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Effects of Stream Channel Improvements on Downstream Floods

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EFFECTS OF STREAM CHANNEL IMPROVEMENTS
ON DOWNSTREAM FLOODS

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ABSTRACT

This report presents a self-calibrating watershed model for predicting the effect of channel improvements on downstream floods. The model is called MOPSET because it is a modified version of OPSET developed several years ago at the University of Kentucky. OPSET is a computerized procedure for determining an optimum set of parameter values by matching synthesized flows with recorded flows. Major modifications include the replacement of the modified Muskingum method of channel routing by a kinematic finite difference method, the division of the watershed into a number of segments, and the inclusion of a storage routing procedure to take care of any reservoirs or flood control structures located in the watershed. The computer program is well documented and can be used not only as a flood predicting model but also as a general model for hydrologic simulations.

The model was applied to three different watersheds in Kentucky. It was found that the optimum set of parameter values obtained automatically by the model was not unique and might not yield the most desirable solution. For this reason, new features were added so that the user can exercise his judgment in selecting the most desirable parameter values.

The synthesized flows obtained from these watersheds are presented and compared with the recorded flows. The effects of channel improvements, flood control structures, and routing procedures on the synthesized flows are discussed.

KEY WORDS: Channel improvement*; computer programs*; downstream floods; finite difference method; flood routing*; hydrographs; model studies*; multiple segments; parameter optimization; rainfall-runoff relationships; streamflow*.

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CHAPTER I

INTRODUCTION

As stated in the original proposal, the purpose of this study was to determine the effect of channel improvements on downstream floods through the use of a