more rapid during

periods of high groundwater storage and then progressively less rapid as the sequence continues (10, pp. 68-69). Recommendations for estimating BFNL R are presented on pp. 170-173.

Subroutine SETFVP  
(First Land Phase Parameter Subroutine)

CONTEXT

The most important single function of OPSET is to estimate an optimum set of values for the six parameters which the sensitivity studies revealed to govern the month by month distribution of flow volume synthesized from a precipitation record. In order of decreasing sensitivity, these are

1. LZC - An index of the moisture storage capacity of the lower zone or soil,
2. BMIR - An index of infiltration rate,
3. SUZC - An index of the degree to which the moisture storage capacity of the upper zone or watershed surface and soil cover increases into the summer season because of vegetation changes, cultivation practices, and other factors,
4. ETLF - An index to the rate of moisture loss through evapotranspiration from the soil,
5. BUZC - An index of the degree to which the upper zone moisture storage capacity decreases with increasing lower zone moisture content because of the development of better surface drainage during wetter conditions,
6. SIAC - An index of the degree to which the infiltration rate increases into the summer.

Each parameter is described in more detail by Ross (25, pp. 47-55) and was originally developed under different names for the Model by Crawford and Linsley (10).

In order to see how and to what degree each parameter affects runoff (annual total as well as distribution over the year), a sensitivity study was made with the data for Elkhorn Creek near Frankfort, Kentucky, for the 1964 water year. First, a best set of parameter values was selected by trial-and-error. Then for each of the six parameters, two more computer runs were made by varying that parameter while all the other parameters were held constant. Each parameter was found to have its own effect on the simulated volumes, and this effect varies by type of flow and with the time of the year. This sensitivity study helps translate each parameter from a variable in an equation to a term with hydrologic meaning and thereby provides very useful guidelines for establishing rules for adjusting each parameter value in a fashion likely to lead to the synthesis of better matching flows.

The adjustment rules for five of these six volume parameters are based on observing differences between synthesized and recorded flows during the months when the synthesized monthly flows are most sensitive to variation in that variable. These five are grouped in Subroutine SETFVP. Better adjustment for BMIR was found to be indexed to matching the recorded and the synthesized base flows during the first three days of selected recession sequences (See Subroutine SETBMI). However, in the process of estimating the flow volume parameters, these two subroutines function as a unit to adjust the six parameters simultaneously between trial flow simulations.

The optimization process begins by simulating a year of stream-flows with a set of initial trial values for the six parameters. Then

each parameter is adjusted by its adjustment rule. The new set of six values is used to simulate another year of streamflows, and the process continues until the adjustments no longer are able to achieve simulated flows which have a smaller value of SSQM as computed by Subroutine SETFDI.

If estimation of the six volume parameters functioned in an ideal manner, each adjustment would improve the match between recorded and simulated flows until the final adjustment achieved exact duplication. Of course, an exact match is impossible with real data and a model which cannot reflect the full complexity of the runoff process. Somewhere, the adjustment will have to be stopped because it can no longer improve the matching.

Perfect matching is prevented by data difficulties and by modeling difficulties. Better data will yield better matching and increase the relative difficulty with the model. No matter which difficulty predominates for a given run with OPSET, it is certain to be monthly asymmetrical. For example, measured precipitation will depart from true watershed precipitation in some months more than in others. If a parameter is indexed on a month with data difficulty, repeated adjustment will overcorrect. Synthesized flows for the other months will worsen. A worsening for many months will overshadow an improvement for an index month, and a larger SSQM will halt the process. If the data difficulty in the index months is large, the worsening may begin early in the adjustment process; and OPSET will be prevented from making a good estimate. For this reason, an alternate adjusting index is developed for each parameter. If overcorrection is leading a parameter into wild values and resulting

in poor matching, the index suggested by the sensitivity studies is abandoned, and the alternate adjustment is tried.

A number of specific observations about this process are useful.

1. The smaller the largest data difficulty, the better will be the final matching.

2. If all the data were accurate and the only difficulty lay with the model, OPSET would adjust the parameters in an attempt to compensate for model imperfections and cause the process to halt as matching worsened in months where the model worked better.

3. Data difficulties in months used to index adjustments to the parameters to which the flows as a whole are the most sensitive are the most likely to quickly terminate the process and produce poor estimates for all six parameters. This suggests indexing these parameters on larger groups of months to minimize susceptibility to this difficulty.

4. These more "sensitive" parameters are estimated by OPSET with greater precision because faulty estimates are more likely to upset the entire annual pattern of simulated flows.

5. Data difficulties in months used to index adjustments to the parameters to which the flows as a whole are least sensitive will more slowly terminate the process. If the index is leading this parameter to bad values but the other parameters are improving, the matching as a whole will likely improve too.

6. These less "sensitive" parameters are estimated by OPSET with less precision. This may in part explain the greater difficulty Ross had in correlating the values estimated for these parameters with his measured watershed characteristics (25, pp.103-118).

7. Simultaneous adjustment of six parameter values leads to interactions as faulty estimates for one parameter upsets the adjust-

ment index for and hence the selected value of another parameter. The sensitivity tables show flows in some months to be more sensitive to the value of a given parameter than are the flows in other months, but flows in all months are to some degree sensitive to all parameters.

8. Months more likely to have data difficulties provide less desirable adjustment indices. October is a poor month for indexing because of the difficulty in estimating initial conditions. Summer months are worse than winter months because the greatest data problems are associated with convective precipitation.

#### PURPOSE

Subroutine SETFVP adjusts the values of five flow volume parameters LZC, SUZC, ETLF, BUZC, and SIAC during the process of estimating the best set of values for the six flow volume parameters.

#### PROCEDURE

The values of the five flow volume parameters are adjusted in Subroutine SETFVP by examining the monthly flow indices determined in Subroutine SETFDI. The specific indices examined were based for a given parameter on months with flows known from the Elkhorn sensitivity studies to be particularly sensitive to that parameter and adjusted as experience was gained with OPSET.

A numerical example of the estimation process is shown on Table 9 based on South Fork Beargrass Creek near Louisville, Kentucky, for 1947 water year. Table 10 provides supplemental information illustrating how overland flow and base flow months were defined in the adjustment of LZC. The information on these tables will be gradually explained as the parameter adjustments are individually presented.

TABLE 9  
 ADJUSTMENT OF FLOW VOLUME PARAMETERS:  
 SOUTH FORK BEARGRASS CREEK, LOUISVILLE, KENTUCKY, 1947

PARAMETER VALUES*						
Cycle	LZC	BMIR	SUZC	ETLF	BUZC	SIAC
1	12.00	1.20	1.30	0.25	1.50	0.90
2	10.16	4.35	0.74	0.19	2.14	0.45
3	8.63	4.37	0.49	0.18	2.72	0.22
4	7.90	3.29	0.47	0.21	4.61	0.11
5	7.53	2.73	0.42	0.24	6.54	0.06
6	7.13	2.83	0.37	0.26	5.42	0.03
7	7.06	3.19	0.33	0.27	4.60	0.02
8	6.56	3.55	0.31	0.27	4.45	0.01
9	6.02	3.73	0.31	0.28	4.59	0.01
10	5.37	3.94	0.31	0.29	4.49	0.01
11	4.67	4.19	0.31	0.29	4.31	0.01
12	3.97	4.49	0.32	0.29	4.05	0.01
13	3.36	4.79	0.32	0.28	3.74	0.01
-----						
1	4.67	4.19	0.31	0.29	4.31	0.01
2	4.45	4.64	0.36	0.31	4.37	0.01
3	3.90	5.43	0.44	0.31	4.37	0.02
4	3.37	6.14	0.52	0.30	3.99	0.02

\* For this station-year, the basic interflow volume factor (BIVF) is 0.0 because no interflow was encountered in Subroutine RECESS. For station-years where interflow is appreciable, BIVF in the Rough cycles is set at 0.90, and its value is adjusted in the Fine cycles by Subroutine SETBIV.



TABLE 9 (cont'd.)

MONTHLY FLOW DEVIATIONS							
Cycle	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.
1	-0.12	0.98	0.19	-0.09	-3.58	-0.22	-0.16
2	-0.07	0.45	0.27	-0.20	-1.00	-0.35	-0.16
3	0.05	1.02	0.74	-0.11	-0.73	-0.18	-0.08
4	-0.03	0.92	0.73	-0.06	-0.78	-0.17	-0.04
5	-0.09	0.50	0.48	-0.08	-0.88	-0.17	-0.04
6	-0.11	0.18	0.28	-0.11	-0.84	-0.18	-0.04
7	-0.12	0.04	0.14	-0.17	-0.76	-0.20	-0.05
8	-0.12	0.11	0.17	-0.16	-0.69	-0.18	-0.04
9	-0.12	-0.05	0.18	-0.15	-0.65	-0.15	-0.02
10	-0.13	-0.11	0.22	-0.12	-0.61	-0.12	-0.00
11	-0.13	-0.15	0.32	-0.08	-0.56	-0.08	0.02
12	-0.13	-0.17	0.49	-0.04	-0.51	-0.03	0.05
13	-0.13	-0.09	0.72	-0.01	-0.47	0.01	0.07
1	-0.05	-0.09	0.34	-0.08	-0.53	-0.08	0.04
2	-0.05	-0.23	0.24	-0.08	-0.47	-0.08	0.03
3	-0.05	-0.23	0.37	-0.06	-0.37	-0.06	0.04
4	-0.05	-0.15	0.58	-0.02	-0.31	-0.02	0.05
Cycle	May	Jun.	Jul.	Aug.	Sep.	SSQM	
1	-0.61	-1.09	-0.70	-0.76	-0.11	16.49	
2	-0.17	-0.42	-0.21	-0.18	-0.05	1.72	
3	0.03	-0.30	0.73	-0.01	-0.01	2.78	
4	0.06	-0.35	0.65	-0.13	-0.06	2.59	
5	0.09	-0.39	0.65	-0.19	-0.09	1.91	
6	0.11	-0.36	0.68	-0.19	-0.09	1.51	
7	0.13	-0.29	0.72	-0.15	-0.09	1.31	
8	0.15	-0.22	0.71	-0.12	-0.10	1.16	
9	0.16	-0.19	0.63	-0.13	-0.10	0.99	
10	0.18	0.18	0.52	-0.16	-0.11	0.83	
11	0.19	-0.16	0.39	-0.20	-0.11	0.71	
12	0.21	-0.15	0.24	-0.26	-0.12	0.73	
13	0.23	-0.13	0.10	-0.31	-0.12	0.94	
1	0.23	-0.09	0.45	-0.13	0.03	0.70	
2	0.19	-0.11	0.28	-0.16	0.03	0.50	
3	0.16	-0.14	0.11	-0.21	0.02	0.44	
4	0.13	-0.14	-0.01	-0.28	0.02	0.58	

TABLE 10  
DIVISION OF SIMULATED FLOWS: S. FK. BEARGRASS CREEK, LOUISVILLE, KY.  
1947

	Recorded Monthly Totals	Recorded Rainfalls (in)	Esti- mated Evapo- trans- piration (in)	Synthesized Flows							
				Cycle 1 (initial)				Cycle 2			
				Over- land flow (in)	Inter- flow (in)	Base- flow (in)	Total* flow (in)	Over- land flow (in)	Inter- flow (in)	Base flow (in)	Total* flow (in)
Oct.	0.099	2.16	3.03	0.079 <sup>a</sup>	0.000	0.011	0.084	0.079 <sup>a</sup>	0.000	0.019	0.090
Nov.	0.279	4.33	1.48	0.306 <sup>a</sup>	0.228	0.068	0.597	0.178	0.084	0.170	0.426
Dec.	0.695	2.92	0.85	0.258	0.265	0.306	0.825	0.119	0.126	0.645 <sup>b</sup>	0.886
Jan.	3.540	5.30	0.64	2.217 <sup>a</sup>	0.563	0.487	3.264	0.633	1.042	1.289	2.960
Feb.	1.042	0.40	0.71	0.000	0.018	0.181 <sup>b</sup>	0.199	0.000	0.043	0.464 <sup>b</sup>	0.504
Mar.	0.636	2.19	0.96	0.167	0.226	0.127	0.516	0.071	0.112	0.282 <sup>b</sup>	0.461
Apr.	3.660	6.26	2.05	1.911 <sup>a</sup>	0.731	0.533	3.165	0.458	1.248	1.473	3.168
May	2.110	6.04	4.03	0.500	0.446	0.380	1.306	0.292	0.615	0.922 <sup>b</sup>	1.809
Jun.	1.390	4.03	5.76	0.205	0.131	0.337 <sup>b</sup>	0.646	0.149	0.138	0.711 <sup>b</sup>	0.972
Jul.	0.472	4.22	6.08	0.142 <sup>a</sup>	0.034	0.111	0.260	0.149	0.120	0.338 <sup>b</sup>	0.578
Aug.	0.158	1.75	6.88	0.054 <sup>a</sup>	0.000	0.033	0.071	0.054	0.000	0.103 <sup>b</sup>	0.128
Sep.	0.108	2.86	4.74	0.089 <sup>a</sup>	0.000	0.010	0.094	0.089 <sup>a</sup>	0.000	0.025	0.102

\* Total flow is the sum of overland flow, interflow, and base flow less stream evaporation.

a. Overland-flow month

b. Base-flow month

Subroutine SETFVP calls Subroutine SETFDI to calculate the monthly flow deviation indices (STFV0008) and then adjusts the five parameters one by one according to the rules which follow.

LZC: LZC is an index of soil-moisture storage capacity. The bigger its value, the more water can be stored in soil. It directly or indirectly controls the simulated rates of infiltration, evapotranspiration, and percolation to groundwater.

The response of different flow components to the variation of LZC can be seen in the results of the Elkhorn Creek sensitivity studies (Tables 11 and 12). Increasing LZC reduces overland flows with the greatest decrease coming toward the end of the wet months when precipitation is less apt to saturate the available storage capacity. The overland flows decrease sharply in those wet months right after long dry season because more water infiltrates into the soil and is stored.

Increasing LZC decreases interflows and base flows in wet winter months but increases them in dry summer months. When the soil storage capacity is increased, more water is stored in the soil during the wet season so that less is available to contribute to interflows and base flows, but the greater soil moisture contributes to base flow in dry months and contributes indirectly to interflow because the soil is wetter during storm periods (Equation 35).

The consistency with which overland flows decrease when LZC increases suggests that the adjustment of LZC be tied to overland flow. Recorded overland flow cannot be readily estimated on a monthly basis, so direct matching is not possible. The next best approach was to use total flow in months where overland flow is known to predominate. An "overland-flow month" is defined as a month in which more than half of the simulated total flow is overland flow. Such months are selected in MAIN0732 to 0746 and tagged by

TABLE 11  
 SENSITIVITY STUDIES: ELKHORN CREEK WATERSHED  
 (1964) with LZC = 3.50, BMIR = 0.65, SUZC = 0.10, ETLF = 0.20,  
 BUZC = 0.75, SIAC = 3.0

	Precipitation (in)	Potential Evapo- transpiration (in)	Overland flow less stream evaporation (in)	Interflow (in)	Baseflow (in)	Total Flow (in)
Oct.	0.46	4.68	0.028	0.000	0.015	0.043
Nov.	1.71	2.42	0.039	0.000	0.001	0.040
Dec.	1.14	0.22	0.034	0.067	0.022	0.123
Jan.	2.75	0.45	0.438	0.809	0.176	1.423
Feb.	2.68	0.32	0.244	0.979	0.279	1.502
Mar.	11.22	0.61	7.031	2.509	0.680	10.220
Apr.	3.32	1.60	0.365	0.570	0.307	1.242
May	1.49	4.12	0.062	0.070	0.153	0.295
June	3.19	5.77	0.063	0.061	0.059	0.183
July	4.03	5.27	0.070	0.015	0.039	0.124
Aug.	2.66	6.50	0.052	0.001	0.010	0.063
Sept.	4.53	5.52	0.068	0.053	0.015	0.136

TABLE 12  
 ELKHORN CREEK WATERSHED SENSITIVITY STUDIES:  
 MONTHLY SYNTHESIZED FLOW VOLUMES AS A FRACTION OF VOLUMES  
 WITH LZC = 3.50

	Overland Flow		Interflow		Base Flow		Total Flow	
	LZC= 7.0	LZC= 24.0	LZC= 7.0	LZC= 24.0	LZC= 7.0	LZC= 24.0	LZC= 7.0	LZC= 24.0
Nov. *	1.00	0.97	1.00	1.00	1.00	2.00	1.00	1.02
Dec.	1.03	1.03	0.48	1.45	0.50	1.41	0.63	1.32
Jan.	0.64	0.60	0.78	0.78	0.49	0.56	0.70	0.70
Feb.	0.52	0.41	0.82	0.68	0.52	0.39	0.72	0.58
Mar. *	0.95	0.86	0.99	0.98	0.61	0.37	0.93	0.85
Apr.	0.87	0.79	0.85	0.68	0.65	0.36	0.81	0.60
May	0.94	0.87	0.93	0.67	0.72	0.39	0.82	0.56
Jun.	1.00	0.97	1.03	0.79	0.92	0.54	0.98	0.77
Jul. *	0.99	0.99	1.33	1.20	1.31	1.03	1.13	1.03
Aug. *	1.00	0.98	3.00	4.00	1.80	1.80	1.15	1.16
Sep. *	0.93	0.87	1.32	1.72	1.53	2.00	1.15	1.32

\* Overland flow month in which synthesized overland flow is more than one-half of total flow (See Table 11).

setting XMPFT for that month equal to 2.0. If there are at least three overland-flow months during the water year, the following steps are used to adjust the value of LZC:

1) Sum the monthly flow deviation indices into SOFMD for the FNOFM overland flow months.

2) Calculate the adjustment factor (FLZC) for LZC as

$$FLZC = \frac{SOFMD}{0.75 * FNOFM} \quad (20)$$

3) If FLZC is positive (the synthesized flow averages greater than the recorded flow in the overland-flow months), increase LZC by multiplying by (1+FLZC) so that the synthesized flow may be decreased. If FLZC is negative, decrease the value of LZC by dividing by the factor (1-FLZC) to make the flows bigger.

The constant 0.75 in Equation 20 (STFV0041) was established in conjunction with values for a set of like constants, one per parameter. The other values are 1.0 for SUZC (STFV0089), 1.2 for ETLF (STFV0134), 0.4 for BUZC (STFV0147), and 1.5 for SIAC (STFV0177). The overall and relative magnitude of these constants were established by trial-and-error. The overall magnitude was adjusted downward as needed to prevent the trial estimates from varying so much that they never stabilized to a good match. It was adjusted upward as needed to prevent excessive computer time from being used. The relative magnitude of the constant for a given parameter was increased as needed to make that parameter arrive at its optimum value about the same time the others did. If a constant was too small, the parameter would not reach an equilibrium value before the process was halted. A single parameter being adjusted in increments too large would upset the estimation process by changing flows faster than changes in the other parameters could bring them in line.

Sometimes, the value of FLZC may be quite large (positive or negative) because of data difficulties in some months or because of the initial trial set of values being far from optimal. If so, the adjusted value would be either so big (when FLZC is positive) or so small (when FLZC is negative) as to upset the whole optimizing process from the very beginning. In order to prevent this, FLZC is limited to the range between -1.0 and 1.0. When the calculated FLZC exceeds 1.0, it is set equal to 1.0; and when FLZC is less than -1.0, it is set equal to -1.0. This limitation confines the adjusted LZC to between one-half and twice the previous value.

Even with the limitation on FLZC, the adjusted value of LZC may eventually become too big or too small if there are data difficulties which cause overcorrection. For example, if an overland-flow month happens to have recorded streamflows too small to be commensurate with the recorded rainfalls, the program would increase the value of LZC time after time in an attempt to decrease the synthesized flows until an unreasonably large value of LZC resulted. As a control, LZC is restricted to values between 30.0 and 2.0 inches.

For a year with many overland-flow months, the above method works well. When there are only one or two, the process may be severely upset by data difficulties in these months. The danger is particularly great when a low flow summer month is an overland-flow month because of overland flow from one summer thunderstorm. When there are no overland flow months, the process breaks down completely. For these reasons, when there are less than three overland-flow months, the average of the monthly flow deviation indices of the two largest runoff months is taken as the adjustment factor FLZC. The two largest total flow months are sensitive to LZC and are less likely than low flow months to be associated with rainfall

measurement problems.

For the example of Tables 9 and 10, the first adjustment of LZC is based on seven overland-flow months (October, November, January, April, July, August, and September) and changes LZC from 12.0 to 10.16 by using Equation 20. The second adjustment is based on the two largest runoff months (April and January) instead of the two overland-flow months (October and September), changes LZC from 10.16 to 8.63 instead of 9.41, and thereby speeds convergence on a final value of 3.90.

The alternate procedure for adjusting LZC is invoked when after six adjustments during the Rough phase LZC exceeds 29.0 (MAIN0777). A value this large hints that something else should be tried. An alternate procedure is not adopted sooner than this because wild values may occur before the program has a chance to recover from initial parameter values which may be quite poor for the watershed at hand. Adjustments by the alternate procedure are handled in MAIN by setting the logical variable LLZC to be TRUE, and the Subroutine SETFVP adjustment is skipped (STFV0030).

The alternate procedure adjusts LZC based on the total annual runoff volume according to the equation (MAIN0781):

$$LZC = PLZC \left( \frac{SATFV}{RATFV} \right) \quad (21)$$

where PLZC = previous trial value of LZC;  
SATFV = synthesized annual total flow volume;  
RATFV = recorded annual total flow volume.

One special case in adjusting LZC comes when there is "Fall trouble" (See Subroutine SETBMI) caused by difficulty in establishing initial soil moisture conditions. If the adjustment is in Rough adjustment cycle and the adjusted value of LZC is less than the



associated estimate of initial soil moisture (LZS) plus 2.0, the value of LZC is set to be LZS + 2.0 (MAIN0283). This step was found to eliminate wild adjustment patterns caused by starting the water year with a saturated watershed.

In the Fine adjustment cycle, the limitation on extreme values (30.0 and 2.0) is removed because a reasonable set of values is estimated during the Rough cycle. If adjustment outside this range can now improve the overall matching, the adjusted value should be accepted for LZC. If during the Rough adjustment, annual runoff volume has been invoked to adjust LZC, then Equation 21 is retained for the Fine adjustment.

SUZC: This parameter is used in the Model to account for the growing season increase in upper zone storage capacity (25, pp. 47-48). The sensitivity studies (Tables 11 and 13) show increases in SUZC to 1) reduce total runoff during summer months, particularly those with significant shower activity, and 2) reduce base flow by an amount which becomes progressively larger through the summer. Each trend makes hydrologic sense. If the watershed surface is able to hold more moisture, summer storms are likely to be adsorbed on the surface (thus reducing runoff from larger rains and eliminating it from smaller rains), and the water will stay near the surface until it evaporates. Summer infiltration will decrease, and groundwater will gradually drain. If little or no rain occurs in August and September, flows in these months will be quite low.

The adjustment of SUZC is based on the monthly flow deviation indices during the two months between April and November inclusive with the greatest recorded rainfall plus indices for August and September when more than half of the flow synthesized for the months during the last trial was base flow. The adjustment is executed in the following steps:

TABLE 13

ELKHORN CREEK WATERSHED SENSITIVITY STUDIES:  
MONTHLY SYNTHESIZED FLOW VOLUMES AS A FRACTION OF VOLUMES  
WITH SUZC = 1.0

	Overland Flow		Interflow		Base Flow		Total Flow	
	SUZC= 0.5	SUZC= 2.0	SUZC= 0.5	SUZC= 2.0	SUZC= 0.5	SUZC= 2.0	SUZC= 0.5	SUZC= 2.0
Nov.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Dec.	1.00	0.97	1.22	0.75	1.09	0.82	1.13	0.82
Jan.	0.99	0.95	1.00	0.96	0.99	1.03	0.99	0.97
Feb.	0.96	0.76	1.00	0.96	1.00	1.14	1.00	0.97
Mar.	1.00	0.98	1.00	1.00	1.00	1.04	1.00	0.99
Apr.	1.47	0.49	1.24	0.76	1.04	0.99	1.26	0.74
May*	1.13	0.92	2.39	0.63	1.10	0.95	1.42	0.87
Jun.	1.60	0.94	4.49	0.15	1.39	0.71	2.50	0.60
Jul.	1.01	0.99	7.27	0.13	2.10	0.49	2.12	0.73
Aug.	1.00	1.00	1.70	0.00	3.00	0.30	1.56	0.87
Sep.	0.63	1.19	3.42	0.11	3.07	0.00	1.98	0.64

\* Base-flow month in which synthesized base flow is more than one-half of total flow.  
(See Table 11).

1. Determine the two months in the period of April through November with the greatest recorded precipitation (STFV0071-0083). For the example given in Table 10, April and May are the two wettest.

2. Check the synthesized flows for August and September to determine whether base flow is over one-half of the total. The example in Table 10 shows August in Cycle 2 to be a base flow month.

3. Sum the monthly flow deviation indices of the two wettest summer months (STFV0084) plus those for Augusts and Septembers which are base flow months (STFR0086, 0089) to get FSUZC.

4. If FSUZC exceeds 1.0, set it equal to 1.0; if it is less than -1.0 set it equal to -1.0.

5. If FSUZC is positive, increase SUZC by multiplying by  $(1 + \text{FSUZC})$  to decrease the index month flows. If FSUZC is negative, decrease SUZC by dividing by a factor  $(1 - \text{FSUZC})$  to increase index month flows.

The extreme accepted values of SUZC are 0.3 and 3.0 during Rough adjustment cycle. However, the limitation is again removed for the Fine adjustment cycle. The use of both wet and dry summer months in the adjustment as described made it unnecessary to develop a secondary adjustment such as those used for the other volume parameters. No further trouble with SUZC was noted once the base flow months were brought in to check unwise adjustments caused by rainy month data difficulties.

ETLF: This parameter is used in estimating the volume of evapotranspiration from the lower zone. Since the Model assumes lower zone evapotranspiration only occurs after the upper zone has dried out and this usually does not occur during winter periods of low evaporation, larger values of ETLF are primarily associated with more thorough drying of the soil during the summer. A larger value of ETLF should reduce runoff from summer rain by an amount which

progressively increases through the season and continues on through the fall period of soil moisture replenishment. The sensitivity studies (Tables 11 and 14) show that when ETLF increases, flows decrease throughout the year, but the most drastic decreases are experienced in summer month flows. The decrease becomes progressively severe until the first major winter storm and is least noticeable during months where most of the flow is from moisture previously stored within the watershed.

The adjustment of ETLF is based on matching the flows in summer months when the rainfall exceeds an arbitrary two inches. When necessary, ETLF is increased to prevent moisture buildup at the end of the water year. The summer months are defined as the months ending with the month beginning the wet season (MBWS) and beginning with the month beginning the dry season (MBDS). MBWS is the first of the months of November and afterwards when the precipitation total exceeds the potential evapotranspiration. MBDS is the month following the first month when the potential evapotranspiration exceeds precipitation (STFV0052-0064). Skipping to the second month allows a month for the upper zone moisture to dry. For the example given in Table 10, MBWS is November, and MBDS is March. Actually, July would probably have been a better choice for this example, but a provision to prevent triggering of the dry season by an extremely dry winter month was not worked into OPSET. The effect on the final estimate of ETLF was minimized by preventing use of an MBDS earlier than June (STFV0111).

If at least one month in the period beginning with MBDS and ending with MBWS has over two inches of recorded precipitation, ETLF is adjusted as follows.

1. Sum the monthly flow deviation indices of WSM wet summer months to get SWSMD. In the example in Table 10, the months are October, November, June, July, and September.

TABLE 14  
 ELKHORN CREEK WATERSHED SENSITIVITY STUDIES:  
 MONTHLY SYNTHESIZED FLOW VOLUMES AS A FRACTION OF VOLUMES  
 WITH ETLF = 0.2

	Overland Flow		Interflow		Base Flow		Total Flow	
	ETLF= 0.1	ETLF= 0.4	ETLF= 0.1	ETLF= 0.4	ETLF= 0.1	ETLF= 0.4	ETLF= 0.1	ETLF= 0.4
Nov.	1.00	0.97	1.00	1.00	1.00	1.00	1.01	0.99
Dec.	1.00	0.91	1.96	0.43	1.91	0.45	1.68	0.57
Jan.	1.09	0.92	1.05	0.95	1.31	0.77	1.10	0.91
Feb.	1.05	0.86	1.02	0.98	1.15	0.88	1.06	0.95
Mar.	1.00	1.00	1.01	0.99	1.05	0.95	1.01	0.99
Apr.	1.01	0.99	1.01	0.99	1.03	0.97	1.02	0.99
May	0.98	1.00	1.16	0.89	1.05	0.97	1.06	0.96
Jun.	1.02	0.97	1.52	0.54	1.68	0.64	1.40	0.72
Jul.	1.00	0.99	2.13	0.33	2.64	0.33	1.66	0.71
Aug.	0.98	1.04	6.00	0.00	4.30	0.00	1.58	0.84
Sep.	0.72	1.12	2.53	0.45	3.87	0.20	1.77	0.76

2. Calculate the ETLF adjustment factor, FETLF, by the equation

$$FETLF = 1.2 \left( \frac{SWSMD}{WSM} \right) \quad (22)$$

3. If FETLF exceeds 1.0, set it equal to 1.0; if it is less than -1.0, set it equal to -1.0.

4. If FETLF is positive, which means the synthesized flows in the index months are too high, increase ETLF by the factor  $(1 + FETLF)$ . If FETLF is negative, decrease the value of ETLF by dividing the factor  $(1 - FETLF)$  in order to increase flows in wet summer months.

Since summers may be so dry that no month has more than two inches of precipitation, an alternate adjustment procedure is provided for this case using the deviation index for the month with the largest runoff volume to adjust ETLF. The following relationship replaces Equation 22:

$$FETLF = 5.0 * MFDP(M1R) \quad (23)$$

where MFDP(M1R) is the deviation index for the month with the largest runoff volume. The relatively high constant of 5.0 is dictated by a low sensitivity.

In the Rough adjustment cycle, the adjusted value of ETLF is bounded by the minimum and maximum values of 0.05 and 0.60. If after six cycles the adjusted ETLF exceeds 0.59, an indication that one wet summer month may have a large positive deviation caused by a precipitation data problem which forces the adjustment to choose a still higher value of ETLF, the largest positive deviation month is thereafter excluded in evaluation of the adjustment factor (STFV0123-0134) for both the remaining Rough cycles and the Fine adjustment cycles.

At the other extreme, circumstances adjusting ETLF to a very low value may cause the soil moisture to build up through the water year. In order to prevent such moisture buildup, the value of ETLF is doubled whenever the end-of-month soil moistures (EMLZC) of both August and September exceed the value of LZC, and annual precipitation is less than 1.5 times annual potential evapotranspiration (STFV0101-0102). Usually this will bring the trial estimates back into line.

BUZC: The basic upper zone storage capacity factor (BUZC) indexes the capacity of the soil surface to hold moisture as interception or depression storage. Sensitivity studies (Tables 11 and 15) show that by increasing BUZC: 1) simulated flows in the spring months slightly increase, and 2) flows in the summer and fall become much smaller with the most severe decrease coming in September through December when these months have significant rainfall. The hydrologic explanation is much the same as it is for SUZC with the added factor of increased late winter or spring runoff caused by a wetter soil surface during this period of low evaporation.

The adjustment of BUZC proceeds as follows.

1. Calculate the adjustment factor (FBUZC) as the product of 0.4 and the sum of monthly flow deviation indices for September, November, and December. The constant 0.4 was found to provide the best adjustment step size. October is omitted to minimize problems caused by poor estimates of initial soil moisture conditions.
2. If FBUZC exceeds 1.0, set it equal to 1.0; if it is less than -1.0, set it equal to -1.0.
3. Adjust BUZC by multiplying the factor  $(1 + \text{FBUZC})$ , if FBUZC is positive. If FBUZC is negative, divide the value of BUZC by the factor  $(1 - \text{FBUZC})$ .

In the Rough adjustment, the value of BUZC is limited to the

TABLE 15

ELKHORN CREEK WATERSHED SENSITIVITY STUDIES:  
 MONTHLY SYNTHESIZED FLOW VOLUMES AS A FRACTION OF VOLUMES  
 WITH BUZC = 0.75

	Overland Flow		Interflow		Base Flow		Total Flow	
	BUZC= 0.25	BUZC= 4.0	BUZC= 0.25	BUZC= 4.0	BUZC= 0.25	BUZC= 4.0	BUZC= 0.25	BUZC= 4.0
Nov.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Dec.	1.00	0.94	1.48	0.00	1.18	0.05	1.29	0.27
Jan.	0.99	0.62	0.99	0.62	0.97	0.82	0.99	0.64
Feb.	0.96	1.20	1.00	1.03	0.99	1.38	1.00	1.13
Mar.	1.00	1.00	1.00	1.01	1.00	1.16	1.00	1.01
Apr.	1.06	0.93	1.03	1.01	1.00	1.10	1.03	1.01
May	0.98	1.00	1.07	0.81	1.01	1.05	1.02	0.98
Jun.	1.00	0.95	1.18	0.43	1.03	0.88	1.07	0.76
Jul.	1.00	1.00	1.27	0.27	1.10	0.67	1.07	0.81
Aug.	1.00	1.02	3.00	0.00	1.20	0.40	1.05	0.89
Sep.	0.94	1.18	1.38	0.09	1.27	0.00	1.15	0.63



range between 0.2 and 4.0. If after six Rough cycles the value exceeds 3.9, it is taken to mean that the three months (September, November, and December) do not provide a suitable adjustment index. Three other months (June, July, and August) are substituted in Step 1 for the remaining Rough cycles and all the Fine cycles.

SIAC: The seasonal infiltration adjustment constant (SIAC) relates infiltration rates to antecedent evaporation to account for more rapid infiltration during warmer periods. A zero value implies a seasonally constant infiltration rate. As the value is made larger, infiltration rates are reduced in the winter and increased in the summer. The sensitivity studies (Tables 11 and 16) show that when SIAC increases, the flows in winter months increase and the flows in summer months decrease.

The adjustment of SIAC uses both summer and winter monthly flow deviation indices. If the previous SIAC value exceeds 1.0, the adjustment is based only on summer months. Table 16 shows winter flows to be almost completely insensitive to changes of SIAC in this range. The particular summer months used are the first three months of the year when rainfall is less than potential evapotranspiration. The procedure is as follows.

1. Average the monthly deviation indices of these three months to get the summer flow deviation index (SFDX).
2. Calculate the adjustment factor FSIAC as 1.5 times SFDX.
3. Confine the value of FSIAC between -1.0 and 1.0.
4. If FSIAC is positive, increase SIAC by multiplying by  $(1 + \text{FSIAC})$  to decrease the synthesized flows in these months. If FSIAC is negative, decrease SIAC by dividing by  $(1 - \text{FSIAC})$ .

When the value of SIAC is less than one, winter flows as well as summer flows are sensitive to value changes and the adjustment

TABLE 16

ELKHORN CREEK WATERSHED SENSITIVITY STUDIES:  
MONTHLY SYNTHESIZED FLOW VOLUMES AS A FRACTION OF VOLUMES  
WITH SIAC = 3.0

	Overland Flow		Interflow		Base Flow		Total Flow	
	SIAC= 0.1	SIAC= 1.0	SIAC= 0.1	SIAC= 1.0	SIAC= 0.1	SIAC= 1.0	SIAC= 0.1	SIAC= 1.0
Nov.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Dec. *	1.00	1.00	0.31	0.91	1.09	1.00	0.64	0.95
Jan.	0.30	1.00	0.79	1.00	1.64	1.00	0.75	1.00
Feb.	0.24	0.96	0.74	1.00	1.91	1.00	0.88	1.00
Mar.	0.63	1.00	1.59	1.00	2.10	1.00	0.97	1.00
Apr.	0.37	0.99	1.02	1.00	2.05	1.00	1.08	1.00
May	0.90	1.00	0.77	0.90	2.04	1.02	1.48	0.99
Jun. **	1.00	0.98	1.48	1.11	1.76	1.02	1.41	1.04
Jul.	0.99	1.00	3.67	2.07	1.38	0.95	1.44	1.11
Aug.	0.98	1.00	8.00	4.00	1.50	1.00	1.16	1.04
Sep.	0.60	0.76	3.68	2.34	0.87	0.87	1.83	1.39

\* December is the month beginning wet season (MBWS)

\*\* June is the month beginning dry season (MBDS)

rule is modified to include winter months. First, the winter flow deviation index (WFDX) is taken as the average deviation index of the first three months in which rainfall exceeds potential evapotranspiration. The value of WFDX is further modified by multiplying the factor  $(1.0 - SIAC)/0.4$ , if the value of SIAC is between 0.6 and 1.0. This factor provides a smooth transition gradually increasing the weight placed on winter deviations as winter flows become more sensitive to differences in SIAC. Because changing the value of SIAC changes simulated flows in the opposite direction in winter from what it does in summer, the adjustment factor FSIAC is calculated as

$$FSIAC = 1.5(SFDX - WFDX). \quad (24)$$

The example given in Table 10 is of this case, and the adjustment is based on the months of November, December, January, February, March, and April.

In the Rough adjustment cycle, if SIAC is adjusted to less than 0.02, it is taken as 0.00; if the adjusted SIAC is greater than 4.0, it is set equal to 4.0. The implication is that if SIAC is less than 0.02, infiltration does not vary by season; however, the value is again set back to 0.02 before each subsequent adjustment to make it possible for the adjusting multipliers to increase the value if the flows so indicate.

## DISCUSSION

The purely empirical approach used to adjust the values of the flow volume parameters in the trial-and-error search for the best match between record and synthesized monthly flow volumes was necessitated by a breakdown in the more formal search procedures tried earlier in the development of OPSET (pp. 21-23). The scheme of indexing the adjustment of each parameter to particular types of

deviations preselected by examining sensitivity studies was used in OPSET because of a better overall performance. The scheme has the following advantages.

1. It stabilizes on a reasonable set of parameter estimates within far less computer time than any other method tried.

2. Single factor variation from the chosen point did not improve the results by changing the values of any of the parameters in either direction.

3. The finally selected set of parameter estimates are relatively independent of the initial set of estimates (p. 33).

4. The method has remarkable recuperative powers when started from or led into bad values by data problems. The other types of adjustment were often not able to recover from such circumstances.

5. Adjusting parameter values based on rules derived from their hydrologic meaning proved far superior to any other method which did not directly use this knowledge.

The chief limitation to Subroutine SETFVP is that it was developed and tested only on 20 Kentucky watersheds. Other watersheds may have flow sequences which the trial-and-error procedure can not handle or handles inefficiently. The user will find many opportunities for programing improvement.

#### Subroutine SETFDI

(Second Land Phase Parameter Subroutine)

#### CONTEXT

The adjustment of the flow volume parameters seeks to improve the seasonal distribution as well as the total magnitude of simulated runoff volume over the water year. Monthly flow totals are

obtained by summing the synthesized and recorded daily values. Their comparison has two aspects. Indices of differences between recorded and synthesized monthly runoff volumes are used to determine how much each parameter needs to be adjusted. Where its flows are found to be sensitive to a particular parameter and the comparison reveals poor matching to occur that month, that parameter is adjusted in Subroutine SETFVP. The second aspect is the need to incorporate all monthly differences in a scalar value to use as an objective function in comparing the relative merit of alternative parameter sets.

### PURPOSE

Given recorded and synthesized monthly flow totals, Subroutine SETFDI is used to index the twelve monthly flow differences for adjusting the flow volume parameters and to establish a single statistic used to evaluate alternative parameter sets.

### PROCEDURE

The monthly flow deviation index (MFDP) was defined as

$$\text{MFDP} = \frac{\text{TMSTF} + 20}{\text{TMRTF} + 20} - 1 \quad (\text{TMSTF} > \text{TMRTF}) \quad (25)$$

or

$$\text{MFDP} = 1 - \frac{\text{TMRTF} + 20}{\text{TMSTF} + 20} \quad (\text{TMSTF} < \text{TMRTF}) \quad (26)$$

where TMSTF is the synthesized monthly total flow volume and TMRTF is the recorded monthly total flow volume, both in second-foot-days. When the synthesized monthly flow volume exceeds the recorded, Equation 25 is used, and the index is positive. When the synthesized monthly total is smaller, Equation 26 is used, and the index is negative. When the recorded and the synthesized flows are identical, both equations are made to indicate zero deviation by using the factor 1.

The simple difference between recorded and the synthesized monthly totals is not used as the monthly flow deviation index because it is too biased toward achieving closeness for a high flow month at the expense of low flow months (6, p. 96). For example, if the recorded monthly flows had a low of 120 sfd and a high of 13200 sfd and the synthesized flows were 20 and 13000 sfd respectively, the differences would be 100 sfd for the low flow month, and 200 sfd for the high flow month. The adjustment would stress further improvement for the high flow month even though that month was really already much better than the low flow month. The above equations attempt to minimize this difficulty by dividing to look at a ratio.

When a ratio index is used, another problem exists when the monthly flow is comparatively small. For example, if the recorded low flow month is 40 sfd, the high flow month is 2000 sfd, and the synthesized low and high flow months are 10 and 500 sfd respectively, the ratios are the same (i. e. , 4); but the synthesized high flow is much poorer than the low flow. In order to dampen this effect and prevent an undefined ratio when either flow is zero, an arbitrary 20 is added to the recorded and synthesized monthly totals before taking the ratios. In the example, this makes the low flow monthly deviation index much smaller (changed from 3 to 1) while the high flow index remains high (changed from 3 to 2.88).

The reason for using two rather than a single equation stems from a third effect. If simple ratios are used and the synthesized flow was twice the recorded flow, the ratio would be 2.00. If the synthesized flow was half the recorded flow, the ratio would be 0.50. The deviations from a perfect match of 1.00 would be +1.00 and -0.50; however, in both cases, the appropriate parameter adjustment would be one which changes the flows by a factor of 2. As deviations for several

months are usually averaged in making parameter adjustments, the average is biased toward increasing the flows. Equations 25 and 26 attempt to restore symmetry to the deviations and thereby speed convergence on optimal parameter sets. Indices which exceed 8 or are less than -8 are set to be 8 and -8 respectively to prevent months with severe data problems from completely upsetting the estimation process.

Monthly flow deviation indices are calculated from Equation 25 or 26, and the minimum sum of the squares of the deviations for 11 months, SSQM, is used to indicate which set of volume parameter values gives the best synthesis of flow volume. October is excluded because flows in the first month of the water year are too dependent on unknown initial conditions (moisture storages entering the water year). To include it will cause the adjustment of parameters when the adjustment of initial moisture storage is really needed.

Subroutine SETFDI is called from Subroutine SETFVP (STFV0008) which uses the monthly flow deviation indices to adjust five flow volume parameters.

### DISCUSSION

The formulation of Equations 25 and 26 was essentially a trial-and-error process in program development seeking the most rapid convergence toward consistent estimates of flow volume parameter sets.

### Subroutine SETBMI

(Third Land Phase Parameter Subroutine)

### CONTEXT

The basic maximum infiltration rate (BMIR) is the parameter used within the Model to control the rate of infiltration of moisture

from the watershed surface into the soil. The parameter represents a concept which is a maximum in two senses. It is a maximum (or capacity) rate which could occur, considering current soil conditions, if moisture supply were no limitation. It is also a maximum in that it represents a rate at the most pervious point in the watershed. More specifically, infiltration over the total watershed is modeled by assuming a linear variation of point infiltration capacity with fraction of the basin area from zero to this maximum value (25, p. 54). The maximum value is adjusted with season of the year and current soil moisture storage (MAIN0418). As the basic index of watershed infiltration, BMIR governs the volume of runoff synthesized during storms by controlling basin recharge and governs the volume of base flow by controlling percolation to groundwater.

In order to determine how to adjust BMIR to bring simulated flows more in line with the corresponding recorded flows, it is necessary to develop some feel for the changes in simulated flows caused by a given change in BMIR. For this purpose, three computer runs were made with data for Elkhorn Creek near Frankfort, Kentucky, for the 1964 water year. All parameters were held constant at a trial-and-error (not an OPSET selected) best fit value except BMIR which was varied from 0.2 to 2.5 in three runs. By inspecting the results, certain synthesized flows were found to vary significantly with changes in BMIR while others were more insensitive. The approach to adjusting BMIR was to single out several of the more sensitive flow categories (specified by time of the year and flow type) and use the differences between their recorded and simulated values as an index for setting the next trial value of BMIR. Several rather than single categories were used to dampen bad estimates caused by poor data particularly affecting a single category.



The degree to which BMIR affects simulated overland flow, interflow, and base flow volumes is illustrated in the summary from the Elkhorn Creek sensitivity studies on Table 17. Increasing the value of BMIR was seen to sharply reduce overland flow during winter low flow months by converting direct runoff to basin recharge, to more moderately reduce overland flow from major flow months because a fixed amount of infiltration is a smaller portion of the precipitation total, and to increase synthesized overland flow from storms following prolonged dry periods by increasing soil moisture storage. Increasing BMIR reduced interflow in low flow months by pushing the interflow triangle (25, p. 54) to the left so that less of its area was below the residual precipitation line and increased interflow in high flow months where higher residual precipitation lines produced opposite results. Increasing BMIR increased base flow by an amount which was largest immediately after major recharge periods and then progressively reduced through low flow months.

The process used in estimating BMIR is basically the same as that used for and is performed simultaneously with the estimation of the other five volume parameters. An initial trial estimate is made for each of the six variables (1.2 for BMIR, MAIN0210). A year of flows is simulated. Each parameter is adjusted by looking at preselected index differences of simulated from recorded flows. A new year of flows is simulated and the process continues until a set of parameter values is accepted. Subroutine SETBMI looks at the index differences for and then adjusts BMIR. Subroutine SETFVP looks at the index differences and then adjusts LZC, SUZC, ETLF, BUZC, and SIAC. BMIR adjustments are performed in a separate subroutine because the adjustment is indexed to a different sort of flow information.

TABLE 17

ELKHORN CREEK WATERSHED SENSITIVITY STUDIES:  
 MONTHLY SYNTHESIZED FLOW VOLUME AS A FRACTION OF VOLUMES  
 WITH BMIR = 0.65

	Overland Flow			Interflow			Base Flow			Total Flow		
	BMIR=			BMIR=			BMIR=			BMIR=		
	0.2 Ratio	0.65 Inch	2.5 Ratio	0.2 Ratio	0.65 Inch	2.5 Ratio	0.2 Ratio	0.65 Inch	2.5 Ratio	0.2 Ratio	0.65 Inch	2.5 Ratio
Dec.	1.38	0.034	0.91	2.76	0.067	0.19	0.50	0.022	1.41	1.97	0.123	0.61
Jan.	2.36	0.438	0.15	0.93	0.809	0.57	0.42	0.176	2.07	1.31	1.423	0.63
Feb.	3.68	0.244	0.21	0.69	0.979	0.47	0.35	0.279	2.56	1.12	1.502	0.82
Mar.	1.31	7.031	0.40	0.41	2.509	1.88	0.31	0.680	3.06	1.02	10.220	0.94
Apr.	1.90	0.365	0.21	0.68	0.570	0.96	0.34	0.307	2.78	0.95	1.242	1.19
May	0.87	0.062	0.90	1.07	0.070	0.61	0.37	0.153	2.56	0.65	0.295	1.72
Jun.	1.13	0.063	0.97	1.82	0.061	0.34	0.32	0.059	2.53	1.10	0.183	1.26
Jul.	1.00	0.070	1.00	2.47	0.015	0.33	0.41	0.039	2.18	1.00	0.124	1.29
Aug.	1.02	0.052	0.98	4.00	0.001	0.00	0.30	0.010	2.40	0.95	0.063	1.19
Sep.	0.65	0.068	1.10	3.26	0.053	0.28	0.47	0.015	1.80	1.65	0.136	0.86

Adjustment of BMIR indexed on total monthly flows was considered first. Table 17 suggests that a reasonable rule might be to increase BMIR if total synthesized flows in mid-winter were too high and total synthesized flows in mid-summer were too low; however, the results were not good. The close tie of BMIR to base flows suggested shifting to adjusting BMIR to better match recorded base flows. Subroutine SETBMI is based on this concept.

#### PURPOSE

Subroutine SETBMI adjusts the value of the parameter controlling infiltration rate (BMIR) by inspecting the match between synthesized and recorded base flows during the search for the best set of six values for the flow volume parameters.

#### PROCEDURE

The distinctive feature of the adjustment of BMIR is that it is indexed on base as contrasted with total flows. Since base flows are not distinguished within the total flows in the raw data, some technique for hydrograph separation is necessary. The technique employed calls on Subroutine SETRBF to estimate a least squares base flow recession line and thereby provide daily base flow totals. The base flows estimated are for the first three days following a flow peak in each recession sequence selected by Subroutine RECESS. The use of every sequence provides the broadest available data base. Three days are enough to minimize the effects on the adjustment of data peculiarities for individual days. More days could be used, but computational time is increased and the index begins to be weighted according to the recession sequence length.

Actually, only the first of each series of three "recorded" base flows needs to be saved in RSBFF because each later value in the linear recession is BFRC times the previous value. The synthesized

base flow totals for each of the same set of days are saved in SBFRS at MAIN0595. The overall relationship between recorded and synthesized base flows is found by dividing each synthesized by the corresponding recorded base flow (STBM0010) checking the ratio for a maximum of 3.0 to dampen the harm caused by sequences with data problems, and averaging the results (STBM0019). An average greater than one indicates BMIR needs to be reduced to decrease synthesized base flows. An average less than one suggests an increase in BMIR.

The data in Table 17 also provide some idea as to the amount of adjustment of BMIR required to achieve a given change in synthesized base flow. The increase of BMIR by a factor of 3.25 from 0.2 to 0.65 increased the average of the ten monthly base flows by a factor of 2.64 (1/0.379). The increase by a further factor of 3.85 to 2.5 increased the average base flow by a factor of 2.34. If the relationship between the two ratios is assumed to be of the form

$$\frac{\text{BMIR}_2}{\text{BMIR}_1} = \left( \frac{\text{BF}_2}{\text{BF}_1} \right)^n \quad (27)$$

the two estimates of n from the tabulated data would be 1.22 and 1.59 respectively. From the viewpoint of quickly converging on a best estimate of BMIR in the context of all the watersheds tested, 1.3 gave the best results.

One of the major problems encountered in OPSET was the development of a satisfactory method for handling estimates of initial (October 1) moisture conditions (pp. 40-43). Despite the rather elaborate procedure developed for adjusting the initial LZS to match LZO (MAIN0274-0285) in the context of the values being used for the six other parameters, difficulty was still occasionally

encountered. The estimates of initial LZS were close enough so that the problem would be minor after the first major storm; but in some situations, the synthesis of the first storm and subsequent recession would be so poor as to corrupt the adjustment of BMIR.

An index called FTX was established to handle this situation. Its value is set to be 1.0 when no problem was encountered. During the first five rough adjustments, the program is left free to try to bring the synthesized flow volumes in line. If the sum of the second and the third (November and December) monthly flow deviation indices exceeds 2.0 or is less than -2.0 after this (p. 43) the first recession sequence is skipped (STBM0018) by changing the value of IFT from 1 to 2 (MAIN 0075).

Some cases were encountered where the method outlined above for adjusting BMIR using an index estimated from base flows just did not work properly. For example, one may encounter a year with only a few well-defined recession sequences and when each of these happens to be associated with a storm where the recorded rainfall is significantly less than what actually fell. The simulated base flow may then well be less than the recorded values until one makes BMIR so big that all simulated flow is base flow and the true storm hydrograph disappears. The programing is handled by developing a second or alternate adjustment rule to be used whenever the first rule is not working properly.

The shift to the alternate rule for adjusting BMIR comes when BMIR exceeds 20.0 after the fifth rough adjustment (MAIN0785). As 20.0 is an unreasonably large value, the alternate adjustment is to reduce BMIR in a sequence of ten-percent reductions (MAIN0773). The overall synthesis is usually much better during this reduction period than it was during the initial increase of BMIR from 1.2 to

over 20.0 because the other parameters have had time to adjust to better values and the increments are smaller. No problem was encountered with faulty data causing an unmanageably small BMIR during operation of the first adjustment rule because the programming in Subroutine RECESS prevents acceptance of recession sequences with very low base flow.

### DISCUSSION

The adjustment of BMIR based on an index keyed to differences between recorded and synthesized base flows supplemented by an alternate adjustment scheme of retreating from very large values when necessary (actually programmed in MAIN rather than Subroutine SETBMI) proved to work well and produce reasonable and consistent (from year to year) results. The user, however, does need to beware of the possibility of encountering data which the programmed rules cannot handle. The programming in OPSET was developed and refined until it worked for the over 20 tested watersheds, but this does not mean that further refinement will not be necessary.

#### Subroutine SETRBF (Fourth Land Phase Parameter Subroutine)

### CONTEXT

The volume of simulated interflow is controlled by the basic interflow volume factor (BIVF). The basic maximum infiltration rate (BMIR) controls infiltration and thereby the volume of base flow by limiting the amount of moisture entering the ground. Consequently, the adjustment of BIVF in Subroutine SETBIV is designed to improve the match between recorded and simulated interflow volumes. The adjustment of the BMIR in Subroutine SETBMI is designed to improve the match between recorded and simulated

base flow volumes. Since recorded flows are not tagged by category, a technique for separating the recorded flows is required.

Recession sequences are selected from the recorded daily streamflows. After the sequences are used to estimate the two recession constants (See Subroutines RECESS, SET2RC, and SET1RC), the volumes of interflow and base flow on the second day of the recession (first or peak days are omitted because they normally include too much direct runoff) can be estimated from the average recession constants and the individual sequences. A least squares method was developed based on assumptions that the two flow components follow linear recession models and the two recession constants are known.

For cases when no interflow is found, an abbreviated least squares method was developed for estimating base flow on the second day. The estimate slightly differs from the recorded total on that day because observed values will not fall exactly on a linear recession line.

#### PURPOSE

Subroutine SETRBF is used to estimate volumes of interflow and base flow during the first day after direct runoff becomes minimal, given the two recession constants and a sequence of recorded recession daily flow volumes beginning with a volume for the day for which an estimate is desired.

#### PROCEDURE

Subroutine RECESS provides a sequence of recession flows  $q_1, q_2, \dots, q_n$  which are assumed to follow the two-component non-linear model

$$q_t = q_{b,o} K_b^t + q_{i,o} K_i^t + \epsilon_t \quad (t=1, 2, \dots, n) \quad (28)$$

where  $q_{b,0}$  and  $q_{i,0}$  are the base flow and interflow components one day before the day for which volumes are to be estimated.  $K_b$  and  $K_i$  are base flow and interflow daily recession constants, and  $\epsilon_t$  is the random error of the  $t^{\text{th}}$  day. If  $K_b$  and  $K_i$  are known, an approximate solution for  $q_{b,0}$  and  $q_{i,0}$  can be obtained by a least squares method. After rewriting the model of Equation 28 in matrix form as

$$\begin{bmatrix} q_1 \\ q_2 \\ \vdots \\ q_n \end{bmatrix} = \begin{bmatrix} K_{b2} & K_{i2} \\ K_b & K_i \\ \vdots & \vdots \\ K_b^n & K_i^n \end{bmatrix} \begin{bmatrix} q_{b,0} \\ q_{i,0} \end{bmatrix} + \begin{bmatrix} \epsilon_1 \\ \epsilon_2 \\ \vdots \\ \epsilon_n \end{bmatrix} \quad (29)$$

then the least squares estimates of  $q_{b,0}$  and  $q_{i,0}$  ( $\hat{q}_{b,0}$  and  $\hat{q}_{i,0}$ ) are the solutions of the "normal equations"

$$\begin{bmatrix} \hat{q}_{b,0} \\ \hat{q}_{i,0} \end{bmatrix} = \begin{bmatrix} n & \sum_{t=1}^n K_b^t & \sum_{t=1}^n q_t \\ \sum_{t=1}^n K_i^t & n & \sum_{t=1}^n q_t \end{bmatrix} \quad (30)$$

Application of Equation 30 requires at least two flows in any sequence. The maximum number of flows used is preassigned to be 12 to avoid extensive calculations which have little effect on the results.

With  $\hat{q}_{b,0}$  and  $\hat{q}_{i,0}$  estimated, the base flow and interflow at the recession beginning day ( $\hat{q}_{b,1}$  and  $\hat{q}_{i,1}$ ) can be readily obtained as

$$\hat{q}_{b,1} = \hat{q}_{b,0} \cdot K_b \quad (31)$$

$$\hat{q}_{i,1} = \hat{q}_{i,0} \cdot K_i \quad (32)$$



When a recession sequence has an interflow recession constant less than 0.3, which was taken (Subroutine RECESS) as meaning that the interflow in that sequence is negligible, a one-component (base flow only) model is used to estimate the first day base flow, and the first day interflow is set equal to zero. The one-component model can be written as

$$q_t = q_{b,0} K_b^t + \epsilon_t \quad (t=1, 2, \dots, n) \quad (33)$$

The least squares estimate of  $q_{b,0}$ , denoted as  $\hat{q}_{b,0}$ , is

$$\hat{q}_{b,0} = \frac{\sum_{t=1}^n K_b^t \cdot q_t}{\sum_{t=1}^n K_b^{2t}} \quad (34)$$

The base flow on the recession sequence beginning day is obtained by using Equation 31.

## DISCUSSION

The results revealed flow sequences with an interflow recession constant greater than 0.3 to yield estimates of interflow and base flow which total very close to the recorded flow. Estimates for sequences with interflow recession constants less than 0.3 indicate slightly less base flow than the recorded total flow. This might be expected because the interflow is neglected.

Subroutine SETBIV  
(Fifth Land Phase Parameter Subroutine)

CONTEXT

The basic interflow volume factor (BIVF) controls the division of moisture not infiltrating into the soil between interflow and direct runoff and thus indirectly governs the time distribution of storm hydrographs. Crawford and Linsley (10, p. 69) defined BIVF (their name was CC) "as an index to the ratio of the increment added to interflow detention to the increment added to surface runoff detention". They recommended trial-and-error adjustment of its value in an effort to better match recorded with simulated hydrograph shape. A larger value of BIVF reduces the hydrograph peak and increases later flows to net a flatter crested hydrograph.

In exploring for a means of estimating BIVF within OPSET, several factors had to be considered. Hydrograph shape is not a very viable criterion for mathematical testing and relates to direct runoff routing as well as to interflow delay. If some other means could be found for estimating BIVF, information on hydrograph peaks and volumes could be reserved for estimating the streamflow routing parameters.

A review of the procedure used in estimating the interflow and base-flow recession constants suggested an independent method. Subroutine SETRBF was developed to estimate the volume of interflow the first day after the peak preceding each recession sequence. These estimates from the recorded flows could then be compared with synthesized interflow volumes, and BIVF could be adjusted to bring the simulated total in line.

Another factor to be considered was the proper timing within OPSET of the adjustment of BIVF. The program first sets six

parameters based on matching the recorded distribution of total runoff among the months of the year and then sets four parameters based on achieving the recorded hydrograph peaks and times. BIVF is an intermediate parameter between the six volume parameters and the four channel routing parameters. It cannot be estimated during the initial adjustment of the six flow volume parameters because simulated interflow volume also depends on these other parameters and their values are being adjusted in increments too large for BIVF to stabilize on a reasonable estimate. BIVF could be estimated in a special TRIP between that for setting the flow volume parameters and that for setting the routing parameters; however, this approach would add significantly to the required computer time.

The compromise was to combine the estimation of BIVF with the final fine adjustment phase of the six flow volume parameters. In the early rough adjustment of the volume parameters, BIVF is taken as 0.90 (MAIN0215). During fine adjustment, the flow volume parameters are not changing in large enough increments from one trial to the next to create major problems in estimating BIVF. Neither do changes in BIVF change the simulated flows from one month to another enough to upset the criterion used to estimate the other parameters. In fact, the small movement of simulated flow among months which does occur is probably beneficial in terms of estimating a better set of all seven parameters taken together.

The fine adjustment of the flow volume parameters is terminated by criteria devised to indicate that the search cannot find a better fit of recorded monthly flow volumes (pp. 39-40). Even though BIVF is being estimated simultaneously, the procedure is seeking the best match of simulated to recorded interflow volumes on

selected days. Each adjustment gives a better match, and the number of fine adjustments required to determine whether the best possible set of volume parameters has been estimated is enough for the adjustment of BIVF to converge on an acceptable value. Therefore, the estimate of BIVF is taken as the final one and not that associated with the minimum value of SSQM.

When the analysis of flow recession data does not detect any appreciable interflow, BIVF is set to 0.00 (MAIN0226) and Subroutine SETBIV is skipped (MAIN0769).

#### PURPOSE

Subroutine SETBIV adjusts the value of the parameter controlling the portion of the moisture which does not infiltrate into the soil that does go into interflow (BIVF) in order to achieve a better match between synthesized and recorded interflow volumes. This match is used as the sole criterion of the adjustment as BIVF does not alter the distribution of simulated flows among months to a degree which warrants basing goodness of fit on SSQM.

#### PROCEDURE

The adjustment of BIVF is based on matching simulated to recorded interflows during the first three days of each recession sequence selected by Subroutine RECESS. It is assumed that interflow and base flow comprise the total flow beginning on the second day of the recession (See Subroutine SET2RC), and the interflow phases out more quickly than baseflow. By using the first three days, the effects of data difficulties on individual days are damped to give a reasonable estimate of the volume ratio between the recorded and the synthesized interflows while largely avoiding the problem of getting a good ratio between two very small values later in the sequence.

The recorded interflow at the recession beginning is determined by Subroutine SETRBF, and the second and third day interflows are determined by multiplying the first day interflow by the estimated interflow recession constant (IFRC) once and twice respectively. The synthesized interflows are saved on the first three days during the simulation process (MAIN0325-0328, 0593-0594) by keeping track of the recession beginning date (RCSS0025, 0120-0122).

A timing differential must be handled before synthesized are compared with recorded interflows. Flows are recorded at the mouth of the watershed while interflows are simulated for points all over the watershed and not separately (from direct runoff) routed to the mouth. In order to compensate for the simulated interflows within the watershed reaching the mouth of the watershed at a later time, the synthesized interflows are adjusted backward in time by the duration estimated for flow to travel from the centroid to the mouth of watershed (STBV0010).

The ratio of the adjusted synthesized interflow to the recorded interflow is calculated for each of the first three days, and an averaged ratio is used to adjust BIVF. Because too small an estimate of recorded interflow will cause the adjustment factor to be unreasonably large, the ratio for each day is limited to a maximum of 3.0 in computing the average.

BIVF enters the streamflow simulation through the equation (MAIN0411, 0419):

$$\text{CIVM} = \text{BIVF} * 2^{(\text{LZS}/\text{LZC})} \quad (35)$$

In order to prevent simulation of negative interflows, CIVM must be held to a minimum of 1.0 (25, p. 54). No interflow is simulated except when CIVM exceeds unity. Equation 35 thus shows how

with BIVF and LZC held constant, interflow will not be simulated during periods when the watershed is very dry (LZS is small). The smaller the value of BIVF, the larger LZS has to be for interflow to commence. However, other simulation controls prevent LZS from becoming much larger than LZC (actual soil moisture storage cannot pass some upper limit). The upper limit on the multiplier of BIVF in Equation 35 is about 2.5. Therefore no interflow will be simulated when BIVF is less than 0.40.

The adjustment of BIVF assumes synthesized interflow to be proportional to (BIVF - 0.40). For example, if the old value of BIVF is 0.90, and the averaged ratio is 0.25 (synthesized interflow volume is one-fourth of recorded interflow), then (by STBV 0023) the new BIVF is

$$0.40 + \frac{(0.90 - 0.40)}{0.25} = 2.40$$

## DISCUSSION

Generally speaking, flow recession becomes more complex as the size of the watershed increases. Two distinct recession constants could be developed from the data for the larger watersheds, but the best modeling for the smaller watersheds came by dividing all flows between direct runoff and baseflow. Consequently, Subroutine SETBIV was not called for most of the small watersheds used to test and develop OPSET because BIVF had already been set equal to zero. Thus, this subroutine, probably more than any other in the program, needs further testing and refinement using additional data.

Subroutine STRHRS  
(First Channel Routing Subroutine)

CONTEXT

The channel routing parameters are estimated by comparing recorded with simulated times and peaks of selected flood hydrographs. As no more than five points on the total annual hydrograph are compared, it would be very inefficient to route all flows throughout the year to try to match so few points. A great deal of computer time is saved by going through the entire annual hydrograph in TRIP 2, storing all channel inflows contributing to the specified peaks, and then performing the repetitive routing on these selected inflows.

In order to use this approach, it is necessary to determine when to start and stop saving channel inflows for each hydrograph peak. The saved inflows should include all runoff contributing to the peak as well as to the rapid rise just before the peak. The saved inflows should continue long enough to make sure that the flows are receding toward base values with no chance of a subsequent rise from the same runoff event producing a new and higher peak for any combination of trial routing parameters.

PURPOSE

The purpose of STRHRS is to set the beginning and ending times of channel inflows anywhere in the basin contributing to selected flood peaks so that simulated inflows between these two times may be saved for the trial routings necessary for selecting an optimal set of channel routing parameters.

PROCEDURE

The day of the year (RHPD) and the hour of the day (RHPH) for each selected hydrograph peak are specified in the input data. The

interval for saving inflows extends both ways from this point in time. Typical hydrograph shapes have recession limbs about three times as long as rising limbs. (21, pp. 197-206). The length of the interval is based on the value of the read parameter INHPT. The flexibility provided by reading this value gives the user an opportunity to adjust the length of this period for any reason. INHPT also represents the number of hours between points printed on the simulated outflow hydrograph. Normally, prescribed values should increase with the size of the watershed.

The method of specifying the total storm period is shown in Figure 3 and takes the following steps.

1. Specify the number of hours from beginning of saved runoff to recorded hydrograph peak (IBTPR) and the number of hours from recorded peak to the end of saved runoff (IPTE). IBTPR is taken as the maximum number of hours (MXTRH) used by OPSET in any time routing (twice the estimated number of time routing hours MAIN0203) plus five times INHPT, and IPTE is taken as 15 times INHPT. MXTRH is included in determining the beginning hour to take into account the time lag from the far end of the watershed. Use of a maximum value ensures that enough inflows will be saved to include flows from the far end of the watershed with the maximum value of NCTRI the program is allowed to use. (SHRS0009, 0010).

2. Convert the points in time IBTPR hours before the peak and IPTE hours after the peak into dates and hours by counting backward and forward respectively from the recorded peak day and peak hour for each recorded hydrograph (SHRS0012-0042).

3. Check the calculated dates and hours of all recorded hydrographs for two types of overlapping. First, flows



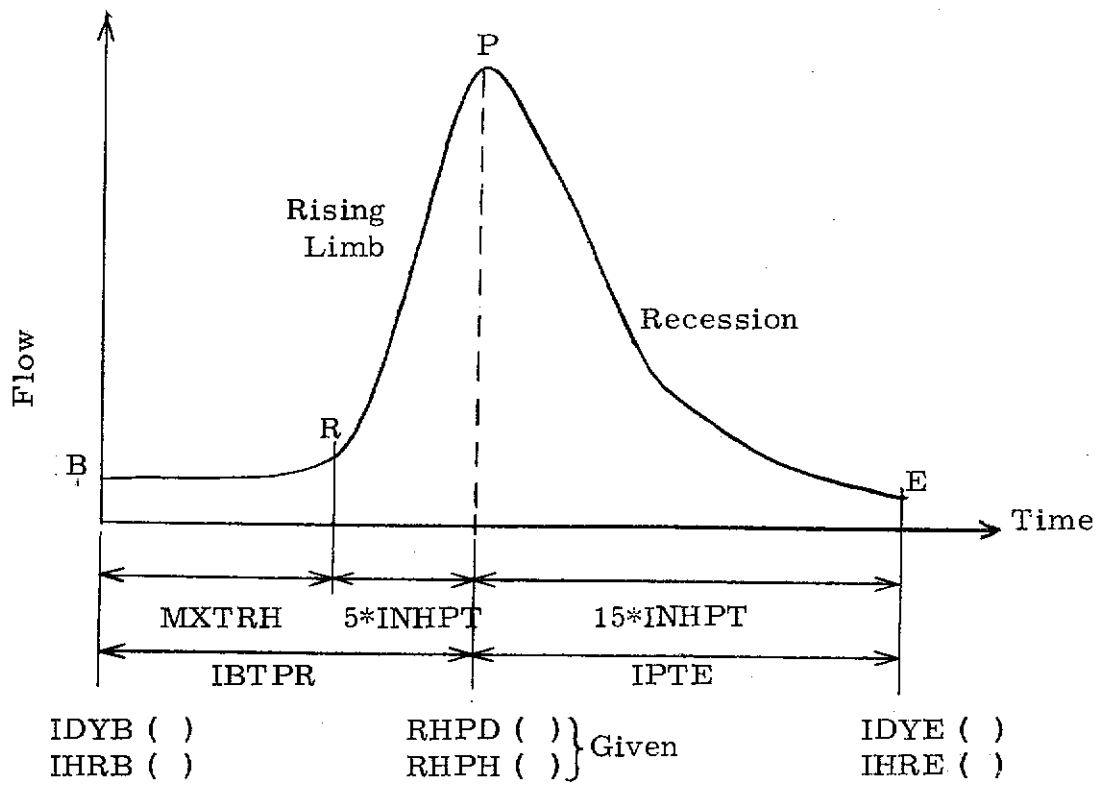


FIG. 3. Determination of Beginning and Ending Times of a Recorded Hydrograph.

contributing to the first recorded flood peak may begin before the start of the water year, or the last recorded hydrograph may extend into the following water year. These cases are handled by confining the beginning and ending to the current water year (SHRS0055-0065). These situations are illustrated in Figures 4(a) and 4(b). Actually, because of the problem of establishing initial conditions, the user should avoid using early October storms unless they represent extraordinarily large events. Second, the ending time of one hydrograph may overlap the beginning time of the next (Figure 4(c)). In this case, the cutoff point of the first hydrograph is shortened to the beginning time of the second (SHRS0051, 0052). A recession limb is arrested by the subsequent rise.

#### DISCUSSION

In choosing INHPT, one should inspect the published daily streamflow data to obtain a general idea of the time base for a typical hydrograph for the watershed. Too large a value of INHPT adds to the computer time, and too small a value may cause the program to drop part of the synthesized inflow which contributes to the peak. If this occurs, the simulated peak will be too low, and OPSET will be biased toward selecting smaller values of NCTRI and SRX in an attempt to compensate by producing a sharper peak.

#### Subroutine ADJHYD

(Second Channel Routing Subroutine)

#### CONTEXT

The criteria for selecting the best set of channel routing parameters seek to minimize differences between recorded and synthesized hydrograph peaks. Differences between recorded

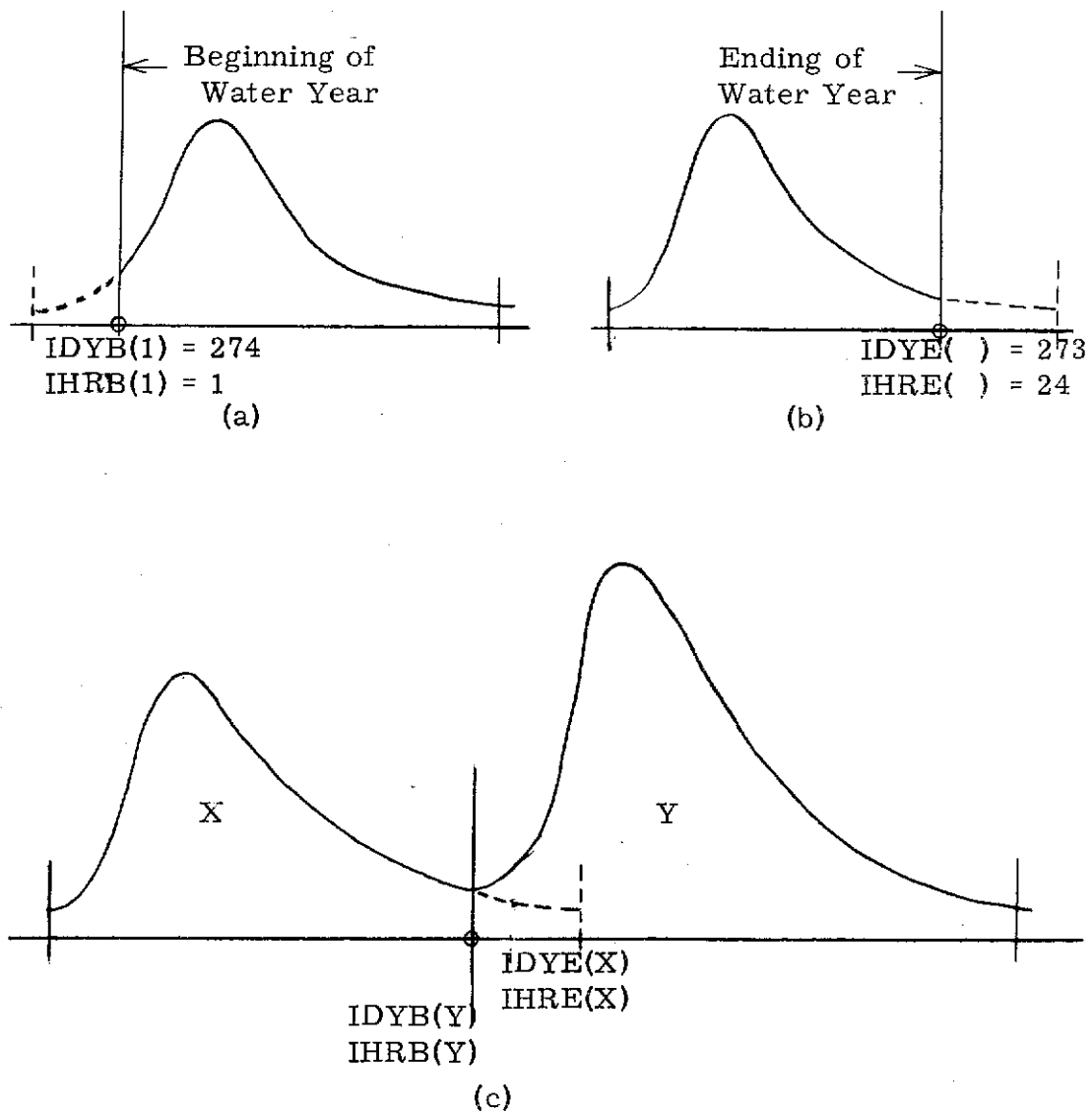


FIG. 4. Elimination of Recorded Hydrograph Overlapping.

peaks and peaks synthesized with the best set of land phase parameters selected in TRIP 1 stem from two causes. First, the set of values for the land phase parameters selected on the basis of providing the best overall estimate of the magnitude and monthly distribution of the annual runoff volume may simulate a volume that is either too high or too low for a given flood. Second, the set of routing parameters may not produce the proper hydrograph shape.

The objective of Subroutine ADJHYD is to adjust each simulated channel inflow hydrograph by a constant multiplier to make the simulated equal the recorded hydrograph volume in order to eliminate differences caused by using an incorrect volume. The goal of TRIP 2 is to select parameters which will do the best job of channel routing, and the results will be distorted if the routing parameters are forced to take on inappropriate values to match a recorded peak having some different total volume. For example, if the peaks selected for fixing the routing parameters happen to be associated with volumes which are too small, the routing parameters selected would produce too sharp a flood hydrograph in an attempt to match the specified peaks.

#### PURPOSE

Subroutine ADJHYD adjusts flow volumes of synthesized channel inflow hydrographs to match the flow volumes of recorded hydrographs in order to overcome bias caused by using different volumes in estimating the two channel routing parameters NCTRI and SRX.

#### PROCEDURE

The flow volume adjustment is accomplished by multiplying each synthesized runoff during the storm period (excluding base

flow which is not modeled by use of channel routing) by a factor equalling the ratio of the recorded to the synthesized storm period runoff volume. This procedure takes three steps:

1. Calculate the recorded and the synthesized direct runoff (including interflow) volumes during the specified storm duration (See Subroutine STRHRS). Hydrograph average base flows are subtracted from daily total flow volumes, and the difference is multiplied by the fraction of the day included within the storm period. Results are summed over all days within the total period, but zero values are added for days with negative differences (i. e. having daily average total flow less than the storm average base flow).

2. Calculate a synthesized hydrograph multiplier (SHM) equal to the ratio of total recorded hydrograph volume (TRHV) to total synthesized hydrograph volume (TSHV).

3. If SHM is less than 8 and greater than  $1/8$ , then adjust each value of synthesized storm runoff (SSR) by multiplying by SHM. If SHM is greater than 8 or less than  $1/8$ , the hydrograph is discarded for use in estimating routing parameters by setting the logical variable LSHA true. The cutoff of 8 is arbitrary, but some value was needed to avoid estimating erroneous routing parameters caused by storms where the direct runoff volume was simulated very poorly, usually because of watershed precipitation measurement problems.

#### Subroutine SETHRP

(Third Channel Routing Subroutine)

#### CONTEXT

The channel routing process incorporated into the earlier versions of the Stanford Watershed Model (10) employs a time-

delay histogram to account for the time required for runoff originating throughout the watershed to reach the mouth and then uses storage routing through a theoretical reservoir to account for the effect of channel storage on the hydrograph (21, p. 304). The time lag routing is based on NCTRI fractions in array CTRI. The storage routing uses the equation

$$O_2 = \bar{I} - SRX (\bar{I} - O_1) \quad (36)$$

where  $O_2$  = routed outflow at the end of the time interval;  
 $\bar{I}$  = average inflow during the time interval;  
 $O_1$  = outflow at the beginning of the time interval;  
 SRX = a storage routing index less than unity in which smaller values imply less storage dampening.

Conceptually, both NCTRI and SRX should vary during the course of a flood hydrograph. NCTRI decreases with increasing flow because the associated faster flow velocities reduce travel time. SRX increases by more closely approaching unity with increasing flow because of greater hydrograph dampening as more water is stored in the channel system, an increase becoming most rapid when large volumes of water begin to spill into the flood plain.

The experience gained by applying the Model to a number of watersheds has shown the variation in SRX to be more pronounced than that in NCTRI. In traditional hydrologic terms, the storage routing coefficients vary more than does the time base of the unit hydrograph. Even though the KWM provides the option of varying NCTRI (25, pp. 23-24), OPSET only considers variation in SRX. The analysis attempts to distinguish a value of SRX for low flows (CSRX) from another one for flood flows (FSRX). The strategy is to estimate one at a time the values of NCTRI and SRX (taken as

independent of flow in the first pass) best matching the times and peak flows of up to five specified hydrograph peaks.

#### PURPOSE

Subroutine SETHRP estimates single best values for NCTRI and SRX for each of the up to five hydrographs specified in the input data. It then averages the hydrograph values of NCTRI to make a single overall best estimate. It calls on Subroutine SETSRP with the hydrograph values of SRX to regress on hydrograph peaks to determine whether the data at hand substantiate the expected tendency for higher values to be associated with higher peaks.

#### PROGRAM DEVELOPMENT

For each station-year, and for up to five flood hydrographs, the flood peak day of the year, the flood peak hour of the day, and flood peak magnitude are read. Subroutine STRHRS is used to determine the beginning and ending hour of runoff contributing to the recorded flood hydrograph, and all synthesized channel inflows between these two times are saved in the pass of TRIP 2 through MAIN. In order to prevent the estimates of NCTRI and SRX from being biased by any difference between recorded and synthesized flood volumes, Subroutine ADJHYD is used to adjust the magnitude of each synthesized flow entering the channel to make the synthesized flow volume equal the recorded volume. Subroutine SETHRP is then called to estimate the two hydrograph routing parameters for each flood hydrograph.

The first step in developing a procedure for this purpose was to study how the magnitude and timing of the synthesized flood peak change with these two variables. Figure 5 shows how NCTRI and SRX affect a simulated flood hydrograph. Two values are shown. The upper one is the synthesized hydrograph peak flow (SHPF), and lower one is the number of time intervals from the recorded to the synthesized

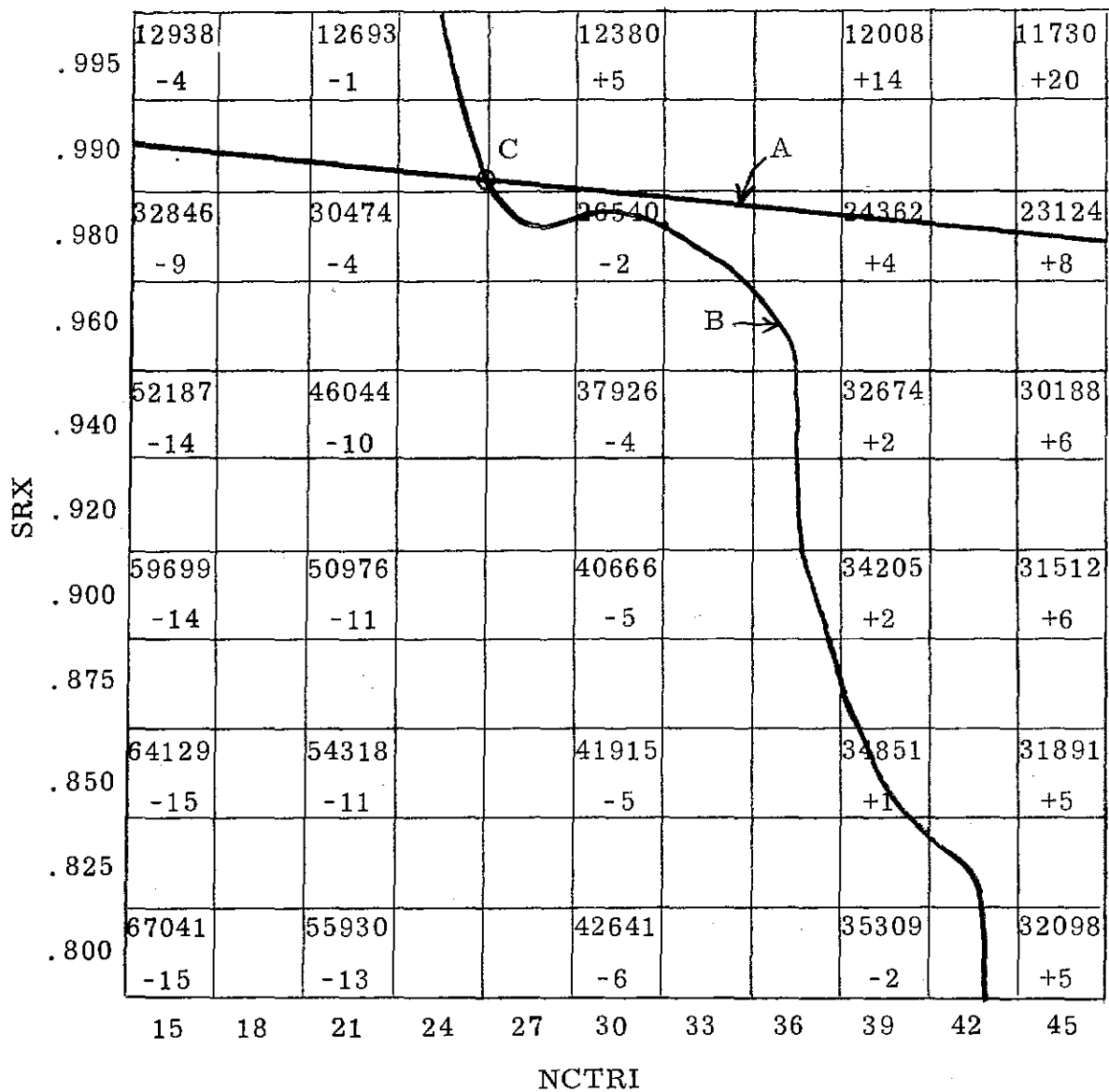


FIG. 5. Response of a Given Flood Hydrograph (Elkhorn Creek, March, 1964) to Changing the Two Hydrograph Routing Parameters NCTRI and SRX. (RHPF = 23200 cfs, upper number is SHPF, lower number is NIRTS).



peak (NIRTS). A negative NIRTS means the synthesized hydrograph peak comes sooner than the recorded; a positive value means it comes later.

Figure 5 reveals what Equation 36 and the time-area routing procedure already imply, namely that 1) with constant NCTRI, SHPF increases and NIRTS decreases as SRX decreases because a smaller storage effect produces a quicker and higher peak, and 2) with constant SRX, SHPF decreases and NIRTS increases as NCTRI increases because longer flow times produce a later and lower peak.

Line A represents the combinations of values of NCTRI and SRX for which the synthesized hydrograph peak (SHPF) equals the recorded hydrograph peak (RHPF). Above line A, SHPF is smaller than RHPF; below line A, SHPF is larger than RHPF. Line B represents the combinations for which the synthesized hydrograph peak hour is identical to recorded hydrograph peak hour, i. e., NIRTS is equal to zero. To the left of line B, NIRTS is negative; and to the right, NIRTS is positive. Point C, at the intersection of lines A and B, indicates the values of NCTRI and SRX for which the synthesized hydrograph would have its time and magnitude equal to the recorded time and magnitude. The consistency with which both lines slope from the upper left to the lower right because of the two rules stated above suggests use of a search procedure for Point C following this same basic pattern. Once it has been established that a better match lies to the right, no further need exists to search to the left. Once it has been established that a better match lies below, no further need exists to search above.

Subroutine SETHRP seeks the combination of values for NCTRI and SRX which produces a synthesized hydrograph with a flood peak most closely matching the recorded peak in magnitude and time. If gaged rainfall closely represents watershed rainfall in amounts and

timing and synchronizes with streamflow measurements and if land surface runoff is adequately simulated, one would expect to always find the solution point (C on Figure 5). Unfortunately, poor quality data may produce lines which do not cross or a solution point outside reasonable limits.

The major difficulty experienced was caused by a difference in timing between the watershed and gaged rainfall. If a storm was slowly moving from the watershed toward the gage and the runoff peak were recorded soon after the gaged rain began, point C might indicate a time of concentration of 15 minutes (NCTRI=1) for a large watershed. With the storm moving the other way, a very long time might be indicated for a small watershed. The consistency of the error depends on the prevailing storm movement pattern.

Subroutine SETHRP is programmed to find the solution point if one exists within the range of reasonable values for NCTRI and SRX. While one might take all solution points, whether or not they are individually reasonable, and average them over many flood peaks to cancel errors in scattered directions and get a reasonable overall best estimate, too few distinct hydrographs normally occur in any given year. More consistent estimates can be found in fewer runs by discarding unacceptable solutions than by saving them and hoping they average out. Thus, Subroutine SETHRP looks for solution points or approximate solution points in a predetermined "reasonable" range and rejects the data when it cannot find one.

Two rules and four boundary limits are used in Subroutine SETHRP. Figure 6 shows the area bounded by the four limits for a typical flood hydrograph. Definitions of lines A and B and Point C are the same as those in Figure 5. Arrows indicate the directions and circled numbers indicate the rule numbers used in converging on Point C.

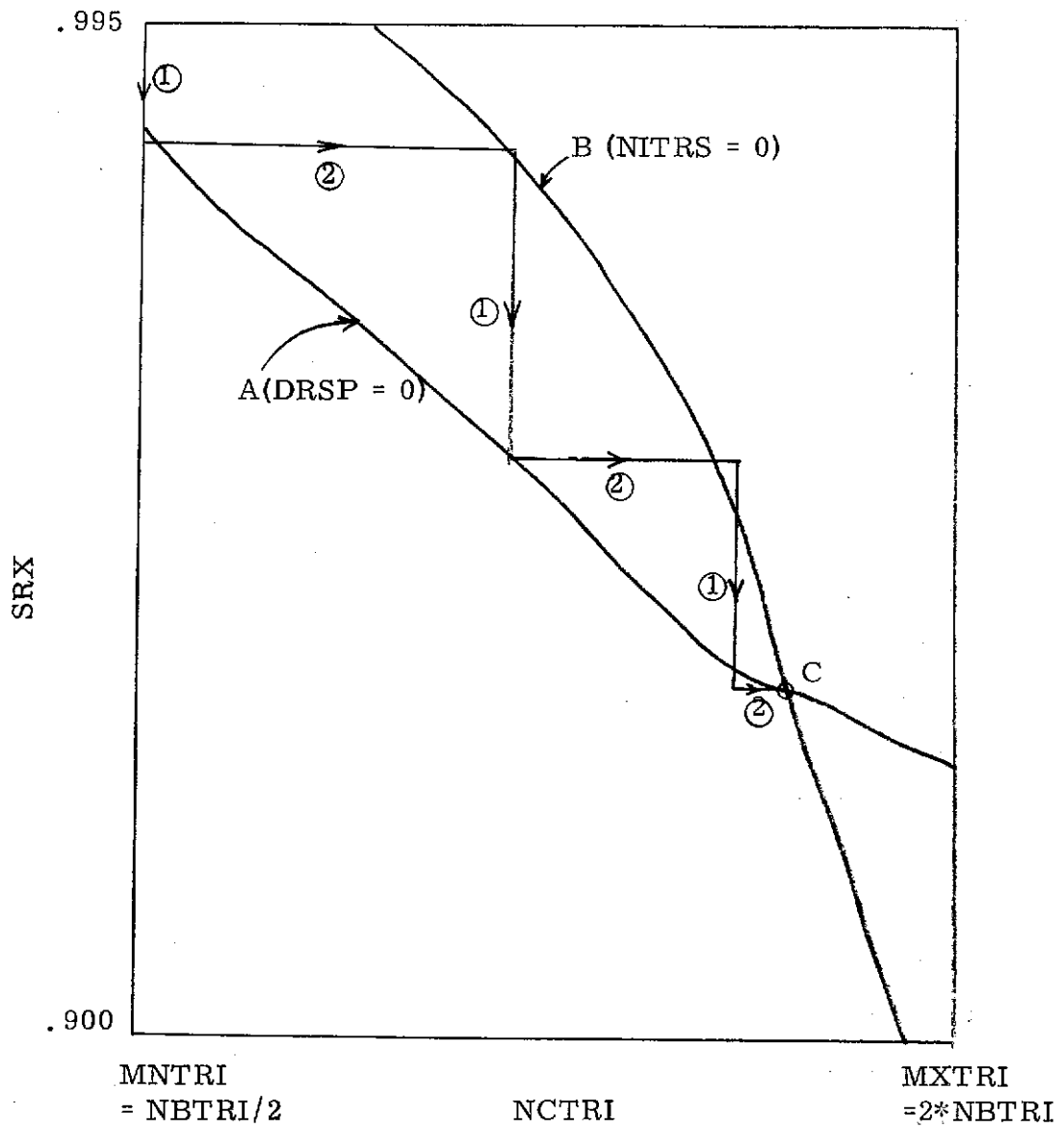


FIG. 6. Rules Used to Find Optimal Set of NCTRI and SRX for Each Flood Hydrograph.

The maximum acceptable value of NCTRI (MXTRI) is twice the number of basic time routing increments (NBTRI) and the minimum value of NCTRI (MNTRI) is one-half NBTRI. The assumption is that the user should be able to estimate the time of concentration (NBTRI) of a basin from topographic information within a factor of two, one way or the other.

The upper limit of SRX is set to 0.995, slightly less than the theoretical upper limit of unity. This is the maximum value which could be used and still avoid computer rounding which increases the volume of the storm during routing unless one resorts to double precision computations. Sensitivity studies show that the effectiveness of SRX in damping the flood peak becomes rapidly more pronounced the closer the value gets to unity. When SRX is decreased to 0.900 or lower, changes in SRX cause rather small changes in simulated flood peak magnitude and time. Therefore, the increments of change in SRX are made smaller for higher values but larger for lower values, and 0.900 is the lower limit of SRX.

The scatter of solution points among storms on a given watershed caused by differences in data quality also suggests that little can be gained by seeking the exact mathematical solution for each of the recorded hydrograph peaks. Computationally, it is much quicker to search among discrete points for the combination of NCTRI and SRX providing a synthesized peak closest to the recorded peak. Six values were chosen for SRX (0.995, 0.99, 0.98, 0.96, 0.93, and 0.90). One is already confined to searching among integer values for NCTRI by the way the variable is defined in the program.

#### OPTIMIZATION RULES AND PROCEDURE

As shown in Figure 6, the estimation procedure starts in upper left hand corner and moves down looking for Line A. After Line A

is found, it moves to the right to find Line B and then moves down to find Line A again. The pattern is repeated until Point C is found.

Examining the process in more detail by using the hydrographs illustrated in Table 18 as numerical examples, it starts with values of NCTRI equal to MNTRI and SRX equal to 0.995. Subroutine FIXTRI is called to establish values of the fractional individual time routing increments (CTRI) to match MNTRI. Then Subroutines TIMERT and STORRT are called to execute channel time routing and channel storage routing of the saved channel inflows. The synthesized flood hydrograph peak (SHPF) and the flood peak time(ITBTS) are used to calculate 1) NIRTS--the number of time routing intervals from the recorded to the synthesized peak and 2) DRSP--the difference in cfs between the recorded and synthesized hydrograph peaks.

The search procedure then chooses between two rules in selecting the discrete point in the field of acceptable points to try next. One rule moves downward by reducing SRX; the other moves to the right by increasing NCTRI. SRX is reduced when it is desirable to increase the synthesized peak flow to more closely match the recorded. NCTRI is increased when it is desirable to more closely match the recorded peak timing.

Rule 1: Hold NCTRI constant and decrease SRX through the series of discrete points until

1. SHPF exceeds RHPF/1.2. If the synthesized flood peak is more than 20 percent too low, a reduction in SRX to improve the matching by increasing the peak flow is preferred over an increase in NCTRI to improve the timing since that can only further reduce the peak. If NIRTS is negative, the adjustment shifts to Rule 2 in an attempt to improve the timing (See trial 6 for the storm of December 22 on Table 18). If NIRTS is zero or positive, the search ends; and SRX

TABLE 18  
ESTIMATION OF NCTRI AND SRX FOR POND CREEK, KY.  
1968, STORM HYDROGRAPHS

Trial No.	Trial Values		NIRTS	DRSP	Make Next Estimate by Rule
	NCTRI	SRX			
Storm of December 2			Recorded Peak = 1330.0 cfs		
1	2	0.995	-1	1112.0	1
2	2	0.990	-1	1029.6	1
3	2	0.985	-2	957.6	1
4	2	0.980	-2	891.1	1
5	2	0.960	-3	687.3	1
6	2	0.930	-4	427.8	1
7	2	0.900	-4	221.8	
Storm of December 22			Recorded Peak = 1550.0 cfs		
1	2	0.995	-3	1275.4	1
2	2	0.990	-3	1126.4	1
3	2	0.985	-3	993.2	1
4	2	0.980	-3	872.7	1
5	2	0.960	-4	428.8	1
6	2	0.930	-4	78.4 <sup>a</sup>	2
7	6	0.930	-2	304.7 <sup>a</sup>	1
8	6	0.900	-2	109.9 <sup>a</sup>	2
9	8	0.900	-1	295.6	
Storm of March 21			Recorded Peak = 3320.0 cfs		
1	2	0.995	-12	2615.4	1
2	2	0.990	-12	2056.9	1
3	2	0.985	-13	1547.3	1
4	2	0.980	-13	1065.6 <sup>b</sup>	1
5	2	0.960	-13	531.5 <sup>b</sup>	2&1
6	8	0.930	-10	505.3 <sup>b</sup>	1
7	8	0.900	-11	1016.6 <sup>b</sup>	
Storm of April 4			Recorded Peak = 4320.0 cfs		
1	2	0.995	-1	3002.6	1
2	2	0.990	-2	2026.4	1
3	2	0.985	-2	1155.2 <sup>a</sup>	1
4	2	0.980	-2	384.5 <sup>a</sup>	2
5	4	0.980	-1	480.2 <sup>a</sup>	2
6	5	0.980	-1	562.2	1
7	5	0.960	-1	1672.3 <sup>a, b</sup>	2
8	6	0.960	-1	1462.6 <sup>b</sup>	

TABLE 18 (cont'd.)

Trial No.	Trial Values		NIRTS	DRSP	Make Next Estimate by Rule
	NCTRI	SRX			
Storm of May 26			Recorded Peak = 1820.0 cfs		
1	2	0.995	-2	1380.5	1
2	2	0.990	-2	1162.4	1
3	2	0.985	-3	1008.8	1
4	2	0.980	-20	877.0	1
5	2	0.960	-20	203.2	2&1
6	8	0.930	-17	249.8	

- a. DRSP not saved in Rule 2 because the rule seeks smaller NIRTS.
- b. Synthesized peak is greater than recorded peak. (In all other cases, the synthesized is less than the recorded peak.)

and NCTRI are taken as their trial values.

2. SRX reaches the lower limit of 0.90 with a value of SHPF which is still too small even though increasing (See the storm of December 2, in Table 18). For such a storm event, SRX is taken as 0.90, and NCTRI is taken as the last value selected for this parameter (MNTRI if Rule 2 was never applied during the search). While SRX could be reduced to lower values, this did not prove wise because a low sensitivity makes a large reduction in SRX required to significantly raise the synthesized peak and averaging the low value (usually caused by a data inconsistency anyway) with those from other storms did not yield a good overall parameter estimate for the watershed.

Rule 2: Hold the value of SRX and adjust NCTRI by subtracting NIRTS. As NIRTS is usually negative (Table 18), the subtraction will lengthen the time base to produce the desired later peak. After each change of NCTRI, Subroutine FIXTRI is called to fix the time routing increments. Then Subroutines TIMERT and STORRT are used to simulate another hydrograph peak, and new values of NIRTS and DRSP are calculated. The adjustment of NCTRI is repeated until

1. NIRTS is reduced to zero or as close to zero as is possible with integer variation of NCTRI. The test for this latter case is whether NIBRS, the absolute value of NCTRI, is decreasing.

2. NCTRI is increased past the maximum acceptable value.

3. NCTRI is increased to the point where the synthesized flood peak is too low to make a larger NCTRI and hence a further reduction in SHFP wise.

Each case needs to be described in further detail.

1. Here, the search has arrived at the best possible estimate of NCTRI associated with the held value of SRX. One of two things is done next: 1) if DRSP is greater than it was for the previously



held value of SRX, the search is interpreted as having passed the solution point. NCTRI and SRX are taken as their trial values for the best point using the previous value of SRX. An example occurs as the program returns from trial 8 to 6 during the tabulated storm for April 4; 2) if DRSP is less than what it was for the previously held value of SRX, return to Rule 1 in an attempt to find a still better value. An example occurs as the program goes from trial 6 to 7 during the April 4 storm.

2. Here, the search is prevented from trying a value of NCTRI outside the acceptable range. The search uses the same criteria as in Case 1 to decide whether to return to Rule 1 and further reduce SRX with NCTRI equal to MXTRI or take a solution from a previous trial value. The tabulated storm of March 21 truncated NCTRI from 15 to 8 and continued the search with trials 6 and 7. In this case to save computer time, the search goes to Rule 2 and back to Rule 1 without performing a routing under Rule 2 (March 21, trial 5).

3. Here, the search returns immediately to Rule 1 to increase the peak (See trial 7 for the storm of December 22).

Close inspection of these rules reveals that they produce an approximate solution point without specifying an explicit criteria for weighting the relative importance attached to peak and time. Operationally, the rules stress peak flow rate when synthesized flows miss by over 20 percent and the time of peak when they do not. Thus, for December 22, trial 9 is preferred over trials 6 or 8. Admittedly, such a search procedure is coarse, but most data do not permit greater precision.

If the highest hydrograph peak synthesized for a given storm is less than one-half of the recorded peak, the set of values of NCTRI and SRX are rejected and that storm is excluded in estimating the parameters.

The average value of NCTRI for a given station-year is taken as the mean of the storm weighted by recorded hydrograph peak flow because a larger flood hydrograph gives a better estimate of NCTRI. The subroutine then calls Subroutine SETSRP to estimate CSRX and FSRX.

### DISCUSSION

Subroutine SETHRP uses a coarse search procedure to estimate a pair of values which simply cannot be estimated with any precision with the quality of data normally encountered by the model. The procedure relies on the mean of a series of estimates to dampen a large random estimating error likely to be encountered in a particular hydrograph. Fifteen estimates may be available in the three years of record recommended in Chapter IV.

### Subroutine FIXTRI (Fourth Channel Routing Subroutine)

### CONTEXT

OPSET is supplied input data describing the shape of the watershed in the form of a time-area histogram. Read are a specified number (NBTRI) of time-area elements (BTRI) and a specification in Control Option 2 as to whether the elements are estimated on a 15-minute or on an hourly basis. However, during the channel routing parameter optimization, it is necessary to perform routings with a value of NCTRI different from NBTRI; therefore, the values of the individual elements must also be adjusted.

For example while data is being collected, a basin may be estimated to have a time of concentration of three hours, and the fractions of the basin from which runoff reaches the mouth of the basin during the first, second, and third hours respectively as

estimated from topographic maps provide the numerical values of the three elements. When a routing is to be done using 2 hours for the time of concentration (NCTRI), a numerical procedure is needed for estimating appropriate values for two elements from the values of three elements.

#### PURPOSE

Subroutine FIXTRI is used to fix values of the individual time routing elements to match the required total number of values. FIXTRI is called whenever there is a change of NCTRI since the last channel time routing.

#### PROCEDURE

For a given watershed, a base number of time routing increments (NBTRI) and the values of increments (BTRI) are read as input data and represent a histogram derived from a topographic map of the watershed. (10, pp. 21-26; 25, pp. 36-39). For purposes of explaining the procedure used in Subroutine FIXTRI, consider a hypothetical basin having three read time-area elements of 0.42, 0.32, and 0.26 and where a two-element array is needed. The following procedure is used.

1. Divide each of the base time routing increments (BTRI) by the number of current time routing increments (NCTRI); thus there will be (NBTRI) x (NCTRI) elements. For the given example, NBTRI is 3, NCTRI is 2; therefore, the resulting six elements will be 0.21, 0.21, 0.16, 0.16, 0.13, and 0.13.

2. In the order of time intervals in time-area histogram, place the elements into NCTRI groups with NBTRI elements in each group. In the example, the six elements are grouped into two groups as (0.21, 0.21, 0.16) and (0.16, 0.13, 0.13).

3. Sum the values of elements in each group, and the values of group sums are assigned to be the values of the current time routing increments. In the example, the resulting two elements are 0.58 and 0.42.

### DISCUSSION

The above procedure will produce the required NCTRI increments. However, the results are approximate because it is assumed that the sub-areas in the base time-area histogram are uniformly distributed in each time interval. In the example, the second original increment of 0.32 was assumed to divide equally between its first and second half hours when in fact a larger share of the basin might be in one of the two. For watersheds with a large NBTRI, this assumption gives better results.

### Subroutine TIMERT (Fifth Channel Routing Subroutine)

### CONTEXT

Two groups of equations are used within the Model to simulate the process whereby the stream channel system translates patterns of runoff from the land surface to an outflow hydrograph. The MAIN program accounts for the time lag from the time runoff enters the channel to when it reaches the mouth by use of a time area histogram (MAIN0484-0498). It then follows with programing to account for the effect of channel storage on hydrograph attenuation (MAIN0500-0504).

During the process wherein OPSET estimates the length of the time-area histogram (NCTRI) and the storage routing index (SRX), a large number of routings with trial values are required. Because the trial routings differ from those in MAIN in that they must save the channel inflow hydrographs for later trials, require some

adjustment in terminology, and are repetitious, special subroutines were developed for both procedures. Subroutine TIMERT is an adaptation of the time routing programming in MAIN for use by Subroutines SETHRP (STHP0035) and SETSRP (STSP0076). Subroutine STORRT is an adaptation of the storage routing programming in MAIN called in Subroutines SETHRP (STHP0038) and SETSRP (STSP0077).

#### PURPOSE

Subroutine TIMERT is used to perform the repetitious trial routing required in the search for the channel routing parameter values which best account for the time lag it takes runoff to pass through the channel system.

#### PROCEDURE

The channel time routing technique originally used by Crawford and Linsley (10, p. 44) can be expressed as

$$I_t = \sum_{x=0}^{x=z-1} R_{t-x} C_{x+1} \quad (37)$$

where  $I_t$  = storm runoff in period  $t$  routed to the mouth of the channel (SRR);  
 $R_{t-x}$  = unrouted synthesized overland flow plus interflow runoff (SSR)  $x$  time periods ago;  
 $C_{x+1}$  = the  $x+1$  time routing increment value (CTRI), expressed as a fraction of the watershed area;  
 $z$  = number of time routing increments in the time-area histogram (NCTRI).

For example, given a series of unrouted synthesized runoffs and a time routing histogram, the routed runoff at the end of the third time interval will be the sum of 1) channel inflow in the first (closest to

the gage) time routing incremental area during the most recent time interval, 2) channel inflow in the second time routing incremental area during the previous time interval, and 3) channel inflow in the third time routing incremental area during the second previous time interval.

### DISCUSSION

In their reports describing the adaption of the method developed by Clark (7) to simulate channel time routing, Crawford and Linsley suggested using a modified time-delay histogram (10, pp. 24-27) rather than the time-area histogram derived in the method described by Ross (25, pp. 36-39) and used in developing the input data for testing OPSET. The time-delay histogram is a time-area histogram slightly delayed to account for the finite time required for flows to traverse the width of the histogram band. The difference between the two histograms decreases as one deals with watersheds divided into an increasingly large number of bands.

The modified or time-delay histogram was not used in gathering the test data for OPSET because it is more difficult to construct. More importantly, it is doubtful that the time-area bands can be plotted sufficiently accurately to justify the refinement as really adding to the precision of the results. Even more importantly, OPSET adjusts the length of the originally read histogram to best match simulated with recorded hydrographs. If a delayed hydrograph produces a better fit, OPSET will automatically provide one by calling Subroutine FIXTRI with a larger optimum value of NCTRI.

Subroutine STORRT  
(Sixth Channel Routing Subroutine)

PURPOSE

Subroutine STORRT serves as a companion to Subroutine TIMERT and is used to perform the repetitious trial routing required in the search for the routing parameter values which best account for observed storage attenuation effects as runoff moves through the channel system.

PROCEDURE

The time-routed synthesized storm runoff (SRR) from Subroutine TIMERT is taken as the inflow to a hypothetical "reservoir." The storage routing is based on Equation 36 and starts at the beginning of the saved synthesized runoff and continues throughout the Subroutine STRHRS selected hydrograph duration. When CONOP2 is specified in the input data as zero, 15-minute routing is used; when CONOP2 = 1, a 60-minute period is used. Base flow is not routed (MAIN0468, 0481-0482); however, a fixed uniform base flow is estimated for each storm hydrograph (MAIN0521, 0664) and added to each routed runoff to determine total flow (SRRT0027). The time (IBTPS) from the beginning of saved runoff to the synthesized hydrograph peak (SHPF) is recorded. SHPF and IBTPS are used in Subroutine SETHRP in the selection of optimum values for NCTRI and SRX.

When Subroutine STORRT is called from Subroutine SETHRP, a single value of SRX is used throughout the entire routing (i. e., FSRX = CSRX) in an attempt to find a value which produces the best estimate of the recorded hydrograph peak flows. However, when it is called from Subroutine SETSRP, SRX is varied as a function of the synthesized total flows in the manner shown on Figure 5 (SRRT0021-0024).

In order to provide information on the shapes of the synthesized storm hydrographs, 21 points from the channel-routed hydrograph (5 points before the time of the recorded hydrograph peak, and 15 points after that time) are tabulated. The interval of printing points (INHPT) is chosen to make the total time base long enough to cover the storm hydrograph (See Subroutine STRHRS). The 21 values are for visual inspection only, and those other than the peak do not enter into parameter estimation. While the recorded hydrographs are set up to all peak at the sixth point, the synthesized peak will not necessarily fall at this time and in fact may not fall at any of the preselected time points. The synthesized peak value is printed below the hydrograph.

Subroutine SETSRP  
(Seventh Channel Routing Subroutine)

CONTEXT

The first pass at estimating a watershed value for SRX begins as presented in Subroutine SETHRP. For each of up to five flow peaks specified in the input data, a value of SRX is found which when used throughout the entire routing produces the best estimate of the recorded flow peak values. The remaining issue is whether the watershed SRX should be taken as the mean of the storm values or whether two values should be used, CSRX when flows are confined to the channel and FSRX when flow enters the flood plain.

Resolution of this issue requires a test. The one used was to regress storm values of SRX against storm flow peaks. If the regression shows SRX to increase with flood peak, the trend to be expected from qualitative analysis of the physical problem, the data is further evaluated to see if separate values of CSRX and FSRX can



be estimated. If the regression shows no (or a reverse) trend, the storm values of SRX can be averaged for a single overall watershed value; CSRX equals FSRX.

#### PURPOSE

Subroutine SETSRP estimates two channel routing parameters. CSRX is the confined flow channel storage routing index and FSRX is the flood flow channel storage routing index.

#### DISCUSSION OF STORAGE ROUTING

In their earlier versions of the Stanford Watershed Model (10, p. 46), Crawford and Linsley used a single value for SRX\* in Equation 36 for all routing through the theoretical reservoir used to account for the effect of channel storage on streamflow. More recently they have used a more sophisticated but computationally more complicated kinematic wave routing in a program capable of simultaneously simulating flows at a large number of points within a given watershed. In developing OPSET, emphasis was placed on estimating the parameters which control the volume and seasonal distribution runoff rather than flood hydrograph shape. The strategy of retaining the theoretical storage reservoir concept provides good results for simulating runoff at the mouth of a single small watershed. Anyone desiring more comprehensive storage routing can readily combine estimates for the other parameters made by OPSET with the routing features of the more recent versions.

While the theoretical reservoir concept was retained, the single parameter approach was modified. Streamflow is often better modeled by varying SRX with stream flow rate in order to incorporate the effectiveness of flood plain storage in dampening flood hydrographs

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\* SRX is equivalent to KS1 as used by Crawford and Linsley.

to a greater degree than can storage in the channel alone. Therefore, the Kentucky Watershed Model selects the value for SRX it uses at any given time according to the magnitude of the streamflow synthesized for the previous routing period (Figure 7). CHCAP is used to index the flow rate at which flood plain, as contrasted with natural channel storage, becomes the predominate influence on storage routing.

When the synthesized streamflow is less than one-half of CHCAP, CSRX is used for routing. If the synthesized flow exceeds twice CHCAP, FSRX is used. When the synthesized flow is between these values, SRX is interpolated from the cubic curve expressed mathematically by the equation

$$SRX = CSRX + (FSRX - CSRX) \times \left( \frac{Q - 0.5CHCAP}{1.5CHCAP} \right)^3 \quad (38)$$

where Q is the synthesized streamflow.

#### PROCEDURE

In order to determine whether the hydrographs used in a given run of OPSET exhibit a trend toward higher values of SRX for larger flows, the values of SRX selected by Subroutine SETHRP (which estimates one value of SRX for use in routing the entire hydrograph) were regressed on the corresponding recorded hydrograph peak flows. An increase of SRX with peak flow suggests flood plain damping. In other words, for smaller hydrographs, flows are confined to the main channel, the damping of the hydrograph is limited, and the result is a low SRX. On the contrary, large floods overflow the channel bank. The much greater flood plain storage damps the flood peaks more drastically, and the value of SRX is higher.

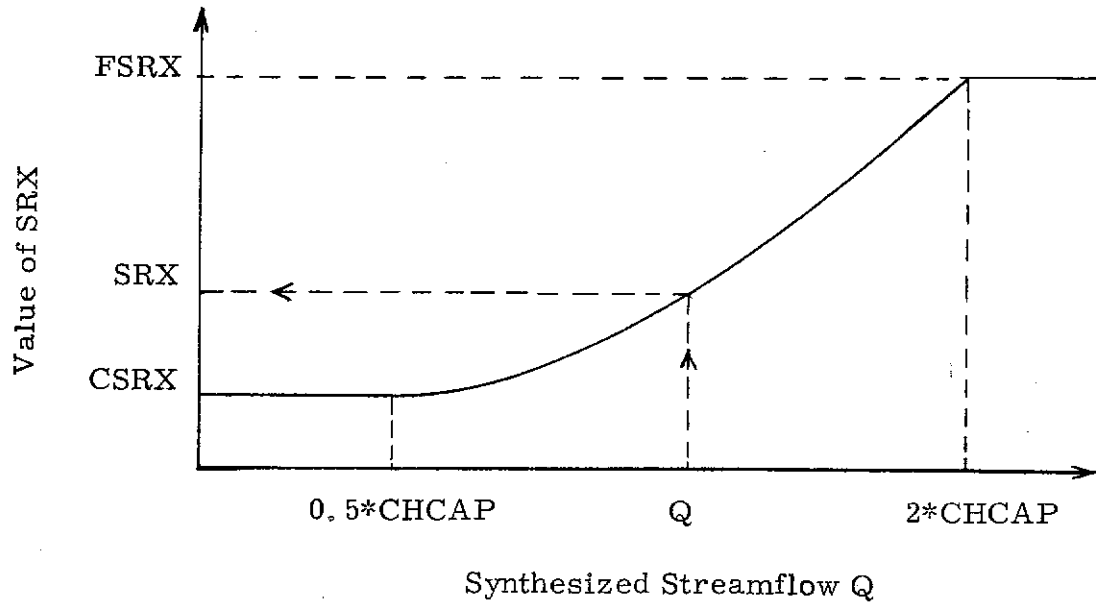


FIG. 7. Relationship Between  $SRX$  and Synthesized Streamflow.

If the coefficient from the regression of the estimates of SRX on the corresponding recorded hydrograph peaks is positive, the first step is to estimate CSRX (Figure 7) as

$$\text{CSRX} = a + b(\text{CHCAP}/2) \quad (39)$$

where the coefficients a and b are the least squares estimates of  $\alpha$  and  $\beta$  in the linear model

$$\text{SRX} = \alpha + \beta \cdot \text{RHPF} + \epsilon \quad (40)$$

If CSRX turns out less than 0.8, CSRX is set equal to 0.8 to prevent the selection of physically unreasonable values caused by hydrographs with nearly equal peak flows having comparatively large differences in estimated SRX.

For floods peaking at a flow less than one-half of CHCAP, CSRX is used for all storage routing throughout the hydrograph. For larger storms, SRX is varied with flow in the manner depicted in Figure 7. FSRX cannot be estimated by substituting twice CHCAP for CHCAP/2 in Equation 39 because the data were derived by routing the entire hydrograph with a fixed SRX. The value of FSRX is instead estimated by trial-and-error adjustment between the values of CSRX and 0.99 used as the upper limit to minimize the effect of rounding errors on flood volume.

Figure 8 illustrates the scheme used to estimate FSRX based on the criterion of minimizing SQPKD, the sum of the squares of the differences between recorded and synthesized hydrograph peaks. The adjustment uses two sizes of increments for FSRX. The big increment (BISRX) is used to rapidly locate the low region in the U-shaped curve whose minimum point indicates the optimal value of FSRX, and the small increment (SISRX) is used to backtrack once

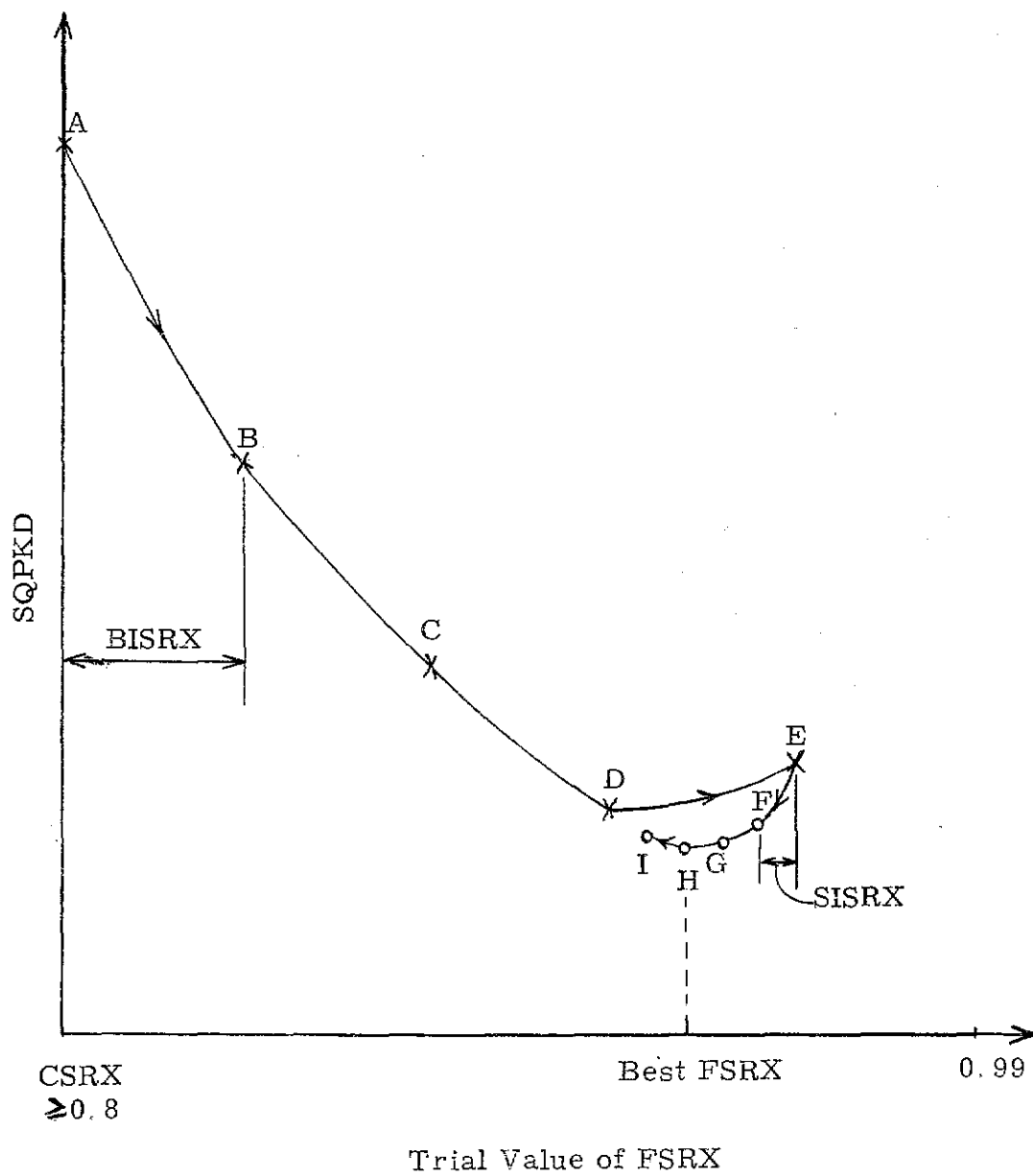


FIG. 8. Determination of FSRX by Trial and Error Method.

the minimum is passed to more closely estimate FSRX. BISRX is taken as one-fifth of the difference between 0.99 and CSRX. SISRX is one-fifth of BISRX.

The iterative procedure starts with FSRX=CSRX to route the selected flood hydrographs. The optimized NCTRI (calculated in Subroutine SETHRP) is used for channel time routing and the trial values of CSRX and FSRX are used for channel storage routing. Then the difference between each pair of recorded and synthesized hydrograph peaks (DRSP), the accumulated sum of DRSP (ADRSP), and the sum of the squares of DRSP (SQPKD) are calculated by the following equations:

$$DRSP_i = SHPF_i - RHPF_i \quad (i=1, 2, \dots, NRHP) \quad (41)$$

$$ADRSP = \sum_{i=1}^{NRHP} DRSP_i \quad (42)$$

$$SQPKD = \sum_{i=1}^{NRHP} (SHPF_i - RHPF_i)^2 \quad (43)$$

where SHPF and RHPF are the synthesized and recorded hydrograph peaks respectively.

When SQPKD is plotted against the first trial value of FSRX (Point A in Fig. 8), the point usually has a rather high value of SQPKD. The next point (Point B in Fig. 8) is obtained as the trial value of FSRX is set equal to the previous value plus BISRX. If SQPKD becomes smaller, the new values of SQPKD and FSRX are saved before going on to try a larger FSRX. The process continues until 1) the new value of SQPKD exceeds the previous one (Point E in Fig. 8), or 2) the minimum value of SQPKD is found at the maximum value for FSRX of 0.99.

The first case indicates that the minimum point of SQPKD has been passed. The direction of search is reversed by determining the next value of FSRX by subtracting the small incremental SRX (SISRX) from the last value. The reverse adjustment of FSRX continues until a new value of SQPKD exceeds the last value, then the last value of FSRX is selected as its best estimate (Point H in Fig. 8).

The case where the smallest SQPKD is for an FSRX of 0.99 may be associated with an ADRSP either larger or smaller than zero. If ADRSP is negative, the search backtracks within the last big increment to find a minimum point in the manner described above. If ADRSP is positive, the synthesized flood peaks are too big but further reduction by increasing FSRX is not practical. In order to make the synthesized peaks smaller, the curve in Fig. 8 is moved to the left by reducing CHCAP to 80 percent of its previous value. Smaller peaks are simulated because a higher value of SRX will now be used for any given flow. The estimate provided in the data is assumed to be in error. With the new values of CSRX (estimated by substituting the new CHCAP in Equation 39) and CHCAP, the procedure of Fig. 8 is again used to estimate FSRX. However, to prevent the program from hanging in a loop, the number of adjustments to CHCAP is limited to four. At this point, OPSET sets the values of both CSRX and FSRX to their upper limit of 0.99 to reduce synthesized flood peaks in a situation where a better estimate is taking too much computer time. A review of the estimated CHCAP as an index of the flow at which flooding begins is recommended. The user may wish to rerun the program starting with a lower estimate.

The two cases in which the above method cannot be applied must be considered separately. They are:

1. Less than two hydrographs may be accepted by Subroutine SETHRP. Subroutine SETSRP is not called and OPSET terminates if no hydrographs are accepted. The user should try another run with better data. Since a slope cannot be estimated from one point, both CSRX and FSRX are set equal to SRX when just one hydrograph is accepted.

2. The estimate of  $b$  in Equation 40 may be negative. Since such a value does not make physical sense, a single estimate is made for both parameters. A mean weighted by the recorded hydrograph peaks is used because the arithmetic mean may be too biased by SRX values arising from a hydrograph with a small magnitude. Small hydrograph peaks are more subject to anomalies between recorded flood peaks and recorded precipitation.

## CONCLUSION

A great many arbitrary sounding empirical rules have been presented in this chapter. Few can be quantitatively verified theoretically. Their merit can best be judged by the results they produce. Space does not permit extensive analysis of results in this report. Ross (25) discusses the results with respect to estimating parameter values by watershed. James (16, pp. 34-36) tabulates the success of estimated parameters values when used in the Model in matching simulated to recorded stream flow. The tables in the following chapter also provide information which can be used in evaluating the results.



## CHAPTER IV

### OPERATION AND APPLICATIONS

The use of OPSET to estimate a set of parameter values for a given watershed is enhanced by a well-planned strategy. Water years of record need to be selected to give the best results. A technique is needed for obtaining the best overall estimate from estimates from individual years. Options are provided in the program to meet the needs of individual users. These topics are discussed in this chapter.

#### OPERATION OF THE OPSET PROGRAM

Even though parameter estimation by OPSET is on a station-year basis, many station years can be handled in a single computer run. The variable NSYT specifies the number of station years included in a given computer run. Since each station year is an independent operational unit, the total number of station years will not affect the result for any station year.

For each station year, a title card for identification and the following six data and operation options are read as input data:

1. CONOPT(1) -- evapotranspiration data option. This is identical to CONOPT(3) in the KWM control options (25, p. 29). When read as 0, daily evaporation data (DPET) and monthly evaporation pan coefficient data (EPCM) are used and read in MAIN0098 and MAIN0103. When read as 1, then 10-day average evaporation data as well as the monthly evaporation pan coefficient data are used. When read as 2, then estimated potential annual evapotranspiration (EPAET) and mean annual number of days with more than

0.01 inch of rain (MNRD) are read and Subroutine EVPDAY is called to estimate daily evapotranspiration values (MAIN0108-0110). The user should choose his option according to the availability of evaporation data. More detailed descriptions are given by Ross (25, pp. 29-35) and in the discussion of Subroutine EVPDAY.

2. CONOPT(2) -- channel routing option. This is identical to CONOPT(12) in the KWM control options (25, p. 23). For larger watersheds, the channel routing in optimizing the hydrograph routing parameters is done hourly so as to save computer time. CONOPT(2) is read as 1, and the input time-area histogram is constructed on an hourly basis. For small watersheds, fifteen-minute routing periods are used in OPSET by setting Option 2 equal to 0, and the input time-area histogram is constructed in 15-minute time intervals.

3. CONOPT(3) -- rain gage moving option. This is identical to CONOPT(8) in the KWM control options. The option specifies whether some event occurred during the water year which would alter the procedure used in combining recording and storage gage amounts to estimate hourly rainfalls on the basin. If it has, CONOPT(3) should be read as 1, and the new storage gage weighting factor (WSG2), the new storage gage reading time (SGRT2), and the day of the change (SGMD) should be read (MAIN0153). If no such event occurred in that station-year, CONOPT(3) should be read as 0, and the other data are not needed. A more detailed description is given by Ross (25, pp. 25-29).

4. MNRC -- minimum number of Rough cycles. In the Rough adjustment of the six volume parameters in TRIP 1, a minimum number of cycles is preassigned to provide the user flexibility to make a more thorough search of parameter value combinations for use in cases where the estimating procedure is experiencing difficulty

in finding a path toward a reasonable solution. Unless the process first finds a set of parameters for which SSQM is less than 0.15 (rough looping cannot be more precise), the estimation requires at least MNRC times of Rough adjustment. Although the last few cycles sometimes get consistently worse, experience has shown that the process may reverse. The opportunity to increase MNRC provides the power to find the best possible combination of the six parameter values without being halted by difficulties in the initial trials which may start from a bad set of values. Smaller values of MNRC will save computer time where the estimation is quicker. A value of 12 was used in most of the studies reported by Ross (25, p. 87).

5. NFTR -- number of the first TRIP to be run for a given station-year of data. If NFTR is 1, the input data is as shown by Ross (25, Appendix B). Sometimes, the user may not want to start with TRIP 1. For example, if the parameter values estimated in TRIP 1 have been established in a previous computer run, and it is desired to repeat the optimization procedure for the channel routing parameters because the hydrograph data was punched incorrectly the first time, the program can be started with TRIP 2 by setting NFTR equal to 2 and adding eight parameter values on punched cards right after the end of hourly precipitation data (MAIN0227):

For the example of the best run of the Fine adjustment cycle for the 1956 Wood Creek data (25, pp. 82-87), the cards would be:

11.14 \* LZC, lower zone storage capacity  
4.23 \* BMIR, basic maximum infiltration rate within watershed  
0.65 \* SUZC, seasonal upper zone storage capacity factor  
0.15 \* ETLF, evapotranspiration loss factor  
1.04 \* BUZC, basic upper zone storage capacity factor  
0.45 \* SIAC, seasonal infiltration adjustment constant

0.00 \* BIVF, basic interflow volume factor

8.58 \* LZS, current lower zone storage.

If it is desired to run the program with the final trip alone, the value of NFTR should be set equal to 3 and six more parameter values should be placed right after the eight parameter values on punched cards as follows:

0.935 \* CSRX, channel storage routing index

0.935 \* FSRX, flood plain storage routing index

3 \* NCTRI, number of current time routing increments

100.0 \* CHCAP, channel capacity - indexed to basin outlet

0.100 \* IFRC, interflow recession constant

0.895 \* BFRC, base flow recession constant

6. NLTR -- number of the last trip to be run for a given station year. This value can be 1, 2, or 3 but must equal or exceed NFTR. The complete run of OPSET will have NFTR equal to 1 and NLTR equal to 3.

#### SELECTION OF WATER YEARS

Data from 20 watersheds in Kentucky were used in developing OPSET. Ross (25, pp. 58-78) describes the criteria followed in selecting the watersheds and gives detailed information on those watersheds selected. Seventeen of the 20 watersheds were classified as rural in that physical change within the watershed was judged not extensive enough to change the value of the parameters with time. The other three were classified as urbanizing as urban development has been changing the face of the watershed. From the published record for each rural watershed, three water years were chosen to represent a wide range of flow patterns. They were (1) the year with largest winter (December-May) flood, (2) the year with the largest summer (June-November) flood, and (3) the year with the

least summer runoff. For each urbanizing watershed, the first four and the last four years of record were obtained to represent both ends of the urbanizing experience.

#### AVERAGING METHOD STUDIES

OPSET estimates parameters from the data for a given water year. If the hydrologic simulation model were perfect, the parameters estimated directly from watershed characteristics were measured correctly, and the climatological data represented watershed experience, OPSET would estimate the same set of parameter values from any year of data (p. 76 ). The exception would be estimates for parameters which relate to processes that do not occur in a year of record. For example, the parameter for routing major floods cannot be estimated from data years when the flow does not leave the banks of the channel, and the parameters best estimated from their effect on summer flows are difficult to estimate from years with very little summer rainfall.

Because parameter estimates do vary from year to year, a suitable method for averaging the results is needed. The issues considered were:

1. Should one use an arithmetic or a geometric mean?
2. Should years with certain extreme flow characteristics be excluded from the averaging?
3. Should parameter estimates obtained from years when simulated flows closely match recorded flows be weighted more heavily than estimates from years when the matching is not so good?
4. How much is the estimate improved by including more years in the averaging?

These issues were resolved through a series of studies. It was recognized ahead of time that the best averaging procedure would

probably vary by parameter. It was also recognized that the available computer time would not permit exhaustive analysis. The studies would have to be exploratory and the conclusions tentative.

Questions 1 and 2: The first study used the Cave Creek watershed where a recording rain gage is located quite close to a small watershed. OPSET was applied to the data for each water year to estimate three sets of parameter values. From these estimates, four different mean values were calculated for each parameter (Table 19). Because of the parameters more sensitive to summer storms, if any year were to be excluded, a year with a dry summer seemed to be the most logical. In all, there were four different means, namely, the arithmetic mean of all three years ( $A_m$ ), the geometric mean of all three years ( $A_g$ ), the arithmetic mean of two wet years ( $A_{mw}$ ), and the geometric mean of two wet years ( $A_{gw}$ ).

In order to examine how parameter estimates based on one water year work on data for another water year, a series of runs were made. For each water year, the set of parameter values obtained from each OPSET run and the four sets of mean values were used to simulate an annual hydrograph (TRIP 3 only). The results of these three computer runs are shown on Tables 20, 21, and 22.

Inspection of the three tables reveals the following features which lend insight useful in choosing the best possible means of averaging parameter values:

1. The best set of parameter values (smallest SSQM) for a given water year is the one obtained from the OPSET run based on that water year. The estimating procedure is doing its job.
2. Averaged parameter values give better simulation results than a set of values based on any other water year (Table 21).
3. The averaged mean values based on all three years are superior to those based on two of the three years. Table 20 shows

TABLE 19  
PARAMETER VALUES USED FOR STUDYING AVERAGING  
METHODS: CAVE CREEK NEAR FORT SPRING, KY.

	1955 (W*)	1958 (S*)	1965 (D*)	Am*	Ag*	Amw*	Agw*
LZC	5.77	1.86	1.37	3.00	2.45	3.81	3.27
BMIR	3.92	4.66	6.06	4.88	4.80	4.29	4.27
SUZC	0.77	0.25	0.94	0.65	0.57	0.51	0.44
ETLF	0.21	0.25	0.17	0.21	0.21	0.23	0.23
BUZC	3.02	1.46	0.65	1.71	1.42	2.24	2.10
SIAC	0.26	0.09	0.40	0.25	0.21	0.18	0.15
BIVF	0.00	0.00	0.00	0.00	0.00	0.00	0.00
BFRC	0.877	0.836	0.877	0.863	0.863	0.857	0.856
IFRC	0.100	0.255	0.128	0.161	0.148	0.178	0.160
CSRX	0.970	0.970	0.984	0.975	0.975	0.970	0.970
FSRX	0.977	0.994	0.984	0.985	0.985	0.986	0.985
NCTRI	2	3	6	4	3	3	2
LZS	3.33	0.95	0.04	1.44	0.50	2.14	1.78

\*W = Winter flood year values

S = Summer flood year values

D = Driest year values

Am =  $(W + S + D)/3$

Ag =  $(W \cdot S \cdot D)^{1/3}$

Amw =  $(W + S)/2$

Agw =  $(W \cdot S)^{1/2}$

TABLE 20

AVERAGING METHOD STUDIES: CAVE CREEK 1955  
THE WINTER FLOOD YEAR (W)

	Recorded Flows (sfd)	Synthesized Flows (monthly deviation indices)						
		W	S	D	Am	Ag	Amw	Agw
Monthly flows (sfd)								
Nov.	3.1	-0.127	-0.090	-0.095	-0.127	-0.138	-0.121	-0.127
Dec.	20.9	-0.033	1.076	1.355	0.406	0.499	0.244	0.389
Jan.	50.7	-0.063	0.420	0.562	0.255	0.293	0.149	0.202
Feb.	320.9	-0.198	0.089	0.101	-0.021	0.016	-0.075	-0.033
Mar.	401.5	0.076	0.176	0.177	0.145	0.156	1.128	1.140
Apr.	40.6	-0.170	-0.015	-0.287	-0.152	-0.098	-0.092	-0.034
May	136.6	0.148	0.625	-0.003	0.246	0.326	0.411	0.471
Jun.	37.9	0.178	0.465	0.159	0.306	0.358	0.383	0.421
Jul.	16.3	0.033	0.113	-0.418	-0.093	-0.103	0.061	0.058
Aug.	19.4	-0.190	0.376	-0.589	-0.271	-0.235	-0.139	-0.091
Sep.	1.7	-0.043	-0.059	-0.074	-0.069	-0.069	-0.059	-0.059
SSQM		0.187	2.146	2.837	0.531	0.692	1.724	1.924
Annual total (sfd)	1051	1037	1369	1222	1172	1214	1176	1220
Flood peaks (cfs)								
Feb. 5	46.0	39.7	50.9	43.7	43.2	44.9	42.3	45.8
Mar. 21	60.0	118.9	102.2	98.4	106.6	107.7	109.0	111.0
May 13	117.0	197.1	138.3	106.5	163.4	170.5	181.7	190.7
May 21	52.0	26.1	71.0	-*	20.7	28.1	40.7	49.9

\* Synthesized flow less than the preassigned minimum value (MINH) which is 10 cfs for this station year.



TABLE 21

AVERAGING METHOD STUDIES: CAVE CREEK 1958  
THE SUMMER FLOOD YEAR (S)

	Recorded Flows (sfd)	Synthesized Flows (monthly deviation indices)						
		W	S	D	Am	Ag	Amw	Agw
Monthly flows								
	(sfd)							
Nov.	72.0	-0.064	0.932	0.712	0.358	0.389	0.287	0.418
Dec.	293.9	-0.366	-0.107	-0.025	-0.187	-0.169	-0.254	-0.226
Jan.	136.1	-0.130	0.001	0.015	-0.066	-0.049	-0.102	-0.082
Feb.	150.5	-0.464	-0.306	-0.258	-0.327	-0.306	-0.377	-0.351
Mar.	79.8	-0.672	-0.363	-0.465	-0.446	-0.420	-0.483	-0.455
Apr.	160.0	-0.326	-0.036	-0.250	-0.180	-0.121	-0.141	-0.108
May	160.8	0.341	0.569	0.380	0.433	0.467	0.462	0.488
Jun.	12.6	-0.342	-0.523	-0.462	-0.424	-0.455	-0.387	-0.399
Jul.	250.7	-0.813	-0.238	-1.286	-0.859	-0.759	-0.644	-0.587
Aug.	115.4	-1.163	-0.643	-0.948	-1.106	-1.021	-1.096	-0.991
Sep.	7.9	-0.177	-0.208	-0.280	-0.229	-0.224	-0.203	-0.203
SSQM		3.206	2.217	3.842	2.886	2.570	2.573	2.339
Annual total	1442	1051	1411	1233	1183	1219	1183	1225
	(sfd)							
Flood peaks								
	(cfs)							
Nov. 18	44.0	47.2	73.3	65.3	65.7	70.5	72.6	82.4
Dec. 7	53.0	50.6	58.2	46.3	51.3	52.3	56.0	57.1
Dec. 26	43.0	95.7	70.1	70.3	85.5	86.8	92.3	94.0
Jul. 24	87.0	128.3	89.0	64.2	106.6	111.9	121.2	127.1

TABLE 22  
 AVERAGING METHOD STUDIES: CAVE CREEK 1965  
 THE DRIEST YEAR (D)

	Recorded Flows (sfd)	Synthesized Flows (monthly deviation indices)						
		W	S	D	Am	Ag	Amw	Agw
Monthly flows								
	(sfd)							
Nov.	4.8	-0.210	-0.143	-0.122	-0.204	-0.204	-0.210	-0.210
Dec.	236.1	-0.781	0.093	0.166	-0.196	-0.095	-0.350	-0.234
Jan.	187.5	-0.601	-0.334	-0.244	-0.409	-0.363	-0.501	-0.451
Feb.	156.4	-0.595	-0.288	-0.251	-0.387	-0.348	-0.453	-0.409
Mar.	305.7	-0.241	-0.081	-0.096	-0.127	-0.109	-0.150	-0.131
Apr.	99.1	0.008	0.188	0.022	0.081	0.128	0.113	0.149
May	21.4	-0.266	-0.251	-0.310	-0.314	-0.310	-0.318	-0.298
Jun.	5.2	-0.235	-0.254	-0.248	-0.241	-0.248	-0.241	-0.241
Jul.	3.2	-0.069	-0.126	-0.149	-0.126	-0.132	-0.105	-0.110
Aug.	0.6	-0.025	-0.030	-0.030	-0.030	-0.030	-0.030	-0.030
Sep.	1.3	-0.060	-0.060	-0.065	-0.065	-0.065	-0.060	-0.065
SSQM		1.562	0.413	0.359	0.597	0.512	0.833	0.673
Annual total	1032	701	945	958	836	874	794	830
	(sfd)							
Flood peaks								
	(cfs)							
Dec. 4	18.0	13.0	96.4	66.6	39.1	56.6	29.1	42.6
Dec. 12	33.5	19.6	46.5	38.5	31.8	36.4	30.8	35.6
Dec. 26	16.5	14.8	20.3	22.4	17.8	18.7	16.9	18.0
Mar. 17	14.0	81.1	80.4	52.9	71.6	72.5	81.3	82.2
Mar. 26	40.0	94.6	92.3	74.9	88.1	88.6	93.2	93.8

how bringing the dry year into the average even improves the simulation for the wet years.

4. The results do not give much grounds for choosing the geometric mean.

Two conclusions came from this study. The first was to prefer the arithmetic mean over the geometric mean because it gives equally good results with less computational effort. The exception was for SIAC where use of the geometric mean seemed intuitively more reasonable because that parameter is an exponent in the modeling equations (MAIN0308, 0551). The second was to include years in the averaging regardless of their flow patterns.

Question 3: The second study examined the merit of weighting the averaged flow volume parameters on SSQM based on the hypothesis that the ability to better match recorded flows is an index of a better estimate. Three types of weighting were tried. Type A takes the arithmetic mean of the parameter values obtained from the various years. That is

$$\bar{X}_A = \frac{\sum_{i=1}^n X_i}{n} \quad (44)$$

where the  $X_i$  are the parameter values obtained from  $n$  single year OPSET runs. Type B weights the parameter values on the reciprocal of SSQM. The mathematical expression is

$$\bar{X}_B = \frac{\sum_{i=1}^n \left(\frac{1}{SSQM_i}\right) X_i}{\sum_{i=1}^n \left(\frac{1}{SSQM_i}\right)} \quad (45)$$

where  $SSQM_i$  is the value of SSQM for the year. Type C weights the parameter values on the portion of the grand total SSQM not pertaining to the year. The expression is

$$\bar{X}_C = \frac{\sum_{i=1}^n \left[ \left( \sum_{i=1}^n SSQM_i \right) - SSQM_i \right] X_i}{(n-1) \sum_{i=1}^n SSQM_i} \quad (46)$$

In all three averaging methods, the logarithms of SIAC were used in keeping with the decision to use its geometric mean. For this study, the parameters other than the land phase parameters were averaged in manners independently developed from reviews of the estimating procedures and presented later.

To test the three types of weighting, three watersheds whose values of SSQM differed substantially from one year to another were selected. Weighting would have no effect if all values of SSQM were equal. The values based on individual years and the averages according to each of the three equations are tabulated in Table 23.

For each of the nine station years, the three types of averaged parameter values were used to run TRIP 3 of OPSET. The monthly flow deviations for each station year are summarized on Table 24. The evidence is not very conclusive for selecting a type of averaging. Type B averaging did the relatively best job for the most months, but it also did the relatively worst job most often. Type A averaging also ranged from one extreme to the other. Type C averaging usually produced a result that was not the best matching or the worst either. The verdict was to recommend unweighted averaging on the grounds that it gave equally good results from much less effort.

TABLE 23  
PARAMETER VALUES WITH DIFFERENT TYPES  
OF AVERAGING

	Water Year			Type of Averaging		
	1956	1957	1961	A	B	C
West Bays Fork near Scottsville, Kentucky						
LZC	8.88	9.96	17.13	11.99	15.69	13.01
BMIR	7.75	5.28	4.96	6.00	5.32	5.96
SUZC	0.40	0.83	0.30	0.51	0.34	0.43
ETLF	0.18	0.14	0.28	0.20	0.26	0.22
BUZC	1.09	0.20	0.79	0.69	0.79	0.78
SIAC	0.00	0.00	0.85	0.07	0.42	0.12
BIVF	0.40	0.40	0.80	0.439	0.727	0.590
BFRC	0.870	0.888	0.939	0.905	0.905	0.905
IFRC	0.234	0.117	0.516	0.332	0.332	0.332
NCTRI	4	4	4	4	4	4
CSRX	0.900	0.880	0.900	0.893	0.893	0.893
FSRX	0.900	0.990	0.900	0.930	0.930	0.930
CHCAP	750	750	750	750	750	750
LZS	5.05	3.51	10.90	--	--	--
SSQM	3.783	7.362	0.560			
McDougal Creek near Hodgenville, Kentucky						
	1954	1958	1966	A	B	C
LZC	9.16	2.18	2.55	4.63	7.67	5.53
BMIR	8.03	2.59	4.20	4.94	6.97	5.14
SUZC	0.20	0.19	0.65	0.35	0.23	0.29
ETLF	0.21	0.16	0.10	0.16	0.19	0.17
BUZC	0.27	0.93	0.23	0.48	0.36	0.48
SIAC	0.02	0.57	0.61	0.19	0.04	0.12
BIVF	0.00	0.00	0.00	0.00	0.00	0.00
BFRC	0.882	0.901	0.923	0.908	0.908	0.908
IFRC	0.100	0.100	0.100	0.100	0.100	0.100
NCTRI	5	5	5	5	5	5
CSRX	0.930	0.925	0.914	0.920	0.920	0.920
FSRX	0.930	0.925	0.914	0.921	0.921	0.921
CHCAP	440	440	440	440	440	440
LZS	4.76	1.77	1.98	--	--	--
SSQM	0.277	1.544	2.802			

TABLE 23 (cont'd.)

Elkhorn Creek near Frankfort, Kentucky						
	1948	1960	1964	A	B	C
LZC	2.64	2.00	1.46	2.03	2.32	2.17
BMIR	10.63	12.24	20.00	14.29	12.28	13.15
SUZC	0.92	0.30	1.30	0.84	0.81	0.78
ETLF	0.10	0.60	0.34	0.35	0.26	0.33
BUZC	2.50	0.20	0.78	1.16	1.75	1.33
SIAC	0.02	4.00	1.29	0.10	0.12	0.31
BIVF	6.48	2.66	1.03	3.39	4.76	3.99
BFRC	0.880	0.864	0.894	0.877	0.877	0.877
IFRC	0.430	0.336	0.442	0.403	0.403	0.403
NCTRI	19	15	17	17	17	17
CSRX	0.874	0.941	0.983	0.927	0.927	0.927
FSRX	0.874	0.941	0.983	0.927	0.927	0.927
CHCAP	1000	1000	1000	1000	1000	1000
LZS	0.97	0.54	0.33	--	--	--
SSQM	1.479	3.407	6.878			

TABLE 24(a)

DEVIATIONS OF MONTHLY SIMULATED FLOWS  
WITH DIFFERENT TYPES OF AVERAGING

Watershed West Bays Fork at Scottsville, Kentucky									
Water Year	1956			1957			1961		
Type	A	B	C	A	B	C	A	B	C
Nov.	0.03	0.22	0.09	-0.27	-0.16	-0.21	-0.40	-0.60	-0.80
Dec.	0.25	0.33	0.27	-0.18	0.11	-0.14	-0.12	0.02	-0.54
Jan.	-0.92	0.04	-0.73	-0.85	-0.57	-0.78	-0.21	0.29	-0.59
Feb.	-0.22	-0.23	0.21	0.26	0.13	0.24	-0.88	-0.50	-1.22
Mar.	0.02	-0.11	0.01	0.27	0.13	0.23	-0.07	0.12	-0.16
Apr.	0.06	0.01	0.05	0.14	0.04	0.12	-0.01	0.06	-0.04
May	-0.08	-0.01	0.03	0.34	0.40	0.40	-0.04	0.10	0.01
Jun.	-1.31	-1.06	-1.16	0.17	0.13	0.15	0.21	0.24	0.22
Jul.	0.84	1.08	1.03	-1.79	-1.58	-1.70	0.14	0.18	0.16
Aug.	0.09	0.20	0.15	0.79	-0.76	-0.78	-0.33	-0.36	-0.36
Sep.	-0.25	-0.20	0.21	0.73	0.61	0.67	-0.69	0.69	-0.69
Watershed McDougal Creek near Hodgenville, Kentucky									
Water Year	1954			1958			1966		
Nov.	-0.27	-0.03	-0.15	-0.10	-0.27	-0.13	-0.82	-0.65	-0.74
Dec.	0.31	0.35	0.35	-0.19	-0.31	-0.24	-0.57	-0.43	-0.53
Jan.	0.57	0.07	0.41	-0.30	-0.54	-0.37	-0.21	-0.43	-0.28
Feb.	0.75	0.33	0.63	-0.26	-0.20	-0.25	-0.54	-0.81	-0.64
Mar.	0.28	0.29	0.29	-0.42	-0.52	-0.47	-0.20	-0.32	-0.22
Apr.	0.11	-0.09	0.05	-0.86	-1.02	-0.89	-0.08	-0.32	-0.13
May	0.19	0.20	0.28	-0.17	-0.18	-0.16	-0.09	-0.05	-0.06
Jun.	-0.16	0.03	-0.05	0.77	0.93	0.87	-0.22	-0.00	-0.14
Jul.	-0.10	-0.06	-0.09	0.27	0.32	0.29	-0.21	-0.03	-0.09
Aug.	-0.62	-0.26	-0.51	-0.94	-0.64	-0.80	1.04	1.08	1.08
Sep.	-0.11	0.10	-0.02	0.38	0.67	0.51	0.40	0.60	0.41

TABLE 24(a) (cont'd.)

Watershed Elkhorn Creek near Frankfort, Kentucky									
Water Year	1948			1960			1964		
Type	A	B	C	A	B	C	A	B	C
Nov.	0.44	0.32	0.41	-0.06	-0.08	-0.06	-0.67	-0.68	-0.68
Dec.	0.02	-0.03	-0.00	0.00	-0.02	-0.01	-1.63	-2.16	-1.85
Jan.	-0.07	-0.10	-0.08	-0.21	-0.22	-0.21	-0.95	-1.38	-1.12
Feb.	-0.85	-0.86	-0.81	-0.16	-0.17	-0.16	-0.14	-0.20	-0.18
Mar.	0.06	0.06	0.07	-0.95	-0.98	-1.00	-0.10	-0.12	-0.10
Apr.	-0.19	-0.19	-0.19	-2.64	-2.59	-2.46	0.50	0.48	0.49
May	0.05	0.05	0.06	0.21	0.41	0.34	-0.04	-0.05	-0.06
Jun.	-1.28	-1.20	-1.28	-0.19	-0.08	-0.14	-0.68	-0.24	-0.55
Jul.	-2.18	-2.08	-2.16	-0.03	-0.04	-0.03	-1.39	-1.14	-1.31
Aug.	0.13	-0.12	-0.13	-1.39	-0.85	-1.12	-0.17	-0.17	-0.17
Sep.	0.42	-0.42	-0.42	-1.16	-0.84	-1.00	1.24	0.95	1.20

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TABLE 24(b)  
SUMMARY STATISTICS

No. of monthly Flow simulations	Type of Average	A	B	C
	Best	41	44	14
	Second Best	16	12	71
	Worst	42	43	14



## AVERAGING RECOMMENDATIONS BY PARAMETER

From the previously described studies and examination of the OPSET estimating programing by parameter, the following procedures are recommended for averaging parameter estimates obtained year by year from OPSET.

1. LZC: Take the arithmetic mean of the annual estimates.
2. BMIR: Take the arithmetic mean of the annual estimates.
3. SUZC: Take the arithmetic mean of the annual estimates.
4. ETLF: Take the arithmetic mean of the annual estimates.
5. BUZC: Take the arithmetic mean of the annual estimates.
6. SIAC: Take the geometric mean of the annual estimates.

In taking the mean, use a value of 0.02 for years where a smaller value was estimated. This rule prevents one zero estimate from causing a zero geometric mean and follows the rules used in estimating SIAC within the program (STFV0185).

7. BIVF: Hinge the estimate on the estimated value of IFRC. If the estimate of IFRC is less than 0.3, take BIVF as 0.0. This is the rule used by OPSET in making an estimate for an individual year. If the estimate exceeds 0.3, take the arithmetic mean of the annual estimates; however, use 0.40 as the annual estimate for years when the OPSET estimate is less than that value (p.116).

8. BFRC: Weight the mean of the annual estimates on the number of days OPSET used within the year to make an estimate. The number of days (ABFSL) is printed out by the program. The weighting formula

$$\overline{\text{BFRC}} = \frac{\sum_{i=1}^n (\text{BFRC}_i \cdot \text{ABFSL}_i)}{\sum_{i=1}^n \text{ABFSL}_i} \quad (47)$$

extends the type of weighting used within the year to weight estimates from the various recession sequences to a multiyear context.

9. IFRC: Take the mean of the annual estimates weighted by the same procedure used for BFRC with the difference that the weighting factor is AIFSL, also printed in the OPSET output. The two weighting factors vary because the program may not detect interflow in some recession sequences and uses a cutoff to exclude tails of very long sequences in the interflow weighting (pp. 68-69). The raw estimate of IFRC should be used for each year. This value is printed before noting a change to a value of 0.1 where the raw value was smaller than 0.3. If  $\overline{\text{IFRC}}$  is less than 0.3, it should be taken as 0.1.

10. NCTRI: Take the mean of the annual estimates weighted on the number of read historical hydrograph peaks accepted by Subroutine SETHRP and round to the nearest integer. The number can be counted from the output data. This procedure is another extension of the method used by OPSET for averaging within the year. Once an estimate of NCTRI has been accepted, it is also necessary to fill the array of time-area increments with the specified number of values. The user has his initial array, and OPSET prints an array corresponding to the best estimate of NCTRI for each year. Usually, one of these arrays will have a correct number of elements. If none do, the user can return to his watershed map and form a new array.

11. CSRX: Take the mean of the annual estimates weighted on the number of read historical hydrograph peaks accepted by Subroutine SETHRP.

12. CHCAP: Take the median of the annual estimates. This prevents extreme estimates caused by timing discrepancies between recorded precipitation and recorded streamflow from causing problems.

13. FSRX: Take the mean of the annual estimates weighted on the number of read historical hydrograph peaks accepted by Sub-routine SETHRP and whose simulated peak exceeds CHCAP as estimated above. This eliminates the impact of smaller events on a parameter which does not pertain to them.

#### A CLOSER LOOK AT MODELING BASE FLOW RECESSION

In the KWM as in the Stanford Watershed Model, base flow is modeled by two parameters, BFRC and BFNL. In OPSET, only BFRC is used, and only BFRC is estimated. The second parameter, BFNL, provides a recession rate which is more rapid at first and gradually becomes slower as flows diminish. The question to be considered here is the value of OPSET in estimating BFNL.

One way to test whether a watershed has nonlinear baseflow recession is to plot the least squares estimate of sequence recession rate against the initial base flow in the sequence. These values are printed in the output from OPSET. Values for the Helton Branch watershed, for which the OPSET estimate of BFRC was 0.915, are plotted on Figure 9. If the points are randomly scattered around a horizontal line estimated by the method recommended above, BFRC should be taken as estimated and BFNL should be taken as 1.00. If a distinct trend toward lower points is noted as one moves toward the right on the plot, BFRC should be increased and BFNL should be reduced. While Figure 9 exhibits some downward trend, it is not strong enough to warrant this modification.

The same type plot for McDougal Creek follows on Figure 10. Here, the trend is distinctly downward to the right. A closer inspection showed the scatter to be reasonably homogeneous around the best estimate for flows less than about 10 cfs but for values of BFRC to be significantly smaller for recessions beginning with

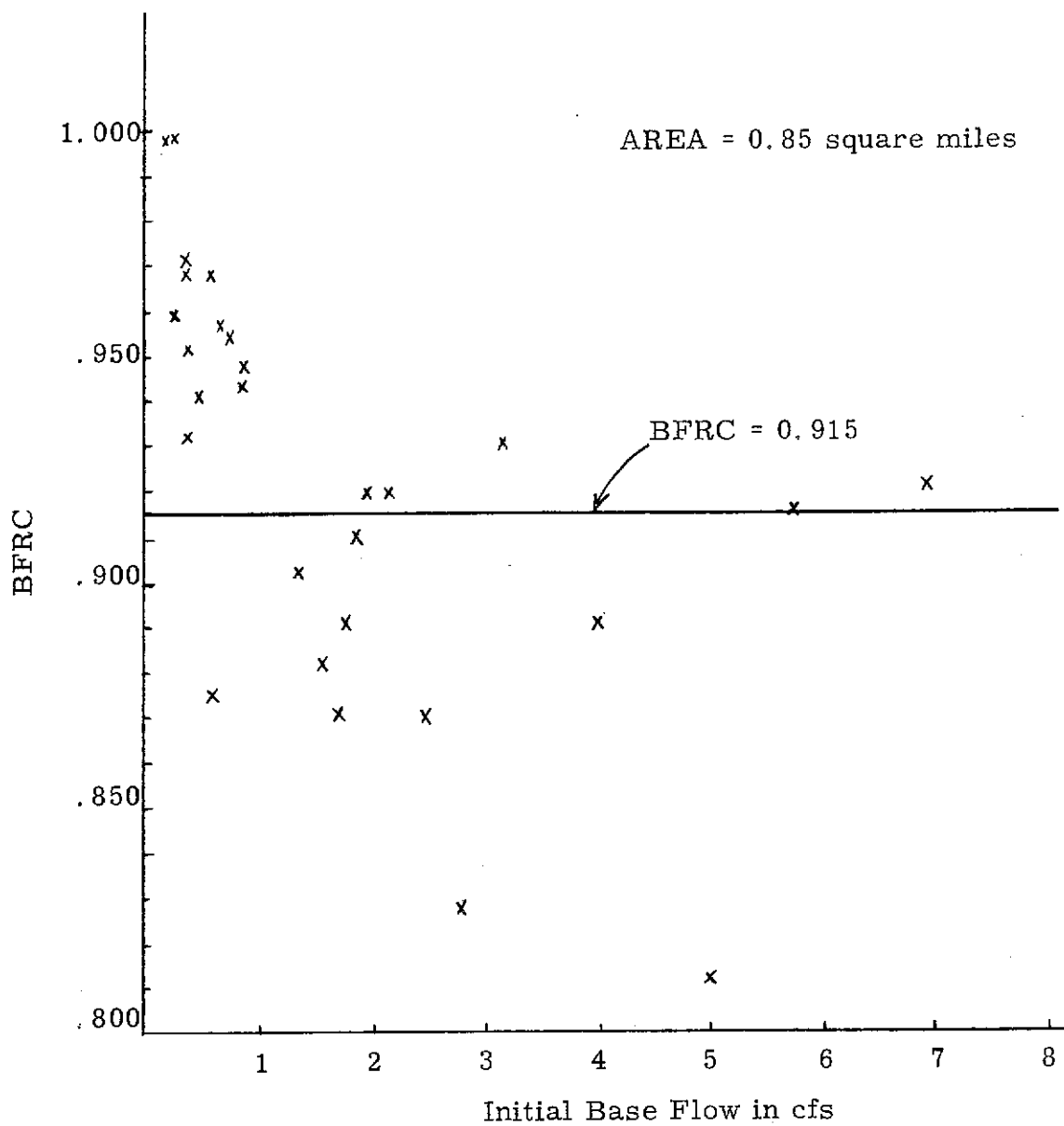


FIG. 9. Relationship Between BFRC and Initial Base Flow, Helton Branch Watershed.

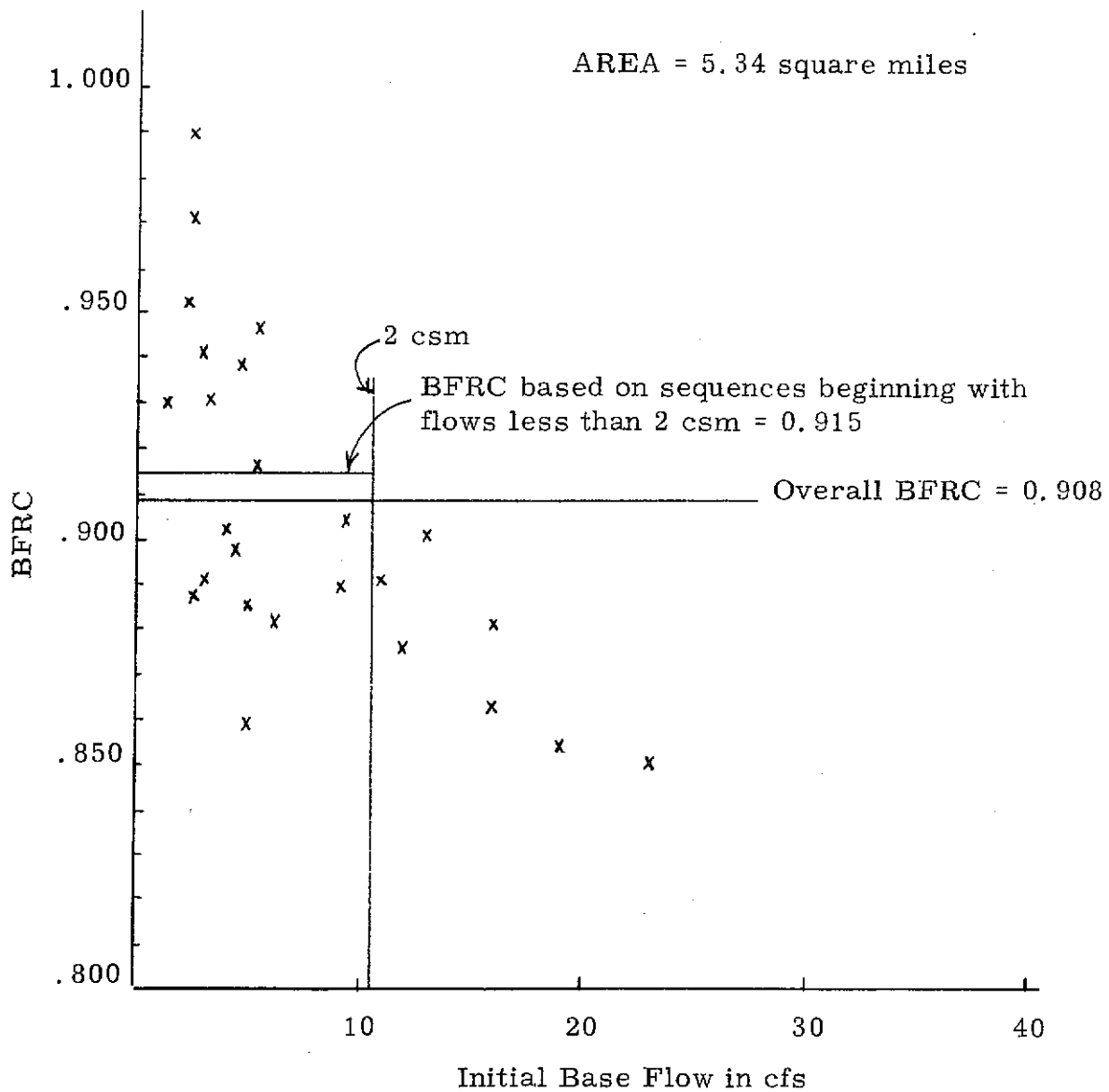


FIG. 10. Relationship Between BFRC and Initial Base Flow, McDougal Creek Watershed.

larger base flows. Using a division line at 10.68 cfs (2.0 csm), BFRC from points to the left is estimated as 0.915 by applying Equation 47 to all the relevant recession sequences. In cases like this, it is recommended that a dividing line be selected and BFRC be estimated in this manner. A good first estimate for BFNLRL is 0.97. The value may be varied by trial-and-error as desired, but the modeling is not too sensitive.

#### NUMBER OF WATER YEARS TO USE IN AVERAGING

Intuitively, one would expect the estimates of parameter values to improve as more years are used. However, the computer cost is directly proportional to the number of years. The purpose of this section is to use the Cave Creek example as a case study for developing some feel for how much the results improve as more years are used.

First, nine water-year-based parameter values were estimated by running TRIP 1 for each of six water years. Then, the arithmetic mean for each parameter and LZS was calculated for three-year, four-year, five-year, and six-year periods. The calculated mean parameter values are shown in Table 25. The channel routing parameters were held at the three-year means since the other three years were run on TRIP 1 alone.

Using the 14 values shown on Table 25, the six water years were run on TRIP 3 of OPSET alone, and Table 26 shows the results. This table shows the degree to which the use of more water years yields better estimates of the annual volume as well as the seasonal distribution (SSQM was used as an index). On the other hand, the highest flood peak seemed to be getting worse. This is probably caused by using the M3 values of the four hydrograph routing parameters for M4, M5, and M6.

TABLE 25  
PARAMETER VALUES WITH DIFFERENT NUMBERS OF YEARS  
AVERAGED: CAVE CREEK WATERSHED

	Water Year						Averaged Values			
	1955	1958	1965	1964	1967	1968	M3*	M4*	M5*	M6*
LZC	6.73	2.24	1.89	1.08	3.67	1.91	3.62	2.99	3.12	2.92
BMIR	5.87	13.88	20.86	7.70	18.05	10.71	13.54	12.08	13.27	12.85
SUZC	0.90	0.20	0.49	1.09	0.63	0.21	0.53	0.67	0.66	0.59
ETLF	0.23	0.21	0.14	0.05	0.14	0.15	0.19	0.16	0.15	0.15
BUZC	2.18	1.88	0.38	0.68	0.31	0.59	1.48	1.28	1.09	1.00
SIAC	0.54	0.08	0.77	0.62	2.30	0.09	0.32	0.38	0.54	0.40
BIVF	0.00	0.00	0.00	0.00	0.00	0.83	0.00	0.00	0.00	0.00
BFRC	0.877	0.836	0.877	0.895	0.876	0.890	0.863	0.871	0.872	0.875
IFRC	0.100	0.100	0.100	0.100	0.100	0.413	0.100	0.100	0.100	0.100
NCTRI	2	4	5	--	--	--	4	4	4	4
CSRX	0.97	0.94	0.98	--	--	--	0.96	0.96	0.96	0.96
FSRX	0.98	0.99	0.98	--	--	--	0.98	0.98	0.98	0.98
CHCAP	50	16	50	--	--	--	39	39	39	39
LZS	4.64	1.46	0.27	0.25	3.12	0.96	2.12	1.66	1.95	1.78

\* M3 = (1955 + 1958 + 1965)/3

M4 = (1955 + 1958 + 1965 + 1964)/4

M5 = (1955 + 1958 + 1965 + 1964 + 1967)/5

M6 = (1955 + 1958 + 1965 + 1964 + 1967 + 1968)/6

TABLE 26

SUMMARY OF RESULTS WITH DIFFERENT  
NUMBERS OF YEARS AVERAGED:  
CAVE CREEK WATERSHED

Water Year	Recorded Flows	Synthesized Flows				
		OPSET	M3	M4	M5	M6
1955 (F)						
Annual (sfd)	1051	1010	1204	1215	1229	1257
SSQM		0.192	0.940	0.803	0.951	1.221
Largest Peak (cfs)	117	164	182	182	180	183
1958 (W)						
Annual (sfd)	1442	1415	1195	1213	1225	1253
SSQM		1.008	1.804	1.950	1.906	1.673
Largest Peak (cfs)	87	82	133	137	135	136
1965 (D)						
Annual (sfd)	1032	960	802	847	849	862
SSQM		0.237	0.894	0.581	0.553	0.555
Largest Peak (cfs)	40	80	74	87	90	87
1964						
Annual (sfd)	788	863	684	719	725	739
SSQM		0.186	2.112	1.322	1.184	1.179
Largest Peak (cfs)	132	152	152	170	174	169
1967						
Annual (sfd)	1226	1173	1008	1027	1038	1060
SSQM		1.447	2.859	2.274	1.952	2.026
Largest Peak (cfs)	135	174	141	140	139	143
1968						
Annual (sfd)	1105	1156	927	946	957	978
SSQM		1.661	5.757	4.176	5.464	3.295
Largest Peak (cfs)	49	63	75	82	89	83
Summary						
Annual $\Sigma$ (Rec.-Sim.)			824	677	621	495
$\Sigma$ SSQM			14.106	11.106	12.010	9.949
Flood Peaks $\Sigma$ (Rec.-Sim.)			-177	-238	-247	-251



More years give better estimates of parameter values, but economy should be taken into consideration. The cost of collecting additional years of data plus the computer time might not make it worthwhile to increase the number of water years. The study verified that the three years of record selected by Ross (25, p. 62) should give fair estimates, although the user might want to use more water years to obtain a closer estimate. Ross found this greater precision helped in studying the effects of urban change on parameter estimates.

## CHAPTER V

### CONCLUSIONS AND RECOMMENDATIONS

Digital hydrologic simulation as exemplified by the Stanford Watershed Model has great potential. Its success at simulating historical stream flows from historical climatological records makes it possible for the planning and design of water resources systems to proceed from a much broader data base. The same ability can be applied to estimate flows for reservoir operation decisions during the time lag between rainfall and runoff. Model success at simulating stream flows with empirical equations representing hydrologic processes provides a challenge and a framework for new research to develop better equations. The ability to simulate changes in flow patterns from changes in parameter values is a powerful tool contributing to a better understanding of the interactions within the runoff process to the student and practitioner of hydrology alike.

Despite this great potential, a number of factors have deterred widespread use of the Stanford Watershed Model. Those most frequently mentioned include:

1. Programing in a little used computer language (6, p. 46).
2. Difficulty in obtaining access to a computer with enough core storage to make modeling feasible and to be able to handle the program without time-consuming adjustments.
3. Difficulty in understanding the program as complicated by its bulk and an unfamiliarity of many hydrologists with digital modeling processes.

4. Disagreements over the appropriateness of specific modeling equations.

5. The inability to directly relate key model parameter values with physical watershed characteristics (6, p. 46).

6. The time and acquired skill required to estimate values for these key parameters by trial and error, the lack of an explicit test for deciding on a best flow matching, and doubts about the Model as a whole caused by difficulties in matching certain flows.

This research has been directly addressed to these factors. A Fortran listing of the Model is provided. The subjective trial-and-error parameter estimating process is standardized. The ability to relate key model parameter values to physical watershed characteristics is improved. The gains are achieved at some sacrifice. The program is bigger and bulkier and hence more imposing to the uninitiated who try to follow its logic. The modeling equations are submerged more deeply into the inner workings of the program and are hence more difficult to change.

In applying OPSET to over 20 Kentucky watersheds which represent a wide range of topographic and soil conditions, it was found to do as good a job of simulating flows and a more consistent job of estimating parameter values than the past trial-and-error approach. Except for summer thunderstorm flood peaks, the simulation was even quite good for watersheds located at a distance from the nearest precipitation gage. The time spent calibrating watershed parameters is greatly reduced. The user does not have to spend so much time familiarizing himself with the program before he can properly use the Model. The program uses standardized criteria to test adequacy of matching and systematic optimization procedures which not only eliminate much of the subjectivity of

choosing which parameters to adjust and determining how large an adjustment to make but also obtain consistent convergence on reasonable parameter values.

The adjustment rules have been thoroughly tested on small watersheds in the climatological setting of Kentucky. However, the extent of the testing done in this research should not be taken as an excuse not to apply OPSET to larger watersheds or locations in another climatological setting. On the contrary, OPSET should be tested in those areas in order to refine and modify it for a wider range of applicability. Likewise, the Model itself needs periodic updating in order to take advantage of better empirical relationships or moisture accounting procedures. Significant changes to the Model will require changes to OPSET to better estimate parameter values in the revised context. The rules used in OPSET to estimate parameter values are not to be taken as the last word and should be modified whenever it would be helpful.

Changes to which special attention is needed include:

1. Expansion of OPSET to estimate parameter values for watersheds where appreciable runoff comes from snowmelt.
2. Checking and adjustment of OPSET to better handle climatological patterns remote from Kentucky.
3. Consideration of alternate equations or combination of equations within the Model and derivation of an objective approach for selecting among these.
4. Refinement of procedures for estimating BFNLR, GWETF, OFSL, and several other parameters which are still not handled by OPSET.
5. Adjustments for simulating flows from climatological data estimated on a different time grid and with a different level of precision.

6. Expansion of the Kentucky Watershed Model to incorporate the power of the Stanford Watershed Model to simulate runoff from watersheds divided into parts and adjustment of OPSET to handle cases where orographic rainfall patterns require simulation by parts.

## APPENDIX

APPENDIX A

LISTING OF KWM

C	KENTUCKY WATERSHED MODEL (VERSION OF JUNE 6, 1970)	MAIN0001
C	BASED ON STANFORD WATERSHED MODELS III & IV	MAIN0002
	DIMENSION BTRI(99), CONOPT(15), CRFMI(22), CTRI(99), DDIW(366),	MAIN0003
	1 DMNT(366), DMXT(366), DPSE(366), DRGPM(366), DRHP(366,24),	MAIN0004
	2 DRSGP(366), DPET(366), DRSF(366), DSSF(366), EDLZS(366),	MAIN0005
	3 EMBFNX(12), EMGWS(12), EMIFS(12), EMLZS(12), EMSIAM(12),	MAIN0006
	4 EMUZC(12), EMUZS(12), EPCM(12), FIRR(15), MEDCY(12), MEDWY(12),	MAIN0007
	5 RICY(37), SATRI(99), SERA(22), SERR(22), SESF(22), SQER(22),	MAIN0008
	6 THSF(24), TITLE(20), TMBF(12), TMFSIL(12), TMIF(12), TMNET(12),	MAIN0009
	7 TMOF(12), TMPET(12), TMPREC(12), TMRPM(12), TMRTF(12), TMSE(12),	MAIN0010
	8 TMSNE(12), TMSTF(12), TMSTFI(12), T20DFH(21), T20PRH(21),	MAIN0011
	9 UHFA(99), YTITLE(20)	MAIN0012
	LOGICAL LSHFT	MAIN0013
	INTEGER CDSDR, CN, CONOPT, DATE, DAY, DPY, EHS GD, HOUR, HRF, HRL, PDAY,	MAIN0014
	1 PRD, RHPD, RHPH, RSD, SGMD, SGRT, SGRT2, YEAR, YR1, YR2	MAIN0015
	REAL IFPRC, IFRC, IFRL, IFS, LZC, LZRX, LZS, LZSR, MHSM, MNRD, MRNSM, NHPT	MAIN0016
	DATA MEDCY/ 0, 31, 59, 90, 120, 151, 181, 212, 243, 273, 304, 334/	MAIN0017
	DATA MEDWY/ 304, 334, 365, 31, 59, 90, 120, 151, 181, 212, 243, 273 /	MAIN0018
100	CONTINUE	MAIN0019
	DO 101 KR D = 1, 14	MAIN0020
101	CALL READ(CONOPT(KRD))	MAIN0021
	DO 102 KIA = 1, 99	MAIN0022
	SATRI(KIA) = 0.0	MAIN0023
	CTRI(KIA) = 0.0	MAIN0024
	BTRI(KIA) = 0.0	MAIN0025
102	UHFA(KIA) = 0.0	MAIN0026
	CALL READ(NCTRI)	MAIN0027
	DO 103 KR D = 1, NCTRI	MAIN0028
103	CALL READ(CTRI(KRD))	MAIN0029

```

IF(CONOPT(7) .NE. 1) GO TO 110
DO 104 KRD = 1,15
104 CALL READ(FIRR(KRD))
DO 105 KRD = 1, 37
105 CALL READ(RICY(KRD))
DO 106 KRD = 274,360,10
106 CALL READ(DPSE(KRD))
DO 107 KRD = 1,273,10
107 CALL READ(DPSE(KRD))
DO 109 IDAY2 = 1, 9
DO 108 IDAY1 = 274,360,10
DAY = IDAY1 + IDAY2
108 DPSE(DAY) = DPSE(IDAY1)
DO 109 IDAY1 = 1,273,10
DAY = IDAY1 + IDAY2
IF(DAY .GT. 273) GO TO 109
DPSE(DAY) = DPSE(IDAY1)
109 CONTINUE
DPSE(366) = DPSE(59)
DPSE(365) = DPSE(363)
DPSE(364) = DPSE(363)
CALL READ(BDDFSM,SPBFLW,SPTWCC,SPM,ELDIF,XDNFS,FFOR,FFSI,MRNSM,
1 DSMGH,PXCSA)
110 CALL READ(RMPF)
CALL READ (RGPMB,AREA,FIMP,FWTR)
CALL READ (VINTMR,BUZO,SUZO,LZO,ETLF,SUBWF,GWETF,SIAC,BMIR,BIVE)
CALL READ (DFSS,DFSL,DFMN,DFMNIS,IFRC)
CALL READ (CSRX,FSRX,CHCAP,EXQPV,BFNLR,BFRC)
BFHRC = BFRC**{(1.0/24.0)}
BFRL = -ALOG(BFHRC)
BFNRL = 0.0
IF(BFNLR .LT. 0.00001 .OR. BFNLR .GT. 0.9999) GO TO 111
BFNHR = BFNLR**{(1.0/24.0)}
BFNRL = -ALOG(BFNHR)
111 IFPRC = IFRC**{(1.0/96.0)}
IFRL = -ALOG(IFPRC)

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MAIN0030
MAIN0031
MAIN0032
MAIN0033
MAIN0034
MAIN0035
MAIN0036
MAIN0037
MAIN0038
MAIN0039
MAIN0040
MAIN0041
MAIN0042
MAIN0043
MAIN0044
MAIN0045
MAIN0046
MAIN0047
MAIN0048
MAIN0049
MAIN0050
MAIN0051
MAIN0052
MAIN0053
MAIN0054
MAIN0055
MAIN0056
MAIN0057
MAIN0058
MAIN0059
MAIN0060
MAIN0061
MAIN0062
MAIN0063
MAIN0064
MAIN0065

```



CALL READ (GWS,UZS,EZS,BFNX,IFS)	MAIN0066
LSHFT = .FALSE.	MAIN0067
IF(CONOPT(13) .NE. 1) GO TO 113	MAIN0068
NBTRI = NCTRI	MAIN0069
FNTRI = NCTRI	MAIN0070
MXTRI = (10.0**EXQPV)*FNTRI + 0.5	MAIN0071
IF(MXTRI .GE. 98) WRITE(6,1)	MAIN0072
1 FORMAT(29HWARNING: EXQPV ARRAY OVER RUN)	MAIN0073
NCSTRI = 99	MAIN0074
DO 112 KIA = 1, NBTRI	MAIN0075
112 BTRI(KIA) = CTRI(KIA)	MAIN0076
TFCFS = 1.0	MAIN0077
CALL RTVARY (CTRI,SATRI,BTRI,CHCAP,NBTRI,MXTRI,NCSTRI,EXQPV,LSHFT,	MAIN0078
1 TFCFS)	MAIN0079
113 EPAET = 0.0	MAIN0080
FPER = 1.0 - FIMP - FWTR	MAIN0081
IF(FPER .GT. 0.01) GO TO 114	MAIN0082
TPLR = 100.0	MAIN0083
FPER = 0.01	MAIN0084
GO TO 115	MAIN0085
114 TPLR = (1.0 - FWTR)/FPER	MAIN0086
115 VINTCR = 0.25*VINTMR	MAIN0087
HSE = 0.0	MAIN0088
NRTRI = 0	MAIN0089
PEAI = 0.0	MAIN0090
SPIF = 0.0	MAIN0091
CBF = GWS*BFRL*(1.0 + BFNRL*BFNX)	MAIN0092
SPDR = 0.0	MAIN0093
OFUS = 0.0	MAIN0094
OFUSIS = 0.0	MAIN0095
OFR = 0.0	MAIN0096
OFRIS = 0.0	MAIN0097
PEIS = 0.0	MAIN0098
RHFO = 0.0	MAIN0099
URHF = 0.0	MAIN0100
AMIF = 0.0	MAIN0101

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```

AMNET = 0.0
AMPET = 0.0
AMSNE = 0.0
AMFSIL = 0.0
SASFX = 0.0
SARAX = 0.0
SRX = CSRX
VWIN = 26.8888*AREA
WCFS = 24.0*VWIN
RHFMC = 0.025/WCFS
TFCFS = CBF*WCFS
SSRT = SQRT(OFSS)
OFRF = 1020.0*SSRT/(OFMN*OFSL)
OFRFIS = 1020.0*SSRT/(OFMNIS*OFSL)
EQDF = 0.00982*((OFMN*OFSL/SSRT)**0.6)
EQDFIS = 0.00982*((OFMNIS*OFSL/SSRT)**0.6)
SOFRF = OFRF
SOFRFI = OFRFIS
SDEPTH = 0.0
ASM = 0.0
IF(CONOPT(7) .EQ. 0) GO TO 116
WT4AM = 60.0
WT4PM = 60.0
SAX = 15.0
TANSM = 0.0
SPTW = 0.0
STMD = 0.7
SFMD = 0.7
ASMRG = 0.0
116 READ(5,2) TITLE
2 FORMAT(20A4)
C BEGIN NEW YEAR
117 BYLZS = LZS
BYUZS = UZS
BYGWS = GWS
BYIFS = IFS

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MAIN0102
MAIN0103
MAIN0104
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MAIN0115
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MAIN0121
MAIN0122
MAIN0123
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MAIN0125
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MAIN0137

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DO 118 KIA = 1,22
  CRFMI(KIA) = 0.0
  SESF(KIA) = 0.0
  SERR(KIA) = 0.0
  SERA(KIA) = 0.0
118 SQER(KIA) = 0.0
  RGPM = RGPMB
  DO 119 KIA = 1,21
    T20DFH(KIA) = 0.0
119 T20PRH(KIA) = 0.0
  DO 120 KIA = 1,12
120 EPCM(KIA) = 1.0
  RDPT = 0.0
  PDAY = 274
  CALL READ (YR1,YR2)
  READ (5,2)YTITLE
  DPY = 365
  IF(MOD(YR2,4) .EQ. 0) DPY = 366
  IF(CONOPT(1) .EQ. 1) CALL READ(CDSDR,NDSDR)
  NSDSP = 0
  MEDWY(5) = 59
  IF(DPY .EQ. 366) MEDWY(5) = 366
C  READ EVAPORATION DATA
  IF(CONOPT(3) .NE. 1) GO TO 125
  DO 121 KRD = 274,360,10
121 CALL READ(DPET(KRD))
  DO 122 KRD = 1,273,10
122 CALL READ(DPET(KRD))
  DO 124 IDAY2 = 1,9
  DO 123 IDAY1 = 274,360,10
  DAY = IDAY1 + IDAY2
123 DPET(DAY) = DPET(IDAY1)
  DO 124 IDAY1 = 1,273,10
  DAY = IDAY1 + IDAY2
  IF(DAY .GT. 273) GO TO 124
  DPET(DAY) = DPET(IDAY1)

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MAIN0138
MAIN0139
MAIN0140
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MAIN0171
MAIN0172
MAIN0173

```

124 CONTINUE	MAIN0174
DPET(366) = DPET(59)	MAIN0175
DPET(365) = DPET(363)	MAIN0176
DPET(364) = DPET(363)	MAIN0177
GO TO 127	MAIN0178
125 IF(CONOPT(3) .EQ. 2) GO TO 130	MAIN0179
DAY = 274	MAIN0180
126 CALL READ(DPET(DAY))	MAIN0181
IF(DAY .EQ. 273) GO TO 127	MAIN0182
CALL DAYNXT(DAY, DPY)	MAIN0183
GO TO 126	MAIN0184
127 DO 128 MONTH = 1,12	MAIN0185
128 CALL READ(EPCM(MONTH))	MAIN0186
IF(EPAET .NE. 0.0) GO TO 133	MAIN0187
DO 129 DAY = 1,DPY	MAIN0188
129 EPAET = EPAET + DPET(DAY)	MAIN0189
IF(EPCM(6) .NE. 1.0) EPAET = 0.7*EPAET	MAIN0190
GO TO 131	MAIN0191
130 CALL READ(EPAET, MNRD)	MAIN0192
EMAET = EPAET*(365.0 + MNRD)/404.0	MAIN0193
CALL EVPDAY(DPET, EMAET)	MAIN0194
131 AETX = 24.0*EPAET/365.0	MAIN0195
AEX96 = 1.2*AETX	MAIN0196
AEX90 = 0.3*AETX	MAIN0197
SIAM = 1.2**SIAC	MAIN0198
UZC = SUZC*AEX90 + BUZC*EXP(-2.7*LZS/LZC)	MAIN0199
IF(UZC .LT. 0.25) UZC = 0.25	MAIN0200
SGRT = 0	MAIN0201
DO 132 DAY = 1,366	MAIN0202
DDIW(DAY) = 0.0	MAIN0203
DRSF(DAY) = 0.0	MAIN0204
DRGPM(DAY) = RGPMB	MAIN0205
DRSGP(DAY) = 0.0	MAIN0206
DO 132 HOUR = 1,24	MAIN0207
132 DRHP(DAY, HOUR) = 0.0	MAIN0208
133 IF(CONOPT(9) .NE. 1) GO TO 135	MAIN0209

```

DAY = 274
DRSF(366) = 0.0
134 CALL READ(DRSF(DAY))
CALL DAYNXT(DAY, DPY)
IF(DAY .NE. 274) GO TO 134
135 IF(CONOPT(11) .NE. 1) GO TO 137
DAY = 274
DDIW(366) = 0.0
136 CALL READ(DDIW(DAY))
CALL DAYNXT(DAY, DPY)
IF(DAY .NE. 274) GO TO 136
137 IF(CONOPT(7) .EQ. 0) GO TO 139
DAY = 274
138 CALL READ(DMXT(DAY), DMNT(DAY))
CALL DAYNXT(DAY, DPY)
IF(DAY .NE. 274) GO TO 138
139 CALL READ(NSGRD)
IF(NSGRD .EQ. 0) GO TO 141
CALL READ(WSG,SGRT)
IF(CONOPT( 8) .EQ. 1) CALL READ(WSG2,SGRT2,SGMD)
DO 140 KRD = 1,NSGRD
CALL READ(ISGRD)
140 CALL READ(DRSGP(ISGRD))
C READ RECORDING RAIN GAGE HOURLY TOTALS
141 CALL READ(IWBG, YEAR, MONTH, DATE, CN)
C PUNCH NO NUMBER AFTER CN ON YEAR .EQ. 98 CARD
IF(YEAR .GE. 98) GO TO 144
HRF = 12*(CN - 1) + 1
HRL = 12*(CN - 1) + 12
DAY = MECCY(MONTH) + DATE
DO 142 HOUR = HRF, HRL
142 CALL READ(DRHP(DAY, HOUR))
IF(DPY .NE. 366 .OR. MONTH .NE. 2 .OR. DATE .NE. 29) GO TO 141
DO 143 HOUR = HRF, HRL
DRHP(366, HOUR) = DRHP(60, HOUR)
143 DRHP(60, HOUR) = 0.0

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MAIN0210
MAIN0211
MAIN0212
MAIN0213
MAIN0214
MAIN0215
MAIN0216
MAIN0217
MAIN0218
MAIN0219
MAIN0220
MAIN0221
MAIN0222
MAIN0223
MAIN0224
MAIN0225
MAIN0226
MAIN0227
MAIN0228
MAIN0229
MAIN0230
MAIN0231
MAIN0232
MAIN0233
MAIN0234
MAIN0235
MAIN0236
MAIN0237
MAIN0238
MAIN0239
MAIN0240
MAIN0241
MAIN0242
MAIN0243
MAIN0244
MAIN0245

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```

GO TO 141
C. CALCULATE PRECIPITATION WEIGHTING FACTORS
144 DAY = 274
    IF(NSGRD .EQ. 0) GO TO 151
    PDAY = 274
    RDPT = 0.0
145 EHS GD = SGRT
    IF(SGRT .EQ. 0) EHS GD = 24
    EHS GDF = EHS GD
146 CONTINUE
    DO 150 HOUR = 1,24
    RDPT = RDPT + DRHP(DAY, HOUR)
    IF(HOUR .NE. EHS GD) GO TO 150
    IF(RDPT .LE. 0.0) GO TO 147
    IF(SGRT .EQ. 0) PDAY = DAY
    DRGPM (PDAY) = (DRSGP(DAY)*WSG + RDPT*(1.0 - WSG))/RDPT
    IF(CONOPT(3) .NE. 0) DPET(PDAY) = 0.5*DPET(PDAY)
    IF(SGRT .NE. 0) PDAY = DAY
    RDPT = 0.0
    GO TO 150
147 IF(DRSGP(DAY) .LE. 0.0) GO TO 149
    DO 148 K HOUR = 1, EHS GD
148 DRHP(DAY, K HOUR) = (WSG*DRSGP(DAY))/EHS GDF
149 IF(SGRT .NE. 0) PDAY = DAY
150 CONTINUE
    CALL DAYNXT(DAY, DPY)
    IF(DAY .EQ. 274) GO TO 151
    IF(CONOPT(8) .EQ. 0) GO TO 146
    IF(DAY .NE. SGMD) GO TO 146
    WSG = WSG2
    SGRT = SGRT2
    GO TO 145
151 MONTH = 1
    MDAY = 273
    AMRPM = 0.0
    AMPREC = 0.0

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MAIN0246
MAIN0247
MAIN0248
MAIN0249
MAIN0250
MAIN0251
MAIN0252
MAIN0253
MAIN0254
MAIN0255
MAIN0256
MAIN0257
MAIN0258
MAIN0259
MAIN0260
MAIN0261
MAIN0262
MAIN0263
MAIN0264
MAIN0265
MAIN0266
MAIN0267
MAIN0268
MAIN0269
MAIN0270
MAIN0271
MAIN0272
MAIN0273
MAIN0274
MAIN0275
MAIN0276
MAIN0277
MAIN0278
MAIN0279
MAIN0280
MAIN0281

```

AMBF = 0.0	MAIN0282
AMSE = 0.0	MAIN0283
AMSTF = 0.0	MAIN0284
AMRTF = 0.0	MAIN0285
WRITE(6,3) (TITLE(KTA), KTA = 1,20)	MAIN0286
3 FORMAT(1H1,10X,20A4)	MAIN0287
WRITE(6,4) (YTITLE(KTA), KTA = 1,20),YR1,YR2	MAIN0288
4 FORMAT(1H0,20A4,2X,13HWATER YEAR 19,12,1H-,12)	MAIN0289
WRITE(6,5)	MAIN0290
5 FORMAT(8H OCTOBER)	MAIN0291
C BEGIN DAY LOOP	MAIN0292
152 TDSF = 0.0	MAIN0293
PET = EPCM(MONTH)*DPET(DAY)	MAIN0294
PETU = PET	MAIN0295
TFMAX = 0.0	MAIN0296
C EVAPOTRANSPIRATION ADJUSTMENTS	MAIN0297
IF(CONOPT(7) .NE. 1) GO TO 153	MAIN0298
IF(DMXT(DAY) - 4.0*ELDIF .LT. 40.0) PET = 0.0	MAIN0299
IF(SPTW .GT. SPTWCC) PET = FFOR*PET	MAIN0300
C CALCULATION OF SNOW EVAPORATION	MAIN0301
IF(DMNT(DAY) .GT. 32.0 .OR. SPTW .LE. DPSE(DAY)) GO TO 153	MAIN0302
SE = DPSE(DAY)	MAIN0303
AMSNE = AMSNE + SE	MAIN0304
SPTW = SPTW - SE	MAIN0305
IF(SFMD .GT. 0.0) SDEPTH = SDEPTH - SE/SFMD	MAIN0306
153 DO 202 HOUR = 1,24	MAIN0307
IF((NSGRD .EQ. 0) .AND. (DRHP(DAY,HOUR) .NE. 0.0) .AND. (PET .EQ.	MAIN0308
1 PETU) .AND. (CONOPT(3) .EQ. 1)) PET = 0.5*PET	MAIN0309
154 IF(HOUR .EQ. SGRT + .1) RGPM = DRGPM(DAY)	MAIN0310
IF(HOUR .EQ. 9) HSE = (FWTR*PET)/12.0	MAIN0311
IF(HOUR .EQ. 21) HSE = 0.0	MAIN0312
PRH = RGPM*DRHP(DAY,HOUR)	MAIN0313
AMPREC = AMPREC + PRH	MAIN0314
C ENTER SNOWMELT SUBROUTINE	MAIN0315
IF(CONOPT(7) .EQ. 1) CALL SNOMEL(BDDFSM,SPTWCC,SPM,ELDIF,DAY,	MAIN0316
1 SPBFLW, XDNFS,FFOR,FFSI,MRNSM,DSMGH,SDEPTH,STMD, PXCSA,HOUR,	MAIN0317

2	SAX, SOFRF, ODRFIS, SOFRFI, AMFSIL, PRH, SPTW, TANSM, SPLW, SFMD, ODRF,	MAIN0318
3	WT4AM, WT4PM, ASM, ASMRG, SASFX, SARAX, DMXT, DMNT, RICY, FIRR)	MAIN0319
155	AMRPM = AMRPM + PRH	MAIN0320
156	TOFR = 0.0	MAIN0321
	ARHF = 0.0	MAIN0322
C 15	MINUTE ACCOUNTING AND ROUTING LOOP	MAIN0323
	DO 187 PRD = 1,4	MAIN0324
	PEBI = 0.0	MAIN0325
	PPI = 0.0	MAIN0326
	OFR = 0.0	MAIN0327
	ODRIS = 0.0	MAIN0328
	WI = 0.0	MAIN0329
	WEIFS = 0.0	MAIN0330
	PMEUZS = 0.0	MAIN0331
	PMELZS = 0.0	MAIN0332
	PMEIFS = 0.0	MAIN0333
	PMEQFS = 0.0	MAIN0334
	PEP = 0.25*PRH	MAIN0335
	IF(CONOPT(2) .EQ. 1) CALL PREPRD(RGPM, DRHP, DAY, HOUR, DPY, PRD, PEP,	MAIN0336
1	PRH)	MAIN0337
	IF(PEP .GT. 0.0) GO TO 157	MAIN0338
	IF(OFUS .GT. 0.0) GO TO 159	MAIN0339
	IF(IFS .GT. 0.0) GO TO 170	MAIN0340
	IF(NRTRI .GT. 0) GO TO 172	MAIN0341
	TRHF = 0.0	MAIN0342
	IF(RHFO .GT. 0.0) GO TO 181	MAIN0343
	GO TO 184	MAIN0344
C	RAINFALL UPPER ZONE INTERACTION	MAIN0345
157	IF(PEP .GE. VINTCR) GO TO 158	MAIN0346
	UZS = UZS + PEP*TPLR	MAIN0347
	VINTCR = VINTCR - PEP	MAIN0348
	PPI = 0.0	MAIN0349
	PEBI = 0.0	MAIN0350
	PMEUZS = PEP	MAIN0351
	IF(OFUS .GT. 0.0) GO TO 159	MAIN0352
	GO TO 170	MAIN0353



```

158 PPI = PEP - VINTCR
    UZS = UZS + VINTCR*TPLR
    VINTCR = 0.0
    LZSR = LZS/LZC
    UZC = SUZC*AEX90 + BUZC*EXP(-2.7*LZSR)
    IF(UZC .LT. 0.25) UZC = 0.25
    UZRX = 2.0*ABS(UZS/UZC - 1.0) + 1.0
    FMR = (1.0/(1.0 + UZRX))**UZRX
    IF(UZS .GT. UZC) FMR = 1.0 - FMR
    PEBI = PPI*FMR
    PMEUSZ = PEP - PEBI
    UZS = UZS + PPI - PEBI

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C LOWER ZONE AND GROUNDWATER INFILTRATION

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159 LZSR = LZS/LZC
    EID = 4.0*LZSR
    IF(LZSR .LE. 1.0) GO TO 160
    EID = 4.0 + 2.0*(LZSR - 1.0)
    IF(LZSR .LE. 2.0) GO TO 160
    EID = 6.0
160 PEBI = PEBI + OFUS
    CMIR = 0.25*SIAM*BMIR/(2.0**EID)
    CIVM = BIVF*2.0**LZSR
    IF(CIVM .LT. 1.0) CIVM = 1.0
    PEAI = PEBI*PEBI/(2.0*CMIR*CIVM)
    WI = PEBI*PEBI/(2.0*CMIR)
    IF(PEBI .GE. CMIR) WI = PEBI - 0.5*CMIR
    IF(PEBI .GE. CMIR*CIVM) PEAI = PEBI - 0.5*CMIR*CIVM
    WEIFS = WI - PEAI
    IF(PEBI .LE. OFUS) GO TO 161
    PMELZS = (PEBI - WI)*((PEBI - OFUS)/PEBI)
    PMEIFS = WEIFS*((PEBI - OFUS)/PEBI)
    PME OFS = PEAI*((PEBI - OFUS)/PEBI)
161 CONTINUE
    IF((PEAI - OFUS) .GT. 0.0) GO TO 162
    EQD = (OFUS + PEAI)/2.0
    GO TO 163

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MAIN0354
MAIN0355
MAIN0356
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MAIN0358
MAIN0359
MAIN0360
MAIN0361
MAIN0362
MAIN0363
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MAIN0367
MAIN0368
MAIN0369
MAIN0370
MAIN0371
MAIN0372
MAIN0373
MAIN0374
MAIN0375
MAIN0376
MAIN0377
MAIN0378
MAIN0379
MAIN0380
MAIN0381
MAIN0382
MAIN0383
MAIN0384
MAIN0385
MAIN0386
MAIN0387
MAIN0388
MAIN0389

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162	EQD = EQDF*((PEAI - OFUS)**0.6)	MAIN0390
163	IF((OFUS + PEAI) .GT. (2.0*EQD)) EQD = 0.5*(OFUS + PEAI)	MAIN0391
	IF((OFUS + PEAI) .LE. 0.001) GO TO 164	MAIN0392
	OFR = 0.25*OFRF*((OFUS + PEAI)*0.5)**1.67)*((1.0 + 0.6*((OFUS +	MAIN0393
	1 PEAI)/(2.0*EQD))**3.0)**1.67)	MAIN0394
	IF(OFR .GT. (0.75*PEAI)) OFR = 0.75*PEAI	MAIN0395
164	IF(FIMP .EQ. 0.0) GO TO 168	MAIN0396
165	PEIS = PPI + OFUSIS	MAIN0397
	IF((PEIS - OFUSIS) .GT. 0.0) GO TO 166	MAIN0398
	EQDIS = (OFUSIS + PEIS)/2.0	MAIN0399
	GO TO 167	MAIN0400
166	EQDIS = EQDFIS*((PEIS - OFUSIS)**0.6)	MAIN0401
167	IF((OFUSIS + PEIS) .GT. (2.0*EQDIS)) EQDIS = 0.5*(OFUSIS + PEIS)	MAIN0402
	IF((OFUSIS + PEIS) .LE. 0.01) GO TO 168	MAIN0403
	OFRIS = 0.25*OFRFIS*((OFUSIS + PEIS)*0.5)**1.67)*((1.0 + 0.6*((	MAIN0404
	1 OFUSIS + PEIS)/(2.0*EQDFIS))**3.0)**1.67)	MAIN0405
	IF(OFRIS .GT. PEIS) OFRIS = PEIS	MAIN0406
168	TOFR = TOFR + FPER*OFR + FIMP*OFRIS + PPI*FWTR	MAIN0407
	OFUSIS = PEIS - OFRIS	MAIN0408
	OFUS = PEAI - OFR	MAIN0409
	IF(OFUS .GE. 0.001) GO TO 169	MAIN0410
	LZS = LZS + OFUS	MAIN0411
	OFUS = 0.0	MAIN0412
	OFRIS = OFRIS + OFUSIS	MAIN0413
	OFUSIS = 0.0	MAIN0414
169	LZRX = 1.5*ABS(LZS/LZC - 1.0) + 1.0	MAIN0415
	FMR = (1.0/(1.0 + LZRX))**LZRX	MAIN0416
	IF(LZS .LT. LZC) FMR = 1.0 - FMR*(LZS/LZC)	MAIN0417
	PLZS = FMR*(PEBI - WI)	MAIN0418
	PGW = (1.0 - FMR)*((PEBI - WI)*(1.0 - SUBWF)*FPER	MAIN0419
	GWS = GWS + PGW	MAIN0420
	BFNX = BFNX + PGW	MAIN0421
	LZS = LZS + PLZS	MAIN0422
	IFS = IFS + WEIFS*FPER	MAIN0423
170	SPIF = IFRL*IFS	MAIN0424
	AMIF = AMIF + SPIF	MAIN0425

IFS = IFS - SPIF	MAIN0426
IF(IFS .GE. 0.0001) GO TO 171	MAIN0427
LZS = LZS + IFS	MAIN0428
IFS = 0.0	MAIN0429
171 UHFA(1) = FPER*OFR + PPI*FWTR + FIMP*OFRIS + SPIF	MAIN0430
SPDR = UHFA(1)	MAIN0431
C ROUTING	MAIN0432
172 IF(CONOPT(12) .NE. 1) GO TO 173	MAIN0433
URHF = URHF + 0.25*UHFA(1)	MAIN0434
IF(PRD .NE. 4) GO TO 181	MAIN0435
UHFA(1) = URHF	MAIN0436
173 TRHF = 0.0	MAIN0437
KTRI = NCTRI	MAIN0438
IF(CONOPT(13) .EQ. 1) KTRI = NCSTRI	MAIN0439
174 URHF = UHFA(KTRI)	MAIN0440
IF(URHF .LE. 0.0) GO TO 176	MAIN0441
175 TRHF = TRHF + URHF*CTRI(KTRI)	MAIN0442
IF(CONOPT(13) .EQ. 1 .AND. LSHFT .AND. KTRI .GE. 2) TRHF = TRHF +	MAIN0443
1 URHF*SATRI(KTRI - 1)	MAIN0444
UHFA(KTRI + 1) = URHF	MAIN0445
GO TO 177	MAIN0446
176 UHFA(KTRI + 1) = 0.0	MAIN0447
177 KTRI = KTRI - 1	MAIN0448
IF(KTRI .GE. 1) GO TO 174	MAIN0449
178 IF(URHF .LE. 0.0) GO TO 179	MAIN0450
NRTRI = NCTRI	MAIN0451
IF(CONOPT(13) .EQ. 1) NRTRI = MXTRI	MAIN0452
179 NRTRI = NRTRI - 1	MAIN0453
UHFA(1) = 0.0	MAIN0454
IF(CONOPT(13) .NE. 1) GO TO 180	MAIN0455
NNSTRI = NCSTRI + 1	MAIN0456
UHFA(NNSTRI) = 0.0	MAIN0457
180 URHF = 0.0	MAIN0458
181 IF(SRX .LE. CSRX) SRX = CSRX	MAIN0459
RHF1 = TRHF - SRX*(TRHF - RHF0)	MAIN0460
RHF0 = RHF1	MAIN0461

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IF(RHFO .LT. RHFC) RHFO = 0.0 MAIN0462
TFCFS = (4.0*RHF1 + CBF - HSE)*WCFS MAIN0463
IF(CONOPT(13) .NE. 1) GO TO 182 MAIN0464
IF(CONOPT(12) .EQ. 1 .AND. PRD .NE. 4) GO TO 182 MAIN0465
CALL RTVARY (CTRI,SATRI,BTRI,CHCAP,NBTRI,MXTRI,NCSTRI,EXQPV,LSHFT, MAIN0466
1 TFCFS) MAIN0467
DATE = MOD(DAY,MDAY) MAIN0468
IF(LSHFT) WRITE(6,6) DATE, HOUR, PRD, NCSTRI MAIN0469
6 FORMAT(2X, I2, 2X, I2, 2X, I2, 2X, I2, 20HHISTOGRAM CHANGES TO, 1X, I2, 1X, MAIN0470
1 8HELEMENTS) MAIN0471
182 CONTINUE MAIN0472
IF(TFCFS .LE. 0.5*CHCAP) SRX = CSRX MAIN0473
IF((TFCFS .GT. 0.5*CHCAP) .AND. (TFCFS .LT. 2.0*CHCAP)) SRX = CSRX MAIN0474
1 + (FSRX - CSRX) * ((TFCFS - 0.5*CHCAP) / (1.5*CHCAP)) ** 3 MAIN0475
IF(TFCFS .GT. 2.0*CHCAP) SRX = FSRX MAIN0476
IF(TFCFS .LE. TFMAX) GO TO 183 MAIN0477
PRDF = PRD MAIN0478
TDFP24 = HOUR MAIN0479
IF(PRD .LE. 3) TDFP24 = (TDFP24 - 1.0) + 0.15*PRDF MAIN0480
TFMAX = TFCFS MAIN0481
183 ARHF = ARHF + RHF1 MAIN0482
C STORM OUTPUT REQUESTED BY CONOPT(1) MAIN0483
184 IF(CONOPT(1) .NE. 1) GO TO 186 MAIN0484
IF(DAY .NE. CSDR) GO TO 186 MAIN0485
IF(HOUR .EQ. 1 .AND. PRD .EQ. 1) WRITE(6,7) MAIN0486
7 FORMAT(1H//, 21X, 19HRAINFALL DEPOSITION, 12X, 16HMOISTURE STORAGE, MAIN0487
1 14X, 17HSTREAMFLOW ORIGIN, 6X, 14HSTREAM OUTFLOW/2X, 116HDY HR PD. RAMAIN0488
2IN EUZS ELZS EIFS EOFS UZS LZS IFS OFS SMAIN0489
3POF SPIF SPBF SPTF INCHES CFS) MAIN0490
DATE = MOD(DAY,MDAY) MAIN0491
OFS = OFUS*EPER + OFUSIS*FIMP MAIN0492
SPOF = OFR*FPER + OFRIS*FIMP + PPI*FWTR MAIN0493
SPBF = 0.25*(CBF-HSE) MAIN0494
SPTF = SPDR + SPBF MAIN0495
SPDR = 0.0 MAIN0496
IF(RHFO .LE. 0.0) TFCFS = (CBF - HSE)*WCFS MAIN0497

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RSPTF = 0.25*TFCFS/WCFS	MAIN0498
WRITE(6,8) DATE, HOUR, PRD, PEP, PMEUSZ, PMELZS, PMEIFS, PMEDFS, UZS, LZS	MAIN0499
1, IFS, OFS, SPOF, SPIF, SPBF, SPTF, RSPTF, TFCFS	MAIN0500
8. FORMAT(2X, I2, 1X, I2, 1X, I1, 5(1X, F6.4), 2X, 4(F7.4), 2X, 5(1X, F6.4), 1X,	MAIN0501
1, F7.1)	MAIN0502
IF(HOUR .EQ. 24 .AND. PRD .EQ. 4) GO TO 185	MAIN0503
GO TO 186	MAIN0504
185 NDSDP = NDSDP + 1	MAIN0505
IF(NDSDR .EQ. NDSDP) GO TO 186	MAIN0506
CALL DAYNXT(CDSDR, DPY)	MAIN0507
186 CONTINUE	MAIN0508
IF(VINTCR .LT. 0.25*VINTMR) VINTCR = VINTCR + DPET(DAY)/96.0	MAIN0509
187 CONTINUE	MAIN0510
C END OF 15 MINUTE LOOP	MAIN0511
IF(CONOPT(5) .NE. 1) GO TO 197	MAIN0512
C HOURLY OVERLAND FLOW AND RAINFALL SORTING	MAIN0513
IF(TOFR .LE. 0.0) GO TO 193	MAIN0514
KT20 = 20	MAIN0515
188 IF(KT20 .LT. 1) GO TO 192	MAIN0516
IF(TOFR .GT. T200FH(KT20)) GO TO 189	MAIN0517
GO TO 190	MAIN0518
189 T200FH(KT20+1) = T200FH(KT20)	MAIN0519
GO TO 191	MAIN0520
190 T200FH(KT20+1) = TOFR	MAIN0521
GO TO 193	MAIN0522
191 KT20 = KT20 - 1	MAIN0523
GO TO 188	MAIN0524
192 T200FH(1) = TOFR	MAIN0525
193 IF(PRH .LE. 0.0) GO TO 197	MAIN0526
KT20 = 20	MAIN0527
194 IF(KT20 .LT. 1) GO TO 196	MAIN0528
IF(PRH .GT. T20PRH(KT20)) GO TO 195	MAIN0529
T20PRH(KT20 + 1) = PRH	MAIN0530
GO TO 197	MAIN0531
195 T20PRH(KT20+1) = T20PRH(KT20)	MAIN0532
KT20 = KT20 - 1	MAIN0533

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GO TO 194
196 T2OPRH(1) = PRH
C ADDING GROUNDWATER FLOW
197 CBF = GWS*BFRL*(1.0 + BFNRL*BFNX)
   GWS = GWS - CBF
   AMBF = AMBF + CBF
   THGR = ARHF + CBF
   IF(HSE .GT. THGR) HSE = THGR
   AMSE = AMSE + HSE
   THSF(HOUR) = (THGR - HSE)*WCFS
   TDSF = TDSF + THSF(HOUR)
C DRAINING OF UPPER ZONE STORAGE
   UZINFX = (UZS/UZC) - (LZS/LZC)
   IF(UZINFX .LE. 0.0) GO TO 198
   LZSR = LZS/LZC
   UZINLZ = 0.003*BMIR*UZC*UZINFX**3.0
   IF(UZINLZ .GT. UZS) UZINLZ = UZS
   UZS = UZS - UZINLZ
   LZRX = 1.5*ABS(LZSR - 1.0) + 1.0
   FMR = (1.0/(1.0 + LZRX))**LZRX
   IF(LZS .LT. LZC) FMR = 1.0 - FMR*LZSR
   PGW = (1.0-FMR)*UZINLZ*(1.0 - SUBWF)*FPER
   PLZS = FMR*UZINLZ
   LZS = LZS + PLZS
   GWS = GWS + PGW
   BFNX = BFNX + PGW
C 4 PM ADJUSTMENTS OF VARIOUS VALUES
198 IF(HOUR .NE. 16) GO TO 202
   AEX90 = 0.9*(AEX90 + PET)
   AEX96 = 0.96*(AEX96 + PET)
C INFILTRATION CORRECTION
   SIAM = (AEX96/AETX)**SIAC
   IF(SIAM .LT. 0.33) SIAM = 0.33
   BFNX = 0.97*BFNX
   IF(PET .EQ. 0.0) GO TO 202
C EVAP-TRANS LOSS FROM GROUNDWATER

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GWET = GWS*GWETF*PET*FPER
GWS = GWS - GWET
BFNX = BFNX - GWET
IF(BFNX .LT. 0.0) BFNX = 0.0
AMPET = AMPET + PET
IF(PET .GE. UZS) GO TO 199
UZS = UZS - PET
AMNET = AMNET + PET
GO TO 202
199 PET = PET - UZS
AMNET = AMNET + UZS
UZS = 0.0
LZSR = LZS/LZC
IF(PET .GE. ETLF*LZSR) GO TO 200
SET = PET*(1.0 - PET/(2.0*ETLF*LZSR))
GO TO 201
200 SET = 0.5*ETLF*LZSR
201 LZS = LZS - SET
AMNET = AMNET + SET
202 CONTINUE
C END OF HOUR LOOP
DSSF(DAY) = TDSF/24.0
IF(CONOPT(11) .EQ. 1) DSSF(DAY) = DSSF(DAY) + DDIW(DAY)
203 AMRTF = AMRTF + DRSF(DAY)
AMSTF = AMSTF + DSSF(DAY)
IF(CONOPT(6) .EQ. 1) EDLZS(DAY) = LZS
C STORE ERRORS AND FLOW DURATION
IF(CONOPT(4) .NE. 1) GO TO 204
ERR = DSSF(DAY) - DRSF(DAY)
IF(DRSF(DAY) .LT. 1.0) KRFMI = 1.0
IF(DRSF(DAY) .GT. 1.0) KRFMI = 2.0*ALOG(DRSF(DAY)) + 2.0
CRFMI(KRFMI) = CRFMI(KRFMI) + 1.0
SERR(KRFMI) = SERR(KRFMI) + ERR
SERA(KRFMI) = SERA(KRFMI) + ABS(ERR)
SQER(KRFMI) = SQER(KRFMI) + ERR*ERR
SESF(KRFMI) = 0.0

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MAIN0604
MAIN0605

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	IF(CRFMI(KRFMI) .GT. 1.0) SESF(KRFMI) = SQRT(ABS((SQER(KRFMI) -	MAIN0606
	1 SERR(KRFMI)**2/CRFMI(KRFMI))/(CRFMI(KRFMI) - 1.0))	MAIN0607
204	IF(DAY .EQ. 366) MDAY = 337	MAIN0608
	DATE = MOD(DAY,MDAY)	MAIN0609
	IF(TFMAX .LE. RMPF) GO TO 206	MAIN0610
	WRITE(6,9) DATE, (THSF(HOUR), HOUR=1,12)	MAIN0611
9	FORMAT(1H/,1X/,1X,I4,2X,2HAM,1X,6F8.1,3X,6F8.1)	MAIN0612
	WRITE(6,10) (THSF(HOUR), HOUR=13,24), DSSF(DAY)	MAIN0613
10	FORMAT(1HJ,6X,2HPM,1X,6F8.1,3X,7F8.1)	MAIN0614
	IF(TDFP24 .LT. 12.0) GO TO 205	MAIN0615
	TDFP12 = TDFP24 - 12.0	MAIN0616
	WRITE(6,11) TFMAX, TDFP12	MAIN0617
11	FORMAT(1H/,10X,8HMAXIMUM=,F8.1,2X,6HC.F.S.,5X,4HTIME,3X,F5.2,2X,	MAIN0618
	1 4HP.M.)	MAIN0619
	GO TO 206	MAIN0620
205	WRITE(6,12) TFMAX, TDFP24	MAIN0621
12	FORMAT(1H/,10X,8HMAXIMUM=,F8.1,2X,6HC.F.S.,5X,4HTIME,3X,F5.2,2X,	MAIN0622
	1 4HA.M.)	MAIN0623
206	IF(CONOPT(7) .EQ. 1 .AND. SDEPTH .GT. 0.0) WRITE(6,13) DATE,	MAIN0624
	1 SDEPTH, STMD, SAX, TANSM, SPLW	MAIN0625
13	FORMAT(3X,I4,2X,7HSDEPTH=,F8.2,2X,5HSTMD=,F6.2,2X,4HSAX=,F6.2,	MAIN0626
	1 2X,6HTANSM=,F6.2,2X,5HSPLW=,F6.2)	MAIN0627
C	MONTHLY SUMMARY STORAGE	MAIN0628
	IF(DAY .NE. MEDWY(MONTH)) GO TO 220	MAIN0629
	TMSTF(MONTH) = AMSTF	MAIN0630
	AMSTF = 0.0	MAIN0631
	TMRTF(MONTH) = AMRTF	MAIN0632
	AMRTF = 0.0	MAIN0633
	EMBFNX(MONTH) = BENX	MAIN0634
	TMPREC(MONTH) = AMPREC	MAIN0635
	AMPREC = 0.0	MAIN0636
	TMRPM(MONTH) = AMRPM	MAIN0637
	AMRPM = 0.0	MAIN0638
	TMBF(MONTH) = AMBF	MAIN0639
	AMBF = 0.0	MAIN0640
	TMIF(MONTH) = AMIF	MAIN0641



AMIF = 0.0	MAIN0642
TMSE(MONTH) = AMSE	MAIN0643
AMSE = 0.0	MAIN0644
TMPET(MONTH) = AMPET	MAIN0645
AMPET = 0.0	MAIN0646
TMNET(MONTH) = AMNET	MAIN0647
AMNET = 0.0	MAIN0648
TMSNE(MONTH) = AMSNE	MAIN0649
AMSNE = 0.0	MAIN0650
TMFSIL(MONTH) = AMFSIL	MAIN0651
AMFSIL = 0.0	MAIN0652
EMGWS(MONTH) = GWS	MAIN0653
UZC = SUZC*AEX90 + BUZC*EXP(-2.7*LZS/LZC)	MAIN0654
IF(UZC.LT. 0.25) UZC = 0.25	MAIN0655
EMUZC(MONTH) = UZC	MAIN0656
EMUZS(MONTH) = UZS	MAIN0657
EMSIAM(MONTH) = SIAM	MAIN0658
EMLZS(MONTH) = LZS	MAIN0659
EMIFS(MONTH) = IFS	MAIN0660
IF(MONTH.EQ. 5) MEDWY(5) = 59	MAIN0661
MDAY = MEDWY(MONTH)	MAIN0662
207 IF(MONTH.NE. 0) GO TO (208,209,210,211,212,213,214,215,216,217,	MAIN0663
1 218,219),MONTH	MAIN0664
208 WRITE(6,14)	MAIN0665
14 FORMAT(1H/,8HNOVEMBER)	MAIN0666
GO TO 219	MAIN0667
209 WRITE(6,15)	MAIN0668
15 FORMAT(1H/,8HDECEMBER)	MAIN0669
GO TO 219	MAIN0670
210 WRITE(6,16)	MAIN0671
16 FORMAT(1H/,7HJANUARY)	MAIN0672
GO TO 219	MAIN0673
211 WRITE(6,17)	MAIN0674
17 FORMAT(1H/,8HFEBRUARY)	MAIN0675
GO TO 219	MAIN0676
212 WRITE(6,18)	MAIN0677

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18 FORMAT(1H/,5HMARCH)
   GO TO 219
213 WRITE(6,19)
19 FORMAT(1H/,5HAPRIL)
   GO TO 219
214 WRITE(6,20)
20 FORMAT(1H/,3HMAY)
   GO TO 219
215 WRITE(6,21)
21 FORMAT(1H/,4HJUNE)
   GO TO 219
2160 WRITE(6,22)
22 FORMAT(1H/,4HJULY)
   GO TO 219
217 WRITE(6,23)
23 FORMAT(1H/,6HAUGUST)
   GO TO 219
218 WRITE(6,24)
24 FORMAT(1H/,9HSEPTEMBER)
219 MONTH = MONTH + 1
220 CALL DAYNXT(DAY,DPY)
   IF(DAY .NE. 274) GO TO 152
C   END OF DAY LOOP
221 CONTINUE
222 WRITE(6,25) (TITLE(KTA), KTA=1,20,1)
25 FORMAT(1H1,10X,20A4)
   WRITE(6,26) (YTITLE(KTA),KTA=1,15,1),YR1,YR2
26 FORMAT(1H/,15A4,3X,14HWATER YEAR 19,I2,1H-,I2,7X,
1. 29H KENTUCKY WATERSHED MODEL )
C   ANNUAL SUMMARY
   SATFV = 0.0
   RATFV = 0.0
   APREC = 0.0
   ABFV = 0.0
   ARPM = 0.0
   ASEV = 0.0

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	ANET = 0.0	MAIN0714
	APET = 0.0	MAIN0715
	AIFV = 0.0	MAIN0716
	ASE = 0.0	MAIN0717
	AFSIL = 0.0	MAIN0718
	DO 223 MONTH = 1,12	MAIN0719
	SATFV = SATFV + TMSTF(MONTH)	MAIN0720
	RATFV = RATFV + TMRTF(MONTH)	MAIN0721
	APREC = APREC + TMPREC(MONTH)	MAIN0722
	ABFV = ABFV + TMBF(MONTH)	MAIN0723
	ARPM = ARPM + TMRPM(MONTH)	MAIN0724
	ASEV = ASEV + TMSE(MONTH)	MAIN0725
	ANET = ANET + TMNET(MONTH)	MAIN0726
	APET = APET + TMPET(MONTH)	MAIN0727
	AIFV = AIFV + TMIF(MONTH)	MAIN0728
	ASE = ASE + TMSNE(MONTH)	MAIN0729
223	AFSIL = AFSIL + TMFSIL(MONTH)	MAIN0730
	IF(CONOPT(14) .NE. 1) GO TO 224	MAIN0731
	WRITE(6,27)	MAIN0732
27	FORMAT(1H///44X,20HRECORDED FLOWS)	MAIN0733
	CALL DAYOUT(DRSF, MEDWY, DPY)	MAIN0734
	WRITE(6,28)	MAIN0735
28	FORMAT(1H///44X,23HSYNTHESIZED FLOWS)	MAIN0736
224	CALL DAYOUT(DSSF, MEDWY, DPY)	MAIN0737
	WRITE(6,29) (TMSTF(KWD), KWD=1,12), SATFV	MAIN0738
29	FORMAT(1X, 9HSYNTHETIC, 3X, 12F8.1, 2X, F10.1, 2X, 3HSFD)	MAIN0739
	DO 225 MONTH = 1,12	MAIN0740
225	TMSTFI(MONTH) = (TMSTF(MONTH))/VWIN	MAIN0741
	SATFVI = SATFV/VWIN	MAIN0742
	WRITE(6,30) (TMSTFI(KWD), KWD=1,12), SATFVI	MAIN0743
30	FORMAT(1X, 5HTOTAL, 8X, 12F8.3, 4X, F7.3, 2X, 6HINCHES)	MAIN0744
	DO 226 MONTH = 1,12	MAIN0745
	TMOF(MONTH) = TMSTFI(MONTH) - TMIF(MONTH) - TMBF(MONTH) +	MAIN0746
	1 TMSE(MONTH)	MAIN0747
226	IF(TMOF(MONTH) .LT. 0.0) TMOF(MONTH) = 0.0	MAIN0748
	AOFV = SATFVI - AIFV - ABFV + ASEV	MAIN0749

	IF(AOFV .LT. 0.0) AOFV = 0.0	MAIN0750
	WRITE(6,31) (TMOF(KWD), KWD=1,12), AOFV	MAIN0751
31	FORMAT(1X,8HOVERLAND,5X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN0752
	WRITE(6,32) (TMIF(KWD), KWD=1,12),AIFV	MAIN0753
32	FORMAT(1X,9HINTERFLOW,4X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN0754
	WRITE(6,33) (TMBF(KWD), KWD=1,12),ABFV	MAIN0755
33	FORMAT(1X,4HBASE,9X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN0756
	WRITE(6,34) (TMSE(KWD), KWD=1,12), ASEV	MAIN0757
34	FORMAT(1X,9HSTRM EVAP,4X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN0758
	IF(CONOPT(9) .EQ. 0) GO TO 227	MAIN0759
	WRITE(6,35) (TMRTF(KWD), KWD=1,12),RATEV	MAIN0760
35	FORMAT(1X,8HRECORDED,4X,12F8.1,2X,F10.1,2X,3HSFD)	MAIN0761
	RATEFVI = RATEV/VWIN	MAIN0762
	WRITE(6,36) RATEFVI	MAIN0763
36	FORMAT(112X,F9.2,2X,6HINCHES)	MAIN0764
227	WRITE(6,37) (TMPREC(KWD), KWD=1,12),APREC	MAIN0765
37	FORMAT(1X,6HPRECIP,7X,12F8.2,3X,F8.2,2X,6HINCHES)	MAIN0766
	IF(CONOPT(7) .EQ.1) WRITE(6,38) (TMRPM(KWD), KWD=1,12),ARPM	MAIN0767
38	FORMAT(1X,9HRAIN+MELT,4X,12F8.2,3X,F8.2,2X,6HINCHES)	MAIN0768
	IF(CONOPT(7) .EQ.1) WRITE(6,39) (TMSNE(KWD), KWD=1,12),ASE	MAIN0769
39	FORMAT(1X,11HSURSNOWEVAP,3X,12F8.3,3X,F7.3,2X,6HINCHES)	MAIN0770
	IF(CONOPT(7) .EQ.1) WRITE(6,40) (TMFSIL(KWD), KWD=1,12),AFSIL	MAIN0771
40	FORMAT(1X,11HINTSNOWLOSS,3X,12F8.3,3X,F7.3,2X,6HINCHES)	MAIN0772
	WRITE(6,41) (TMNET(KWD), KWD=1,12),ANET	MAIN0773
41	FORMAT(1X,12HEVP/TRAN-NET,2X,12F8.3,3X,F7.3,2X,6HINCHES)	MAIN0774
	WRITE(6,42) (TMPET(KWD), KWD=1,12),APET	MAIN0775
42	FORMAT(3X,10H-POTENTIAL,2X,12F8.3,3X,F7.3,2X,6HINCHES)	MAIN0776
	WRITE(6,43) (EMUZS(KWD), KWD=1,12)	MAIN0777
43	FORMAT(1X,12HSTORAGES-UZS,2X,12F8.3,12X,6HINCHES)	MAIN0778
	WRITE(6,44) (EMLZS(KWD), KWD=1,12)	MAIN0779
44	FORMAT(10X,3HLZS,2X,12F8.3,12X,6HINCHES)	MAIN0780
	WRITE(6,45) (EMIFS(KWD), KWD=1,12)	MAIN0781
45	FORMAT(10X,3HIFS,2X,12F8.3,12X,6HINCHES)	MAIN0782
	WRITE(6,46) (EMGWS(KWD), KWD=1,12)	MAIN0783
46	FORMAT(10X,3HGWS,2X,12F8.3,12X,6HINCHES)	MAIN0784
	WRITE(6,47) (EMUZC(KWD), KWD=1,12)	MAIN0785

47	FORMAT(1X,12HINDICES= UZC,2X,12F8.3)	MAIN0786
	WRITE(6,48) (EMBFNX(KWD), KWD=1,12)	MAIN0787
48	FORMAT(9X,4HBFNX,2X,12F8.3)	MAIN0788
	WRITE(6,49) (EMSIAM(KWD), KWD=1,12)	MAIN0789
49	FORMAT(9X,4HSIAM,2X,12F8.3)	MAIN0790
	IF(CONOPT(7) .NE. 1) SPM = 1.0	MAIN0791
	AMBER = (LZS - BYLZS + IFS - BYIFS)*FPER + (UZS - BYUZS + GWS -	MAIN0792
	1 BYGWS)*(1.0 - FWTR) + SATFV/VWIN + ANET*FPER + ASEV - APREC	MAIN0793
	2 + ASE + AFSIL - ((SPM - 1.0)/SPM)*ASM	MAIN0794
	WRITE(6,50) AMBER	MAIN0795
50	FORMAT(1H/7HBALANCE,5X,F10.4,2X,6HINCHES)	MAIN0796
	IF(CONOPT(7) .NE. 1) GO TO 228	MAIN0797
	WRITE(6,51) ASM, ASMRG	MAIN0798
51	FORMAT(1H/,13HCHECK ON SNOW,5X,F10.4,5X,F10.4)	MAIN0799
	ASM = 0.0	MAIN0800
	ASMRG = 0.0	MAIN0801
228	CONTINUE	MAIN0802
	IF(CONOPT(4) .NE. 1) GO TO 232	MAIN0803
	WRITE(6,52)	MAIN0804
52	FORMAT(1H1,10X,35HDAILY FLOW DURATION AND ERROR TABLE)	MAIN0805
	WRITE(6,53)	MAIN0806
53	FORMAT(1H/,10X,13HFLOW INTERVAL,5X,5HCASES,3X,8HAV.ERROR,3X,	MAIN0807
	1 16H AVR. ABS. ERROR,3X,14HSTANDARD ERROR)	MAIN0808
	SSESF = 0.0	MAIN0809
	SSERA = 0.0	MAIN0810
	SSERR = 0.0	MAIN0811
	ACRFMI = 0.0	MAIN0812
	DO 230 KRFMI = 1,22	MAIN0813
	IF(KRFMI .EQ. 1) ETIBF = 0.0	MAIN0814
	IF(KRFMI .EQ. 2) ETIBF = 1.0	MAIN0815
	FKRFMI = KRFMI	MAIN0816
	IF(KRFMI .GT. 2) ETIBF = EXP((FKRFMI/2.0) - 1.0)	MAIN0817
	CCRFMI = CRFMI(KRFMI)	MAIN0818
	IF(CCRFMI .EQ. 0.0) WRITE(6,54) ETIBF, CCRFMI	MAIN0819
54	FORMAT(1X,13X,F8.1,1H-,F9.1,F12.1,5X,F8.2,5X,F8.2)	MAIN0820
	IF(CCRFMI .EQ. 0.0) GO TO 229	MAIN0821

SERAV = SERA(KRFMI)/CCRFMI	MAIN0822
SERRV = SERR(KRFMI)/CCRFMI	MAIN0823
IF(CCRFMI .EQ. 1) WRITE(6,54) ETIBF,CCRFMI,SERRV,SERAV	MAIN0824
IF(CCRFMI .NE. 1) WRITE(6,54) ETIBF,CCRFMI,SERRV,SERAV,	MAIN0825
1\$ESF(KRFMI)	MAIN0826
229 ACRFMI = ACRFMI + CRFMI(KRFMI)	MAIN0827
IF(ACRFMI .EQ. 0.0) GO TO 230	MAIN0828
SSERR = SSERR + SERR(KRFMI)	MAIN0829
SSERRV = SSERR/ACRFMI	MAIN0830
SSERA = SSERA + SERA(KRFMI)	MAIN0831
SSERAV = SSERA/ACRFMI	MAIN0832
230 S\$ESF = S\$ESF + \$ESF(KRFMI)	MAIN0833
WRITE(6,55) ACRFMI,SSERRV,SSERAV,S\$ESF	MAIN0834
55 FORMAT(1H/,22X,F9.1,F12.1,5X,F8.2,5X,F8.2)	MAIN0835
FDPY = DPY	MAIN0836
SADF = SATFV/FDPY	MAIN0837
RADF = RATFV/FDPY	MAIN0838
RA1 = 0.0	MAIN0839
RA2 = 0.0	MAIN0840
RA3 = 0.0	MAIN0841
DO 231 DAY = 1,DPY	MAIN0842
DRAF = DRSF(DAY) - RADF	MAIN0843
DSAF = DSSF(DAY) - SADF	MAIN0844
RA1 = RA1 + DRAF*DRAF	MAIN0845
RA2 = RA2 + DSAF*DSAF	MAIN0846
231 RA3 = RA3 + DRAF*DSAF	MAIN0847
DFCC = RA3/SQRT(RA1*RA2)	MAIN0848
WRITE(6,56) DFCC	MAIN0849
56 FORMAT(1H/,10X,31HCORRELATION COEFFICIENT (DAILY),3X,F10.4)	MAIN0850
232 CONTINUE	MAIN0851
IF(CONOPT(5) .NE. 1) GO TO 233	MAIN0852
C OUTPUT MAXIMUM RUNOFF, PRECIPITATION AT END OF YEARS	MAIN0853
WRITE(6,57)	MAIN0854
57 FORMAT(1H/,10X,58HTWENTY HIGHEST CLOCKHOUR RAINFALL EVENTS IN THE	MAIN0855
1WATER YEAR)	MAIN0856
WRITE(6,58) (T20PRH(KT20), KT20=1,20)	MAIN0857

58	FORMAT(1H/,5X,20F6.3)	MAIN0858
	WRITE(6,59)	MAIN0859
59	FORMAT(1H/,10X,70HTWENTY HIGHEST CLOCKHOUR OVERLAND FLOW RUNOFF EV	MAIN0860
	MENTS IN THE WATER YEAR)	MAIN0861
	WRITE(6,58) (T20DFH(KT20), KT20=1,20)	MAIN0862
233	CONTINUE	MAIN0863
	IF(CONOPT(6) .EQ. 0) GO TO 234	MAIN0864
	WRITE(6,60)	MAIN0865
60	FORMAT(1H1,30X,27HDAILY SOIL MOISTURE OUTPUT )	MAIN0866
	CALL DAYOUT(EDLZS,MEDWY,DPY)	MAIN0867
234	CONTINUE	MAIN0868
	IF(CONOPT(10) .EQ. 1) GO TO 100	MAIN0869
	GO TO 117	MAIN0870
	END	MAIN0871

	SUBROUTINE DAYNXT(DAY,DPY)	DYNX0001
C	DETERMINES NUMBER OF NEXT DAY OF THE YEAR	DYNX0002
	INTEGER DAY,DPY	DYNX0003
	DAY = DAY + 1	DYNX0004
	IF(DAY .EQ. 366) DAY = 1	DYNX0005
	IF(DAY .EQ. 60 .AND. DPY .EQ. 366) DAY = 366	DYNX0006
	IF(DAY .EQ. 367) DAY = 60	DYNX0007
	RETURN	DYNX0008
	END	DYNX0009

	SUBROUTINE DAYOUT(VDCY,MEDWY,DPY)	DYOT0001
C	PRINTS TABLE OF DAILY VALUES	DYOT0002
	DIMENSION MEDWY(12),VDCY(366),VDMO(12)	DYOT0003
	INTEGER DATE,DAY,DPY	DYOT0004
100	WRITE(6,1)	DYOT0005

1	FORMAT(7X,3HDAY,7X,3HOCT,5X,3HNOV,5X,3HDEC,5X,3HJAN,5X,3HFEB,5X,	DYOT0006
1	3HMAR,5X,3HAPR,5X,3HMAY,5X,3HJUN,5X,3HJUL,5X,3HAUG,5X,4HSEPT)	DYOT0007
	MEDWY(3) = 0	DYOT0008
	DO 104 DATE = 1,28,1	DYOT0009
	IF(MOD(DATE,5) .NE. 1) GO TO 102	DYOT0010
	DO 101 KMO = 1,12	DYOT0011
	DAY = MEDWY(KMO) + DATE	DYOT0012
101	VDMD(KMO) = VDCY(DAY)	DYOT0013
	WRITE(6,2) DATE,VDMD(12),{VDMD(KWD)}, KWD=1,11)	DYOT0014
	2 FORMAT(1H0,3X,16,3X,12F8.1)	DYOT0015
	GO TO 104	DYOT0016
102	DO 103 KMO = 1,12	DYOT0017
	DAY = MEDWY(KMO) + DATE	DYOT0018
103	VDMD(KMO) = VDCY(DAY)	DYOT0019
	WRITE(6,3) DATE,VDMD(12),{VDMD(KWD)}, KWD = 1,11)	DYOT0020
	3 FORMAT(1X,3X,16,3X,12F8.1)	DYOT0021
104	CONTINUE	DYOT0022
	IF(DPY .NE. 366) GO TO 106	DYOT0023
	DATE = 29	DYOT0024
	VDCY(60) = VDCY(366)	DYOT0025
	DO 105 KMO = 1,12	DYOT0026
	DAY = MEDWY(KMO) + DATE	DYOT0027
105	VDMD(KMO) = VDCY(DAY)	DYOT0028
	WRITE(6,3) DATE,VDMD(12),{VDMD(KWD)}, KWD=1,11)	DYOT0029
	GO TO 107	DYOT0030
106	CONTINUE	DYOT0031
	WRITE(6,4) VDCY(302),VDCY(333),VDCY(363),VDCY(29),VDCY(88),	DYOT0032
	1VDCY(119),VDCY(149),VDCY(180),VDCY(210),VDCY(241),VDCY(272)	DYOT0033
	4 FORMAT(1X,7X,2H29,3X,4F8.1,8X,7F8.1)	DYOT0034
107	CONTINUE	DYOT0035
108	WRITE(6,5) VDCY(303),VDCY(334),VDCY(364),VDCY(30),VDCY(89),	DYOT0036
	1VDCY(120),VDCY(150),VDCY(181),VDCY(211),VDCY(242),VDCY(273)	DYOT0037
	5 FORMAT(1X,7X,2H30,3X,4F8.1,8X,7F8.1)	DYOT0038
	WRITE(6,6) VDCY(304),VDCY(365),VDCY(31),VDCY(90),VDCY(151),	DYOT0039
	1VDCY(212),VDCY(243)	DYOT0040
	6 FORMAT(1H/,7X,2H31,3X,F8.1,8X,2F8.1,8X,F8.1,8X,F8.1,8X,2F8.1)	DYOT0041



MEDWY(3) = 365  
RETURN  
END

DYOT0042  
DYOT0043  
DYOT0044

SUBROUTINE EVPDAY(DPET, EMAET)  
C DETERMINES DATED PAN EVAPORATION TOTALS  
DIMENSION DPET(366)  
INTEGER DAY  
DO 100 DAY = 1,5  
100 DPET(DAY) = 0.00060\*EMAET  
DPET( 6) = 0.00059\*EMAET  
DPET( 7) = DPET( 6)  
DO 101 DAY = 8,10  
101 DPET(DAY) = 0.00058\*EMAET  
DO 102 DAY = 11,16  
102 DPET(DAY) = 0.00057\*EMAET  
DPET( 17) = DPET( 8)  
DO 103 DAY = 18,20  
103 DPET(DAY) = DPET( 6)  
DO 104 DAY = 21,32  
104 DPET(DAY) = DPET( 1)  
DPET( 33) = 0.00061\*EMAET  
DO 105 DAY = 34,38  
105 DPET(DAY) = 0.00062\*EMAET  
DPET( 39) = 0.00063\*EMAET  
DPET( 40) = DPET( 39)  
DPET( 41) = 0.00064\*EMAET  
DPET( 42) = 0.00065\*EMAET  
DPET( 43) = 0.00066\*EMAET  
DO 106 DAY = 44,50  
106 DPET(DAY) = 0.00067\*EMAET  
DO 107 DAY = 51,55  
107 DPET(DAY) = 0.00068\*EMAET  
DPET( 56) = 0.00069\*EMAET

EVDY0001  
EVDY0002  
EVDY0003  
EVDY0004  
EVDY0005  
EVDY0006  
EVDY0007  
EVDY0008  
EVDY0009  
EVDY0010  
EVDY0011  
EVDY0012  
EVDY0013  
EVDY0014  
EVDY0015  
EVDY0016  
EVDY0017  
EVDY0018  
EVDY0019  
EVDY0020  
EVDY0021  
EVDY0022  
EVDY0023  
EVDY0024  
EVDY0025  
EVDY0026  
EVDY0027  
EVDY0028  
EVDY0029  
EVDY0030

DD 108 DAY = 57,61  
108 DPET(DAY) = 0.00070\*EMAET  
DPET( 62) = 0.00071\*EMAET  
DPET( 63) = 0.00072\*EMAET  
DPET( 64) = DPET( 63)  
DPET( 65) = 0.00073\*EMAET  
DPET( 66) = 0.00074\*EMAET  
DPET( 67) = 0.00075\*EMAET  
DPET( 68) = 0.00076\*EMAET  
DPET( 69) = 0.00077\*EMAET  
DPET( 70) = DPET( 69)  
DPET( 71) = 0.00078\*EMAET  
DPET( 72) = DPET( 71)  
DPET( 73) = 0.00079\*EMAET  
DPET( 74) = DPET( 73)  
DPET( 75) = 0.00080\*EMAET  
DPET( 76) = 0.00081\*EMAET  
DPET( 77) = 0.00082\*EMAET  
DPET( 78) = 0.00084\*EMAET  
DPET( 79) = 0.00086\*EMAET  
DPET( 80) = 0.00088\*EMAET  
DPET( 81) = 0.00090\*EMAET  
DPET( 82) = 0.00092\*EMAET  
DPET( 83) = 0.00094\*EMAET  
DPET( 84) = 0.00097\*EMAET  
DPET( 85) = 0.00099\*EMAET  
DPET( 86) = 0.00102\*EMAET  
DPET( 87) = 0.00106\*EMAET  
DPET( 88) = 0.00109\*EMAET  
DPET( 89) = 0.00113\*EMAET  
DPET( 90) = 0.00118\*EMAET  
DPET( 91) = 0.00122\*EMAET  
DPET( 92) = 0.00128\*EMAET  
DPET( 93) = 0.00132\*EMAET  
DPET( 94) = 0.00137\*EMAET  
DPET( 95) = 0.00142\*EMAET

EVDY0031  
EVDY0032  
EVDY0033  
EVDY0034  
EVDY0035  
EVDY0036  
EVDY0037  
EVDY0038  
EVDY0039  
EVDY0040  
EVDY0041  
EVDY0042  
EVDY0043  
EVDY0044  
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EVDY0046  
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EVDY0048  
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EVDY0050  
EVDY0051  
EVDY0052  
EVDY0053  
EVDY0054  
EVDY0055  
EVDY0056  
EVDY0057  
EVDY0058  
EVDY0059  
EVDY0060  
EVDY0061  
EVDY0062  
EVDY0063  
EVDY0064  
EVDY0065  
EVDY0066

DPET( 96) = 0.00147\*EMAET  
DPET( 97) = 0.00151\*EMAET  
DPET( 98) = 0.00157\*EMAET  
DPET( 99) = 0.00163\*EMAET  
DPET(100) = 0.00168\*EMAET  
DPET(101) = 0.00173\*EMAET  
DPET(102) = 0.00178\*EMAET  
DPET(103) = 0.00185\*EMAET  
DPET(104) = 0.00193\*EMAET  
DPET(105) = 0.00201\*EMAET  
DPET(106) = 0.00208\*EMAET  
DPET(107) = 0.00214\*EMAET  
DPET(108) = 0.00221\*EMAET  
DPET(109) = 0.00227\*EMAET  
DPET(110) = 0.00234\*EMAET  
DPET(111) = 0.00241\*EMAET  
DPET(112) = 0.00249\*EMAET  
DPET(113) = 0.00256\*EMAET  
DPET(114) = 0.00262\*EMAET  
DPET(115) = 0.00268\*EMAET  
DPET(116) = 0.00276\*EMAET  
DPET(117) = 0.00281\*EMAET  
DPET(118) = 0.00287\*EMAET  
DPET(119) = 0.00293\*EMAET  
DPET(120) = 0.00299\*EMAET  
DPET(121) = 0.00305\*EMAET  
DPET(122) = 0.00310\*EMAET  
DPET(123) = 0.00317\*EMAET  
DPET(124) = 0.00322\*EMAET  
DPET(125) = 0.00328\*EMAET  
DPET(126) = 0.00333\*EMAET  
DPET(127) = 0.00338\*EMAET  
DPET(128) = 0.00344\*EMAET  
DPET(129) = 0.00348\*EMAET  
DPET(130) = 0.00354\*EMAET  
DPET(131) = 0.00359\*EMAET

EVDY0067  
EVDY0068  
EVDY0069  
EVDY0070  
EVDY0071  
EVDY0072  
EVDY0073  
EVDY0074  
EVDY0075  
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EVDY0077  
EVDY0078  
EVDY0079  
EVDY0080  
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EVDY0090  
EVDY0091  
EVDY0092  
EVDY0093  
EVDY0094  
EVDY0095  
EVDY0096  
EVDY0097  
EVDY0098  
EVDY0099  
EVDY0100  
EVDY0101  
EVDY0102

DPET(132) = 0.00365\*EMAET  
DPET(133) = 0.00370\*EMAET  
DPET(134) = 0.00374\*EMAET  
DPET(135) = 0.00378\*EMAET  
DPET(136) = 0.00382\*EMAET  
DPET(137) = 0.00387\*EMAET  
DPET(138) = 0.00391\*EMAET  
DPET(139) = 0.00394\*EMAET  
DPET(140) = 0.00399\*EMAET  
DPET(141) = 0.00402\*EMAET  
DPET(142) = 0.00407\*EMAET  
DPET(143) = 0.00411\*EMAET  
DPET(144) = 0.00417\*EMAET  
DPET(145) = 0.00420\*EMAET  
DPET(146) = 0.00426\*EMAET  
DPET(147) = 0.00430\*EMAET  
DPET(148) = 0.00436\*EMAET  
DPET(149) = 0.00440\*EMAET  
DPET(150) = 0.00446\*EMAET  
DPET(151) = 0.00450\*EMAET  
DPET(152) = 0.00455\*EMAET  
DPET(153) = 0.00460\*EMAET  
DPET(154) = 0.00466\*EMAET  
DPET(155) = 0.00470\*EMAET  
DPET(156) = 0.00473\*EMAET  
DPET(157) = 0.00478\*EMAET  
DPET(158) = 0.00482\*EMAET  
DPET(159) = 0.00487\*EMAET  
DPET(160) = 0.00491\*EMAET  
DPET(161) = 0.00495\*EMAET  
DPET(162) = 0.00500\*EMAET  
DPET(163) = 0.00504\*EMAET  
DPET(164) = 0.00508\*EMAET  
DPET(165) = 0.00510\*EMAET  
DPET(166) = 0.00512\*EMAET  
DPET(167) = 0.00514\*EMAET

EVDY0103  
EVDY0104  
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EVDY0131  
EVDY0132  
EVDY0133  
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EVDY0135  
EVDY0136  
EVDY0137  
EVDY0138

DPET(168) = 0.00515\*EMAET  
DPET(169) = 0.00517\*EMAET  
DPET(170) = 0.00519\*EMAET  
DPET(171) = 0.00520\*EMAET  
DPET(172) = 0.00521\*EMAET  
DPET(173) = DPET(172)  
DPET(174) = DPET(172)  
DPET(175) = 0.00522\*EMAET  
DPET(176) = 0.00523\*EMAET  
DPET(177) = 0.00524\*EMAET  
DPET(178) = 0.00525\*EMAET  
DPET(179) = 0.00527\*EMAET  
DPET(180) = 0.00528\*EMAET  
DPET(181) = DPET(180)  
DPET(182) = 0.00529\*EMAET  
DPET(183) = 0.00530\*EMAET  
DPET(184) = DPET(183)  
DPET(185) = 0.00531\*EMAET  
DPET(186) = 0.00532\*EMAET  
DPET(187) = 0.00533\*EMAET  
DPET(188) = 0.00534\*EMAET  
DPET(189) = DPET(188)  
DPET(190) = 0.00535\*EMAET  
DPET(191) = 0.00536\*EMAET  
DPET(192) = 0.00537\*EMAET  
DPET(193) = 0.00538\*EMAET  
DPET(194) = DPET(193)  
DPET(195) = 0.00539\*EMAET  
DPET(196) = 0.00540\*EMAET  
DPET(197) = DPET(196)  
DPET(198) = 0.00541\*EMAET  
DPET(199) = 0.00542\*EMAET  
DPET(200) = 0.00543\*EMAET  
DPET(201) = 0.00545\*EMAET  
DPET(202) = 0.00546\*EMAET  
DPET(203) = 0.00547\*EMAET

EVDY0139  
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EVDY0167  
EVDY0168  
EVDY0169  
EVDY0170  
EVDY0171  
EVDY0172  
EVDY0173  
EVDY0174

DPET(204) = 0.00548*EMAET	EVDY0175
DPET(205) = 0.00549*EMAET	EVDY0176
DPET(206) = 0.00550*EMAET	EVDY0177
DPET(207) = 0.00551*EMAET	EVDY0178
DPET(208) = 0.00552*EMAET	EVDY0179
DPET(209) = 0.00553*EMAET	EVDY0180
DPET(210) = 0.00555*EMAET	EVDY0181
DPET(211) = 0.00557*EMAET	EVDY0182
DPET(212) = 0.00558*EMAET	EVDY0183
DPET(213) = 0.00560*EMAET	EVDY0184
DPET(214) = DPET(213)	EVDY0185
DPET(215) = 0.00561*EMAET	EVDY0186
DPET(216) = 0.00562*EMAET	EVDY0187
DPET(217) = 0.00563*EMAET	EVDY0188
DPET(218) = 0.00565*EMAET	EVDY0189
DPET(219) = 0.00567*EMAET	EVDY0190
DPET(220) = DPET(219)	EVDY0191
DO 109 DAY = 221,226	EVDY0192
109 DPET(DAY) = 0.00568*EMAET	EVDY0193
DO 110 DAY = 227,229	EVDY0194
110 DPET(DAY) = DPET(219)	EVDY0195
DPET(230) = 0.00566*EMAET	EVDY0196
DPET(231) = 0.00564*EMAET	EVDY0197
DPET(232) = DPET(217)	EVDY0198
DPET(233) = DPET(216)	EVDY0199
DPET(234) = DPET(213)	EVDY0200
DPET(235) = 0.00559*EMAET	EVDY0201
DPET(236) = DPET(211)	EVDY0202
DPET(237) = DPET(210)	EVDY0203
DPET(238) = DPET(209)	EVDY0204
DPET(239) = DPET(206)	EVDY0205
DPET(240) = DPET(203)	EVDY0206
DPET(241) = DPET(199)	EVDY0207
DPET(242) = DPET(193)	EVDY0208
DPET(243) = DPET(190)	EVDY0209
DPET(244) = DPET(185)	EVDY0210

DPET(245) = DPET(179)  
DPET(246) = DPET(175)  
DPET(247) = DPET(169)  
DPET(248) = 0.00511\*EMAET  
DPET(249) = DPET(163)  
DPET(250) = 0.00497\*EMAET  
DPET(251) = 0.00490\*EMAET  
DPET(252) = DPET(158)  
DPET(253) = 0.00476\*EMAET  
DPET(254) = 0.00468\*EMAET  
DPET(255) = 0.00461\*EMAET  
DPET(256) = 0.00454\*EMAET  
DPET(257) = DPET(150)  
DPET(258) = 0.00437\*EMAET  
DPET(259) = 0.00427\*EMAET  
DPET(260) = 0.00418\*EMAET  
DPET(261) = DPET(142)  
DPET(262) = 0.00397\*EMAET  
DPET(263) = DPET(137)  
DPET(264) = 0.00377\*EMAET  
DPET(265) = 0.00367\*EMAET  
DPET(266) = 0.00356\*EMAET  
DPET(267) = 0.00347\*EMAET  
DPET(268) = 0.00337\*EMAET  
DPET(269) = 0.00329\*EMAET  
DPET(270) = DPET(124)  
DPET(271) = 0.00315\*EMAET  
DPET(272) = 0.00308\*EMAET  
DPET(273) = 0.00303\*EMAET  
DPET(274) = 0.00300\*EMAET  
DPET(275) = 0.00298\*EMAET  
DPET(276) = 0.00294\*EMAET  
DPET(277) = 0.00290\*EMAET  
DPET(278) = 0.00286\*EMAET  
DPET(279) = 0.00283\*EMAET  
DPET(280) = 0.00279\*EMAET

EVDY0211  
EVDY0212  
EVDY0213  
EVDY0214  
EVDY0215  
EVDY0216  
EVDY0217  
EVDY0218  
EVDY0219  
EVDY0220  
EVDY0221  
EVDY0222  
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EVDY0224  
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EVDY0229  
EVDY0230  
EVDY0231  
EVDY0232  
EVDY0233  
EVDY0234  
EVDY0235  
EVDY0236  
EVDY0237  
EVDY0238  
EVDY0239  
EVDY0240  
EVDY0241  
EVDY0242  
EVDY0243  
EVDY0244  
EVDY0245  
EVDY0246

DPET(281) = DPET(116)  
DPET(282) = 0.00271\*EMAET  
DPET(283) = DPET(115)  
DPET(284) = DPET(114)  
DPET(285) = 0.00259\*EMAET  
DPET(286) = 0.00254\*EMAET  
DPET(287) = 0.00252\*EMAET  
DPET(288) = 0.00247\*EMAET  
DPET(289) = 0.00244\*EMAET  
DPET(290) = 0.00239\*EMAET  
DPET(291) = DPET(110)  
DPET(292) = 0.00230\*EMAET  
DPET(293) = 0.00225\*EMAET  
DPET(294) = 0.00222\*EMAET  
DPET(295) = 0.00217\*EMAET  
DPET(296) = 0.00213\*EMAET  
DPET(297) = 0.00210\*EMAET  
DPET(298) = 0.00206\*EMAET  
DPET(299) = 0.00200\*EMAET  
DPET(300) = 0.00197\*EMAET  
DPET(301) = 0.00194\*EMAET  
DPET(302) = 0.00189\*EMAET  
DPET(303) = 0.00186\*EMAET  
DPET(304) = 0.00183\*EMAET  
DPET(305) = 0.00180\*EMAET  
DPET(306) = 0.00177\*EMAET  
DPET(307) = 0.00174\*EMAET  
DPET(308) = 0.00172\*EMAET  
DPET(309) = DPET(100)  
DPET(310) = DPET(99)  
DPET(311) = 0.00160\*EMAET  
DPET(312) = 0.00156\*EMAET  
DPET(313) = 0.00152\*EMAET  
DPET(314) = 0.00149\*EMAET  
DPET(315) = 0.00146\*EMAET  
DPET(316) = DPET(95)

EVDY0247  
EVDY0248  
EVDY0249  
EVDY0250  
EVDY0251  
EVDY0252  
EVDY0253  
EVDY0254  
EVDY0255  
EVDY0256  
EVDY0257  
EVDY0258  
EVDY0259  
EVDY0260  
EVDY0261  
EVDY0262  
EVDY0263  
EVDY0264  
EVDY0265  
EVDY0266  
EVDY0267  
EVDY0268  
EVDY0269  
EVDY0270  
EVDY0271  
EVDY0272  
EVDY0273  
EVDY0274  
EVDY0275  
EVDY0276  
EVDY0277  
EVDY0278  
EVDY0279  
EVDY0280  
EVDY0281  
EVDY0282



DPET(317) = 0.00138*EMAET	EVDY0283
DPET(318) = 0.00135*EMAET	EVDY0284
DPET(319) = 0.00131*EMAET	EVDY0285
DPET(320) = 0.00127*EMAET	EVDY0286
DPET(321) = 0.00124*EMAET	EVDY0287
DPET(322) = 0.00120*EMAET	EVDY0288
DPET(323) = DPET( 90)	EVDY0289
DPET(324) = 0.00116*EMAET	EVDY0290
DPET(325) = DPET( 89)	EVDY0291
DPET(326) = 0.00110*EMAET	EVDY0292
DPET(327) = 0.00107*EMAET	EVDY0293
DPET(328) = 0.00104*EMAET	EVDY0294
DPET(329) = DPET( 86)	EVDY0295
DPET(330) = 0.00100*EMAET	EVDY0296
DPET(331) = 0.00098*EMAET	EVDY0297
DPET(332) = 0.00097*EMAET	EVDY0298
DPET(333) = 0.00095*EMAET	EVDY0299
DPET(334) = 0.00093*EMAET	EVDY0300
DPET(335) = DPET( 81)	EVDY0301
DPET(336) = DPET( 80)	EVDY0302
DPET(337) = 0.00087*EMAET	EVDY0303
DPET(338) = DPET( 79)	EVDY0304
DPET(339) = DPET( 78)	EVDY0305
DPET(340) = DPET( 77)	EVDY0306
DPET(341) = DPET( 75)	EVDY0307
DPET(342) = DPET( 73)	EVDY0308
DPET(343) = DPET( 71)	EVDY0309
DPET(344) = DPET( 71)	EVDY0310
DPET(345) = DPET( 69)	EVDY0311
DPET(346) = DPET( 68)	EVDY0312
DPET(347) = DPET( 66)	EVDY0313
DPET(348) = DPET( 63)	EVDY0314
DPET(349) = DPET( 62)	EVDY0315
DPET(350) = DPET( 57)	EVDY0316
DPET(351) = DPET( 57)	EVDY0317
DPET(352) = DPET( 56)	EVDY0318

```

DO 111 DAY = 353,355
111 DPET(DAY) = DPET( 51)
   DPET(356) = DPET( 44)
   DPET(357) = DPET( 44)
   DPET(358) = DPET( 42)
   DPET(359) = DPET( 41)
   DPET(360) = DPET( 39)
   DPET(361) = DPET( 34)
   DPET(362) = DPET( 33)
DO 112 DAY = 363,365
112 DPET(DAY) = DPET( 1)
   DPET(366) = DPET( 57)
RETURN
END

```

```

EVDY0319
EVDY0320
EVDY0321
EVDY0322
EVDY0323
EVDY0324
EVDY0325
EVDY0326
EVDY0327
EVDY0328
EVDY0329
EVDY0330
EVDY0331
EVDY0332

```

```

SUBROUTINE PREPRD(RGPM,DRHP,DAY,HOUR,DPY,PRD,PEP,PRH)
C DIVIDES HOURLY PRECIPITATION TOTALS AMONG PERIODS FOR SMALL BASINS
  DIMENSION DRHP(366,24), PE4P(4)
  INTEGER DAY,DPY,HOUR,PRD
  PEP = 0.0
  IF(PRH .EQ. 0.0) RETURN
  IF(PRD .EQ. 1) GO TO 100
  PEP = PE4P(PRD)
  RETURN
100 LHOOR = HOUR - 1
   LDAY = DAY
   IF(LHOOR .GE. 1) GO TO 101
   LHOOR = 24
   LDAY = DAY - 1
   IF(LDAY .EQ. 0) LDAY = 365
   IF(LDAY .EQ. 365) LDAY = 59
   IF(LDAY .EQ. 59 .AND. DPY .EQ. 366) LDAY = 366
101 PRLH = RGPM*DRHP(LDAY,LHOOR)
   NHOOR = HOUR + 1

```

```

PREP0001
PREP0002
PREP0003
PREP0004
PREP0005
PREP0006
PREP0007
PREP0008
PREP0009
PREP0010
PREP0011
PREP0012
PREP0013
PREP0014
PREP0015
PREP0016
PREP0017
PREP0018
PREP0019

```

```

      NDAY = DAY
      IF(NHOUR .LE. 24) GO TO 102
      NHOUR = 1
      CALL DAYNXT(NDAY,DPY)
102  PRNH = RGPM*DRHP(NDAY,NHOUR)
      IF(PRH .GT. PRLH .AND. PRH .GT. PRNH) GO TO 103
      GO TO 104
103  PE4P(1) = 0.10
      PE4P(2) = 0.28
      PE4P(3) = 0.46
      PE4P(4) = 0.16
      GO TO 108
104  IF(PRH .LT. PRLH .AND. PRH .LT. PRNH) GO TO 105
      GO TO 106
105  PE4P(1) = 0.28
      PE4P(2) = 0.10
      PE4P(3) = 0.16
      PE4P(4) = 0.46
      GO TO 108
106  IF(PRNH .GE. PRLH) GO TO 107
      PE4P(1) = 0.46
      PE4P(2) = 0.16
      PE4P(3) = 0.28
      PE4P(4) = 0.10
      GO TO 108
107  PE4P(1) = 0.10
      PE4P(2) = 0.28
      PE4P(3) = 0.16
      PE4P(4) = 0.46
108  DO 109 KPRD = 1,4
109  PE4P(KPRD) = PE4P(KPRD)*PRH
      PEP = PE4P(1)
      RETURN
      END

```

```

PREP0020
PREP0021
PREP0022
PREP0023
PREP0024
PREP0025
PREP0026
PREP0027
PREP0028
PREP0029
PREP0030
PREP0031
PREP0032
PREP0033
PREP0034
PREP0035
PREP0036
PREP0037
PREP0038
PREP0039
PREP0040
PREP0041
PREP0042
PREP0043
PREP0044
PREP0045
PREP0046
PREP0047
PREP0048
PREP0049
PREP0050
PREP0051
PREP0052
PREP0053

```

<pre> SUBROUTINE RTVARY(CTRI,SATRI,BTRI,CHCAP,NBTRI,MXTRI,NCTRI,EXQPV, 1 LSHFT,TFCFS) DIMENSION AWSBIT(99),BTRI(99),CTRI(99),SATRI(99) LOGICAL LSHFT DO 100 KIA = 1,MXTRI SATRI(KIA) = 0.0 100 AWSBIT(KIA) = 0.0 LSHFT = .FALSE. FMXTRI = MXTRI FNBTRI = NBTRI FNPTRI = NCTRI TFX = TFCFS TFMRT = 0.1*CHCAP IF(TFX .LT. TFMRT) TFX = TFMRT IF(FNPTRI .EQ. FMXTRI .AND. TFX .EQ. TFMRT) RETURN FNTRI = FNBTRI*(CHCAP/TFX)**EXQPV + 0.5 IF(FNTRI .LT. 1.0) FNTRI = 1.01 NCTRI = FNTRI FNSTRI = NCTRI IF(FNSTRI .NE. FNPTRI) LSHFT = .TRUE. IF(.NOT. LSHFT) RETURN IF(FNPTRI .GT. 98.5) GO TO 101 FCNTRI = ABS(FNSTRI - FNPTRI) IF(FCNTRI .LE. 1.1) GO TO 101 IF(FNSTRI .GT. FNPTRI) FNSTRI = FNPTRI + 1.0 IF(FNSTRI .LT. FNPTRI) FNSTRI = FNPTRI - 1.0 NCTRI = FNSTRI 101 KB1 = 0 KB2 = 1 KB3 = 0 102 KB1 = KB1 + 1 IF(KB1 .GT. NBTRI) GO TO 105 KB4 = 0 WSBIT = BTRI(KB1)/FNSTRI 103 KB4 = KB4 + 1 </pre>	<pre> RTVY0001 RTVY0002 RTVY0003 RTVY0004 RTVY0005 RTVY0006 RTVY0007 RTVY0008 RTVY0009 RTVY0010 RTVY0011 RTVY0012 RTVY0013 RTVY0014 RTVY0015 RTVY0016 RTVY0017 RTVY0018 RTVY0019 RTVY0020 RTVY0021 RTVY0022 RTVY0023 RTVY0024 RTVY0025 RTVY0026 RTVY0027 RTVY0028 RTVY0029 RTVY0030 RTVY0031 RTVY0032 RTVY0033 RTVY0034 RTVY0035 </pre>
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IF(KB4 .GT. NCTRI) GO TO 102	RTVY0036
AWSBIT(KB2) = AWSBIT(KB2) + WSBIT	RTVY0037
KB3 = KB3 + 1	RTVY0038
IF(KB3 .LT. NBTRI) GO TO 104	RTVY0039
KB3 = 0	RTVY0040
KB2 = KB2 + 1	RTVY0041
104 GO TO 103	RTVY0042
105 IF(FNPTRI .GT. 98.5) GO TO 108	RTVY0043
DO 107 KB6 = 1,NCTRI	RTVY0044
DO 106 KB7 = 1,KB6	RTVY0045
106 SATRI(KB6) = SATRI(KB6) + AWSBIT(KB7) - CTRI(KB7)	RTVY0046
107 CONTINUE	RTVY0047
108 DO 109 KB5 = 1,MXTRI	RTVY0048
109 CTRI(KB5) = AWSBIT(KB5)	RTVY0049
RETURN	RTVY0050
END	RTVY0051

- 220 -

SUBROUTINE SNOMEL(BDDFSM, SPTWCC, SPM, ELDIF, DAY, SPBFLW, XDNFS, FFOR,	SNOW0001
1 FFSI, MRNSM, DSMGH, SDEPTH, STMD, PXCSA, HOUR, SAX, SOFRF, OFRFIS, SOFRFI,	SNOW0002
2 AMFSIL, PRH, SPTW, TANSM, SPLW, SFMD, OFRF, WT4AM, WT4PM, ASM, ASMRG,	SNOW0003
3 SASFX, SARAX, DMXT, DMNT, RICY, FIRR)	SNOW0004
C SNOWMELT COMPUTATION	SNOW0005
DIMENSION DMNT(366), DMXT(366), FIRR(15), RICY(37)	SNOW0006
INTEGER DAY, HOUR	SNOW0007
REAL MFSM, MRNSM	SNOW0008
IF((DAY .NE. 274) .OR. (HOUR .NE. 1)) GO TO 100	SNOW0009
SPLW = 0.0	SNOW0010
XELR = 0.0	SNOW0011
SDSC = 0.0278	SNOW0012
FDSC = 0.0	SNOW0013
FTA = 0.0	SNOW0014
RICD = 0.0	SNOW0015
KRIA = 0	SNOW0016
100 CONTINUE	SNOW0017

C	CALCULATION OF HOURLY AIR TEMPERATURE	SNOW0018
C	DMXT CURRENT DAY, DMNT NEXT DAY	SNOW0019
	IF(HOUR .NE. 4) GO TO 101	SNOW0020
	FDSC = 0.0	SNOW0021
	FTA = FDSC	SNOW0022
	WT4PM = DMXT(DAY) - 4.0*ELDIF + (XELR/4.0)*0.7*ELDIF	SNOW0023
101	IF(HOUR .EQ. 10) SDSC = -0.0278	SNOW0024
	IF(HOUR .EQ. 22) SDSC = 0.0278	SNOW0025
	IF(HOUR .NE. 16) GO TO 102	SNOW0026
	NDAY = DAY + 1	SNOW0027
	IF(NDAY .EQ. 366) NDAY = 1	SNOW0028
	IF(NDAY .EQ. 60 .AND. DMXT(366) .NE. 0.0) NDAY = 366	SNOW0029
	IF(NDAY .EQ. 367) NDAY = 60	SNOW0030
	WT4AM = DMNT(NDAY) - (XELR/4.0)*3.3*ELDIF	SNOW0031
102	IF(PRH .LE. 0.0 .OR. XELR .GE. 4.0) GO TO 103	SNOW0032
	WT4AM = WT4AM - 0.825*ELDIF	SNOW0033
	WT4PM = WT4PM + 0.175*ELDIF	SNOW0034
	XELR = XELR + 1.0	SNOW0035
103	IF(PRH .NE. 0.0 .OR. XELR .LE. 0.0) GO TO 104	SNOW0036
	WT4AM = WT4AM + 0.825*ELDIF	SNOW0037
	WT4PM = WT4PM - 0.175*ELDIF	SNOW0038
	XELR = XELR - 1.0	SNOW0039
104	TEH = WT4AM + FTA*(WT4PM - WT4AM)	SNOW0040
	FDSC = FDSC + SDSC	SNOW0041
	FTA = FTA + FDSC	SNOW0042
	IF(PRH+SPTW .EQ. 0.0) GO TO 128	SNOW0043
	IF(HOUR .NE. 24) GO TO 105	SNOW0044
C	CALCULATION OF TIME AGING OF THE SNOWPACK	SNOW0045
	SAX = SAX + 1.0	SNOW0046
	IF(SAX .GT. 15.0) SAX = 15.0	SNOW0047
105	IF(TEH .GT. 32.0) GO TO 110	SNOW0048
C	PRECIPITATION IN FORM OF SNOW - CALCULATE INTERCEPTION DENSITY OF NEWS	SNOW0049
C	SNOW COMPACTION, AND SETTLING SNOW PACK AND THE EFFECT ON ALBEDO	SNOW0050
	IF(PRH .LE. 0.0) GO TO 110	SNOW0051
	PRH = SPM*PRH	SNOW0052
	HSE = PRH	SNOW0053

ASM = ASM + HSF	SNOW0054
PRH = (1.0 - (FFSI*FFOR))*PRH	SNOW0055
HSPRG = PRH	SNOW0056
ASMRG = ASMRG + HSPRG	SNOW0057
FSIL = FFSI*FFOR*HSF	SNOW0058
AMFSIL = AMFSIL + FSIL	SNOW0059
IF(TEH .LE. 0.0) GO TO 106	SNOW0060
DNFS = XDNFS + ((0.01*TEH)**2)	SNOW0061
GO TO 107	SNOW0062
106 DNFS = XDNFS	SNOW0063
107 IF(SPTW .GT. 0.0 .AND. SDEPTH .GT. SPTW) SDEPTH = SDEPTH - ((PRH*	SNOW0064
1 SDEPTH/SPTW)*((0.10*SDEPTH)**0.25))	SNOW0065
SPTW = SPTW + PRH	SNOW0066
SDEPTH = SDEPTH + (PRH/DNFS)	SNOW0067
SASFX = SASFX + PRH	SNOW0068
IF(SASFX .GE. PXCSA) GO TO 108	SNOW0069
GO TO 109	SNOW0070
108 SAX = SAX - 1.0	SNOW0071
IF(SAX .LT. 0.0) SAX = 0.0	SNOW0072
SASFX = SASFX - PXCSA	SNOW0073
109 PRH = 0.0	SNOW0074
110 CONTINUE	SNOW0075
IF(SPTW .LE. 0.0) GO TO 127	SNOW0076
C SEASONAL MELT FACTOR ADJUSTMENT	SNOW0077
FDAY = DAY	SNOW0078
KAAO = KRIA	SNOW0079
KRIA = 1.0 + (FDAY/10.0)	SNOW0080
IF(KAAO .NE. KRIA) RICD = RICY(KRIA)	SNOW0081
IF(TEH .LE. 32.0) GO TO 111	SNOW0082
GO TO 114	SNOW0083
C CALCULATION OF NEGATIVE MELT	SNOW0084
111 IF(TANSM .LE. 11.5*MRNSM) GO TO 112	SNOW0085
IF(TANSM .LT. 1.0) TANSM = TANSM + ((5.0*MRNSM)**(1.3 + 2.0*	SNOW0086
1 TANSM))	SNOW0087
GO TO 113	SNOW0088
112 TANSM = TANSM + MRNSM	SNOW0089

113	IF(TANSM .GT. 0.08*SPTW) TANSM = 0.08*SPTW	SNOW0090
	GO TO 127	SNOW0091
C	EFFECT OF RAIN ON ALBEDO	SNOW0092
114	SARAX = SARAX + PRH	SNOW0093
	IF(SARAX .LT. PXCSA/2.0) GO TO 115	SNOW0094
	SAX = SAX + 1.0	SNOW0095
	IF(SAX .GT. 15.0) SAX = 15.0	SNOW0096
	SASFX = 0.0	SNOW0097
	SARAX = SARAX - (PXCSA/2.0)	SNOW0098
115	IF(TEH .GT. 32.0) HSM = (TEH - 32.0)*BDDFSM	SNOW0099
	IF(TEH .LT. 32.0) HSM = 0.0	SNOW0100
	HSM = HSM*RICD	SNOW0101
	KAA = 1.0 + SAX	SNOW0102
	IF(SAX .LT. 15.0) HSM = HSM*(1.0 - ((1.0 - FFOR)*FIRR(KAA)))	SNOW0103
	IF(SAX .EQ. 15.0) HSM = HSM*(1.0 - ((1.0 - FFOR)*FIRR(15)))	SNOW0104
	IF(PRH .GT. 0.0) HSM = HSM + ((TEH - 32.0)*(PRH/144.0))	SNOW0105
	IF(STMD .GT. 0.3 .AND. SPTW .LT. SPTWCC) GO TO 116	SNOW0106
	GO TO 117	SNOW0107
116	MHSM = HSM	SNOW0108
	HSM = (SPTW/SPTWCC)*HSM	SNOW0109
	IF(HSM .LT. 0.1*MHSM) HSM = 0.1*MHSM	SNOW0110
117	IF(HSM .LT. SPTW) GO TO 118	SNOW0111
	HSM = SPTW	SNOW0112
	SDEPTH = 0.0	SNOW0113
	SPTW = 0.0	SNOW0114
	SPLW = 0.0	SNOW0115
	RICD = 0.0	SNOW0116
	TANSM = 0.0	SNOW0117
	SAX = 15.0	SNOW0118
	OFRF = SOFRF	SNOW0119
	OFRFIS = SOFRFI	SNOW0120
	GO TO 122	SNOW0121
118	SPTW = SPTW - HSM	SNOW0122
	IF(SFMD .LE. 0.0) GO TO 122	SNOW0123
	IF(SAX .GE. 15.0) GO TO 121	SNOW0124
	IF(SAX .GE. 6.0) GO TO 119	SNOW0125



SDEPTH = SDEPTH - (HSM/(0.5*SFMD))	SNOW0126
GO TO 122	SNOW0127
119 IF(SAX .LE. 10.0) GO TO 120	SNOW0128
SDEPTH = SDEPTH - (HSM/(0.9*SFMD))	SNOW0129
GO TO 122	SNOW0130
120 SDEPTH = SDEPTH - (HSM/(0.7*SFMD))	SNOW0131
GO TO 122	SNOW0132
121 SDEPTH = SDEPTH - (HSM/SFMD)	SNOW0133
122 CONTINUE	SNOW0134
IF(SPTW .LT. 0.00001) SPTW = 0.0	SNOW0135
C CALCULATION OF LIQUID-WATER-HOLDING CAPACITY	SNOW0136
SPLWC = SPBFLW*SPTW	SNOW0137
IF(SFMD .GT. 0.6) SPLWC = SPBFLW*(3.0 - 3.33*SFMD)*SPTW	SNOW0138
IF(SPLWC .LT. 0.0) SPLWC = 0.0	SNOW0139
C ACCOUNTING OF MELT WATER AND RAIN	SNOW0140
IF((SPLW + HSM + PRH) .GT. (SPLWC + TANSM)) GO TO 123	SNOW0141
GO TO 124	SNOW0142
123 PRH = HSM + PRH + SPLW - SPLWC - TANSM	SNOW0143
SPLW = SPLWC	SNOW0144
SPTW = SPTW + TANSM	SNOW0145
TANSM = 0.0	SNOW0146
GO TO 127	SNOW0147
124 IF((HSM + PRH) .LE. TANSM) GO TO 126	SNOW0148
125 SPTW = SPTW + TANSM	SNOW0149
SPLW = SPLW + HSM + PRH - TANSM	SNOW0150
PRH = 0.0	SNOW0151
TANSM = 0.0	SNOW0152
GO TO 127	SNOW0153
126 TANSM = TANSM - HSM - PRH	SNOW0154
SPTW = SPTW + HSM + PRH	SNOW0155
PRH = 0.0	SNOW0156
127 CONTINUE	SNOW0157
HSM = 0.0	SNOW0158
C CALCULATION OF DENSITY AND ADJUSTMENT OF OVERLAND FLOW TIME	SNOW0159
IF(SDEPTH .LE. 0.0 .OR. SPTW .GE. SDEPTH) GO TO 128	SNOW0160
STMD = (SPTW + SPLW)/SDEPTH	SNOW0161

SFMD = SPTW/SDEPTH	SNOW0162
OFRF = 0.33*SOFRF	SNOW0163
IF(SPTW .LE. SPTWCC) OFRF = (1.0 - (SPTW/SPTWCC)*0.67)*SOFRF	SNOW0164
128 IF(SDEPTH .LE. 0.0) OFRF = SOFRF	SNOW0165
OFRFIS = SOFRFI*OFRF/SOFRF	SNOW0166
C CALCULATION OF GROUND MELT	SNOW0167
IF(HOUR .NE. 12 .OR. SPTW .LE. 0.0) RETURN	SNOW0168
IF(SPTW .LE. DSMGH) GO TO 129	SNOW0169
PRH = PRH + DSMGH	SNOW0170
SPTW = SPTW - DSMGH	SNOW0171
IF(STMD .LT. 0.50 .AND. SDEPTH .GT. 2.0*DSMGH) SDEPTH = SDEPTH -	SNOW0172
1 2.0*DSMGH	SNOW0173
RETURN	SNOW0174
129 PRH = SPTW + PRH + SPLW	SNOW0175
TANSM = 0.0	SNOW0176
RIGD = 0.0	SNOW0177
SPLW = 0.0	SNOW0178
SDEPTH = 0.0	SNOW0179
SPTW = 0.0	SNOW0180
SAX = 15.0	SNOW0181
OFRF = SOFRF	SNOW0182
OFRFIS = SOFRFI	SNOW0183
RETURN	SNOW0184
END	SNOW0185

APPENDIX B

LISTING OF OPSET

C OPSET MAIN0001  
C A SELF-CALIBRATING VERSION OF THE STANFORD WATERSHED MODEL MAIN0002  
C BASIC LOGIC OF INNER LOOP BASED ON STANFORD WATERSHED MODELS III & IV MAIN0003  
C VERSION OF NOVEMBER 12, 1970 MAIN0004  
DIMENSION BTRI(99), CONOPT(5), CTRI(99), DRGPM(366), DRHP(366,24), MAIN0005  
1 DRSGP(366), DPET(366), DRSF(366), DSSF(366), EMGWS(12), MAIN0006  
2 EMIFS(12), EMLZS(12), EMSIAM(12), EMUZC(12), EMUZS(12), MAIN0007  
3 EPCM(12), HBF(5), IDYB(5), IDYE(5), IHRB(5), IHRE(5), KPSH(5), MAIN0008  
4 LSHA(5), MEDCY(12), MEDWY(12), RHPD(5), RHPF(5), RHPH(5), MAIN0009  
5 RSBBF(20),RSBD(20),RSBIF(20),SBFRS(3,20),SIFRS(3,20),SSR(5,170), MAIN0010  
6 THSF(24), TITLE(20), TMBF(12), TMIF(12), TMNET(12), TMOF(12), MAIN0011  
7 TMPET(12), TMPREC(12), TMRTF(12), TMSE(12), TMSTF(12), MAIN0012  
8 TMSTFI(12), UHFA(99), XMPFT(12) MAIN0013  
LOGICAL LBMIR, LBUZC, LETLF, LLZC, LNPR, LRC, LSHA, LSHP MAIN0014  
INTEGER CN, CONOPT, DATE, DAY, DPY, EHSGD, HOUR, HRF, HRL, PDAY, MAIN0015  
1 PRD, RHPD, RHPH, RSBD, SGMD, SGRT, SGRT2, TRIP, YEAR, YR1, MAIN0016  
2 YR2 MAIN0017  
REAL IFPRC, IFRC, IFRL, IFS, LZC, LZRX, LZS, LZSR, MNRD, NHPT MAIN0018  
DATA MEDCY/ 0, 31,59,90,120,151,181,212,243,273,304,334/ MAIN0019  
DATA MEDWY/304,334,365,31,59,90,120,151,181,212,243,273 / MAIN0020  
C SPECIFY NUMBER OF STATION-YEARS INCLUDED IN COMPUTER RUN MAIN0021  
NSYC = 0 MAIN0022  
CALL READ(NSYT) MAIN0023  
100 NSYC = NSYC + 1 MAIN0024  
C READ TITLE TO COMPUTER RUN MAIN0025  
READ (5,1) TITLE MAIN0026  
1 FORMAT(20A4) MAIN0027  
C READ CONTROL OPTIONS MAIN0028  
DO 101 KRD = 1,3 MAIN0029

```

101 CALL READ(CONOPT(KRD))
    CALL READ(MNRC,NFTR,NLTR)
C READ BASIC TIME-AREA HISTOGRAM
    DO 102 KIA = 1,99
        BTRI(KIA) = 0.0
102 UHFA(KIA) = 0.0
    CALL READ(NBTRI)
    DO 103 KRD = 1,NBTRI
103 CALL READ(BTRI(KRD))
C SET INITIAL CONDITIONS
    IFT = 1
    LRC = .TRUE.
    LLZC = .FALSE.
    LBUZC = .FALSE.
    LBMIR = .FALSE.
    LETLF = .FALSE.
    LNPR = .FALSE.
    IF(CONOPT(2) .EQ. 0 .AND. NBTRI .LE. 6) LNPR = .TRUE.
    KRC = 1
    KBRC = 0
    KFFC = 0
    SSSQM = 950.0
    SGRT = 0
C READ FIXED PARAMETERS
104 CALL READ(RMPF,CHCAP)
    CALL READ(RGPMB,AREA,FIMP,FWTR)
    CALL READ(VINTMR,SUBWF,GWETF,DESS,DEMN,DEMNIS,OFSL,DIV)
C CALCULATE CONSTANTS SET BY FIXED PARAMETERS
    FPER = 1.0 - FIMP - FWTR
    IF(FPER .GT. 0.01) GO TO 105
    TPLR = 100.0
    FPER = 0.01
    GO TO 106
105 TPLR = (1.0 - FWTR)/FPER
106 VWIN = 26.8888*AREA
    WCFS = 24.0*VWIN

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MAIN0030
MAIN0031
MAIN0032
MAIN0033
MAIN0034
MAIN0035
MAIN0036
MAIN0037
MAIN0038
MAIN0039
MAIN0040
MAIN0041
MAIN0042
MAIN0043
MAIN0044
MAIN0045
MAIN0046
MAIN0047
MAIN0048
MAIN0049
MAIN0050
MAIN0051
MAIN0052
MAIN0053
MAIN0054
MAIN0055
MAIN0056
MAIN0057
MAIN0058
MAIN0059
MAIN0060
MAIN0061
MAIN0062
MAIN0063
MAIN0064
MAIN0065

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```

RHFMC = 0.025/WCFS
SSRT = SQRT(OFSS)
QFRF = 1020.0*SSRT/(OFMN*OFSL)
QFRFIS = 1020.0*SSRT/(OFMNIS*OFSL)
EQDF = 0.00982*((OFMN*OFSL/SSRT)**0.6)
EQDFIS = 0.00982*((OFMNIS*OFSL/SSRT)**0.6)
RGPM = RGPMB
C READ WATER YEAR
  CALL READ(YR1,YR2)
  DPY = 365
  IF(MOD(YR2,4) .EQ. 0) DPY = 366
C READ EVAPORATION DATA
  IF(CONOPT(1) .NE. 1) GO TO 111
  DO 107 KRD = 274,360,10
107 CALL READ(DPET(KRD))
  DO 108 KRD = 1,273,10
108 CALL READ(DPET(KRD))
  DO 110 IDAY2 = 1,9
  DO 109 IDAY1 = 274,360,10
  DAY = IDAY1 + IDAY2
109 DPET(DAY) = DPET(IDAY1)
  DO 110 IDAY1 = 1,273,10
  DAY = IDAY1 + IDAY2
  IF(DAY .GT. 273) GO TO 110
  DPET(DAY) = DPET(IDAY1)
110 CONTINUE
  DPET(366) = DPET(59)
  DPET(365) = DPET(363)
  DPET(364) = DPET(363)
  GO TO 113
111 IF(CONOPT(1) .EQ. 2) GO TO 116
  DAY = 274
112 CALL READ (DPET(DAY))
  IF(DAY .EQ. 273) GO TO 113
  CALL DAYNXT(DAY, DPY)
  GO TO 112

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MAIN0066
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MAIN0070
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MAIN0100
MAIN0101

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113 DO 114 MONTH = 1,12	MAIN0102
114 CALL READ(EPCM(MONTH))	MAIN0103
DO 115 DAY = 1,DPY	MAIN0104
115 EPAET = EPAET + DPET(DAY)	MAIN0105
IF(EPCM(6) .NE. 1.0) EPAET = 0.7*EPAET	MAIN0106
GO TO 117	MAIN0107
116 CALL READ(EPAET,MNRD)	MAIN0108
EMAET = EPAET*(365.0 + MNRD)/404.0	MAIN0109
CALL EVPDAY(DPET,EMAET)	MAIN0110
C READ DAILY FLOW DATA	MAIN0111
117 DRSF(366) = 0.0	MAIN0112
118 DAY = 274	MAIN0113
119 CALL READ(DRSF(DAY))	MAIN0114
CALL DAYNXT(DAY,DPY)	MAIN0115
IF(DAY .NE. 274) GO TO 119	MAIN0116
IF(DIV .EQ. 0.0) GO TO 122	MAIN0117
DO 121 DAY = 1,DPY	MAIN0118
IF(DRSF(DAY) .GT. DIV) GO TO 120	MAIN0119
DRSF(DAY) = 0.0	MAIN0120
GO TO 121	MAIN0121
120 DRSF(DAY) = DRSF(DAY) - DIV	MAIN0122
121 CONTINUE	MAIN0123
122 WRITE(6,2) (TITLE(KTA), KTA = 1,20)	MAIN0124
2 FORMAT(1H1,25X,20A4)	MAIN0125
C WRITE DAILY FLOWS	MAIN0126
CALL DAYSUM(DRSF,MEDCY,DPY,RATFV,TMRTF)	MAIN0127
WRITE(6,3)	MAIN0128
3 FORMAT(1H0,42X,'RECORDED FLOWS')	MAIN0129
CALL DAYOUT(DRSF,MEDWY,DPY)	MAIN0130
WRITE(6,4) (TMRTF(KWD), KWD = 1,12),RATFV	MAIN0131
4 FORMAT(6X,'TOTAL',2X,12F8.1,2X,F10.1,2X,3HSFD)	MAIN0132
C READ STORM HYDROGRAPH DATA	MAIN0133
CALL READ(NRHP,NHPT)	MAIN0134
IF(NRHP .EQ. 0) GO TO 124	MAIN0135
DO 123 KRDP = 1,NRHP	MAIN0136
CALL READ(RHPD(KRDP),RHPH(KRDP),RHPF(KRDP))	MAIN0137

123	WRITE(6,5) KRD,NHPT,RHPD(KRD),RHPH(KRD),RHPF(KRD)	MAIN0138
5	FORMAT(/5X,'RECORDED HYDROGRAPH',I3/10X,'HYDROGRAPH INTERVAL =',	MAIN0139
	1F5.2,1X,5HHOURS/10X,'CALENDAR DAY OF PEAK =',I5,5X,'HOUR OF DAY ='	MAIN0140
	2,I4,5X,'PEAK FLOW =',F8.1,1X,3HCFS)	MAIN0141
C	INITIALIZE PRECIPITATION DATA ARRAYS	MAIN0142
124	DO 125 DAY = 1,366	MAIN0143
	DRGPM(DAY) = RGPMB	MAIN0144
	DSSF(DAY) = 0.0	MAIN0145
	DRSGP(DAY) = 0.0	MAIN0146
	DO 125 HOUR = 1,24	MAIN0147
125	DRHP(DAY,HOUR) = 0.0	MAIN0148
C	READ AUXILIARY RAIN GAGE DAILY TOTALS	MAIN0149
	CALL READ(NSGRD)	MAIN0150
	IF(NSGRD.EQ.0) GO TO 127	MAIN0151
	CALL READ(WSG,SGRT)	MAIN0152
	IF(CONOPT(3).EQ.1) CALL READ(WSG2,SGRT2,SGMD)	MAIN0153
	DO 126 KRD = 1,NSGRD	MAIN0154
	CALL READ(ISGRD)	MAIN0155
126	CALL READ(DRSGP(ISGRD))	MAIN0156
C	READ RECORDING RAIN GAGE HOURLY TOTALS	MAIN0157
127	CALL READ(IWBG,YEAR,MONTH,DATE,CN)	MAIN0158
C	PUNCH NO NUMBERS AFTER CN ON YEAR.EQ.98 CARD	MAIN0159
	IF(YEAR.GE.98) GO TO 130	MAIN0160
	HRF = 12*(CN-1) + 1	MAIN0161
	HRL = 12*(CN-1) + 12	MAIN0162
	DAY = MECCY(MONTH) + DATE	MAIN0163
	DO 128 HOUR = HRF,HRL	MAIN0164
128	CALL READ(DRHP(DAY,HOUR))	MAIN0165
	IF(DPY.NE.366.OR.MONTH.NE.2.OR.DATE.NE.29) GO TO 127	MAIN0166
	DO 129 HOUR = HRF,HRL	MAIN0167
	DRHP(366,HOUR) = DRHP(60,HOUR)	MAIN0168
129	DRHP(60,HOUR) = 0.0	MAIN0169
	GO TO 127	MAIN0170
C	CALCULATE PRECIPITATION WEGHTING FACTORS	MAIN0171
130	IF(NSGRD.EQ.0) GO TO 137	MAIN0172
	PDAY = 274	MAIN0173

	RDPT = 0.0	MAIN0174
	DAY = 274	MAIN0175
131	EHS GD = SGRT	MAIN0176
	IF(SGRT .EQ. 0) EHS GD = 24	MAIN0177
	EHS GDF = EHS GD	MAIN0178
132	CONTINUE	MAIN0179
	DO 136 HOUR = 1,24	MAIN0180
	RDPT = RDPT + DRHP(DAY,HOUR)	MAIN0181
	IF(HOUR .NE. EHS GD) GO TO 136	MAIN0182
	IF(RDPT .LE. 0.0) GO TO 133	MAIN0183
	IF(SGRT .EQ. 0) PDAY = DAY	MAIN0184
	DRGPM (PDAY) = (DRSGP(DAY)*WSG + RDPT*(1.0 - WSG))/RDPT	MAIN0185
	IF(CONOPT(1) .NE. 0) DPET(PDAY) = 0.5*DPET(PDAY)	MAIN0186
	IF(SGRT .NE. 0) PDAY = DAY	MAIN0187
	RDPT = 0.0	MAIN0188
	GO TO 136	MAIN0189
133	IF(DRSGP(DAY) .LE. 0.0) GO TO 135	MAIN0190
	DO 134 K HOUR = 1,EHS GD	MAIN0191
134	DRHP(DAY,K HOUR) = (WSG*DRSGP(DAY))/EHS GDF	MAIN0192
135	IF(SGRT .NE. 0) PDAY = DAY	MAIN0193
136	CONTINUE	MAIN0194
	CALL DAYNXT(DAY,DPY)	MAIN0195
	IF(DAY .EQ. 274) GO TO 137	MAIN0196
	IF(CONOPT(3) .EQ. 0) GO TO 132	MAIN0197
	IF(DAY .NE. SGMD) GO TO 132	MAIN0198
	WSG = WSG2	MAIN0199
	SGRT = SGRT2	MAIN0200
	GO TO 131	MAIN0201
C	ADJUST RAINFALL ANOMALIES	MAIN0202
137	MXTRH = 2*NBTRI	MAIN0203
	IF(CONOPT(2) .EQ. 0) MXTRH = (2*NBTRI - 1)/4 + 1	MAIN0204
	NATRH = MXTRH/2	MAIN0205
	IF(NFTR .GE. 2) GO TO 138	MAIN0206
	IF(NATRH .LT. 12) CALL PRECHK(DRGPM,DRHP,DRSF,VWIN,SGRT,NATRH)	MAIN0207
C	SET INITIAL VALUES OF VARIABLE PARAMETERS TO BE OPTIMIZED	MAIN0208
	LZC = 12.0	MAIN0209



	BMIR = 1.2	MAIN0210
	SUZC = 1.3	MAIN0211
	ETLF = 0.25	MAIN0212
	BUZC = 1.50	MAIN0213
	SIAC = 0.90	MAIN0214
	BIVF = 0.90	MAIN0215
138	IF(NFTR .EQ. 3) GO TO 139	MAIN0216
	SRX = 0.98	MAIN0217
	NCTRI = NBTRI	MAIN0218
	CSRX = SRX	MAIN0219
	FSRX = SRX	MAIN0220
	CALL RECESS(DRSF,DPY,BFRC,IFRC,AREA,RSBD,RSBIF,NRS,RSBBF)	MAIN0221
	IF(IFRC .GE. 0.3) GO TO 139	MAIN0222
	WRITE(6,6) IFRC	MAIN0223
6	FORMAT(/10X,'REJECTED IFRC =',F8.4)	MAIN0224
	IFRC = 0.1	MAIN0225
	BIVF = 0.0	MAIN0226
139	IF(NFTR .GE. 2) CALL READ(LZC, BMIR, SUZC, ETLF, BUZC, SIAC, BIVF, LZS)	MAIN0227
	IF(NFTR .EQ. 3) CALL READ(CSRX,FSRX,NCTRI,CHCAP,IFRC,BFRC)	MAIN0228
140	BFHRC = BFRC**((1.0/24.0)	MAIN0229
	BFRL = -ALOG(BFHRC)	MAIN0230
	CALL FIXTRI(CTRI,BTRI,NBTRI,NCTRI)	MAIN0231
	TRIP = NFTR	MAIN0232
	SRX = CSRX	MAIN0233
	KHYD = 1	MAIN0234
	LSHP = .FALSE.	MAIN0235
	DO 141 KIA = 1,5	MAIN0236
	KPSH(KIA) = 0	MAIN0237
141	HBF(KIA) = 0.0	MAIN0238
C	POINT OF RETURN FOR NEW TRIP	MAIN0239
142	IF(KRC .LE. 5) FTX = 1.0	MAIN0240
	IF(DPY .EQ. 366) MEDWY(5) = 366	MAIN0241
	PPH = 1.0	MAIN0242
	IF(.NOT. LRC) PPH = 3.0	MAIN0243
	IF(TRIP .NE. 1) PPH = 4.0	MAIN0244
	IPPH = PPH	MAIN0245

FHPP = 1.0/PPH	MAIN0246
IFPRC = IFRC** (FHPP/24.0)	MAIN0247
IFRL = -ALOG(IFPRC)	MAIN0248
VINTCR = FHPP*VINTMR	MAIN0249
NCTRH = NCTRI	MAIN0250
IF(CONOPT(2) .EQ. 0) NCTRH = (NCTRI - 1)/4 + 1	MAIN0251
C DETERMINE STORM HOURS FOR ADJUSTING HYDROGRAPH SHAPE VARIABLES	MAIN0252
IF(NRHP .NE. 0 .AND. TRIP .EQ. 2) CALL STRHRS(RHPD,RHPH,IOYB,	MAIN0253
1 IDYE,IHRB,IHRE,NHPT,MXTRH,DPY,NRHP,IBTPR)	MAIN0254
HSE = 0.0	MAIN0255
NRTRI = 0	MAIN0256
PEAI = 0.0	MAIN0257
SPIF = 0.0	MAIN0258
OFUS = 0.0	MAIN0259
OFUSIS = 0.0	MAIN0260
RHFO = 0.0	MAIN0261
URHF = 0.0	MAIN0262
AMIF = 0.0	MAIN0263
AMNET = 0.0	MAIN0264
AMPET = 0.0	MAIN0265
AMPREC = 0.0	MAIN0266
AMBF = 0.0	MAIN0267
AMSE = 0.0	MAIN0268
KRS = 1	MAIN0269
KDRS = 400	MAIN0270
UZS = 0.0	MAIN0271
IFS = 0.0	MAIN0272
IF(NFTR .GE. 2) GO TO 145	MAIN0273
IF(KRC .NE. 1) GO TO 143	MAIN0274
BYLZS = 6.00	MAIN0275
LZS = BYLZS	MAIN0276
GO TO 145	MAIN0277
143 IF(EMLZS(11) .LT. LZS) LZS = EMLZS(11)	MAIN0278
LZS = LZS*LZC/PLZC	MAIN0279
IF(LLZC) LZS = LZC - (LZC-LZS)*(SAFEV/RATEV)	MAIN0280
IF(ABS(FTX - 1.0) .LT. 0.02) GO TO 144	MAIN0281

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LZS = FTX*BBYLZS*LZC/BLZC
IF(LRC .AND. LZC-LZS .LT. 2.0) LZC = LZS + 2.0
144 IF(TRIP .EQ. 3 .OR. KFFC .EQ. 1) LZS = BBYLZS
KFFC = 0
145 OCT1BF = 0.05*TMRTF(1)
IF(DRSF(274) .LT. 0.05*TMRTF(1)) OCT1BF = DRSF(274)
IF(DRSF(276) .LT. OCT1BF*BFRC**2) OCT1BF = DRSF(276)/BFRC**2
BYGWS = OCT1BF/(WCFS*BFRL*SQRT(BFRC))
GWS = BYGWS
BYLZS = LZS
BFNX = GWS*BFRL
TFCFS = BFNX*WCFS
WRITE(6,7) TRIP,LZC,BMIR,SUZC,ETLF,BUZC,SIAC,BIVF,BFRC,IFRC,
ICSRX,FSRX,NCTRI,CHCAP
7 FORMAT(1H1,3X,'TRIAL RUN NUMBER',I3/5X,'PARAMETER VALUES'/10X,
1 5HLZC =,3X,F8.4,2X,6HBMIR =,2X,F8.4,2X,6HSUZC =,2X,F8.4,2X,
2 6HETLF =,2X,F8.4,2X,6HBUZC =,2X,F8.4,2X,6HSIAC =,2X,F8.4/10X,
3 6HBIVF =,2X,F8.4,2X,6HBFRC =,2X,F8.4,2X,6HIFRC =,2X,F8.4,2X,
4 6HCSRX =,2X,F8.4,2X,6HFSRX =,2X,F8.4,2X,7HNCTRI =,1X,I8/10X,
5 7HCHCAP =,1X,F8.0)
WRITE(6,8) LZS,GWS
8 FORMAT(/5X,'INITIAL MOISTURE STORAGES, LZS =',F9.4,5X,'GWS =',
1 F9.4)
AETX = 24.0*EPAET/365.0
AEX96 = 1.2*AETX
AEX90 = 0.3*AETX
SIAM = 1.2**SIAC
UZC = SUZC*AEX90 + BUZC*EXP(-2.7*LZS/LZC)
IF(UZC .LT. 0.25) UZC = 0.25
MONTH = 1
MDAY = 273
IF(TRIP .EQ. 1) GO TO 147
146 WRITE(6,9) (TITLE(KTA), KTA=1,20)
9 FORMAT(25X,20A4)
WRITE(6,10) YR1,YR2
10 FORMAT(03X,61HOPTIMIZATION OF MODEL INPUT PARAMETERS BASED ON WATER

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MAIN0317

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1R YEAR 19,I2,IH-,I2)	MAIN0318
WRITE(6,11)	MAIN0319
11 FORMAT(8H OCTOBER)	MAIN0320
C BEGIN DAY LOOP	MAIN0321
147 DAY = 274	MAIN0322
148 CONTINUE	MAIN0323
IF(TRIP .NE. 1) GO TO 149	MAIN0324
KDRS = KDRS + 1	MAIN0325
IF(RSBD(KRS) .NE. DAY) GO TO 149	MAIN0326
KDRS = 1	MAIN0327
KRS = KRS + 1	MAIN0328
149 CONTINUE	MAIN0329
ADIF = 0.0	MAIN0330
ADBF = 0.0	MAIN0331
TDSF = 0.0	MAIN0332
PET = DPET(DAY)	MAIN0333
IF(CONOPT(1) .NE. 2) PET = PET*EPCM(MONTH)	MAIN0334
PETU = PET	MAIN0335
TFMAX = 0.0	MAIN0336
DO 190 HOUR = 1,24	MAIN0337
IF(TRIP .NE. 2) GO TO 152	MAIN0338
C LOGICAL VARIABLE 'LSHP' SET TRUE DURING DURATION OF RECORDED HYDRO-	MAIN0339
C GRAPH SO SYNTHESIZED DATA MAY BE SAVED DURING CORRESPONDING PERIOD	MAIN0340
IF(KHYD .GT. NRHP) GO TO 152	MAIN0341
IF(IDYB(KHYD) .EQ. DAY .AND. IHRB(KHYD) .EQ. HOUR) LSHP = .TRUE.	MAIN0342
IF(KHYD .GE. NRHP) GO TO 150	MAIN0343
IF(IDYB(KHYD+1) .EQ. DAY .AND. IHRB(KHYD+1) .EQ. HOUR) KHYD =	MAIN0344
1 KHYD + 1	MAIN0345
150 IF(IDYE(KHYD) .NE. DAY .OR. IHRE(KHYD) .NE. HOUR) GO TO 151	MAIN0346
KHYD = KHYD + 1	MAIN0347
LSHP = .FALSE.	MAIN0348
151 IF(.NOT. LSHP) GO TO 152	MAIN0349
KPSH(KHYD) = KPSH(KHYD) + 1	MAIN0350
IF(KPSH(KHYD) .LT. 171) GO TO 152	MAIN0351
WRITE(6,12)	MAIN0352
12 FORMAT(5X, 'FLOOD HYDROGRAPH ARRAY EXCEEDED, SHORTEN NHPT OR SHIFT	MAIN0353

1 TO HOURLY ROUTING)	MAIN0354
GO TO 228	MAIN0355
152 CONTINUE	MAIN0356
IF((NSGRD .EQ. 0) .AND. (DRHP(DAY, HOUR) .NE. 0.0) .AND. (PET .EQ.	MAIN0357
1 PETU) .AND. (CONOPT(1) .NE. 0)) PET = 0.5*PET	MAIN0358
153 IF(HOUR .EQ. SGRT+1) RGPM = DRGPM(DAY)	MAIN0359
IF(HOUR .EQ. 9) HSE = (FWTR*PET)/12.0	MAIN0360
IF(HOUR .EQ. 21) HSE = 0.0	MAIN0361
PRH = RGPM*DRHP(DAY, HOUR)	MAIN0362
AMPREC = AMPREC + PRH	MAIN0363
ARHF = 0.0	MAIN0364
C 15 MIN ACCOUNTING AND ROUTING LOOP (60 MINUTES USED FOR ROUGH	MAIN0365
C ADJUSTMENT, AND 20 MINUTES FOR FINE ADJUSTMENT IN TRIP 1)	MAIN0366
DO 182 PRD = 1, IPPH	MAIN0367
IF(LSHP .AND. CONOPT(2) .EQ. 0 .AND. PRD .NE. 1) KPSH(KHYD) =	MAIN0368
1 KPSH(KHYD) + 1	MAIN0369
PEBI = 0.0	MAIN0370
PPI = 0.0	MAIN0371
OFR = 0.0	MAIN0372
OFRIS = 0.0	MAIN0373
WI = 0.0	MAIN0374
WEIFS = 0.0	MAIN0375
PEP = FHPP*PRH	MAIN0376
IF(TRIP .GE. 2 .AND. LNPR) CALL PREPRD(RGPM, DRHP, DAY, HOUR, DPY, PRD,	MAIN0377
1 PEP, PRH)	MAIN0378
IF(PEP .GT. 0.0) GO TO 155	MAIN0379
IF(OFUS .GT. 0.0) GO TO 157	MAIN0380
IF(IIFS .GT. 0.0) GO TO 167	MAIN0381
IF(TRIP .EQ. 1) GO TO 181	MAIN0382
IF(NRTRI .GT. 0) GO TO 169	MAIN0383
TRHF = 0.0	MAIN0384
IF(.NOT. LSHP) GO TO 154	MAIN0385
KHPT = KPSH(KHYD)	MAIN0386
SSR(KHYD, KHPT) = 0.0	MAIN0387
154 CONTINUE	MAIN0388
IF(RHFO .GT. 0.0) GO TO 178	MAIN0389

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GO TO 181
C RAINFALL UPPER ZONE INTERACTION
155 IF(PEP .GE. VINTCR) GO TO 156
    UZS = UZS + PEP*TPLR
    VINTCR = VINTCR - PEP
    PPI = 0.0
    PEBI = 0.0
    IF(OFUS .GT. 0.0) GO TO 157
    GO TO 167
156 PPI = PEP - VINTCR
    UZS = UZS + VINTCR*TPLR
    VINTCR = 0.0
    LZSR = LZS/LZC
    UZC = SUZC*AEX90 + BUZC*EXP(-2.7*LZSR)
    IF(UZC .LT. 0.25) UZC = 0.25
    UZRX = 2.0*ABS(UZS/UZC - 1.0) + 1.0
    FMR = (1.0/(1.0 + UZRX))*UZRX
    IF(UZS .GT. UZC) FMR = 1.0 - FMR
    PEBI = PPI*FMR
    UZS = UZS + PPI - PEBI
C LOWER ZONE AND GROUNDWATER INFILTRATION
157 LZSR = LZS/LZC
    EID = 4.0*LZSR
    IF(LZSR .LE. 1.0) GO TO 158
    EID = 4.0 + 2.0*(LZSR - 1.0)
    IF(LZSR .LE. 2.0) GO TO 158
    EID = 6.0
158 PEBI = PEBI + OFUS
    CMIR = FHPP*SIAM*BMIR/(2.0**EID)
    CIVM = BIVF*2.0**LZSR
    IF(CIVM .LT. 1.0) CIVM = 1.0
    PEAI = PEBI*PEBI/(2.0*CMIR*CIVM)
    WI = PEBI*PEBI/(2.0*CMIR)
    IF(PEBI .GE. CMIR) WI = PEBI - 0.5*CMIR
    IF(PEBI .GE. CMIR*CIVM) PEAI = PEBI - 0.5*CMIR*CIVM
    WEIFS = WI - PEAI

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IF((PEAI - OFUS) .GT. 0.0) GO TO 159	MAIN0426
EQD = (OFUS + PEA1)/2.0	MAIN0427
GO TO 160	MAIN0428
159 EQD = EQDF*((PEAI - OFUS)**0.6)	MAIN0429
160 IF((OFUS + PEA1) .GT. (2.0*EQD)) EQD = 0.5*(OFUS + PEA1)	MAIN0430
IF((OFUS + PEA1) .LE. 0.001) GO TO 161	MAIN0431
OFR = FHPP*DFRF*((OFUS + PEA1)*0.5)**1.67)*((1.0 + 0.6*((OFUS +	MAIN0432
1 PEA1)/(2.0*EQD))**3.0)**1.67)	MAIN0433
IF(OFR .GT. (0.75*PEAI)) OFR = 0.75*PEAI	MAIN0434
161 IF(FIMP .EQ. 0.0) GO TO 165	MAIN0435
162 PEIS = PPI + OFUSIS	MAIN0436
IF((PEIS - OFUSIS) .GT. 0.0) GO TO 163	MAIN0437
EQDIS = (OFUSIS + PEIS)/2.0	MAIN0438
GO TO 164	MAIN0439
163 EQDIS = EQDFIS*((PEIS - OFUSIS)**0.6)	MAIN0440
164 IF((OFUSIS + PEIS) .GT. (2.0*EQDIS)) EQDIS = 0.5*(OFUSIS + PEIS)	MAIN0441
IF((OFUSIS + PEIS) .LE. 0.01) GO TO 165	MAIN0442
OFRIS = FHPP*DFRFIS*((OFUSIS + PEIS)*0.5)**1.67)*((1.0 + 0.6*((	MAIN0443
1 OFUSIS + PEIS)/(2.0*EQDFIS))**3.0)**1.67)	MAIN0444
IF(OFRIS .GT. PEIS) OFRIS = PEIS	MAIN0445
165 OFUSIS = PEIS - OFRIS	MAIN0446
OFUS = PEA1 - OFR	MAIN0447
IF(OFUS .GE. 0.001) GO TO 166	MAIN0448
LZS = LZS + OFUS	MAIN0449
OFUS = 0.0	MAIN0450
OFRIS = OFRIS + OFUSIS	MAIN0451
OFUSIS = 0.0	MAIN0452
166 LZRX = 1.5*ABS(LZS/LZC - 1.0) + 1.0	MAIN0453
FMR = (1.0/(1.0 + LZRX))**LZRX	MAIN0454
IF(LZS .LT. LZC) FMR = 1.0 - FMR*(LZS/LZC)	MAIN0455
PLZS = FMR*(PEBI - WI)	MAIN0456
PGW = (1.0 - FMR)*(PEBI - WI)*(1.0 - SUBWF)*FPER	MAIN0457
GWS = GWS + PGW	MAIN0458
LZS = LZS + PLZS	MAIN0459
IFS = IFS + WEIFS*FPER	MAIN0460
167 SPIF = IFRL*IFS	MAIN0461

AMIF = AMIF + SPIF	MAIN0462
ADIF = ADIF + SPIF	MAIN0463
IFS = IFS - SPIF	MAIN0464
IF(IFS .GE. 0.0001) GO TO 168	MAIN0465
LZS = LZS + IFS	MAIN0466
IFS = 0.0	MAIN0467
168 UHFA(1) = FPER*QFR + PPI*FWTR + FIMP*QFRIS + SPIF	MAIN0468
IF(TRIP .NE. 1) GO TO 169	MAIN0469
ARHF = ARHF + UHFA(1)	MAIN0470
GO TO 181	MAIN0471
C ROUTING	MAIN0472
169 IF(CONOPT( 2) .NE. 1) GO TO 170	MAIN0473
URHF = URHF + 0.25*UHFA(1)	MAIN0474
IF(PRD .NE. 4) GO TO 178	MAIN0475
UHFA(1) = URHF	MAIN0476
C SAVE SYNTHESIZED DIRECT RUNOFF AND INTERFLOW ENTERING STREAM DURING	MAIN0477
C DURATION OF RECORDED HYDROGRAPH	MAIN0478
170 IF(.NOT. LSHP) GO TO 171	MAIN0479
KHPT = KPSH(KHYD)	MAIN0480
IF(CONOPT(2) .EQ. 1) SSR(KHYD,KHPT) = 4.0*URHF*WCFS	MAIN0481
IF(CONOPT(2) .EQ. 0) SSR(KHYD,KHPT) = 4.0*UHFA(1)*WCFS	MAIN0482
171 CONTINUE	MAIN0483
TRHF = 0.0	MAIN0484
KTRI = NCTRI	MAIN0485
172 URHF = UHFA(KTRI)	MAIN0486
IF(URHF .LE. 0.0) GO TO 174	MAIN0487
173 TRHF = TRHF + URHF*CTRI(KTRI)	MAIN0488
UHFA(KTRI + 1) = URHF	MAIN0489
GO TO 175	MAIN0490
174 UHFA(KTRI+ 1) = 0.0	MAIN0491
175 KTRI = KTRI - 1	MAIN0492
IF(KTRI .GE. 1) GO TO 172	MAIN0493
176 IF(URHF .LE. 0.0) GO TO 177	MAIN0494
NRTRI = NCTRI	MAIN0495
177 NRTRI = NRTRI - 1	MAIN0496
UHFA(1) = 0.0	MAIN0497



URHF = 0.0	MAIN0498
178 IF(TRIP .LE. 2) GO TO 179	MAIN0499
IF(TFCFS .LE. 0.5*CHCAP) SRX = CSRX	MAIN0500
IF(TFCFS .GT. 0.5*CHCAP) .AND. (TFCFS .LT. 2.0*CHCAP) SRX = CSRX	MAIN0501
1. +(FSRX - CSRX)*((TFCFS - 0.5*CHCAP)/(1.5*CHCAP))**3	MAIN0502
IF(TFCFS .GT. 2.0*CHCAP) SRX = FSRX	MAIN0503
179 RHF1 = TRHF - SRX*(TRHF - RHFO)	MAIN0504
RHFO = RHF1	MAIN0505
IF(RHFO .LT. RHFC) RHFO = 0.0	MAIN0506
TFCFS = (4.0*RHF1 + CBF - HSE)*WCFS	MAIN0507
IF(TFCFS .LE. TFMAX) GO TO 180	MAIN0508
PRDF = PRD	MAIN0509
TDFP24 = HOUR	MAIN0510
IF(PRD .LE. 3) TDFP24 = (TDFP24 - 1.0) + 0.15*PRDF	MAIN0511
TFMAX = TFCFS	MAIN0512
180 ARHF = ARHF + RHF1	MAIN0513
181 IF(VINTCR .LT. FHPP*VINTMR) VINTCR = VINTCR + DPET(DAY)/(24.0/	MAIN0514
1 - FHPP)	MAIN0515
182 CONTINUE	MAIN0516
C END OF 15 MINUTE LOOP	MAIN0517
C ADDING GROUNDWATER FLOW	MAIN0518
183 CBF = GWS*BFRL	MAIN0519
IF(KHYD .GT. NRHP) GO TO 184	MAIN0520
IF(LSHP .AND. (HBF(KHYD) .EQ. 0.0)) HBF(KHYD) = CBF*WCFS	MAIN0521
184 GWS = GWS - CBF	MAIN0522
AMBF = AMBF + CBF	MAIN0523
THGR = ARHF + CBF	MAIN0524
C EVAPORATION FROM STREAM SURFACE	MAIN0525
185 IF(HSE .GT. THGR) HSE = THGR	MAIN0526
IF(CBF .GT. HSE) ADBF = ADBF + CBF - HSE	MAIN0527
AMSE = AMSE + HSE	MAIN0528
THSF(HOUR) = (THGR - HSE)*WCFS	MAIN0529
IF(TFMAX .LE. 0.0) TFMAX = THSF(HOUR)	MAIN0530
TDSF = TDSF + THSF(HOUR)	MAIN0531
C DRAINING OF UPPER ZONE STORAGE	MAIN0532
UZINFX = (UZS/UZC) - (LZS/LZC)	MAIN0533

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IF(UZINFX .LE. 0.0) GO TO 186
LZSR = LZS/LZC
UZINLZ = 0.003*BMIR*UZC*UZINFX**3.0
IF(UZINLZ .GT. UZS) UZINLZ = UZS
UZS = UZS - UZINLZ
LZRX = 1.5*ABS(LZSR - 1.0) + 1.0
FMR = (1.0/(1.0 + LZRX))**LZRX
IF(LZS .LT. LZC) FMR = 1.0 - FMR*LZSR
PGW = (1.0-FMR)*UZINLZ*(1.0 - SUBWF)*FPER
PLZS = FMR*UZINLZ
LZS = LZS + PLZS
GWS = GWS + PGW
C 4 PM ADJUSTMENTS OF VARIOUS VALUES
186 IF(HOUR .NE. 16) GO TO 190
AEX90 = 0.9*(AEX90 + PET)
AEX96 = 0.96*(AEX96 + PET)
C INFILTRATION CORRECTION
SIAM = (AEX96/AETX)**SIAC
IF(SIAM .LT. 0.33) SIAM = 0.33
IF(PET .EQ. 0.0) GO TO 190
C EVAP-TRANS LOSS FROM GROUNDWATER
GWET = GWS*GWETF*PET*FPER
GWS = GWS - GWET
AMPET = AMPET + PET
IF(PET .GE. UZS) GO TO 187
UZS = UZS - PET
AMNET = AMNET + PET
GO TO 190
187 PET = PET - UZS
AMNET = AMNET + UZS
UZS = 0.0
LZSR = LZS/LZC
IF(PET .GE. ETLF*LZSR) GO TO 188
SET = PET*(1.0 - PET/(2.0*ETLF*LZSR))
GO TO 189
188 SET = 0.5*ETLF*LZSR

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189 LZS = LZS - SET
    AMNET = AMNET + SET
190 CONTINUE
C END OF HOUR LOOP
    DSSF(DAY) = TDSF/24.0
    IF(TRIP.EQ. 1) GO TO 192
    IF(TFMAX.LE. RMPF) GO TO 192
    IF(DAY.EQ. 366) MDAY = 337
    DATE = MOD(DAY,MDAY)
    WRITE(6,13) DATE, (THSF(HOUR), HOUR=1,12)
13  FORMAT(1H/,1X/,1X,I4,2X,2HAM,1X,6F8.1,3X,6F8.1)
    WRITE(6,14) (THSF(HOUR), HOUR=13,24), DSSF(DAY)
14  FORMAT(1HJ,6X,2HPM,1X,6F8.1,3X,7F8.1)
    IF(TDFP24.LT. 12.0) GO TO 191
    TDFP12 = TDFP24 - 12.0
    WRITE(6,15) TFMAX, TDFP12
15  FORMAT(1H/,10X,8HMAXIMUM=,F8.1,2X,6HC.F.S.,5X,4HTIME,3X,F5.2,2X,
1    4HP.M.)
    GO TO 192
191 WRITE(6,16) TFMAX, TDFP24
16  FORMAT(1H/,10X,8HMAXIMUM=,F8.1,2X,6HC.F.S.,5X,4HTIME,3X,F5.2,2X,
1    4HA.M.)
192 CONTINUE
    IF( (TRIP.EQ. 1 .AND. .NOT. LRC .AND. KDRS.LE. 3 .AND. IFRC.GT.
1    0.1) SIFRS(KDRS,KRS-1) = ADIF*VWIN
    IF( (TRIP.EQ. 1 .AND. KDRS.LE. 3) SBFRS(KDRS,KRS-1) = ADBF*VWIN
C MONTHLY SUMMARY STORAGE
    IF(DAY.NE. MEDWY(MONTH)) GO TO 206
    TMPREC(MONTH) = AMPREC
    AMPREC = 0.0
    TMBF(MONTH) = AMBF
    AMBF = 0.0
    TMIF(MONTH) = AMIF
    AMIF = 0.0
    TMSE(MONTH) = AMSE
    AMSE = 0.0

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TMPET(MONTH) = AMPET
AMPET = 0.0
TMNET(MONTH) = AMNET
AMNET = 0.0
EMGWS(MONTH) = GWS
UZC = SUZC*AEX90 + BUZC*EXP(-2.7*LZS/LZC)
IF(UZC .LT. 0.25) UZC = 0.25
EMUZC(MONTH) = UZC
EMUZS(MONTH) = UZS
EMSIAM(MONTH) = SIAM
EMLZS(MONTH) = LZS
EMIFS(MONTH) = IFS
IF(MONTH .EQ. 5) MEDWY(5) = 59
MDAY = MEDWY(MONTH)
IF(TRIP .EQ. 1) GO TO 205
193 GO TO (194,195,196,197,198,199,200,201,202,203,204,
1 205),MONTH
194 WRITE(6,17)
17 FORMAT(1H/,8HNOVEMBER)
GO TO 205
195 WRITE(6,18)
18 FORMAT(1H/,8HDECEMBER)
GO TO 205
196 WRITE(6,19)
19 FORMAT(1H/,7HJANUARY)
GO TO 205
197 WRITE(6,20)
20 FORMAT(1H/,8HFEBRUARY)
GO TO 205
198 WRITE(6,21)
21 FORMAT(1H/,5HMARCH)
GO TO 205
199 WRITE(6,22)
22 FORMAT(1H/,5HAPRIL)
GO TO 205
200 WRITE(6,23)

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23	FORMAT(1H/,3HMAY)	MAIN0642
	GO TO 205	MAIN0643
201	WRITE(6,24)	MAIN0644
24	FORMAT(1H/,4HJUNE)	MAIN0645
	GO TO 205	MAIN0646
2020	WRITE(6,25)	MAIN0647
25	FORMAT(1H/,4HJULY)	MAIN0648
	GO TO 205	MAIN0649
203	WRITE(6,26)	MAIN0650
26	FORMAT(1H/,6HAUGUST)	MAIN0651
	GO TO 205	MAIN0652
204	WRITE(6,27)	MAIN0653
27	FORMAT(1H/,9HSEPTEMBER)	MAIN0654
205	MONTH = MONTH + 1	MAIN0655
C	END OF DAY LOOP	MAIN0656
206	CALL DAYNXT(DAY,DPY)	MAIN0657
	IF(DAY .NE. 274) GO TO 148	MAIN0658
	IF(TRIP .NE. 2) GO TO 208	MAIN0659
C	ADJUST BASE FLOW FOR AVERAGE VALUE DURING STORM:	MAIN0660
	DO 207 KHYD = 1, NRHP	MAIN0661
	DAY = IDYB(KHYD)	MAIN0662
	IF(DSSF(DAY) .GT. HBF(KHYD)) GO TO 207	MAIN0663
	HBF(KHYD) = (HBF(KHYD) + DSSF(DAY))/2.0	MAIN0664
207	CONTINUE	MAIN0665
208	IF(TRIP .NE. 1) WRITE(6,28) (TITLE(KTA), KTA=1,20,1)	MAIN0666
	28 FORMAT(1H1,25X,20A4)	MAIN0667
C	ANNUAL SUMMARY	MAIN0668
	APREC = 0.0	MAIN0669
	ABFV = 0.0	MAIN0670
	ASEV = 0.0	MAIN0671
	ANET = 0.0	MAIN0672
	APET = 0.0	MAIN0673
	AIFV = 0.0	MAIN0674
	DO 209 MONTH = 1,12	MAIN0675
	APREC = APREC + TMPREC(MONTH)	MAIN0676
	ABFV = ABFV + TMBF(MONTH)	MAIN0677

ASEV = ASEV + TMSE(MONTH)	MAIN0678
ANET = ANET + TMNET(MONTH)	MAIN0679
APET = APET + TMPET(MONTH)	MAIN0680
209 AIFV = AIFV + TMIF(MONTH)	MAIN0681
WRITE(6,29)	MAIN0682
29 FORMAT(1H//44X,23HSYNTHESIZED FLOWS)	MAIN0683
210 IF(TRIP.EQ. 1) WRITE(6,30)	MAIN0684
30 FORMAT(//5X,'SUMMARY WHILE OPTIMIZING VOLUME VARIABLES')	MAIN0685
211 CALL DAYSUM(DSSF,MEDCY,DPY,SATFV,TMSTF)	MAIN0686
IF(TRIP.EQ. 1) GO TO 212	MAIN0687
CALL DAYOUT(DSSF,MEDWY,DPY)	MAIN0688
212 WRITE(6,31) (TMSTF(KWD), KWD=1,12), SATFV	MAIN0689
DO 213 MONTH = 1,12	MAIN0691
31 FORMAT(1X, 9HSYNTHETIC,3X,12F8.1,2X,F10.1,2X,3HSFD)	MAIN0690
213 TMSTFI(MONTH) = TMSTF(MONTH)/VWIN	MAIN0692
SATFVI = SATFV/VWIN	MAIN0693
WRITE(6,32) (TMSTFI(KWD), KWD=1,12), SATFVI	MAIN0694
32 FORMAT(1X,5HTOTAL,8X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN0695
DO 214 MONTH = 1,12	MAIN0696
TMOF(MONTH) = TMSTFI(MONTH) - TMIF(MONTH) - TMBF(MONTH) +	MAIN0697
1 TMSE(MONTH)	MAIN0698
214 IF(TMOF(MONTH) .LT. 0.0) TMOF(MONTH) = 0.0	MAIN0699
AOFV = SATFVI - AIFV - ABFV + ASEV	MAIN0700
IF(AOFV .LT. 0.0) AOFV = 0.0	MAIN0701
WRITE(6,33) (TMOF(KWD), KWD=1,12), AOFV	MAIN0702
33 FORMAT(1X,8HOVERLAND,5X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN0703
WRITE(6,34) (TMIF(KWD), KWD=1,12), AIFV	MAIN0704
34 FORMAT(1X,9HINTERFLOW,4X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN0705
WRITE(6,35) (TMBF(KWD), KWD=1,12), ABFV	MAIN0706
35 FORMAT(1X,4HBASE,9X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN0707
WRITE(6,36) (TMSE(KWD), KWD=1,12), ASEV	MAIN0708
36 FORMAT(1X,9HSTRM EVAP,4X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN0709
WRITE(6,37) (TMPREC(KWD), KWD=1,12), APREC	MAIN0710
37 FORMAT(1X,6HPRECIP,7X,12F8.2,3X,F8.2,2X,6HINCHES)	MAIN0711
WRITE(6,38) (TMNET(KWD), KWD=1,12), ANET	MAIN0712
38 FORMAT(1X,12HEVP/TRAN-NET,2X,12F8.3,3X,F7.3,2X,6HINCHES)	MAIN0713

	WRITE(6,39) ((TMPET(KWD), KWD=1,12), APET	MAIN0714
39	FORMAT(3X,10H-POTENTIAL,2X,12F8.3,3X,F7.3,2X,6HINCHES)	MAIN0715
	WRITE(6,40) ((EMUZS(KWD), KWD=1,12)	MAIN0716
40	FORMAT(1X,12HSTORAGES-UZS,2X,12F8.3,12X,6HINCHES)	MAIN0717
	WRITE(6,41) ((EMLZS(KWD), KWD=1,12)	MAIN0718
41	FORMAT(10X,3HLZS,2X,12F8.3,12X,6HINCHES)	MAIN0719
	WRITE(6,42) ((EMIFS(KWD), KWD=1,12)	MAIN0720
42	FORMAT(10X,3HIFS,2X,12F8.3,12X,6HINCHES)	MAIN0721
	WRITE(6,43) ((EMGWS(KWD), KWD=1,12)	MAIN0722
43	FORMAT(10X,3HGWS,2X,12F8.3,12X,6HINCHES)	MAIN0723
	WRITE(6,44) ((EMUZC(KWD), KWD=1,12)	MAIN0724
44	FORMAT(1X,12HINDICES-UZC,2X,12F8.3)	MAIN0725
	WRITE(6,45) ((EMSIAM(KWD), KWD=1,12)	MAIN0726
45	FORMAT(9X,4HSIAM,2X,12F8.3)	MAIN0727
	AMBER = (LZS - BYLZS)*FPER + (UZS + IFS + GWS - BYGWS)*(1.0 - FWTR	MAIN0728
	1 ) + SATFVI + ANET*FPER + ASEV - APREC	MAIN0729
	WRITE(6,46) AMBER	MAIN0730
46	FORMAT(1H/7H BALANCE,5X,F10.4,2X,6HINCHES)	MAIN0731
C	ESTABLISH WHETHER MONTH IS PREDOMINATELY BASE FLOW OR DIRECT RUNOFF	MAIN0732
	NOFM = 0	MAIN0733
	MONTH1 = 1	MAIN0734
	IF(FTX .LT. 0.95) MONTH1 = 4	MAIN0735
	DO 216 MONTH = 1,12	MAIN0736
	XMPFT(MONTH) = 0.0	MAIN0737
	IF(MONTH .LT. MONTH1) GO TO 216	MAIN0738
	IF(TMSTFI(MONTH) .GT. 0.001) GO TO 215	MAIN0739
	XMPFT(MONTH) = 1.0	MAIN0740
	GO TO 216	MAIN0741
215	IF((TMBF(MONTH)/TMSTFI(MONTH) .GT. 0.5) XMPFT(MONTH) = 1.0	MAIN0742
	IF((TMOF(MONTH)/TMSTFI(MONTH) .LT. 0.5) GO TO 216	MAIN0743
	NOFM = NOFM + 1	MAIN0744
	XMPFT(MONTH) = 2.0	MAIN0745
216	CONTINUE	MAIN0746
C	NATURE OF TRIPS	MAIN0747
C	TRIP 1 OPTIMIZE VOLUME VARIABLES WHILE BYPASSING ROUTING	MAIN0748
C	TRIP 2 SET FLOOD HYDROGRAPH VARIABLES: CSRX,FSRX,NCTRI,CHCAP	MAIN0749

C	TRIP 3 FINAL RUN WITH OPTIMIZED VALUES	MAIN0750
	217 IF(TRIP .EQ. 1) GO TO 218	MAIN0751
	KRC = MNRC + 1	MAIN0752
	IF(TRIP .EQ. 2) GO TO 226	MAIN0753
	GO TO 228	MAIN0754
C	SYSTEMATIC ADJUSTMENT OF VOLUME VARIABLES CONVERGING ON OPTIMUM VALUES	MAIN0755
	218 KRC = KRC + 1	MAIN0756
	KBRC = KBRC + 1	MAIN0757
	PLZC = LZC	MAIN0758
	PBMIR = BMIR	MAIN0759
	PSUZC = SUZC	MAIN0760
	PETLF = ETLF	MAIN0761
	PBUZC = BUZC	MAIN0762
	PSIAC = SIAC	MAIN0763
C	ADJUST FIVE VOLUME VARIABLES: LZC, SUZC, ETLF, BUZC, SIAC	MAIN0764
	CALL SETFVP(LZC, SUZC, ETLF, BUZC, SIAC, TMSTF, TMRTF, TMPREC, TMPET,	MAIN0765
	1 EMLZS, SSQM, LRC, XMPFT, FTX, NOFM, LBUZC, LETLF, LLZC, APREC, APET)	MAIN0766
C	ADJUST INTERFLOW VOLUME CONSTANT DURING FINE ADJUSTMENT PHASE	MAIN0767
	FNCTRH = NCTRH	MAIN0768
	IF(.NOT. LRC .AND. IFRC .GT. 0.1) CALL SETBIV(BIVF, NRS, IFRC, RSBIF,	MAIN0769
	1 SIFRS, FNCTRH)	MAIN0770
C	ADJUST INFILTRATION RATE CONSTANT: BMIR	MAIN0771
	IF(.NOT. LBMIR) GO TO 219	MAIN0772
	BMIR = 0.9*BMIR	MAIN0773
	GO TO 220	MAIN0774
	219 IF(ABS(FTX-1.0) .GT. 0.02 .AND. KRC .GT. 5) IFT = 2	MAIN0775
	CALL SETBMI(BMIR, NRS, BFRC, RSBBF, SBFRS, FNCTRH, IFT)	MAIN0776
	220 IF((KRC .GT. 6) .AND. (LZC .GT. 29.0)) LLZC = .TRUE.	MAIN0777
	IF((KRC .GT. 6) .AND. (ETLF .GT. 0.59)) LETLF = .TRUE.	MAIN0778
	IF((KRC .GT. 6) .AND. (BUZC .GT. 3.9)) LBUZC = .TRUE.	MAIN0779
	IF(.NOT. LLZC) GO TO 221	MAIN0780
	LZC = PLZC*SATEV/RATFV	MAIN0781
	WRITE(6,47) LZC	MAIN0782
	47 FORMAT(/2X, 'LZC WAS CHANGED TO', F6.2, ' BASED ON ANNUAL RUNOFF VOLU	MAIN0783
	1ME')	MAIN0784
	221 IF(KRC .LT. 6 .OR. BMIR .LT. 20.0) GO TO 222	MAIN0785



	LB MIR = .TRUE.	MAIN0786
	BMIR = 20.0	MAIN0787
222	IF(SSSQM .LE. SSQM .AND. ((KRC .GE. MNRC .AND. KBRC .GE. 2) .OR.	MAIN0788
1	(.NOT. LRC))) GO TO 224	MAIN0789
	IF(SSSQM .LE. SSQM) GO TO 142	MAIN0790
	IF(KRC .GE. MNRC) KRC = KRC - 1	MAIN0791
	BLZC = PLZC	MAIN0792
	BBMIR = PB MIR	MAIN0793
	BSUZC = PSUZC	MAIN0794
	BETLF = PETLF	MAIN0795
	BBUZC = PBUZC	MAIN0796
	BSIAC = PSIAC	MAIN0797
	SSSQM = SSQM	MAIN0798
	BBYLZS = BYLZS	MAIN0799
	KBRC = 0	MAIN0800
	IF(SSSQM .LT. 0.15 .AND. LRC) GO TO 223	MAIN0801
	GO TO 142	MAIN0802
223	LRC = .FALSE.	MAIN0803
	WRITE(S,48)	MAIN0804
48	FORMAT('5X, 'SHIFT TO FINE ADJUSTMENT BEGINNING AT BEST ROUGH ADJUSTMENT POINT')	MAIN0805
	SSSQM = 1000.0	MAIN0806
	GO TO 225	MAIN0807
224	CONTINUE	MAIN0808
	IF(LRC) GO TO 223	MAIN0809
	IF(TRIP .GE. NLTR) GO TO 228	MAIN0810
	TRIP = TRIP + 1	MAIN0811
225	LZC = BEZC	MAIN0812
	BMIR = BBMIR	MAIN0813
	SUZC = BSUZC	MAIN0814
	ETLF = BETLF	MAIN0815
	BUZC = BBUZC	MAIN0816
	SIAC = BSIAC	MAIN0817
	KFFC = 1	MAIN0818
	GO TO 142	MAIN0819
226	IF(NRHP .EQ. 0) GO TO 227	MAIN0820
		MAIN0821

C	CORRECT SYNTHESIZED RUNOFF TO RECORDED VOLUMES	MAIN0822
	CALL ADJHYD(IDYB, IDYE, IHRB, IHRE, KPSH, DPY, HBF, NRHP, DSSF, DRSF, SSR,	MAIN0823
	1 LSHA)	MAIN0824
C	ESTABLISH STORM AND OVERALL OPTIMUM VALUES FOR SRX AND NCTRI	MAIN0825
	CALL SETHRP(CTRI, BTRI, WCFS, CONOPT(2), HBF, LSHA, SSR, NHPT, KPSH,	MAIN0826
	1 IBTPR, SRX, CSRX, FSRX, CHCAP, NRHP, RHPF, NCTRI, NBTRI)	MAIN0827
	IF(NCTRI .EQ. 0) GO TO 228	MAIN0828
227	IF(TRIP .GE. NLTR) GO TO 228	MAIN0829
	TRIP = TRIP + 1	MAIN0830
	GO TO 142	MAIN0831
228	CONTINUE	MAIN0832
	IF(NSYC .LT. NSYT) GO TO 100	MAIN0833
	END	MAIN0834

	SUBROUTINE ADJHYD(IDYB, IDYE, IHRB, IHRE, KPSH, DPY, HBF, NRHP, DSSF,	ADJH0001
	1 DRSF, SSR, LSHA)	ADJH0002
C	ADJUSTS SYNTHESIZED FLOW VOLUME TO MATCH RECORDED VOLUME FOR SETTING	ADJH0003
C	HYDROGRAPH ROUTING PARAMETERS	ADJH0004
	DIMENSION IDYB(5), IDYE(5), IHRB(5), IHRE(5), KPSH(5), SSR(5,170),	ADJH0005
	1 DSSF(366), DRSF(366), HBF(5), LSHA(5)	ADJH0006
	LOGICAL LSHA, LSHP	ADJH0007
	INTEGER DAY, DPY	ADJH0008
	LSHP = .FALSE.	ADJH0009
	KRHP = 1	ADJH0010
	DAY = 274	ADJH0011
100	CONTINUE	ADJH0012
	IF(LSHP) GO TO 102	ADJH0013
	IF(IDYB(KRHP) .NE. DAY) GO TO 107	ADJH0014
101	HTH = IHRB(KRHP)	ADJH0015
	HBFM = 1.0	ADJH0016
	IF(DSSF(DAY) .LT. HBF(KRHP)) HBFM = 0.0	ADJH0017
	TSHV = (24.0 - HTH)*(DSSF(DAY) - HBF(KRHP))*HBFM/24.0	ADJH0018

```

    HBFM = 1.0
    IF(DRSF(DAY) .LT. HBF(KRHP)) HBFM = 0.0
    TRHV = (24.0 - HTH)*(DRSF(DAY) - HBF(KRHP))*HBFM/24.0
    IF(IDYE(KRHP) .EQ. DAY) GO TO 104
    LSHP = .TRUE.
    GO TO 107
102 IF(DSSF(DAY) .LT. HBF(KRHP)) GO TO 103
    TSHV = TSHV + DSSF(DAY) - HBF(KRHP)
103 IF(DRSF(DAY) .LT. HBF(KRHP)) GO TO 104
    TRHV = TRHV + DRSF(DAY) - HBF(KRHP)
104 CONTINUE
    IF(IDYE(KRHP) .NE. DAY) GO TO 107
    HTH = IHRE(KRHP)
    TSHV = TSHV - (24.0 - HTH)*(DSSF(DAY) - HBF(KRHP))/24.0
    TRHV = TRHV - (24.0 - HTH)*(DRSF(DAY) - HBF(KRHP))/24.0
    LSHP = .FALSE.
    SHM = TRHV/TSHV
    LSHA(KRHP) = .TRUE.
    IF(SHM .GT. 8.0 .OR. SHM .LT. 0.125) LSHA(KRHP) = .FALSE.
    IF(.NOT. LSHA(KRHP)) GO TO 106
    KPCH = KPSH(KRHP)
    DO 105 KHPT = 1, KPCH
105 SSR(KRHP, KHPT) = SHM*SSR(KRHP, KHPT)
106 WRITE(6,1) KRHP, SHM
    1 FORMAT(//10X, 'VOLUME ADJUSTMENT FACTOR FOR HYDROGRAPH', I2,
    1 ' EQUALS', F10.4)
    KRHP = KRHP + 1
    IF(KRHP .GT. NRHP) RETURN
    IF(IDYB(KRHP) .EQ. IDYE(KRHP-1)) GO TO 101
107 CALL DAYNXT(DAY, DPY)
    IF(DAY .NE. 274) GO TO 100
    RETURN
    END

```

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ADJH0019
ADJH0020
ADJH0021
ADJH0022
ADJH0023
ADJH0024
ADJH0025
ADJH0026
ADJH0027
ADJH0028
ADJH0029
ADJH0030
ADJH0031
ADJH0032
ADJH0033
ADJH0034
ADJH0035
ADJH0036
ADJH0037
ADJH0038
ADJH0039
ADJH0040
ADJH0041
ADJH0042
ADJH0043
ADJH0044
ADJH0045
ADJH0046
ADJH0047
ADJH0048
ADJH0049
ADJH0050
ADJH0051

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<pre> SUBROUTINE DAYSUM(DRSF, MEDCY, DPY, ATFV, TMTFWY) C SUMS DAILY VALUES TO GET MONTHLY AND ANNUAL TOTALS   DIMENSION DRSF(366), EMATF(13), MEDCY(12), TMTFCY(12), TMTFWY(12)   INTEGER DAY, DPY C SUM ANNUAL AND CUMULATIVE MONTHLY FLOWS   EMATF(1) = 0.0   ATF = 0.0   DO 101 DAY = 1, 365     ATF = ATF + DRSF(DAY)   DO 100 KMO = 2, 12 100 IF(DAY .EQ. MEDCY(KMO)) EMATF(KMO) = ATF 101 CONTINUE     EMATF(13) = ATF     ATFV = ATF + DRSF(366) C CALCULATE MONTHLY FLOWS   DO 102 KMO = 1, 12 102 TMTFCY(KMO) = EMATF(KMO + 1) - EMATF(KMO)     TMTFCY(2) = TMTFCY(2) + DRSF(366) C CONVERT MONTHLY FLOWS TO A WATER YEAR ORDER   DO 103 KMO = 1, 9 103 TMTFWY(KMO+3) = TMTFCY(KMO)   DO 104 KMO = 10, 12 104 TMTFWY(KMO-9) = TMTFCY(KMO)   RETURN   END </pre>	<pre> DYSM0001 DYSM0002 DYSM0003 DYSM0004 DYSM0005 DYSM0006 DYSM0007 DYSM0008 DYSM0009 DYSM0010 DYSM0011 DYSM0012 DYSM0013 DYSM0014 DYSM0015 DYSM0016 DYSM0017 DYSM0018 DYSM0019 DYSM0020 DYSM0021 DYSM0022 DYSM0023 DYSM0024 DYSM0025 </pre>
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<pre> SUBROUTINE FIXTRI(CTRI, BTRI, NBTRI, NCTRI) C FIX VALUES OF THE TIME ROUTING INCREMENTS TO MATCH REQUIRED TOTAL C NUMBER OF VALUES   DIMENSION AWSBIT(99), BTRI(99), CTRI(99)   IF(NCTRI .GT. 99) GO TO 101   IF(NBTRI .NE. NCTRI) GO TO 102   DO 100 KRDI = 1, 99 100 CTRI(KRDI) = BTRI(KRDI) </pre>	<pre> FXTI0001 FXTI0002 FXTI0003 FXTI0004 FXTI0005 FXTI0006 FXTI0007 FXTI0008 </pre>
---	--

RETURN	FXTI0009
101 WRITE(6,1) NCTRI	FXTI0010
1 FORMAT(5X,'NCTRI OF',I5,IX,'EXCEEDS MAXIMUM VALUE OF 99, 99 USED')	FXTI0011
NCTRI = 99	FXTI0012
102 DO 103 KIA = 1,99	FXTI0013
103 AWSBIT(KIA) = 0.0	FXTI0014
FNTRI = NCTRI	FXTI0015
KB1 = 0	FXTI0016
KB2 = 1	FXTI0017
KB3 = 0	FXTI0018
104 KB1 = KB1 + 1	FXTI0019
IF(KB1 .GT. NBTRI) GO TO 107	FXTI0020
KB4 = 0	FXTI0021
WSBIT = BTRI(KB1)/FNTRI	FXTI0022
105 KB4 = KB4 + 1	FXTI0023
IF(KB4 .GT. NCTRI) GO TO 104	FXTI0024
AWSBIT(KB2) = AWSBIT(KB2) + WSBIT	FXTI0025
KB3 = KB3 + 1	FXTI0026
IF(KB3 .LT. NBTRI) GO TO 106	FXTI0027
KB3 = 0	FXTI0028
KB2 = KB2 + 1	FXTI0029
106 GO TO 105	FXTI0030
107 DO 108 KB5 = 1,99	FXTI0031
108 CTRI(KB5) = AWSBIT(KB5)	FXTI0032
RETURN	FXTI0033
END	FXTI0034

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SUBROUTINE PRECHK(DRGPM,DRHP,DRSF,VWIN,SGRT,NATRH)	PRCK0001
C CHECKS PRECIPITATION-STREAMFLOW ANOMALIES AND ADJUSTS PRECIPITATION	PRCK0002
C WHERE NECESSARY	PRCK0003
DIMENSION DRGPM(366),DRHP(366,24),DRSF(366)	PRCK0004
INTEGER DAY,HOUR,SGRT	PRCK0005
AHP = 0.0	PRCK0006
NRHA = 24 - NATRH	PRCK0007

RGPM = DRGPM(90)	PRCK0008
DAY = 90	PRCK0009
RMWR = 1.25	PRCK0010
100 DAY = DAY + 1	PRCK0011
IF(DAY .GT. 200 .OR. VWIN .GT. 750.0) RMWR = 2.00	PRCK0012
RFRISE = (DRSF(DAY) - DRSF(DAY-1))/VWIN	PRCK0013
DO 101 HOUR = 1,24	PRCK0014
IF(HOUR .EQ. SGRT+1) RGPM = DRGPM(DAY)	PRCK0015
AHP = AHP + DRHP(DAY,HOUR)*RGPM	PRCK0016
IF(HOUR .NE. NRHA) GO TO 101	PRCK0017
RWRAIN = AHP	PRCK0018
AHP = 0.0	PRCK0019
101 CONTINUE	PRCK0020
IF(RFRISE .GT. RWRAIN .AND. RFRISE .GT. 0.1) GO TO 102	PRCK0021
IF((RWRAIN .GT. RMWR .AND. RFRISE .LT. 0.02*RWRAIN) .OR. (RWRAIN	PRCK0022
1 .GT. 3.00 .AND. RFRISE .LT. 0.05*RWRAIN)) GO TO 104	PRCK0023
GO TO 108	PRCK0024
102 IF(RWRAIN .GT. 0.05) GO TO 103	PRCK0025
RAA = RFRISE*2.0 - RWRAIN + 1.0	PRCK0026
DRHP(DAY,13) = RAA	PRCK0027
WRITE(6,1) DAY, RAA	PRCK0028
1 FORMAT(/10X, 'FOR DAY', I4, 1X, 'RAIN ADDED OF', F7.2)	PRCK0029
GO TO 108	PRCK0030
103 RAM = 2.0*RFRISE/RWRAIN	PRCK0031
GO TO 105	PRCK0032
104 RAM = 10.0*RFRISE/RWRAIN	PRCK0033
105 IF(RAM .LT. 0.0) GO TO 108	PRCK0034
WRITE(6,2) DAY, RAM, RWRAIN	PRCK0035
2 FORMAT(/5X, 'FOR DAY', I4, 1X, 'RAIN ADJUSTMENT MULTIPLIER IS', F8.4,	PRCK0036
1 1X, 'RECORDED RAIN IS', F7.2)	PRCK0037
DO 106 HOUR = 1, NRHA	PRCK0038
106 DRHP(DAY, HOUR) = DRHP(DAY, HOUR)*RAM	PRCK0039
IF(NATRH .EQ. 0) GO TO 108	PRCK0040
NFRHA = NRHA + 1	PRCK0041
DO 107 HOUR = NFRHA, 24	PRCK0042
107 DRHP(DAY-1, HOUR) = DRHP(DAY-1, HOUR)*RAM	PRCK0043

108 IF(DAY .NE. 273) GO TO 100  
RETURN  
END

PRCK0044  
PRCK0045  
PRCK0046

SUBROUTINE RECESS(DRSF,DPY,BFRC,IFRC,AREA,RSBD,RSBIF,NRS,RSBBF) RCSS0001  
C ESTABLISHES RECESSION SEQUENCES RCSS0002  
DIMENSION DRSF(366),LBFO(20),NDRS(20),RSBBF(20),RSBD(20), RCSS0003  
1 RSBFRC(20),RSBIF(20),RSIFRC(20),RSTF(50,20) RCSS0004  
LOGICAL LBFO RCSS0005  
INTEGER DAY,DPY,RSBD,RSL RCSS0006  
REAL IFRC RCSS0007  
MRSL = 9 RCSS0008  
BFRC = 0.9 RCSS0009  
IFRC = 0.05 RCSS0010  
FRERS = 0.1\*SQRT(AREA) RCSS0011  
100 DO 101 KSD = 1,50 RCSS0012  
DO 101 KRS = 1,20 RCSS0013  
101 RSTF(KSD,KRS) = 0.0 RCSS0014  
KRS = 0 RCSS0015  
DAY = 274 RCSS0016  
C BEGIN NEW SEQUENCE RCSS0017  
102 IF(KRS .GE. 20) GO TO 109 RCSS0018  
KRS = KRS + 1 RCSS0019  
KSD = 1 RCSS0020  
RSF1 = DRSF(DAY) RCSS0021  
CALL DAYNXT(DAY,DPY) RCSS0022  
IF(DAY .EQ. 274) GO TO 107 RCSS0023  
RSF2 = DRSF(DAY) RCSS0024  
RSBD(KRS) = DAY RCSS0025  
IF(RSF2 .LT. RSF1+FRERS .AND. (RSF2 .GT. 0.4\*AREA .OR. RSF2 .GT. RCSS0026  
1 10.0)) GO TO 103 RCSS0027  
KRS = KRS - 1 RCSS0028  
GO TO 102 RCSS0029  
103 RSTF(1,KRS) = RSF2 RCSS0030

RSFM = RSF2	RCSS0031
104 KSD = KSD + 1	RCSS0032
CALL DAYNXT(DAY,DPY)	RCSS0033
IF(DAY .EQ. 274) GO TO 107	RCSS0034
RSFN = DRSF(DAY)	RCSS0035
IF(RSFN .LT. (RSFM + FRERS) .AND. RSFN .GT. 0.0) GO TO 106	RCSS0036
IF(KSD .GE. MRSL) GO TO 102	RCSS0037
NDRS(KRS) = 0	RCSS0038
DO 105 KSD = 1,MRSL	RCSS0039
105 RSTF(KSD,KRS) = 0.0	RCSS0040
KRS = KRS - 1	RCSS0041
GO TO 102	RCSS0042
106 IF(RSFN .LT. RSFM) RSFM = RSFN	RCSS0043
RSTF(KSD,KRS) = RSFN	RCSS0044
NDRS(KRS) = KSD	RCSS0045
IF(KSD .GE. 50) GO TO 102	RCSS0046
GO TO 104	RCSS0047
107 IF(KSD .GE. MRSL) GO TO 109	RCSS0048
NTRS = KRS - 1	RCSS0049
DO 108 KSD = 1,MRSL	RCSS0050
108 RSTF(KSD,KRS) = 0.0	RCSS0051
GO TO 110	RCSS0052
109 NTRS = KRS	RCSS0053
110 CONTINUE	RCSS0054
IF(NTRS .GE. 3) GO TO 111	RCSS0055
IF(MRSL .LT. 7) RETURN	RCSS0056
MRSL = 6	RCSS0057
GO TO 100	RCSS0058
C WRITE OUT ESTABLISHED ARRAY OF FLOW SEQUENCES	RCSS0059
111 WRITE(6,1)	RCSS0060
1 FORMAT(/5X,'FLOW SEQUENCES USED TO ESTIMATE RECESSION CONSTANTS')	RCSS0061
DO 113 KRS = 1,NTRS	RCSS0062
NDRSC = NDRS(KRS)	RCSS0063
DO 112 KSD = 2,NDRSC	RCSS0064
112 RSTF(KSD-1,KRS) = RSTF(KSD,KRS)	RCSS0065
NDRS(KRS) = NDRS(KRS) - 1	RCSS0066



NDRSC = NDRSC - 1	RCSS0067
WRITE(6,2) KRS,(RSTF(KSD,KRS);KSD=1,NDRSC)	RCSS0068
2 FORMAT(/10X,I2,5(10F8.1/12X))	RCSS0069
113 CONTINUE	RCSS0070
C DETERMINE RECESSION CONSTANTS FROM EACH SEQUENCE	RCSS0071
114 DO 116 KRS = 1,NTRS	RCSS0072
IF((RSTF(1,KRS) .LT. 0.4*AREA) .AND. (RSTF(2,KRS) .GT. 0.8*	RCSS0073
1 (RSTF(1,KRS)))) GO TO 115	RCSS0074
LBFO(KRS) = .FALSE.	RCSS0075
CALL SET2RC(RSTF,KRS,NDRS(KRS),RSIFRC(KRS),RSBFRC(KRS),LBFO(KRS))	RCSS0076
IF(LBFO(KRS) .OR. RSBFRC(KRS) .GT. 1.2 .OR. RSBFRC(KRS) .LT. 0.6	RCSS0077
1 .OR. RSIFRC(KRS) .GT. 0.8 .OR. RSIFRC(KRS) .LT. -0.4) GO TO 115	RCSS0078
GO TO 116	RCSS0079
115 LBFO(KRS) = .TRUE.	RCSS0080
CALL SET1RC(RSTF,KRS,NDRS(KRS),RSBFRC(KRS))	RCSS0081
116 CONTINUE	RCSS0082
C CALCULATE WEIGHTED AVERAGE RECESSION CONSTANTS	RCSS0083
BFRC = 0.0	RCSS0084
IFRC = 0.0	RCSS0085
ABFSL = 0.0	RCSS0086
AIFSL = 0.0	RCSS0087
DO 118 KRS = 1,NTRS	RCSS0088
IF(RSBFRC(KRS) .GT. 1.2 .OR. RSBFRC(KRS) .LT. 0.6) GO TO 117	RCSS0089
RSL = NDRS(KRS)	RCSS0090
BFRC = BFRC + RSBFRC(KRS)*RSL	RCSS0091
ABFSL = ABFSL + RSL	RCSS0092
IF(LBFO(KRS)) GO TO 118	RCSS0093
IF(RSL .GE. 20.0) RSL = 20.0	RCSS0094
IFRC = IFRC + RSIFRC(KRS)*RSL	RCSS0095
AIFSL = AIFSL + RSL	RCSS0096
GO TO 118	RCSS0097
117 WRITE(6,3) KRS	RCSS0098
3 FORMAT(10X,'SEQUENCE',I3,1X,'OMITTED IN AVERAGING')	RCSS0099
118 CONTINUE	RCSS0100
WRITE(6,4) ABFSL,AIFSL	RCSS0101
4 FORMAT(10X,'BASE FLOW DAYS =',F5.0,2X,'INTERFLOW DAYS =',F5.0)	RCSS0102

BFRC = BFRC/ABFSL	RCSS0103
IFRC = IFRC/AIFSL	RCSS0104
IF(BFRC .GT. 0.99) BFRC = 0.99	RCSS0105
IF(BFRC .LT. 0.70) BFRC = 0.70	RCSS0106
KSQ = 0	RCSS0107
DO 119 KRS = 1,NTRS	RCSS0108
IF(LBFO(KRS)) GO TO 119	RCSS0109
CALL SETRBF(RSTF,NDRS,KRS,BFRC,IFRC,CRSBIF,CRSBBF)	RCSS0110
IF(CRSBIF .GT. 95000.0 .OR. CRSBBF .LT. 0.0) GO TO 119	RCSS0111
IF(CRSBIF .LT. 0.0) CRSBIF = 0.0	RCSS0112
KSQ = KSQ + 1	RCSS0113
RSBD(KSQ) = RSBD(KRS)	RCSS0114
RSBIF(KSQ) = CRSBIF	RCSS0115
RSBBF(KSQ) = CRSBBF	RCSS0116
119 CONTINUE	RCSS0117
NRS = KSQ	RCSS0118
DO 120 KSQ = 1,NRS	RCSS0119
DAY = RSBD(KSQ)	RCSS0120
CALL DAYNXT(DAY,DPY)	RCSS0121
120 RSBD(KSQ) = DAY	RCSS0122
DO 121 KSQ = 1,NRS	RCSS0123
CRSBTF = RSBIF(KSQ) + RSBBF(KSQ)	RCSS0124
121 WRITE(6,5) KSQ,RSBD(KSQ),RSBIF(KSQ),RSBBF(KSQ),CRSBTF	RCSS0125
5 FORMAT(/10X,'REVISED FLOW SEQUENCE',I3,1X,1X,'BEGINS ON DAY',I4,	RCSS0126
1 1X,'AT INTERFLOW =',F7.2,1X,'CFS, BASE FLOW =',F7.2,1X,'CFS,	RCSS0127
1 TOTAL =',F7.2,1X,'CFS')	RCSS0128
RETURN	RCSS0129
END	RCSS0130

SUBROUTINE SETBIV(BIVF,NRS,IFRC,RSBIF,SIFRS,FNCTRH)	STBV0001
C SETS BEST VALUE OF BASIC INTERFLOW VOLUME FACTOR	STBV0002
DIMENSION RSBIF(20),SIFRS(3,20)	STBV0003
REAL IFRC	STBV0004
ARSTR = 0.0	STBV0005

DO 101 KRS = 1,NRS	STBV0006
RIF = RSBIF(KRS)/IFRC	STBV0007
DO 100 KDY = 1,3	STBV0008
RIF = RIF*IFRC	STBV0009
SIF = SIFRS(KDY,KRS)/IFRC**(FNCTR/48.0)	STBV0010
RSTR = 0.0	STBV0011
IF(RIF .GT. 0.0) RSTR = SIF/RIF	STBV0012
IF(RSTR .GT. 3.0 .OR. (SIF .GT. 0.0 .AND. RIF .EQ. 0.0))RSTR=3.0	STBV0013
ARSTR = ARSTR + RSTR	STBV0014
WRITE(6,1) KRS,KDY,SIF,RIF	STBV0015
1 FORMAT(10X,'KRS =',I3,2X,'KDY =',I2,2X,'SIF =',F7.1,5X,'RIF =',	STBV0016
1 F7.1)	STBV0017
100 CONTINUE	STBV0018
101 CONTINUE	STBV0019
TIRD = NRS*3	STBV0020
PBIVF = BIVF	STBV0021
BIVF = 0.40	STBV0022
IF(ARSTR .GT. 0.0) BIVF = ((PBIVF - 0.40)*TIRD)/ARSTR + 0.40	STBV0023
WRITE(6,2) PBIVF,BIVF	STBV0024
2 FORMAT(5X,'BIVF CHANGED FROM',F6.2,2X,'TO',F6.2//)	STBV0025
RETURN	STBV0026
END	STBV0027

SUBROUTINE SETBMI(BMIR,NRS,BFRC,RSBBF,SBFRS,FNCTR,IFT)	STBM0001
C SETS BEST VALUE OF BASIC MAXIMUM INFILTRATION RATE WITHIN WATERSHED	STBM0002
DIMENSION RSBBF(20),SBFRS(3,20)	STBM0003
ARSTR = 0.0	STBM0004
DO 101 KRS = IFT,NRS	STBM0005
RBF = RSBBF(KRS)/BFRC	STBM0006
DO 100 KDY = 1,3	STBM0007
RBF = RBF*BFRC	STBM0008
SBF = SBFRS(KDY,KRS)/BFRC**(FNCTR/48.0)	STBM0009
RSTR = SBF/RBF	STBM0010
IF(RSTR .GT. 3.0) RSTR = 3.0	STBM0011

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ARSTR = ARSTR + RSTR
WRITE(6,1) KRS,KDY,SBF,RBF
1 FORMAT(10X,'KRS =',I3,2X,'KDY =',I2,2X,'SBF =',F7.1,5X,'RBF =',
1 F7.1)
100 CONTINUE
101 CONTINUE
TBRD = (NRS + 1 - IFT)*3
ARSTR = ARSTR/TBRD
ARSTR = ARSTR**1.3
PBMIR = BMIR
BMIR = PBMIR/ARSTR
WRITE(6,2) PBMIR,BMIR
2 FORMAT(5X,'BMIR CHANGED FROM',F6.2,2X,'TO',F6.2//)
RETURN
END

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STBM0012
STBM0013
STBM0014
STBM0015
STBM0016
STBM0017
STBM0018
STBM0019
STBM0020
STBM0021
STBM0022
STBM0023
STBM0024
STBM0025
STBM0026

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```

SUBROUTINE SETFDI(MFDP, TMSTF, TMRTF, SSQM)
C SETS VALUES OF FLOW DEVIATION INDICES
DIMENSION MFDP(12), TMRTF(12), TMSTF(12)
REAL MFDP
DO 101 MONTH = 1,12
IF(MONTH .LE. 2) SSQM = 0.0
SMFX = TMSTF(MONTH) + 20.0
RMFX = TMRTF(MONTH) + 20.0
MFDP(MONTH) = SMFX/RMFX - 1.0
IF(MFDP(MONTH) .GT. 8.0) MFDP(MONTH) = 8.0
IF(MFDP(MONTH) .LT. 0.0) MFDP(MONTH) = 1.0 - RMFX/SMFX
IF(MFDP(MONTH) .LT. -8.0) MFDP(MONTH) = -8.0
100 SSQM = SSQM + MFDP(MONTH)*MFDP(MONTH)
101 CONTINUE
WRITE(6,1) (MFDP(MONTH), MONTH=1,12), SSQM
1 FORMAT(//2X,'MONTHLY DEVIATIONS',/16X,12(F7.3,1X),'SSQM =',F7.3)
RETURN
END

```

```

STFD0001
STFD0002
STFD0003
STFD0004
STFD0005
STFD0006
STFD0007
STFD0008
STFD0009
STFD0010
STFD0011
STFD0012
STFD0013
STFD0014
STFD0015
STFD0016
STFD0017
STFD0018

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	SUBROUTINE SETFVP(LZC,SUZC,ETLF,BUZO,SIAC,TMSTF,MRTE,TMPREC,	STFV0001
	1 TMPET,EMLZS,SSQM,LRC,XMPFT,FTX,NOFM,LBUZO,LETLF,LLZO,APREC,APET)	STFV0002
C	SETS BEST VALUES OF FLOW VOLUME PARAMETERS	STFV0003
	DIMENSION EMLZS(12),MFD(12),MXA(12),TMPET(12),TMPREC(12),	STFV0004
	1 TMRTF(12),TMSTF(12),XMPFT(12)	STFV0005
	LOGICAL LBUZO,LETLF,LLZO,LRC	STFV0006
	REAL LZC,MFD	STFV0007
	CALL SETFDI(MFD,TMSTF,MRTE,SSQM)	STFV0008
	IF((MFD(2) + MFD(3)) .GT. 2.0 .AND. FTX .LT. 1.05) FTX = 0.9	STFV0009
	IF((MFD(2) + MFD(3)) .LT. -2.0 .AND. FTX .GT. 0.95) FTX = 1.1	STFV0010
C	ADJUSTMENT OF LZC BASED ON MONTHS WHERE OVER HALF OF TOTAL	STFV0011
C	SYNTHESIZED RUNOFF IS OVERLAND FLOW, MINIMUM OF TWO MONTHS	STFV0012
C	WITH GREATEST RUNOFF USED	STFV0013
	PLZO = LZC	STFV0014
	FNOFM = NOFM	STFV0015
	IF(NOFM .GT. 2) GO TO 103	STFV0016
	M1R = 2	STFV0017
	M2R = 1	STFV0018
	IF(TMRTF(2) .GT. TMRTF(1)) GO TO 100	STFV0019
	M1R = 1	STFV0020
	M2R = 2	STFV0021
100	DO 102 MONTH = 3,12	STFV0022
	IF(TMRTF(MONTH) .LT. TMRTF(M2R)) GO TO 102	STFV0023
	IF(TMRTF(MONTH) .GT. TMRTF(M1R)) GO TO 101	STFV0024
	M2R = MONTH	STFV0025
	GO TO 102	STFV0026
101	M2R = M1R	STFV0027
	M1R = MONTH	STFV0028
102	CONTINUE	STFV0029
	IF(LLZO) GO TO 106	STFV0030
	FLZO = (MFD(M1R) + MFD(M2R))/2.0	STFV0031
	GO TO 105	STFV0032
103	SDFMD = 0.0	STFV0033

<pre> KMI = 0 DO 104 MONTH = 1,12 IF(XMPFT(MONTH) .LT. 1.5) GO TO 104 SOFMD = SOFMD + MFDP(MONTH) KMI = KMI + 1 MXA(KMI) = MONTH 104 CONTINUE FLZC = SOFMD/(FNDFM*0.75) 105 IF(FLZC .GT. 1.0) FLZC = 1.0 IF(FLZC .LT. -1.0) FLZC = -1.0 IF(FLZC .GT. 0.0) LZC = (FLZC + 1.0)*LZC IF(FLZC .LE. 0.0) LZC = LZC/(1.0 - FLZC) IF(NDFM .LE. 2) WRITE(6,1) LZC,M1R,M2R 1 FORMAT(/5X,'LZC WAS CHANGED TO',F6.2,' BASED ON MONTHS',2I3) IF(NDFM .GT. 2) WRITE(6,2) LZC,(MXA(KWD), KWD = 1,NDFM) 2 FORMAT(/5X,'LZC WAS CHANGED TO',F6.2,' BASED ON MONTHS',12I3) IF(LZC .LT. 2.0 .AND. LRC) LZC = 2.0 IF(LZC .GT. 30.0 .AND. LRC) LZC = 30.0 C SELECTION OF MONTHS BEGINNING WET AND BEGINNING DRY SEASONS 106 MBWS = 0 MBDS = 0 DO 109 MONTH = 2,10 IF(TMPET(MONTH) .GT. TMPREC(MONTH)) GO TO 108 IF(MBWS .NE. 0) GO TO 107 MBWS = MONTH GO TO 109 107 MBDS = MONTH + 1 GO TO 109 108 IF(MBDS .NE. 0) GO TO 110 109 CONTINUE 110 MBDS = MBDS + 1 C ADJUSTMENT OF SUZC BASED ON TWO WETTEST SUMMER MONTHS AND LAST TWO C BASE FLOW MONTHS M11 = 0 M12 = 0 PRM1 = 0.0 </pre>	<pre> STFV0034 STFV0035 STFV0036 STFV0037 STFV0038 STFV0039 STFV0040 STFV0041 STFV0042 STFV0043 STFV0044 STFV0045 STFV0046 STFV0047 STFV0048 STFV0049 STFV0050 STFV0051 STFV0052 STFV0053 STFV0054 STFV0055 STFV0056 STFV0057 STFV0058 STFV0059 STFV0060 STFV0061 STFV0062 STFV0063 STFV0064 STFV0065 STFV0066 STFV0067 STFV0068 STFV0069 </pre>
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M1SP = 0	STFV0070
DO 112 MNX = 7,14	STFV0071
MONTH = MNX	STFV0072
IF(MNX .GT. 12) MONTH = MNX - 12	STFV0073
IF(TMPREC(MONTH) .LE. PRM1) GO TO 111	STFV0074
M2SP = M1SP	STFV0075
PRM2 = PRM1	STFV0076
M1SP = MONTH	STFV0077
PRM1 = TMPREC(MONTH)	STFV0078
GO TO 112	STFV0079
111 IF(TMPREC(MONTH) .LE. PRM2) GO TO 112	STFV0080
M2SP = MONTH	STFV0081
PRM2 = TMPREC(MONTH)	STFV0082
112 CONTINUE	STFV0083
FSUZC = MFD(P(M1SP) + MFD(P(M2SP)	STFV0084
IF(ABS(XMPFT(12) - 1.0) .GT. 0.2) GO TO 113	STFV0085
FSUZC = FSUZC + MFD(P(12)	STFV0086
M12 = 12	STFV0087
113 IF(ABS(XMPFT(11) - 1.0) .GT. 0.2) GO TO 114	STFV0088
FSUZC = FSUZC + MFD(P(11)	STFV0089
M11 = 11	STFV0090
114 IF(FSUZC .GT. 1.0) FSUZC = 1.0	STFV0091
IF(FSUZC .LT. -1.0) FSUZC = -1.0	STFV0092
IF(FSUZC .GT. 0.0) SUZC = (FSUZC + 1.0)*SUZC	STFV0093
IF(FSUZC .LE. 0.0) SUZC = SUZC/(1.0 - FSUZC)	STFV0094
WRITE(6,3) SUZC,M1SP,M2SP,M11,M12	STFV0095
3 FORMAT(4X,'SUZC WAS CHANGED TO',F6.2,' BASED ON MONTHS',4I3)	STFV0096
IF(SUZC .LT. 0.3 .AND. LRC) SUZC = 0.3	STFV0097
IF(SUZC .GT. 3.0 .AND. LRC) SUZC = 3.0	STFV0098
C ADJUSTMENT OF ETLF BASED ON SUMMER MONTHS OF RAINFALL EXCEEDING TWO	STFV0099
C INCHES OR NEED TO PREVENT MOISTURE BUILDUP	STFV0100
IF(EMLZS(12) .LT. PLZC .OR. EMLZS(11) .LT. PLZC .OR. APREC .GT.	STFV0101
1 1.5*APET) GO TO 115	STFV0102
FETLF = 1.0	STFV0103
MXA(1) = 13	STFV0104
KWSM = 1	STFV0105

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GO TO 120
115 SWSMD = 0.0
    KWSM = 0
    DO 116 MONTH = 1,12
    IF((MONTH .GT. MBWS .OR. MONTH .GT. 2) .AND. (MONTH .LT. MBDS
1 .AND. MONTH .LT. 9)) GO TO 116
    IF(TMPREC(MONTH) .LT. 2.0) GO TO 116
    SWSMD = SWSMD + MFDP(MONTH)
    KWSM = KWSM + 1
    MXA(KWSM) = MONTH
116 CONTINUE
    IF(KWSM .GE. 1) GO TO 117
    MXA(1) = MIR
    KWSM = 1
    FETLF = 5.0*MFDP(MIR)
    GO TO 120
117 WSM = KWSM
    IF(.NOT. LETLF .OR. KWSM .EQ. 1) GO TO 119
    EMFDP = 0.0
    DO 118 MONTH = 1,KWSM
    KM1 = MXA(MONTH)
    IF(MFDP(KM1) .LT. EMFDP) GO TO 118
    EMFDP = MFDP(KM1)
    KM2 = MONTH
118 CONTINUE
    MXA(KM2) = 0
    SWSMD = SWSMD - EMFDP
    WSM = WSM - 1.0
119 FETLF = 1.2*SWSMD/WSM
120 IF(FETLF .GT. 1.0) FETLF = 1.0
    IF(FETLF .LT. -1.0) FETLF = -1.0
    IF(FETLF .GT. 0.0) ETLF = (FETLF + 1.0)*ETLF
    IF(FETLF .LT. 0.0) ETLF = ETLF/(1.0 - FETLF)
    WRITE(6,4) ETLF,(MXA(KWD), KWD = 1,KWSM)
4 FORMAT(4X,'ETLF WAS CHANGED TO',F6.2,' BASED ON MONTHS',12I3)
    IF(ETLF .LT. 0.05 .AND. LRC) ETLF = 0.05

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STFV0106
STFV0107
STFV0108
STFV0109
STFV0110
STFV0111
STFV0112
STFV0113
STFV0114
STFV0115
STFV0116
STFV0117
STFV0118
STFV0119
STFV0120
STFV0121
STFV0122
STFV0123
STFV0124
STFV0125
STFV0126
STFV0127
STFV0128
STFV0129
STFV0130
STFV0131
STFV0132
STFV0133
STFV0134
STFV0135
STFV0136
STFV0137
STFV0138
STFV0139
STFV0140
STFV0141

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      IF(ETLF .GT. 0.6 .AND. LRC) ETLF = 0.6
C   ADJUSTMENT OF BUZC BASED ON SEPTEMBER, NOVEMBER, AND DECEMBER
      KM1 = 12
      KM2 = 2
      KM3 = 3
      FBUZC = 0.4*(MFDP(12) + MFDP(2) + MFDP(3))
      IF(.NOT. LBUZC) GO TO 121
      FBUZC = 0.4*(MFDP(9) + MFDP(10) + MFDP(11))
      KM1 = 9
      KM2 = 10
      KM3 = 11
121  IF(FBUZC .GT. 1.0) FBUZC = 1.0
      IF(FBUZC .LT. -1.0) FBUZC = -1.0
      IF(FBUZC .GT. 0.0) BUZC = (FBUZC + 1.0)*BUZC
      IF(FBUZC .LE. 0.0) BUZC = BUZC/(1.0 - FBUZC)
      WRITE(6,5) BUZC,KM1,KM2,KM3
      5  FORMAT(4X,'BUZC WAS CHANGED TO',F6.2,' BASED ON MONTHS',3I3)
      IF(BUZC .LT. 0.2 .AND. LRC) BUZC = 0.2
      IF(BUZC .GT. 4.0 .AND. LRC) BUZC = 4.0
C   ADJUSTMENT OF SIAC BASED ON THREE FIRST MOISTURE EXCESS AND THREE
C   FIRST MOISTURE DEFICIENT MONTHS
      KM1 = MBDS
      KM2 = MBDS + 1
      KM3 = MBDS - 1
      KM4 = 0
      KM5 = 0
      KM6 = 0
      WFDX = 0.0
      IF(SIAC .GT. 1.0) GO TO 122
      WFDX = (MFDP(MBWS) + MFDP(MBWS+1) + MFDP(MBWS+2))/3.0
      IF(SIAC .GT. 0.6) WFDX = WFDX*(1.0 - SIAC)/0.4
      KM4 = MBWS
      KM5 = MBWS + 1
      KM6 = MBWS + 2
122  SFDX = (MFDP(MBDS) + MFDP(MBDS+1) + MFDP(MBDS-1))/3.0
      FSIAC = 1.5*(SFDX - WFDX)

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STFV0142
STFV0143
STFV0144
STFV0145
STFV0146
STFV0147
STFV0148
STFV0149
STFV0150
STFV0151
STFV0152
STFV0153
STFV0154
STFV0155
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STFV0168
STFV0169
STFV0170
STFV0171
STFV0172
STFV0173
STFV0174
STFV0175
STFV0176
STFV0177

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IF(FSIAC .GT. 1.0) FSIAC = 1.0	STFV0178
IF(FSIAC .LE. -1.0) FSIAC = -1.0	STFV0179
IF(SIAC .LT. 0.02) SIAC = 0.02	STFV0180
IF(FSIAC .GT. 0.0) SIAC = (FSIAC + 1.0)*SIAC	STFV0181
IF(FSIAC .LE. 0.0) SIAC = SIAC/(1.0 - FSIAC)	STFV0182
WRITE(6,6) SIAC,KM4,KM5,KM6,KM3,KM1,KM2	STFV0183
6 FORMAT(4X,'SIAC WAS CHANGED TO',F6.2,' BASED ON MONTHS',6I3)	STFV0184
IF(SIAC .LT. 0.02 .AND. LRC) SIAC = 0.00	STFV0185
IF(SIAC .GT. 4.0 .AND. LRC) SIAC = 4.0	STFV0186
RETURN	STFV0187
END	STFV0188

SUBROUTINE SETHRP(CTRI,BTRI,WCFS,CONOP2,HBF,LSHA,SSR,NHPT,KPSH,	STHP0001
1 IBTPR,SRX,CSRX,FSRX,CHCAP,NRHP,RHPF,NCTRI,NBTRI)	STHP0002
C SETS BEST VALUES OF HYDROGRAPH ROUTING PARAMETERS	STHP0003
DIMENSION BTRI(99),CTRI(99),HBF(5),HSRX(5),KPSH(5),LSHA(5),	STHP0004
1 HNTRI(5),RHPF(5),SRR(5,170),SSR(5,170),TSRX(7)	STHP0005
LOGICAL LSHA	STHP0006
INTEGER CONOP2,HNTRI,SNTRI	STHP0007
REAL NHPT	STHP0008
MHTP = 1	STHP0009
IF(CONOP2 .EQ. 0) MHTP = 4	STHP0010
MXTRI = 2*NBTRI	STHP0011
MNTRI = NBTRI/2	STHP0012
TSRX(1) = 0.995	STHP0013
TSRX(2) = 0.99	STHP0014
TSRX(3) = 0.985	STHP0015
TSRX(4) = 0.98	STHP0016
TSRX(5) = 0.96	STHP0017
TSRX(6) = 0.93	STHP0018
TSRX(7) = 0.90	STHP0019
LNIBRS = 0	STHP0020
DD 112 KHYD = 1,NRHP	STHP0021
IF(.NOT. LSHA(KHYD)) GO TO 112	STHP0022

KPCH = KPSH(KHYD)	STHP0023
NCTRI = MNTRI	STHP0024
CALL FIXTRI(CTRI,BTRI,NBTRI,NCTRI)	STHP0025
KH1 = 1	STHP0026
KH2 = 1	STHP0027
KH3 = 1	STHP0028
SDRSP = 80.0*CHCAP	STHP0029
SNTRI = MXTRI	STHP0030
100 SRX = TSRX(KH1)	STHP0031
IF(KH2 .EQ. 2) LNIBRS = NIBRS	STHP0032
WRITE(6,1) NCTRI,SRX	STHP0033
1 FORMAT(//15X,'TRIAL VALUE OF NCTRI =',I3,', SRX =',F6.3)	STHP0034
CALL TIMERT(SSR,SRR,CTRI,NCTRI,KHYD,KPCH)	STHP0035
CSRX = SRX	STHP0036
FSRX = SRX	STHP0037
CALL STORRT(SRR,CSRX,FSRX,CHCAP,CONOP2,IBTPS,SHPF,KHYD,HBF(KHYD),	STHP0038
1 NHPT,KPCH,IBTPR)	STHP0039
LNTRI = NCTRI	STHP0040
NIRTS = IBTPS - IBTPR*MHTP	STHP0041
NIBRS = IABS(NIRTS)	STHP0042
DRSP = ABS(SHPF - RHPF(KHYD))	STHP0043
IF(NIRTS .EQ. 0 .OR. (KH2 .EQ. 2 .AND. NIBRS .GE. LNIBRS) .OR.	STHP0044
1 RHPF(KHYD) .GT. 1.2*SHPF) GO TO 103	STHP0045
IF(NIRTS .GE. 1) GO TO 109	STHP0046
101 NCTRI = NCTRI - NIRTS	STHP0047
IF(NCTRI .LT. MNTRI) NCTRI = MNTRI	STHP0048
IF(NCTRI .GT. MXTRI) GO TO 106	STHP0049
102 CALL FIXTRI(CTRI,BTRI,NBTRI,NCTRI)	STHP0050
KH2 = 2	STHP0051
GO TO 100	STHP0052
103 IF(DRSP .GT. SDRSP) GO TO 108	STHP0053
SNTRI = LNTRI	STHP0054
SDRSP = DRSP	STHP0055
KH3 = 2	STHP0056
104 KH1 = KH1 + 1	STHP0057
IF(KH1 .EQ. 8) GO TO 105	STHP0058

	KH2 = 1	STHP0059
	GO TO 100	STHP0060
105	HNTRI(KHYD) = LNTRI	STHP0061
	HSRX(KHYD) = SRX	STHP0062
	GO TO 111	STHP0063
106	IF(KH1 .GE. 2 .AND. KH3 .EQ. 2 .AND. DRSP .GE. SDRSP) GO TO 108	STHP0064
	NCTRI = MXTRI	STHP0065
	CALL FIXTRI(CTRI,BTRI,NBTRI,NCTRI)	STHP0066
	IF(KH2 .EQ. 2 .AND. KH1 .EQ. 1 .AND. SHPF .GT. RHPF(KHYD)) GO	STHP0067
	1 TO 107	STHP0068
	IF(KH2 .EQ. 2 .OR. KH1 .GE. 2) GO TO 109	STHP0069
	GO TO 102	STHP0070
107	HNTRI(KHYD) = MXTRI	STHP0071
	HSRX(KHYD) = 0.995	STHP0072
	GO TO 111	STHP0073
108	HSRX(KHYD) = TSRX(KH1-1)	STHP0074
	HNTRI(KHYD) = SNTRI	STHP0075
	GO TO 111	STHP0076
109	IF(NCTRI .GT. MNTRI .AND. NCTRI .LT. MXTRI) GO TO 101	STHP0077
	IF(DRSP .GT. SDRSP) GO TO 110	STHP0078
	SDRSP = DRSP	STHP0079
	SNTRI = LNTRI	STHP0080
	GO TO 104	STHP0081
110	HNTRI(KHYD) = NCTRI	STHP0082
	HSRX(KHYD) = 0.995	STHP0083
	IF(KH1 .GE. 2) HSRX(KHYD) = TSRX(KH1 - 1)	STHP0084
111	IF(HSRX(KHYD) .LT. 0.91 .AND. SHPF .LT. 0.5*RHPF(KHYD)) LSHA(KHYD)	STHP0085
	1 = .FALSE.	STHP0086
	IF(.NOT. LSHA(KHYD)) GO TO 112	STHP0087
	WRITE(6,2) KHYD,HNTRI(KHYD),HSRX(KHYD)	STHP0088
	2 FORMAT(10X,'FOR STORM ',I2,' NCTRI =',I3,' SRX =',F6.3)	STHP0089
112	CONTINUE	STHP0090
	KPA = 1	STHP0091
113	ARHPF = 0.0	STHP0092
	APPKP = 0.0	STHP0093
	DO 114 KHYD = 1,NRHP	STHP0094

IF(.NOT. LSHA(KHYD)) GO TO 114	STHP0095
CHPV = HNTRI(KHYD)	STHP0096
IF(KPA .EQ. 2) CHPV = HSRX(KHYD)	STHP0097
APPKP = APPKP + CHPV*RHPF(KHYD)	STHP0098
ARHPF = ARHPF + RHPF(KHYD)	STHP0099
114 CONTINUE	STHP0100
WAPV = APPKP/ARHPF	STHP0101
IF(KPA .EQ. 2) GO TO 115	STHP0102
NCTRI = WAPV + 0.5	STHP0103
WRITE(6,3) NCTRI	STHP0104
3 FORMAT(/ /10X, 'OPTIMUM NCTRI =', I3)	STHP0105
IF(NCTRI .EQ. 0) RETURN	STHP0106
CALL FIXTRI(CTRI, BTRI, NBTRI, NCTRI)	STHP0107
WRITE(6,4) (CTRI(KTRI), KTRI = 1, NCTRI)	STHP0108
4 FORMAT(18X, 'CTRI ARE' /9(16X, 11F8.4/))	STHP0109
WRITE(6,5)	STHP0110
5 FORMAT(18X, 'WARNING: THE USER MAY HAVE TO ADJUST THESE VALUES TO	STHP0111
1 MAKE THEM ADD TO ONE TO COMPENSATE FOR ROUNDING.')	STHP0112
KPA = 2	STHP0113
GO TO 113	STHP0114
115 SRX = WAPV	STHP0115
CSRX = SRX	STHP0116
FSRX = SRX	STHP0117
CALL SETSRP(CONOP2, NRHP, LSHA, RHPF, HSRX, CHCAP, SSR, SRR, CTRI, CSRX,	STHP0118
1 FSRX, KHYD, IBTPS, SHPF, NCTRI, HBF, NHPT, KPSH, IBTPR)	STHP0119
SRX = CSRX	STHP0120
RETURN	STHP0121
END	STHP0122

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SUBROUTINE SETRBFIRSTF, NDRS, KRS, BFCR, IFRC, CRSBIF, CRSBBF)	STRB0001
C SETS VALUES OF INTERFLOW AND BASE FLOW AT RECESSON BEGINNING	STRB0002
DIMENSION RSTF(50,20), NDRS(20)	STRB0003
REAL*8 RA1, RA2, RA3, RA4, RA5, RA6	STRB0004
REAL IFRC	STRB0005

RA1 = 0.0	STRB0006
RA2 = 0.0	STRB0007
RA3 = 0.0	STRB0008
RA4 = 0.0	STRB0009
RA5 = 0.0	STRB0010
MNDRS = 12	STRB0011
IF(NDRS(KRS) .LT. 12) MNDRS = NDRS(KRS)	STRB0012
IF(IFRC .GE. 0.3) GO TO 101	STRB0013
CRSBIF = 0.0	STRB0014
DO 100 KSD = 1, MNDRS	STRB0015
RA1 = RA1 + BFRC**(2*KSD)	STRB0016
100 RA4 = RA4 + RSTF(KSD, KRS)*(BFRC**KSD)	STRB0017
CRSOBF = RA4/RA1	STRB0018
CRSBIF = CRSOBF*BFRC	STRB0019
RETURN	STRB0020
101 CRSBIF = 100000.0	STRB0021
DO 102 KSD = 1, MNDRS	STRB0022
RA1 = RA1 + BFRC**(2*KSD)	STRB0023
RA2 = RA2 + IFRC**(2*KSD)	STRB0024
RA3 = RA3 + (BFRC*IFRC)**KSD	STRB0025
RA4 = RA4 + RSTF(KSD, KRS)*(BFRC**KSD)	STRB0026
RA5 = RA5 + RSTF(KSD, KRS)*(IFRC**KSD)	STRB0027
102 CONTINUE	STRB0028
RA6 = RA1*RA2 - RA3**2	STRB0029
IF(RA6 .EQ. 0.0) RETURN	STRB0030
CRSOIF = -(RA3/RA6)*RA4 + (RA1/RA6)*RA5	STRB0031
CRSBIF = CRSOIF*IFRC	STRB0032
CRSOBF = (RA2/RA6)*RA4 - (RA3/RA6)*RA5	STRB0033
CRSBIF = CRSOBF*BFRC	STRB0034
RETURN	STRB0035
END	STRB0036

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SUBROUTINE SETSRP(CONOP2, NRHP, LSHA, RHPF, HSRX, CHCAP, SSR, SRR, GTRI, STSP0001  
1 CSRX, FSRX, KHYD, IBTPS, SHPF, NCTRI, HBF, NHPT, KPSH, IBTPR) STSP0002

C	SETS BEST VALUES OF STORAGE ROUTING PARAMETERS	STSP0003
	DIMENSION CTRI(99), HBF(5), HSRX(5), KPSH(5), LSHA(5), RHPF(5),	STSP0004
	1 SRR(5,170), SSR(5,170)	STSP0005
	LOGICAL LSHA	STSP0006
	INTEGER CONOP2	STSP0007
	REAL NHPT	STSP0008
	KLCCA = 1	STSP0009
	SRX = CSRX	STSP0010
	EPS = 0.000001	STSP0011
	NORHP = NRHP	STSP0012
	DO 100 KHYD = 1, NORHP	STSP0013
	IF(.NOT. LSHA(KHYD)) NRHP = NRHP - 1	STSP0014
100	CONTINUE	STSP0015
	IF(NRHP .LE. 2) GO TO 103	STSP0016
C	FIND REGRESSION LINE FOR DETERMINING CSRX, WHEN NRHP EXCEEDS 3	STSP0017
	RA1 = 0.0	STSP0018
	RA2 = 0.0	STSP0019
	RA3 = 0.0	STSP0020
	RA4 = 0.0	STSP0021
C	FNRHP = NRHP	STSP0022
	DO 101 KHYD = 1, NORHP	STSP0023
	IF(.NOT. LSHA(KHYD)) GO TO 101	STSP0024
	RA1 = RA1 + RHPF(KHYD)	STSP0025
	RA2 = RA2 + HSRX(KHYD)	STSP0026
	RA3 = RA3 + RHPF(KHYD)*HSRX(KHYD)	STSP0027
	RA4 = RA4 + RHPF(KHYD)**2	STSP0028
101	CONTINUE	STSP0029
	AVRHPF = RA1/FNRHP	STSP0030
	ASRX = RA2/FNRHP	STSP0031
	RSLP = (RA3 - RA1*ASRX)/(RA4 - RA1**2/FNRHP)	STSP0032
	IF(RSLP .LE. EPS) GO TO 106	STSP0033
	RINT = ASRX - RSLP*AVRHPF	STSP0034
102	CSRX = RINT + RSLP*(0.5*CHCAP)	STSP0035
	IF(CSRX .GE. 0.99) RETURN	STSP0036
	IF(CSRX .LE. 0.8) CSRX = 0.8	STSP0037
	GO TO 107	STSP0038

103	K1AH = 0	STSP0039
	DO 104 KHYD = 1, NORHP	STSP0040
	IF(.NOT. LSHA(KHYD)) GO TO 104	STSP0041
	IF(K1AH .EQ. 0) K1AH = KHYD	STSP0042
	IF(K1AH .GT. 0) K2AH = KHYD	STSP0043
104	CONTINUE	STSP0044
	IF(NRHP .EQ. 1) GO TO 105	STSP0045
C	FIT THE STRAIGHT LINE WHEN NRHP = 2	STSP0046
	RSLP = (HSRX(K1AH) - HSRX(K2AH)) / (RHPF(K1AH) - RHPF(K2AH))	STSP0047
	IF(RSLP .LE. EPS) GO TO 106	STSP0048
	RINT = HSRX(K1AH) - RSLP * RHPF(K1AH)	STSP0049
	GO TO 102	STSP0050
105	CONTINUE	STSP0051
	CSRX = HSRX(K1AH)	STSP0052
	FSRX = CSRX	STSP0053
	GO TO 115	STSP0054
106	CONTINUE	STSP0055
	CSRX = SRX	STSP0056
	FSRX = CSRX	STSP0057
	WRITE(6,1)	STSP0058
	1 FORMAT(/ /10X, 'REGRESSION LINE HAS NEGATIVE SLOPE')	STSP0059
	GO TO 115	STSP0060
107	CONTINUE	STSP0061
	BISRX = 0.2 * (0.99 - CSRX)	STSP0062
	SISRX = 0.04 * (0.99 - CSRX)	STSP0063
	TFSRX = CSRX	STSP0064
	KISRX = 0	STSP0065
108	KISRX = KISRX + 1	STSP0066
	FSRX = TFSRX	STSP0067
	WRITE(6,2) KISRX, CSRX, FSRX, CHCAP	STSP0068
	2 FORMAT(/ /15X, 'TRIAL', I3, ', ', CSRX = ', F8.5, ', ', FSRX = ', F8.5,	STSP0069
	1 ', CHCAP = ', F10.0)	STSP0070
	SQPKD = 0.0	STSP0071
	ADRSP = 0.0	STSP0072
	DO 109 KHYD = 1, NORHP	STSP0073
	IF(.NOT. LSHA(KHYD)) GO TO 109	STSP0074



KPGH = KPSH(KHYD)	STSP0075
CALL TIMERT(SSR,SRR,CTRI,NCTRI,KHYD,KPCH)	STSP0076
CALL STORRT(SRR,CSRX,FSRX,CHCAP,CONDP2,IBTPS,SHPF,KHYD,HBF(KHYD),	STSP0077
1 NHPT,KPCH,IBTPR)	STSP0078
DRSP = SHPF - RHPP(KHYD)	STSP0079
SQPKD = SQPKD + DRSP**2	STSP0080
ADRSP = ADRSP + DRSP	STSP0081
109 CONTINUE	STSP0082
WRITE(6,3) SQPKD	STSP0083
3 FORMAT(/25X,'SQPKD =',F14.0)	STSP0084
IF(KISRX .NE. 1) GO TO 110	STSP0085
TFSRX = CSRX + BISRX	STSP0086
SSQPKD = SQPKD	STSP0087
BFSRX = FSRX	STSP0088
GO TO 108	STSP0089
110 IF(SQPKD .GT. SSQPKD) GO TO 113	STSP0090
IF(KISRX .EQ. 6 .AND. ADRSP .GT. 0.0) GO TO 111	STSP0091
SSQPKD = SQPKD	STSP0092
BFSRX = FSRX	STSP0093
IF(KISRX .GE. 11) GO TO 114	STSP0094
IF(KISRX .LE. 5) TFSRX = TFSRX + BISRX	STSP0095
IF(KISRX .GE. 6) TFSRX = TFSRX - SISRX	STSP0096
GO TO 108	STSP0097
111 KLCCA = KLCCA + 1	STSP0098
IF(KLCCA .GE. 5) GO TO 112	STSP0099
CHCAP = 0.8*CHCAP	STSP0100
CSRX = RINT + RSLP*(0.5*CHCAP)	STSP0101
GO TO 107	STSP0102
112 CSRX = 0.990	STSP0103
FSRX = 0.990	STSP0104
GO TO 115	STSP0105
113 IF(KISRX .GT. 6) GO TO 114	STSP0106
KISRX = 6	STSP0107
SSQPKD = SQPKD	STSP0108
BFSRX = FSRX	STSP0109
TFSRX = TFSRX - SISRX	STSP0110

```

      GO TO 108
114 FSRX = BFSRX
      WRITE(6,4) CSRX,FSRX,SSQPKD
      4 FORMAT(/,10X,'CSRX =',F7.4,10X,'FSRX =',F7.4,10X,'SQPKD =',F15.2)
      RETURN
115 WRITE(6,5) CSRX,FSRX
      5 FORMAT(/,10X,'CSRX =',F7.4,10X,'FSRX =',F7.4)
      RETURN
      END

```

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STSP0111
STSP0112
STSP0113
STSP0114
STSP0115
STSP0116
STSP0117
STSP0118
STSP0119

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      SUBROUTINE SET1RC(RSTF,KRS,NDRSC,BFRC)
C SETS BEST VALUE FOR ONE RECESSION CONSTANT
      DIMENSION RSTF(50,20)
      RA1 = 0.0
      RA2 = 0.0
      NDRSC1 = NDRSC - 1
      DO 100 KSD = 1,NDRSC1
      RA1 = RA1 + RSTF(KSD,KRS)**2
100 RA2 = RA2 + RSTF(KSD,KRS)*RSTF(KSD+1,KRS)
      BFRC = RA2/RA1
      WRITE(6,1) KRS,BFRC
      1 FORMAT(15X,'KRS =',I3,5X,'BFRC =',F8.4)
      RETURN
      END

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```

ST1R0001
ST1R0002
ST1R0003
ST1R0004
ST1R0005
ST1R0006
ST1R0007
ST1R0008
ST1R0009
ST1R0010
ST1R0011
ST1R0012
ST1R0013
ST1R0014

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      SUBROUTINE SET2RC(RSTF,KRS,NDRSC,IFRC,BFRC,LBFO)
C SETS BEST VALUES FOR TWO RECESSION CONSTANTS
      DIMENSION RSTF(50,20)
      LOGICAL LBFO
      REAL IFRC
      REAL*8 RA1,RA2,RA3,RA4,RA5,CRSTF(50),RA6,DBFRC,DIFRC,RA,RB,RD
      DO 100 KSD = 1,NDRSC

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```

ST2R0001
ST2R0002
ST2R0003
ST2R0004
ST2R0005
ST2R0006
ST2R0007

```

100	CRSTF(KSD) = RSTF(KSD,KRS)	ST2R0008
	NDRSC2 = NDRSC - 2	ST2R0009
	RA1 = 0.0	ST2R0010
	RA2 = 0.0	ST2R0011
	RA3 = 0.0	ST2R0012
	DO 101 KSD = 1, NDRSC2	ST2R0013
	RA1 = RA1 + CRSTF(KSD)**2	ST2R0014
	RA2 = RA2 + CRSTF(KSD)*CRSTF(KSD+1)	ST2R0015
101	RA3 = RA3 + CRSTF(KSD)*CRSTF(KSD+2)	ST2R0016
	RA4 = RA1 + CRSTF(NDRSC-1)**2 - CRSTF(1)**2	ST2R0017
	RA5 = RA2 + CRSTF(NDRSC-1)*CRSTF(NDRSC) - CRSTF(1)*CRSTF(2)	ST2R0018
	RA6 = RA4*RA1 - RA2**2	ST2R0019
	IF(RA6 .EQ. 0.0) GO TO 102	ST2R0020
	RA5 = RA5/RA6	ST2R0021
	RA3 = RA3/RA6	ST2R0022
	RA = -RA1*RA5 - RA2*RA3	ST2R0023
	RB = RA4*RA3 - RA2*RA5	ST2R0024
	RD = RA**2 + 4.0*RB	ST2R0025
	IF(RD .LT. 0.0) GO TO 102	ST2R0026
	DBFRC = (RA + RD**0.5)/2.0	ST2R0027
	DIFRC = RA - DBFRC	ST2R0028
	BFRC = DBFRC	ST2R0029
	IFRC = DIFRC	ST2R0030
	WRITE(6,1) KRS,BFRC,IFRC	ST2R0031
	1 FORMAT(15X,'KRS =',I3,5X,'BFRC =',F8.4,5X,'IFRC =',F8.4)	ST2R0032
	GO TO 103	ST2R0033
102	LBFO = .TRUE.	ST2R0034
	WRITE(6,2) KRS	ST2R0035
	2 FORMAT(/15X,'IMAGINARY VALUES ENCOUNTERED IN SET2RC, SEQUENCE =',	ST2R0036
	1 I3)	ST2R0037
103	RETURN	ST2R0038
	END	ST2R0039

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SUBROUTINE STORRT(SRR,CSRX,FSRX,CHCAP,CONOP2,IBTPS,SHPF,KHYD, SRRT0001

<pre> 1 CHBF,NHPT,KPCH,IBTPR) C PERFORMS CHANNEL STORAGE ROUTING   DIMENSION ASRR(5,21),SRR(5,170)   INTEGER CONOP2,PRD   REAL NHPT   WRITE(6,1) CHBF 1 FORMAT(/25X,'BASE FLOW =',F7.1,' CFS')   TFCFS = CHBF   INHPT = NHPT   MHTP = 1   IF(CONOP2 .EQ. 0) MHTP = 4   INHPT = MHTP*INHPT   SHPF = 0.0   RHFO = 0.9*SRR(KHYD,1)   KAFH = 0   DO 102 KHPT = 1,KPCH   PRD = 0 100 PRD = PRD + 1   TRHF = SRR(KHYD,KHPT)   IF(TFCFS .LE. 0.5*CHCAP) SRX = CSRX   IF((TFCFS .GT. 0.5*CHCAP) .AND. (TFCFS .LT. 2.0*CHCAP)) SRX = 1 CSRX + (FSRX - CSRX)*((TFCFS - 0.5*CHCAP)/(1.5*CHCAP))**3   IF(TFCFS .GE. 2.0*CHCAP) SRX = FSRX   RHF1 = TRHF - SRX*(TRHF - RHFO)   RHFO = RHF1   TFCFS = RHF1 + CHBF   IF(TFCFS .LT. SHPF) GO TO 101   SHPF = TFCFS   IBTPS = KHPT 101 IF(PRD .LE. 3 .AND. CONOP2 .EQ. 1) GO TO 100   KAHP = KHPT - IBTPR*MHTP + 5*INHPT   IF(KAHP .LT. 0) GO TO 102   IF(MOD(KAHP,INHPT) .NE. 0) GO TO 102   KAFH = KAFH + 1   ASRR(KHYD,KAFH) = TFCFS 102 CONTINUE </pre>	<pre> SRRT0002 SRRT0003 SRRT0004 SRRT0005 SRRT0006 SRRT0007 SRRT0008 SRRT0009 SRRT0010 SRRT0011 SRRT0012 SRRT0013 SRRT0014 SRRT0015 SRRT0016 SRRT0017 SRRT0018 SRRT0019 SRRT0020 SRRT0021 SRRT0022 SRRT0023 SRRT0024 SRRT0025 SRRT0026 SRRT0027 SRRT0028 SRRT0029 SRRT0030 SRRT0031 SRRT0032 SRRT0033 SRRT0034 SRRT0035 SRRT0036 SRRT0037 </pre>
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IF(KAFH .EQ. 21) GO TO 104.
KAFH = KAFH + 1
DO 103 KIA = KAFH, 21
103 ASRR(KHYD, KIA) = 0.0
104 WRITE(6, 2) (KHYD, NHPT, (ASRR(KHYD, KWD), KWD = 1, 21))
2 FORMAT(/25X, 'SYNTHESIZED HYDROGRAPH', I3, ' INTERVAL =', F5.2,
1 ' HOURS' /3(22X, 7F10.1/))
WRITE(6, 3) SHPF
3 FORMAT(25X, 'FLOOD PEAK =', F10.1, ' CFS')
RETURN
END

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SRRT0038
SRRT0039
SRRT0040
SRRT0041
SRRT0042
SRRT0043
SRRT0044
SRRT0045
SRRT0046
SRRT0047
SRRT0048

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SUBROUTINE STRHRS(RHPD, RHPH, IDYB, IDYE, IHRB, IHRE, NHPT, MXTRH, DPY,
1 NRHP, IBTPR)
C SETS BEGINNING AND END TIMES OF RUNOFF ENTERING RECORDED HYDROGRAPHS
DIMENSION RHPD(5), RHPH(5), IDYB(5), IDYE(5), IHRB(5), IHRE(5)
INTEGER DAY, DPY, RHPD, RHPH
REAL NHPT
C ESTIMATE HOURS EACH WAY FROM PEAK
INHPT = NHPT
IBTPR = 5*INHPT + MXTRH
IPTE = 15*INHPT
C DETERMINE TIME OF BEGINNING AND ENDING FOR EACH STORM
DO 106 KRHP = 1, NRHP
KHBCK = IBTPR - RHPH(KRHP)
IF(KHBCK .LT. 0) GO TO 101
KDBCK = KHBCK/24 + 1
IHRB(KRHP) = 24*KDBCK - KHBCK
DAY = RHPD(KRHP)
100 DAY = DAY - 1
IF(DAY .EQ. 59 .AND. DPY .EQ. 366) DAY = 366
IF(DAY .EQ. 365) DAY = 59
IF(DAY .EQ. 0) DAY = 365
KDBCK = KDBCK - 1

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SHRS0001
SHRS0002
SHRS0003
SHRS0004
SHRS0005
SHRS0006
SHRS0007
SHRS0008
SHRS0009
SHRS0010
SHRS0011
SHRS0012
SHRS0013
SHRS0014
SHRS0015
SHRS0016
SHRS0017
SHRS0018
SHRS0019
SHRS0020
SHRS0021
SHRS0022

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	IF(KDBCK .GT. 0) GO TO 100	SHRS0023
	IDYB(KRHP) = DAY	SHRS0024
	GO TO 102	SHRS0025
101	IDYB(KRHP) = RHPD(KRHP)	SHRS0026
	IHRB(KRHP) = RHPH(KRHP) - IBTPR	SHRS0027
102	KHFOR = IPTE + RHPH(KRHP)	SHRS0028
	IF(KHFOR .LE. 24) GO TO 105	SHRS0029
	KDFOR = KHFOR/24	SHRS0030
	IHRE(KRHP) = KHFOR - 24*KDFOR	SHRS0031
	IF(IHRE(KRHP) .NE. 0) GO TO 103	SHRS0032
	KDFOR = KDFOR - 1	SHRS0033
103	DAY = RHPD(KRHP)	SHRS0034
104	CALL DAYNXT(DAY,DPY)	SHRS0035
	KDFOR = KDFOR - 1	SHRS0036
	IF(KDFOR .GT. 0) GO TO 104	SHRS0037
	IDYE(KRHP) = DAY	SHRS0038
	GO TO 106	SHRS0039
105	IDYE(KRHP) = RHPD(KRHP)	SHRS0040
	IHRE(KRHP) = RHPH(KRHP) + IPTE	SHRS0041
106	CONTINUE	SHRS0042
C	ELIMINATE HYDRDGRAPH OVERLAPPING	SHRS0043
	NRHP1 = NRHP - 1	SHRS0044
	IF(NRHP1 .EQ. 0) GO TO 109	SHRS0045
	DO 108 KRHP = 1, NRHP1	SHRS0046
	IF(((IDYE(KRHP) .GT. IDYB(KRHP+1)) .AND. (.NOT. ((IDYE(KRHP) .GE. 1	SHRS0047
	1 274 .AND. IDYB(KRHP+1) .LE. 273) .OR. IDYE(KRHP) .EQ. 366))) .OR.	SHRS0048
	2 ((IDYE(KRHP) .EQ. IDYB(KRHP+1)) .AND. IHRE(KRHP) .GT. IHRB(KRHP+1))	SHRS0049
	3 )) GO TO 107	SHRS0050
	GO TO 108	SHRS0051
107	IDYE(KRHP) = IDYB(KRHP+1)	SHRS0052
	IHRE(KRHP) = IHRB(KRHP+1)	SHRS0053
108	CONTINUE	SHRS0054
109	IF((IDYB(1) .LE. 273 .AND. RHPD(1) .GE. 274 .AND. RHPD(1) .NE. 366)	SHRS0055
	1 GO TO 110	SHRS0056
	GO TO 111	SHRS0057
110	IDYB(1) = 274	SHRS0058

IHRB(1) = 1	SHRS0059
111 IF(IDYE(NRHP) .GE. 274 .AND. RHPD(NRHP) .LE. 273 .AND. IDYE(NRHP)	SHRS0060
1 .NE. 366) GO TO 112	SHRS0061
GO TO 113	SHRS0062
112 IDYE(NRHP) = 273	SHRS0063
IHRE(NRHP) = 24	SHRS0064
113 CONTINUE	SHRS0065
DO 114 KRHP = 1, NRHP	SHRS0066
WRITE(6,1) KRHP, IDYB(KRHP), IHRB(KRHP), IDYE(KRHP), IHRE(KRHP)	SHRS0067
1 FORMAT(5X, 'RUNOFF CONTRIBUTING TO RECORDED HYDROGRAPH', I2/10X,	SHRS0068
1 'BEGINS ON DAY', I4, ' AT HOUR', I3/10X, 'AND ENDS ON DAY', I4,	SHRS0069
2 ' AT HOUR', I3)	SHRS0070
114 CONTINUE	SHRS0071
RETURN	SHRS0072
END	SHRS0073

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SUBROUTINE TIMERT(SSR, SRR, CTRI, NCTRI, KRHP, KPCH)	TMRT0001
C PERFORMS CHANNEL TIME ROUTING	TMRT0002
DIMENSION SSR(5,170), SRR(5,170), CTRI(99)	TMRT0003
DO 100 KHPT = 1, KPCH	TMRT0004
100 SRR(KRHP, KHPT) = 0.0	TMRT0005
KTRI = 1	TMRT0006
101 CONTINUE	TMRT0007
DO 102 KHPT = KTRI, KPCH	TMRT0008
NRTRI = KHPT - KTRI + 1	TMRT0009
102 SRR(KRHP, KHPT) = CTRI(KTRI)*SSR(KRHP, NRTRI) + SRR(KRHP, KHPT)	TMRT0010
KTRI = KTRI + 1	TMRT0011
IF(KTRI .LE. NCTRI) GO TO 101	TMRT0012
RETURN	TMRT0013
END	TMRT0014

APPENDIX C

DICTIONARY OF VARIABLES

USED IN

THE KENTUCKY WATERSHED MODEL AND OPSET

ITEM 1 - VARIABLE NAME

ITEM 2 - WHETHER VARIABLE IS REAL, INTEGER, OR LOGICAL

ITEM 3 - VARIABLE DIMENSIONS

ITEM 4 - UNITS

ITEM 5 - DEFINITION OF THE VARIABLE

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1	2	3	4	5
ABFSL	R	1	DAY	ACCUMULATED BASE FLOW SEQUENCE LENGTH
ABFV	R	1	IN	ANNUAL BASE FLOW VOLUME
ACRFMI	R	1	-	ACCUMULATED CASES IN ALL RECORDED FLOOD MAGNITUDE INTERVALS
ADBF	R	1	IN	ACCUMULATED DAILY BASE FLOW
ADIF	R	1	IN	ACCUMULATED DAILY INTERFLOW
ADRSP	R	1	CFS	ACCUMULATED SUM OF DRSP
AETX	R	1	IN	ANNUAL EVAPOTRANSPIRATION INDEX
AEX90	R	1	IN	ANTECEDENT EVAPORATION INDEX, DECAY RATE = 0.9
AEX96	R	1	IN	ANTECEDENT EVAPORATION INDEX, DECAY RATE = 0.96
AFSIL	R	1	IN	ANNUAL FOREST SNOW INTERCEPTION LOSS
AHP	R	1	IN	ACCUMULATED HOURLY PRECIPITATION



AIFSL	R	1 DAY	ACCUMULATED INTERFLOW SEQUENCE LENGTH
AIFV	R	1 IN	ANNUAL INTERFLOW VOLUME
AMBER	R	1 IN	ANNUAL MOISTURE BALANCE ERROR
AMBF	R	1 IN	ACCUMULATED MONTHLY BASE FLOW
AMFSIL	R	1 IN	ACCUMULATED MONTHLY FOREST SNOW INTERCEPTION LOSS
AMIF	R	1 IN	ACCUMULATED MONTHLY INTERFLOW
AMNET	R	1 IN	ACCUMULATED MONTHLY NET EVAPOTRANSPIRATION
AMPET	R	1 IN	ACCUMULATED MONTHLY POTENTIAL EVAPOTRANSPIRATION
AMPREC	R	1 IN	ACCUMULATED MONTHLY PRECIPITATION
AMRPM	R	1 IN	ACCUMULATED MONTHLY RAIN PLUS MELT
AMRTF	R	1 CFS	ACCUMULATED MONTHLY RECORDED TOTAL FLOW
AMSE	R	1 IN	ACCUMULATED MONTHLY STREAM EVAPORATION
AMSNE	R	1 IN	ACCUMULATED MONTHLY SNOW EVAPORATION
AMSTF	R	1 CFS	ACCUMULATED MONTHLY SYNTHESIZED TOTAL FLOW
ANET	R	1 IN	ANNUAL NET EVAPOTRANSPIRATION
ADFV	R	1 IN	ANNUAL OVERLAND FLOW VOLUME
APET	R	1 IN	ANNUAL POTENTIAL EVAPOTRANSPIRATION
APPKP	R	1 -	ACCUMULATED PARAMETER PEAK PRODUCTS
APREC	R	1 IN	ANNUAL PRECIPITATION
AREA	R	1 SQ MI	AREA OF WATERSHED
ARHF	R	1 IN	ACCUMULATED ROUTED HYDROGRAPH FLOW
ARHPF	R	1 CFS	ACCUMULATED RECORDED HYDROGRAPH PEAK FLOWS
ARPM	R	1 IN	ANNUAL RAIN PLUS MELT
ARSTR	R	1 -	ACCUMULATED RATIO OF SYNTHESIZED TO RECORDED FLOWS
ASE	R	1 IN	ANNUAL SNOW EVAPORATION
ASEV	R	1 IN	ANNUAL STREAM EVAPORATION VOLUME
ASM	R	1 IN	ANNUAL SNOWFALL MOISTURE
ASMRG	R	1 IN	ANNUAL SNOWFALL MOISTURE REACHING GROUND
ASRR	R	5,21 CFS	ABSTRACTED SYNTHESIZED ROUTED RUNOFFS
ASRX	R	1 -	AVERAGE VALUE OF SRX
ATF	R	1 SFD	ACCUMULATED TOTAL FLOW
ATFV	R	1 SFD	ANNUAL TOTAL FLOW VOLUME
AVRHPF	R	1 CFS	AVERAGE VALUE OF RHPF
AWSBIT	R	99 -	ACCUMULATOR FOR WATERSHED BITS
BBMIR	R	1 IN/HR	CURRENT BEST ESTIMATE OF BASIC MAXIMUM INFILTRATION RATE
BBUZC	R	1 -	CURRENT BEST ESTIMATE OF BASIC UPPER ZONE STORAGE

CAPACITY FACTOR

BBYLZS	R	1	IN	CURRENT BEST ESTIMATE OF BEGINNING OF YEAR LOWER ZONE STORAGE
BDDFSM	R	1	IN/HR	BASIC DEGREE DAY FACTOR FOR SNOW MELT
BETLF	R	1	-	CURRENT BEST ESTIMATE OF EVAPOTRANSPIRATION LOSS FACTOR
BFHRC	R	1	-	BASE FLOW HOURLY RECESSION CONSTANT
BFNHR	R	1	-	BASE FLOW HOURLY NONLINEAR RECESSION ADJUSTMENT FACTOR
BFNLR	R	1	-	BASE FLOW NONLINEAR RECESSION ADJUSTMENT FACTOR
BFNRL	R	1	-	BASE FLOW NONLINEAR RECESSION LOGARITHM
BFNX	R	1	IN	CURRENT VALUE OF BASE FLOW NONLINEAR RECESSION INDEX
BFRC	R	1	-	BASE FLOW RECESSION CONSTANT
BFRL	R	1	-	BASE FLOW RECESSION LOGARITHM
BFSRX	R	1	-	CURRENT BEST ESTIMATE OF FSRX
BISRX	R	1	-	BIG INCREMENTAL STORAGE ROUTING INDEX
BIVF	R	1	-	BASIC INTERFLOW VOLUME FACTOR
BLZC	R	1	IN	CURRENT BEST ESTIMATE OF LOWER ZONE STORAGE CAPACITY
BMIR	R	1	IN/HR	BASIC MAXIMUM INFILTRATION RATE WITHIN WATERSHED
BSIAC	R	1	-	CURRENT BEST ESTIMATE OF SEASONAL INFILTRATION ADJUSTMENT FACTOR
BSUZC	R	1	-	CURRENT BEST ESTIMATE OF SEASONAL UPPER ZONE STORAGE CAPACITY FACTOR
BTRI	R	99	-	BASE TIME ROUTING INCREMENTS
BUZC	R	1	-	BASIC UPPER ZONE STORAGE CAPACITY FACTOR
BYGWS	R	1	IN	BEGINNING OF YEAR GROUNDWATER STORAGE
BYIFS	R	1	IN	BEGINNING OF YEAR INTERFLOW STORAGE
BYLZS	R	1	IN	BEGINNING OF YEAR LOWER ZONE STORAGE
BYUZS	R	1	IN	BEGINNING OF YEAR UPPER ZONE STORAGE
CBF	R	1	IN/HR	CURRENT BASE FLOW
CCRFMI	R	1	-	CASES IN CURRENT RECORDED FLOW MAGNITUDE INTERVAL
CDSDR	I	1	-	CURRENT DAY FOR WHICH STORM DETAILS REQUESTED
CHBF	R	1	CFS	CURRENT HYDROGRAPH BASE FLOW
CHCAP	R	1	CFS	CHANNEL CAPACITY - INDEXED TO BASIN OUTLET
CHPV	R	1	-	CURRENT HYDROGRAPH PARAMETER VALUE
CIVM	R	1	-	CURRENT INTERFLOW VOLUME MULTIPLIER
CMIR	R	1	IN	CURRENT MAXIMUM INFILTRATION RATE DURING PERIOD
CN	I	1	-	1 = A. M., 2 = P. M.

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CONOPT I	15	-	CONTROL OPTION
CONOP2 I	1	-	SECOND CONTROL OPTION
CRFMI R	22	-	CASES RECORDED IN FLOW MAGNITUDE INTERVAL
CRSBBF R	1	CFS	CURRENT RECESSION SEQUENCE BEGINNING BASE FLOW
CRSBIF R	1	CFS	CURRENT RECESSION SEQUENCE BEGINNING INTERFLOW
CRSBTF R	1	CFS	CURRENT RECESSION SEQUENCE BEGINNING TOTAL FLOW
CRSTF R	50	CFS	CURRENT RECESSION SEQUENCE TOTAL FLOWS
CRSOBF R	1	CFS	CURRENT RECESSION SEQUENCE BASE FLOW ON DAY ZERO
CRSOIF R	1	CFS	CURRENT RECESSION SEQUENCE INTERFLOW ON DAY ZERO
CSRX R	1	-	CHANNEL STORAGE ROUTING INDEX
CTRI R	99	-	CURRENT TIME ROUTING INCREMENTS
DATE I	1	-	CURRENT DAY OF THE MONTH
DAY I	1	-	CURRENT DAY OF THE YEAR
DBFRC R	1	-	DOUBLE PRECISION BFRC
DDIW R	366	CFS	DATED DIVERSIONS INTO WATERSHED
DFCC R	1	-	DAILY FLOW CORRELATION COEFFICIENT
DIFRC R	1	-	DOUBLE PRECISION IFRC
DIV R	1	CFS	DIVERSION INTO BASIN, MEAN DAILY FLOW
DMNT R	366	DEGF	DATED MINIMUM TEMPERATURE
DMXT R	366	DEGF	DATED MAXIMUM TEMPERATURE
DNFS R	1	-	DENSITY OF NEW FALLEN SNOW
DPET R	366	IN	DATED POTENTIAL EVAPOTRANSPIRATION
DPSE R	366	IN	DATED POTENTIAL SNOW EVAPORATION
DPY I	1	-	DAYS PER YEAR
DRAF R	1	CFS	DIFFERENCE BETWEEN RECORDED AND AVERAGE FLOW
DRGPM R	366	-	DATED RECORDING GAGE PRECIPITATION MULTIPLIER
DRHP R	366,24	IN	DATED RECORDED HOURLY PRECIPITATION
DRSF R	366	CFS	DATED RECORDED STREAMFLOW
DRSGP R	366	IN	DATED RECORDED STORAGE GAGE PRECIPITATION
DRSP R	1	CFS	DIFFERENCE BETWEEN RECORDED AND SYNTHESIZED HYDROGRAPH PEAKS
DSAF R	1	CFS	DIFFERENCE BETWEEN SYNTHESIZED AND AVERAGE FLOW
DSMGH R	1	IN	RATE OF DAILY SNOWMELT FROM GROUND HEAT
DSSF R	366	CFS	DATED SYNTHESIZED STREAMFLOW
EDLZS R	366	IN	END OF DAY VALUES OF LZS
EHSGD I	1	-	ENDING HOUR OF STORAGE GAGE DAY

EHSGDF	R	1	-	ENDING HOUR OF STORAGE GAGE DAY - FLOATING POINT
EID	R	1	-	EXPONENT OF INFILTRATION RATE DECAY WITH INCREASED SOIL MOISTURE CONTENT
ELDIF	R	1	1000FT	ELEVATION DIFFERENCE BETWEEN BASE THERMOMETER AND BASIN MEAN ELEVATION
EMAET	R	1	IN	ESTIMATED MAXIMUM ANNUAL EVAPOTRANSPIRATION
EMATF	R	13	SFD	END OF MONTH ACCUMULATED TOTAL FLOWS
EMBFNX	R	12	IN	END OF MONTH BASE FLOW NONLINEAR RECESSION INDEX
EMFDP	R	1	-	EXTREME MONTHLY FLOW DEVIATION PARAMETER
EMGWS	R	12	IN	END OF MONTH GROUNDWATER STORAGE
EMIFS	R	12	IN	END OF MONTH INTERFLOW STORAGE
EMLZS	R	12	IN	END OF MONTH LOWER ZONE STORAGE
EMSIAM	R	12	-	END OF MONTH SEASONAL INFILTRATION ADJUSTMENT MULTIPLIER
EMUZC	R	12	IN	END OF MONTH UPPER ZONE STORAGE CAPACITY
EMUZS	R	12	IN	END OF MONTH UPPER ZONE STORAGE
EPAET	R	1	IN	ESTIMATED POTENTIAL ANNUAL EVAPOTRANSPIRATION
EPCM	R	12	-	EVAPORATION PAN COEFFICIENT FOR MONTH
EPS	R	1	-	MAXIMUM REQUIRED ESTIMATING TOLERANCE
EQD	R	1	IN	EQUILIBRIUM DEPTH OF OVERLAND FLOW
EQDF	R	1	-	EQUILIBRIUM DEPTH FACTOR FOR OVERLAND FLOW
EQDFIS	R	1	-	EQUILIBRIUM DEPTH FACTOR FOR OVERLAND FLOW, IMPERVIOUS SURFACES
EQDIS	R	1	IN	EQUILIBRIUM DEPTH OF OVERLAND FLOW IMPERVIOUS SURFACES
ERR	R	1	CFS	DIFFERENCE BETWEEN RECORDED AND SYNTHESIZED DATED STREAMFLOW
ETIBF	R	1	CFS	ERROR TABLE INTERVAL BOUNDARY FLOODS
ETLF	R	1	-	EVAPOTRANSPIRATION LOSS FACTOR
EXQPV	R	1	-	EXPONENT OF FLOW PROPORTIONAL TO VELOCITY
FBUZC	R	1	-	ADJUSTMENT FACTOR FOR BUZC
FCNTRI	R	1	-	FLOATING POINT CHANGE IN NUMBER OF TIME ROUTING INCREMENTS
FDAY	R	1	-	FLOATING POINT CURRENT DAY OF THE YEAR
FDPY	R	1	-	FLOATING POINT DAYS PER YEAR
FDSC	R	1	-	FIRST DIFFERENTIAL OF SINE CURVE MAGNITUDE
FETLF	R	1	-	ADJUSTMENT FACTOR FOR ETLF
FFOR	R	1	-	FRACTION OF THE WATERSHED BEING FOREST

FFSI R	1 -	FRACTION OF SNOW ON FOREST INTERCEPTED
FHPP R	1 -	FRACTIONAL HOUR PER PERIOD
FIMP R	1 -	FRACTION OF THE WATERSHED BEING IMPERVIOUS
FIRR R	15 -	FRACTION OF INCOMING RADIATION REFLECTED BY SNOW SURFACE AS A FUNCTION OF AGE
FKRFMI R	1 -	FLOATING POINT VALUE OF KRFMI
FLZC R	1 -	ADJUSTMENT FACTOR FOR LZC
FMR R	1 -	FRACTION OF MOISTURE RETENTION
FMXTRI R	1 -	FLOATING POINT MAXIMUM NUMBER OF TIME ROUTING INCREMENTS
FNBTRI R	1 -	FLOATING POINT NUMBER OF BASIC TIME ROUTING INCREMENTS
FNCTRH R	1 -	FLOATING POINT NUMBER OF CURRENT TIME ROUTING HOURS
FNOFM R	1 -	FLOATING POINT NUMBER OF OVERLAND FLOW MONTHS
FNPTRI R	1 -	FLOATING POINT NUMBER OF PREVIOUS TIME ROUTING INCREMENTS
FNRHP R	1 -	FLOATING POINT NUMBER OF RECORDED HYDROGRAPH PEAKS
FNSTRI R	1 -	FLOATING POINT NUMBER OF SUBSEQUENT TIME ROUTING INCREMENTS
FNTRI R	1 -	FLOATING POINT NUMBER OF TIME ROUTING INCREMENTS
FPER R	1 -	FRACTION OF THE WATERSHED BEING PERVIOUS
FRERS R	1 CFS	FLOW RISE ENDING RECESSION SEQUENCE
FSIAC R	1 -	ADJUSTMENT FACTOR FOR SIAC
FSIL R	1 IN	HOURLY FOREST SNOW INTERCEPTION LOSS
FSRX R	1 -	FLOOD PLAIN STORAGE ROUTING INDEX
FSUZC R	1 -	ADJUSTMENT FACTOR FOR SUZC
FTA R	1 -	FACTOR FOR ESTIMATING DIURNAL TEMPERATURE VARIATION BASED ON SINE CURVE
FTX R	1 -	FALL TROUBLE INDEX
FWTR R	1 -	FRACTION OF THE WATERSHED BEING WATER
GWET R	1 IN	CURRENT HOURLY GROUNDWATER EVAPOTRANSPIRATION
GWETF R	1 -	GROUNDWATER EVAPOTRANSPIRATION FACTOR
GWS R	1 IN	CURRENT GROUNDWATER STORAGE
HBF R	5 CFS	HYDROGRAPH BASE FLOW
HBFM R	1 -	HYDROGRAPH BASE FLOW MULTIPLIER
HNTRI I	5 -	HYDROGRAPH NUMBER OF TIME ROUTING INCREMENTS
HOUR I	1 -	CURRENT HOUR OF THE DAY
HOURF R	1 HR	CURRENT HOUR OF THE DAY, FLOATING POINT
HRF I	1 -	FIRST HOUR OF LOOP

HRL I	1 -	LAST HOUR OF LOOP
HSE R	1 IN	CURRENT HOURLY STREAM EVAPORATION
HSF R	1 IN	HOURLY SNOWFALL
HSFRG R	1 IN	HOURLY SNOWFALL REACHING GROUND
HSM R	1 IN	HOURLY SNOWMELT RATE
HSRX R	5 -	HYDROGRAPH STORAGE ROUTING INDEX
HTH R	1 -	HOURS INTO DAY WHEN HYDROGRAPH STOPS OR STARTS
IBTPR I	1 HR	TIME FROM BEGINNING OF SAVED RUNOFF TO RECORDED HYDROGRAPH PEAK
IBTPS I	1 -	TIME FROM BEGINNING OF SAVED RUNOFF TO SYNTHESIZED HYDROGRAPH PEAK
IDAY1 I	1 -	INDEX TO 10-DAY PERIOD
IDAY2 I	1 -	INDEX WITHIN 10-DAY PERIOD
IDYB I	5 -	DAY OF ROUTING HYDROGRAPH BEGINNING
IDYE I	5 -	DAY OF ROUTING HYDROGRAPH ENDING
IFPRC R	1 -	INTERFLOW PERIOD RECESSIOIN CONSTANT
IFRC R	1 -	INTERFLOW RECESSIOIN CONSTANT
IFRL R	1 -	INTERFLOW RECESSIOIN LOGARITHM
IFS R	1 IN	INTERFLOW STORAGE
IFT I	1 -	INDICATOR OF FALL TROUBLE (SKIP FIRST RECESSIOIN IN EVALUATION OF BMIR)
IHRB I	5 -	HOUR OF DAY OF ROUTING HYDROGRAPH BEGINNING
IHRE I	5 -	HOUR OF DAY OF ROUTING HYDROGRAPH ENDING
INHPT I	1 HR	INTEGER NUMBER OF HOURS BETWEEN HYDROGRAPH PRINTING POINTS
IPPH I	1 -	INTEGER PERIODS PER HOUR
IPTE I	1 HR	TIME FROM PEAK OF RECORDED HYDROGRAPH TO END OF SAVED RUNOFF
ISGRD I	1 -	CURRENT STORAGE GAGE RAINFALL DAY
IWBG I	1 -	INDEX NUMBER OF WEATHER BUREAU PRECIPITATION GAGE
KA A I	1 -	COUNTER OF APPROPRIATE ELEMENT FROM ALBEDO ARRAY
KA A O I	1 -	PRECEDING VALUE OF KAA
KAFH I	1 -	COUNTER FOR ABSTRACTED FLOW HYDROGRAPH
KAHP I	1 -	COUNTER FOR ABSTRACTING HYDROGRAPH POINTS
KBRC I	1 -	COUNTER OF ROUGH CYCLES SINCE BEST ONE
KB1-7 I	1 -	COUNTERS FOR COMBINING WATERSHED BITS

KDBCK	I	1 -	BACKWARD DAY COUNTER
KDFOR	I	1 -	FORWARD DAY COUNTER
KDRS	I	1 -	COUNTER OF CURRENT DAY IN RECESSION SEQUENCE
KDY	I	1 -	COUNTER FOR DAY
KFFC	I	1 -	COUNTER EQUALLING ONE ON FIRST FINE ADJUSTMENT CYCLE
KHBCK	I	1 -	BACKWARD HOUR COUNTER
KHFOR	I	1 -	FORWARD HOUR COUNTER
KHOUR	I	1 -	COUNTER FOR HOUR OF DAY
KHPT	I	1 -	COUNTER OF CURRENT HYDROGRAPH POINT
KHYD	I	1 -	COUNTER SPECIFYING CURRENT HYDROGRAPH
KHI-3	I	1 -	COUNTERS FOR FIXING HYDROGRAPH ROUTING PARAMETERS
KIA	I	1 -	COUNTER FOR INITIALIZING ARRAYS
KISRX	I	1 -	COUNTER FOR INCREMENTING STORAGE ROUTING INDEX
KLCCA	I	1 -	COUNTER FOR LIMITING NUMBER OF CHANNEL CAPACITY ADJUSTMENTS
KMD	I	1 -	COUNTER INDEXING MONTH OF THE YEAR
KMI-6	I	1 -	MONTH COUNTERS
KPA	I	1 -	COUNTER DESIGNATING PARAMETER TO BE AVERAGED
KPCH	I	1 -	COUNTED POINTS IN CURRENT HYDROGRAPH
KPRD	I	1 -	COUNTER FOR PERIOD
KPSH	I	5 -	COUNTER POINTS IN SUBSCRIPTED HYDROGRAPH
KRC	I	1 -	COUNTER OF CURRENT ROUGH CYCLE
KRD	I	1 -	COUNTER FOR READING DATA ARRAYS
KRFMI	I	1 -	COUNTER FOR RECORDED FLOW MAGNITUDE INTERVAL
KRHP	I	1 -	COUNTER FOR RECORDED HYDROGRAPH PEAKS
KRIA	I	1 -	COUNTER OF APPROPRIATE ELEMENT FROM RADIATION INCIDENCE ARRAY
KRS	I	1 -	COUNTER FOR RECESSION SEQUENCE NUMBER
KSD	I	1 -	COUNTER FOR RECESSION SEQUENCE DAYS
KSQ	I	1 -	COUNTER FOR REVISED SEQUENCES
KTA	I	1 -	COUNTER FOR TITLE ARRAY
KTRI	I	1 -	COUNTER FOR TIME ROUTING INCREMENTS
KT20	I	1 -	COUNTER FOR TOP 20 VALUES
KWD	I	1 -	COUNTER FOR WRITING DATA ARRAYS
KWSM	I	1 -	COUNTER OF WET SUMMER MONTHS
KIAH	I	1 -	COUNTER FOR FIRST ACCEPTED HYDROGRAPH

K2AH	I	1	-	COUNTER FOR SECOND ACCEPTED HYDROGRAPH
LBFO	L	20	-	LOGICAL VARIABLE SET TRUE WHERE BASE FLOW ONLY ENCOUNTERED
LBMIR	L	1	-	LOGICAL VARIABLE SET TRUE WHEN EXERCIZING SUBSTITUTE APPROACH FOR EVALUATING BMIR
LBUZC	L	1	-	LOGICAL VARIABLE SET TRUE WHEN EXERCIZING SUBSTITUTE APPROACH FOR EVALUATING BUZC
LDAY	I	1	-	LAST DAY OF YEAR
LETLF	L	1	-	LOGICAL VARIABLE SET TRUE WHEN EXERCIZING SUBSTITUTE APPROACH FOR EVALUATING ETLF
LHOUR	I	1	-	LAST HOUR OF DAY
LLZC	L	1	-	LOGICAL VARIABLE SET TRUE WHEN EXERCIZING SUBSTITUTE APPROACH FOR EVALUATING LZC
LNIBRS	I	1	-	LAST VALUE OF NIBRS
LNPR	L	1	-	LOGICAL VARIABLE SET TRUE FOR NONEQUAL PERIOD RAINFALL
LNTRI	I	1	-	LAST NUMBER OF TIME ROUTING INCREMENTS
LRC	L	1	-	LOGICAL VARIABLE SET TRUE DURING ROUGH ADJUSTMENT CYCLES
LSHA	L	5	-	LOGICAL VARIABLE KEPT TRUE WHILE SYNTHESIZED HYDROGRAPH IS ACCEPTED FOR COMPARISON WITH RECORDED HYDROGRAPH
LSHFT	L	1	-	LOGICAL VARIABLE SET TRUE WHILE SHIFTING THE NUMBER OF TIME ROUTING INCREMENTS
LSHP	L	1	-	LOGICAL VARIABLE SET TRUE DURING STORM HYDROGRAPH PERIODS
LZC	R	1	IN	LOWER ZONE STORAGE CAPACITY
LZRX	R	1	-	LOWER ZONE MOISTURE RETENTION INDEX
LZS	R	1	IN	CURRENT LOWER ZONE STORAGE
LZSR	R	1	-	CURRENT LOWER ZONE STORAGE RATIO (LZS/LZC)
MBDS	I	1	-	MONTH BEGINNING DRY SEASON
MBWS	I	1	-	MONTH BEGINNING WET SEASON
MDAY	I	1	-	DAY OF YEAR OF LAST DAY OF PREVIOUS MONTH
MEDCY	I	12	-	MONTH END DATES - CALENDAR YEAR
MEDWY	I	12	-	MONTH END DATES - WATER YEAR
MFDPR	R	12	-	MONTHLY FLOW DEVIATION PARAMETER
MHSM	R	1	IN	MINIMUM HOURLY SNOWMELT RATE
MHTP	I	1	-	MULTIPLIER CONVERTING FROM HOURS TO PERIODS
MNDRS	I	1	-	MAXIMUM NUMBER OF DAYS IN RECESSION SEQUENCE
MNRC	I	1	-	MINIMUM NUMBER OF ROUGH CYCLES



MNRD	R	1	-	MEAN ANNUAL NUMBER OF RAINY DAYS
MNTRI	I	1	-	MINIMUM NUMBER OF TIME ROUTING INCREMENTS
MX	I	1	-	MONTH INDEX
MONTH	I	1	-	CURRENT MONTH OF THE YEAR
MONTH1	I	1	-	COUNTER FOR BEGINNING MONTH
MRNSM	R	1	IN	MAXIMUM RATE OF NEGATIVE SNOWMELT (SNOW CHILLING)
MRSL	I	1	DAY	MINIMUM RECESSON SEQUENCE LENGTH
MXA	I	12	-	MONTH INDEX ARRAY (SPECIFYING MONTHS USED IN PARAMETER ADJUSTMENT)
MXTRH	I	1	-	MAXIMUM NUMBER OF TIME ROUTING HOURS
MXTRI	I	1	-	MAXIMUM NUMBER OF TIME ROUTING INCREMENTS
MIR	I	1	-	MONTH WITH MOST RUNOFF
M1SP	I	1	-	MONTH WITH MOST SUMMER PRECIPITATION
M11	I	1	-	SET AT 11 IF AUGUST IS A BASE FLOW MONTH
M12	I	1	-	SET AT 12 IF SEPTEMBER IS A BASE FLOW MONTH
M2R	I	1	-	MONTH WITH SECOND MOST RUNOFF
M2SP	I	1	-	MONTH WITH SECOND MOST SUMMER PRECIPITATION
NATRH	I	1	-	NUMBER OF ANTICIPATED TIME ROUTING HOURS
NBTRI	I	1	-	NUMBER OF BASE TIME ROUTING INCREMENTS
NCSTRI	I	1	-	NUMBER OF CURRENT TIME ROUTING INCREMENTS DURING SHIFTING
NCTRH	I	1	-	NUMBER OF CURRENT TIME ROUTING HOURS
NCTRI	I	1	-	NUMBER OF CURRENT TIME ROUTING INCREMENTS
NDAY	I	1	-	NEXT DAY OF YEAR
NDRS	I	20	DAY	NUMBER OF DAYS IN RECESSON SEQUENCE
NDRSC	I	1	DAY	NUMBER OF DAYS IN CURRENT RECESSON SEQUENCE
NDRSC1	I	1	DAY	NUMBER OF DAYS IN CURRENT RECESSON SEQUENCE LESS 1
NDRSC2	I	1	DAY	NUMBER OF DAYS IN CURRENT RECESSON SEQUENCE LESS 2
NSDP	I	1	DAY	NUMBER OF DAYS FOR WHICH STORM DETAILS HAVE ALREADY BEEN PRINTED
NSDDR	I	1	DAY	NUMBER OF DAYS FOR WHICH STORM DETAILS REQUESTED
NFRHA	I	1	-	NUMBER OF FIRST RAINFALL HOUR ADJUSTED, PREVIOUS DAY
NFTR	I	1	-	NUMBER OF FIRST TRIP TO BE RUN FOR A GIVEN STATION YEAR
NHOUR	I	1	-	NEXT HOUR OF DAY
NHPT	R	1	HR	NUMBER OF HOURS BETWEEN HYDROGRAPH PRINTING POINTS
NIBRS	I	1	-	NUMBER OF TIME ROUTING INTERVALS BETWEEN RECORDED AND SYNTHESIZED PEAKS

NIRTS	I	1 -	NUMBER OF TIME ROUTING INTERVALS FROM RECORDED TO SYNTHESIZED PEAK
NLTR	I	1 -	NUMBER OF LAST TRIP TO BE RUN FOR A GIVEN STATION YEAR
NNSTRI	I	1 -	NUMBER OF NEXT TIME ROUTING INCREMENT DURING SHIFTING
NOFM	I	1 -	NUMBER OF OVERLAND FLOW MONTHS
NORHP	I	1 -	NUMBER OF ORIGINAL RECORDED HYDROGRAPH PEAKS
NRHA	I	1 -	NUMBER OF RAINFALL HOURS ADJUSTED, CURRENT DAY
NRHP	I	1 -	NUMBER OF RECORDED HYDROGRAPH PEAKS
NRHP1	I	1 -	NUMBER OF RECORDED HYDROGRAPH PEAKS LESS ONE
NRS	I	1 -	NUMBER OF RECESSION SEQUENCES
NRTRI	I	1 -	NUMBER OF TIME ROUTING INCREMENTS REMAINING TO BE ROUTED
NSGRD	I	1 -	NUMBER OF STORAGE GAGE RAINFALL DAYS
NSYC	I	1 -	NUMBER OF STATION YEAR, CURRENT ONE BEING RUN
NSYT	I	1 -	NUMBER OF STATION YEARS, TOTAL INCLUDED IN A GIVEN JOB
NTRS	I	1 -	NUMBER OF TENTATIVE RECESSION SEQUENCES
OCT1BF	R	1 -	OCTOBER FIRST BASE FLOW
OFMN	R	1 -	OVERLAND FLOW MANNING'S N
OFMNIS	R	1 -	OVERLAND FLOW MANNING'S N, IMPERVIOUS SURFACES
OFR	R	1 IN	CURRENT OVERLAND FLOW RUNOFF
OFRF	R	1 -	OVERLAND FLOW ROUTING FACTOR
OFRFIS	R	1 -	OVERLAND FLOW ROUTING FACTOR, IMPERVIOUS SURFACES
OFRIS	R	1 IN	CURRENT OVERLAND FLOW RUNOFF, IMPERVIOUS SURFACES
OFS	R	1 IN	OVERLAND FLOW STORAGE
OFSL	R	1 FT	OVERLAND FLOW SURFACE LENGTH
OFSS	R	1 -	OVERLAND FLOW SURFACE SLOPE
OFUS	R	1 IN	CURRENT OVERLAND FLOW UNROUTED STORAGE
OFUSIS	R	1 IN	CURRENT OVERLAND FLOW UNROUTED STORAGE, IMPERVIOUS SURFACES
PBIVF	R	1 -	PREVIOUS VALUE OF BIVF
PBMIR	R	1 IN/HR	PREVIOUS VALUE OF BMIR
PBUZC	R	1 -	PREVIOUS ESTIMATE OF BASIC UPPER ZONE STORAGE CAPACITY FACTOR
PDAY	I	1 -	PREVIOUS DAY OF THE YEAR
PEAI	R	1 IN	PRECIPITATION EXCESS AFTER INFILTRATION
PEBI	R	1 IN	PRECIPITATION EXCESS, BEFORE INFILTRATION
PEIS	R	1 IN	PRECIPITATION EXCESS ON IMPERVIOUS SURFACES

PEP R	1 IN	PRECIPITATION ESTIMATED FOR PERIOD
PET R	1 IN	CURRENT DAILY POTENTIAL EVAPOTRANSPIRATION
PETLF R	1 -	PREVIOUS ESTIMATE OF EVAPOTRANSPIRATION LOSS FACTOR
PETU R	1 IN	UNADJUSTED CURRENT DAILY POTENTIAL EVAPOTRANSPIRATION
PE4P R	4 IN	PRECIPITATION ESTIMATES FOR 4 PERIODS
PGW R	1 IN	PERCOLATION TO GROUND WATER
PLZC R	1 IN	PREVIOUS ESTIMATE OF LZC
PLZS R	1 IN	PERCOLATION TO LOWER ZONE STORAGE
PMEIFS R	1 IN	PERIOD MOISTURE ENTERING INTERFLOW STORAGE
PMELZS R	1 IN	PERIOD MOISTURE ENTERING LOWER ZONE STORAGE
PMEOFS R	1 IN	PERIOD MOISTURE ENTERING OVERLAND FLOW STORAGE
PMEUZS R	1 IN	PERIOD MOISTURE ENTERING UPPER ZONE STORAGE
PPH R	1 -	PERIODS PER HOUR
PPI R	1 IN	PRECIPITATION PASSING INTERCEPTION
PRD I	1 -	CURRENT PERIOD OF THE HOUR
PRDF R	1 -	CURRENT PERIOD OF THE HOUR-FLOATING POINT
PRH R	1 IN	PRECIPITATION RECORDED FOR HOUR
PRLH R	1 IN	PRECIPITATION RECORDED FOR LAST HOUR
PRM1 R	1 IN	PRECIPITATION DURING WETTEST MONTH
PRM2 R	1 IN	PRECIPITATION DURING SECOND WETTEST MONTH
PRNH R	1 IN	PRECIPITATION RECORDED FOR NEXT HOUR
PSIAC R	1 -	PREVIOUS ESTIMATE OF SEASONAL INFILTRATION ADJUSTMENT
PSUZC R	1 -	PREVIOUS ESTIMATE OF SEASONAL UPPER ZONE STORAGE CAPACITY FACTOR
PXCSA R	1 IN	PRECIPITATION INDEX FOR CHANGING SNOW ALBEDO
RA R	1 -	RECESSION ALPHA
RAA R	1 IN	RAINFALL ADJUSTMENT ADDITION
RADF R	1 CFS	RECORDED AVERAGE DAILY FLOW
RAM R	1 -	RAINFALL ADJUSTMENT MULTIPLIER
RATFV R	1 SFD	RECORDED ANNUAL TOTAL FLOW VOLUME
RAI-6 R	1 -	REGRESSION ACCUMULATORS
RB R	1 -	RECESSION BETA
RBF R	1 CFS	RECORDED BASE FLOW
RD R	1 -	RECESSION DISCRIMINANT
RDPT R	1 IN	RECORDED DAILY PRECIPITATION TOTAL
RFRISE R	1 IN	RECORDED FLOW RISE

RGPM	R	1	-	RECORDING GAGE PRECIPITATION MULTIPLIER
RGPMB	R	1	-	RECORDING GAGE PRECIPITATION MULTIPLIER - BASIC
RHFMC	R	1	IN	ROUTED HYDROGRAPH FLOW AT MINIMUM CUTOFF
RHFO	R	1	IN	PRECEDING ROUTED HYDROGRAPH FLOW
RHFI	R	1	IN	CURRENT ROUTED HYDROGRAPH FLOW (EXCLUDING BASE FLOW)
RHPD	I	5	-	RECORDED HYDROGRAPH PEAK DAY
RHPF	R	5	CFS	RECORDED HYDROGRAPH PEAK FLOW
RHPH	I	5	-	RECORDED HYDROGRAPH PEAK HOUR
RICD	R	1	-	RADIATION INCIDENCE FOR THE CURRENT DAY
RICY	R	37	-	RADIATION INCIDENCE OVER THE CALENDAR YEAR
RIF	R	1	CFS	RECORDED INTERFLOW
RINT	R	1	-	REGRESSION INTERCEPT
RMFX	R	1	-	RECORDED MONTHLY FLOW INDEX
RMPF	R	1	CFS	REQUESTED MINIMUM DAILY PEAK FLOW TO BE PRINTED
RMWR	R	1	IN	RAINFALL MAXIMUM WITHOUT RUNOFF
RSBBF	R	20	CFS	ESTIMATED BASE FLOW AT BEGINNING OF RECESSION SEQUENCE
RSBD	I	20	-	RECESSION SEQUENCE BEGINNING DAY
RSBFRC	R	20	-	RECESSION SEQUENCE BASE FLOW RECESSION CONSTANT
RSBIF	R	20	CFS	ESTIMATED INTERFLOW AT BEGINNING OF RECESSION SEQUENCE
RSFM	R	1	CFS	RECESSION SEQUENCE FLOW MINIMUM
RSFN	R	1	CFS	RECORDED STREAMFLOW ON NEW DAY
RSF1	R	1	CFS	RECORDED STREAMFLOW ON DAY 1
RSF2	R	1	CFS	RECORDED STREAMFLOW ON DAY 2
RSIFRC	R	20	-	RECESSION SEQUENCE INTERFLOW RECESSION CONSTANT
RSL	I	1	DAY	CURRENT RECESSION SEQUENCE LENGTH
RSLP	R	1	-	REGRESSION SLOPE
RSPTF	R	1	IN	ROUTED SYNTHESIZED PERIOD TOTAL FLOW
RSTF	R	50,20	CFS	RECESSION SEQUENCE TOTAL FLOWS
RSTR	R	1	-	RATIO OF SYNTHESIZED TO RECORDED FLOW
RWRAIN	R	1	IN	RECORDED WATERSHED RAINFALL
SADF	R	1	CFS	SYNTHESIZED AVERAGE DAILY FLOW
SARAX	R	1	IN	SNOW ALBEDO RAINFALL AGING INDEX
SASFX	R	1	IN	SNOW ALBEDO SNOWFALL FRESHENING INDEX
SATFV	R	1	SFD	SYNTHESIZED ANNUAL TOTAL FLOW VOLUME
SATFVI	R	1	IN	SYNTHESIZED ANNUAL TOTAL FLOW VOLUME IN INCHES
SATRI	R	99	-	SHIFT ADJUSTMENTS FOR TIME ROUTING INCREMENTS

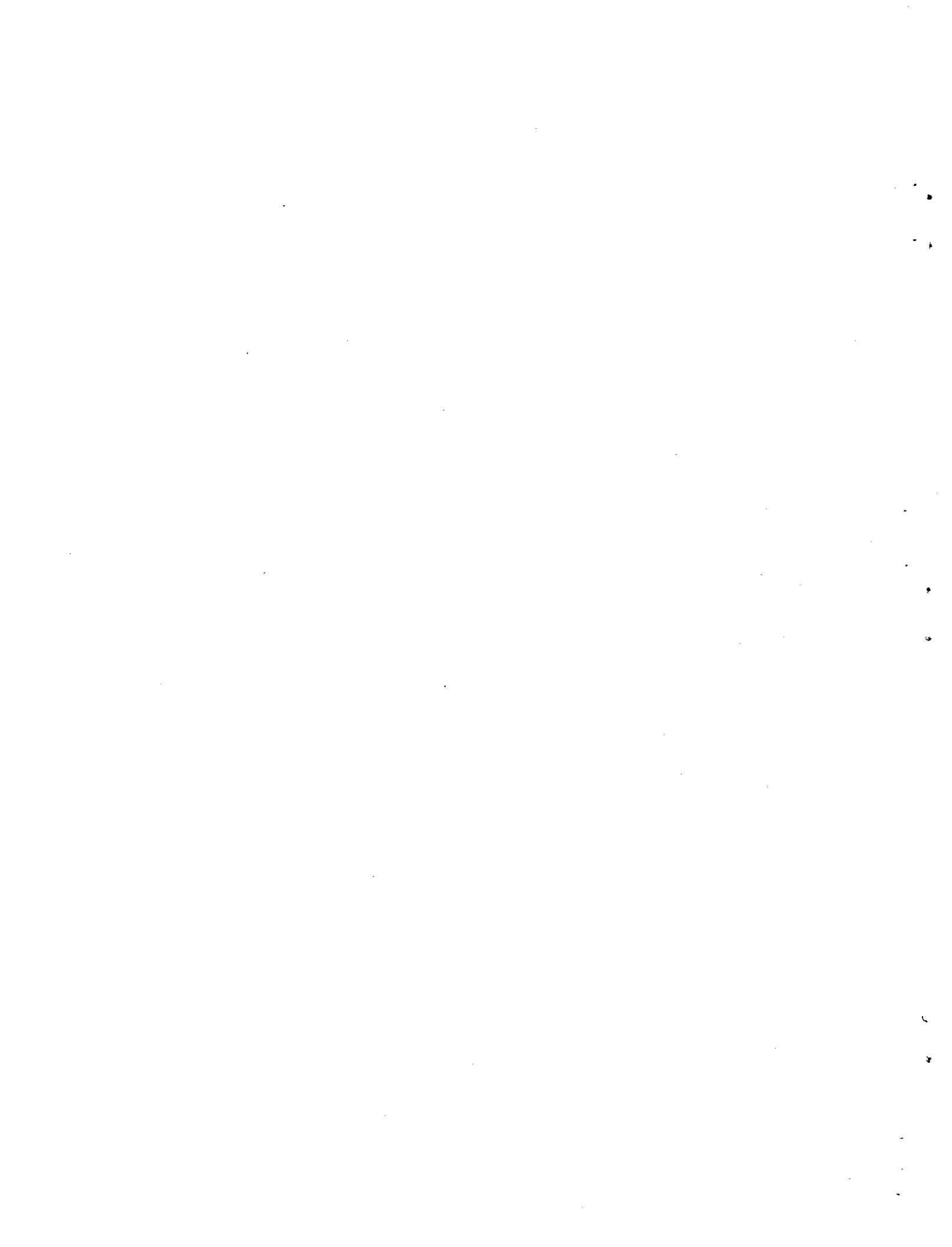
SAX R	1 -	SNOW ALBEDO INDEX
SBF R	1 CFS	SYNTHESIZED BASE FLOW
SBFRS R	3,20 CFS	SYNTHESIZED BASE FLOW DURING THE FIRST THREE DAYS OF EACH RECESSON SEQUENCE
SDEPTH R	1 IN	AVERAGE DEPTH OF SNOW ON GROUND
SDRSP R	1 -	SMALLEST VALUE OF DRSP
SDSC R	1 -	SECOND DIFFERENTIAL OF SINE CURVE MAGNITUDE
SE R	1 IN	CURRENT DAILY SNOW EVAPORATION
SERA R	22 CFS	ACCUMULATED ABSOLUTE DIFFERENCES BETWEEN RECORDED AND SYNTHESIZED DAILY STREAMFLOWS FOR INTERVAL
SERAV R	1 CFS	AVERAGE INTERVAL ABSOLUTE DIFFERENCE BETWEEN RECORDED AND SYNTHESIZED DAILY STREAMFLOWS
SERR R	22 CFS	ACCUMULATED DIFFERENCES BETWEEN RECORDED AND SYNTHESIZED DAILY STREAMFLOWS FOR INTERVAL
SERRV R	1 CFS	AVERAGE INTERVAL DIFFERENCE BETWEEN RECORDED AND SYNTHESIZED DAILY STREAMFLOWS
SESF R	22 CFS	STANDARD ERROR OF SYNTHESIZED FLOWS BY MAGNITUDE INTERVAL
SET R	1 IN	CURRENT HOURLY SOIL EVAPOTRANSPIRATION
SFDX R	1 -	SUMMER FLOW DEVIATION INDEX
SFMD R	1 -	SNOW FROZEN MOISTURE DENSITY
SGMD I	1 -	STORAGE GAGE MOVING DAY (WHEN IT IS MOVED DURING WATER YEAR)
SGRT I	1 -	STORAGE GAGE READING TIME
SGRT2 I	1 -	SECOND STORAGE GAGE READING TIME
SHM R	1 -	SYNTHESIZED HYDROGRAPH MULTIPLIER
SHPF R	1 CFS	SYNTHESIZED HYDROGRAPH PEAK FLOW
SIAC R	1 -	SEASONAL INFILTRATION ADJUSTMENT CONSTANT
SIAM R	1 -	SEASONAL INFILTRATION ADJUSTMENT MULTIPLIER
SIF R	1 CFS	SYNTHESIZED INTERFLOW
SIFRS R	3,20 CFS	SYNTHESIZED INTERFLOW DURING THE FIRST THREE DAYS OF EACH RECESSON SEQUENCE
SISRX R	1 -	SMALL INCREMENTAL STORAGE ROUTING INDEX
SMFX R	1 -	SYNTHESIZED MONTHLY FLOW INDEX
SNTRI I	1 -	SAVED NUMBER OF TIME ROUTING INCREMENTS
SOFMD R	1 -	SUM OF OVERLAND FLOW MONTH DEVIATIONS
SOFRF R	1 -	SNOW OVERLAND FLOW ROUTING FACTOR

SORFFI R	1 -	SNOW OVERLAND FLOW ROUTING FACTOR IMPERVIOUS SURFACES
SPBF R	1 IN	SYNTHESIZED PERIOD BASE FLOW
SPBFLW R	1 -	SNOW PACK BASIC MAXIMUM FRACTION IN LIQUID WATER
SPDR R	1 IN	SYNTHESIZED PERIOD DIRECT RUNOFF
SPIF R	1 IN	SYNTHESIZED PERIOD INTERFLOW
SPLW R	1 IN	SNOW PACK LIQUID WATER CONTENT
SPLWC R	1 IN	SNOWPACK LIQUID WATER HOLDING CAPACITY
SPM R	1 -	SNOW PRECIPITATION MULTIPLIER
SPOF R	1 CFS	SYNTHESIZED PERIOD OVERLAND FLOW (INCLUDING CHANNEL PRECIPITATION)
SPTF R	1 IN	SYNTHESIZED PERIOD TOTAL FLOW
SPTW R	1 IN	SNOW PACK TOTAL WATER CONTENT
SPTWCC R	1 IN	SNOWPACK MINIMUM TOTAL WATER FOR COMPLETE BASIN COVERAGE
SQER R	22 CFS	ACCUMULATED SQUARES OF DIFFERENCES BETWEEN RECORDED AND SYNTHESIZED DAILY STREAMFLOWS
SQPKD R	1 -	SUM OF SQUARED PEAK DIFFERENCES
SRR R	5,170 CFS	STORM RUNOFF ROUTED DOWN CHANNELS
SRX R	1 -	CURRENT STORAGE ROUTING INDEX
SSERA R	1 CFS	ACCUMULATED ABSOLUTE DIFFERENCES BETWEEN RECORDED AND SYNTHESIZED FLOWS OVER INTERVALS
SSERAV R	1 CFS	OVERALL AVERAGE ABSOLUTE DIFFERENCE BETWEEN RECORDED AND SYNTHESIZED FLOWS
SSERR R	1 CFS	ACCUMULATED DIFFERENCES BETWEEN RECORDED AND SYNTHESIZED FLOWS OVER INTERVALS
SSERRV R	1 CFS	OVERALL AVERAGE DIFFERENCE BETWEEN RECORDED AND SYNTHESIZED FLOWS
SSESF R	1 CFS	ACCUMULATED STANDARD ERROR OF SYNTHESIZED FLOW OVER INTERVALS
SSQM R	1 -	SUM OF THE SQUARES OF THE MONTHLY FLOW DEVIATIONS
SSQPKD R	1 -	SMALLEST VALUE OF SQPKD
SSR R	5,170 CFS	SYNTHESIZED STORM RUNOFF (NOT CHANNEL ROUTED)
SSRT R	1 -	SQUARE ROOT OF OVERLAND FLOW SURFACE SLOPE
SSSQM R	1 -	CURRENT SMALLEST ESTIMATE OF SSQM
STMD R	1 -	SNOW TOTAL MOISTURE DENSITY
SUBWF R	1 -	SUBSURFACE WATER FLOW OUT OF THE BASIN
SUZC R	1 -	SEASONAL UPPER ZONE STORAGE CAPACITY FACTOR

SWSMD	R	1	-	SUM OF WET SUMMER MONTH DEVIATIONS
TANSM	R	1	IN	TOTAL ACCUMULATED NEGATIVE SNOWMELT (SNOW CHILLING)
TBRD	R	1	-	TOTAL BASE FLOW RECESSON DAYS
TDFP12	R	1	-	TIME OF DAILY FLOOD PEAK, 12-HOUR CLOCK
TDFP24	R	1	-	TIME OF DAILY FLOOD PEAK, 24-HOUR CLOCK
TDSF	R	1	CFS	TOTAL DAILY STREAMFLOW
TEH	R	1	DEGF	TEMPERATURE ESTIMATED FOR HOUR
TFCFS	R	1	CFS	CURRENT TOTAL FLOW
TFMAX	R	1	CFS	MAXIMUM TOTAL FLOW DURING CURRENT DAY
TFMRT	R	1	CFS	TOTAL STREAMFLOW AT MAXIMUM STREAM ROUTING TIME
TFSRX	R	1	-	TRIAL VALUE OF FSRX
TFX	R	1	CFS	TOTAL STREAMFLOW INDEX
THGR	R	1	IN/HR	TOTAL HOURLY GROSS RUNOFF
THSF	R	24	CFS	TOTAL HOURLY STREAMFLOW
TIRD	R	1	-	TOTAL INTERFLOW RECESSON DAYS
TITLE	A	20	-	TITLE OF CURRENT STATION YEAR (STREAMGAGE LOCATION AND DATE)
TMBF	R	12	IN	TOTALS OF MONTHLY BASE FLOW
TMFSIL	R	12	IN	TOTALS OF MONTHLY FOREST SNOW INTERCEPTION LOSS
TMIF	R	12	IN	TOTALS OF MONTHLY INTERFLOW
TMNET	R	12	IN	TOTALS OF MONTHLY NET EVAPOTRANSPIRATION
TMDF	R	12	IN	TOTALS OF MONTHLY OVERLAND FLOW
TMPEP	R	12	IN	TOTALS OF MONTHLY POTENTIAL EVAPOTRANSPIRATION
TMPREC	R	12	IN	TOTALS OF MONTHLY PRECIPITATION
TMRPM	R	12	IN	TOTALS OF MONTHLY RAIN PLUS MELT
TMRTF	R	12	SFD	TOTALS OF MONTHLY RECORDED TOTAL FLOW
TMSE	R	12	IN	TOTALS OF MONTHLY STREAM EVAPORATION
TMSNE	R	12	IN	TOTALS OF MONTHLY SNOW EVAPORATION
TMSTF	R	12	SFD	TOTALS OF MONTHLY SYNTHESIZED TOTAL FLOW
TMSTFI	R	12	IN	TOTALS OF MONTHLY SYNTHESIZED TOTAL FLOW IN INCHES
TMTCY	R	12	SFD	TOTALS OF MONTHLY TOTAL FLOW BY CALENDAR YEAR
TMTFWY	R	12	SFD	TOTALS OF MONTHLY TOTAL FLOW BY WATER YEAR
TOFR	R	1	IN	CURRENT TOTAL OVERLAND FLOW RUNOFF
TPLR	R	1	-	TOTAL TO PERVIOUS LAND RATIO
TRHF	R	1	IN/HR	CURRENT TIME ROUTED HYDROGRAPH FLOW
TRHV	R	1	-	TOTAL RECORDED HYDROGRAPH VOLUME

TRIP	I	1 -	COUNTER SPECIFYING PROGRAM PORTIONS
TSHV	R	1 -	TOTAL SYNTHESIZED HYDROGRAPH VOLUME
TSRX	R	7 -	ARRAY OF TRIAL STORAGE ROUTING INDICES
T20DFH	R	21 IN	TOP 20 VALUES DURING THE YEAR OF HOURLY OVERLAND FLOW
T20PRH	R	21 IN	TOP 20 VALUES DURING THE YEAR OF HOURLY PRECIPITATION
UHFA	R	99 IN	UNROUTED HYDROGRAPH FLOW ARRAY
URHF	R	1 IN	CURRENT UNROUTED HYDROGRAPH FLOW
UZC	R	1 IN	UPPER ZONE STORAGE CAPACITY
UZINFX	R	1 -	UPPER ZONE INFILTRATION INDEX
UZINLZ	R	1 IN/HR	CURRENT UPPER ZONE INFILTRATION TO LOWER ZONE
UZR	R	1 -	UPPER ZONE MOISTURE RETENTION INDEX
UZS	R	1 IN	CURRENT UPPER ZONE STORAGE
VDCY	R	366 -	VALUE DATED BY CALENDAR DAY
VDMD	R	12 -	VALUE DATED BY MONTH DAY
VINTCR	R	1 IN	VEGETATIVE INTERCEPTION - CURRENT RATE PER PERIOD
VINTMR	R	1 IN/HR	VEGETATIVE INTERCEPTION - MAXIMUM RATE
VWIN	R	1 SFD	VOLUME OF AN INCH OF RUNOFF FROM WATERSHED
WAPV	R	1 -	WEIGHTED AVERAGE PARAMETER VALUE
WCFS	R	1 CFS	WATERSHED CFS EQUALLING ONE INCH PER HOUR
WEIFS	R	1 IN	WATER ENTERING INTERFLOW STORAGE
WFDX	R	1 -	WINTER FLOW DEVIATION INDEX
WI	R	1 IN	WATER INFILTRATION
WSBIT	R	1 -	WATERSHED BIT FOR RESTRUCTURING TIME-AREA HISTOGRAM
WSG	R	1 -	WEIGHTING FACTOR FOR STORAGE RAIN GAGE
WSG2	R	1 -	SECOND WEIGHTING FACTOR FOR STORAGE RAIN GAGE
WSM	R	1 -	NUMBER OF WET SUMMER MONTHS
WT4AM	R	1 DEGF	AVERAGE 4 A.M. TEMPERATURE OVER WATERSHED
WT4PM	R	1 DEGF	AVERAGE 4 P.M. TEMPERATURE OVER WATERSHED
XDNFS	R	1 -	INDEX DENSITY OF NEW-FALLEN SNOW
XELR	R	1 -	RAIN INDEX FOR ESTIMATING LAPSE RATE 0.0 = DRY, 4.0 = RAIN
XMPFT	R	12 -	INDEX OF MONTHLY PREDOMINATE FLOW TYPE
YEAR	I	1 -	LAST TWO DIGITS OF CURRENT YEAR
YR1	I	1 -	LAST TWO DIGITS OF FIRST CALENDAR YEAR IN WATER YEAR
YR2	I	1 -	LAST TWO DIGITS OF SECOND CALENDAR YEAR IN WATER YEAR
YTITLE	A	20 -	YEAR TITLE





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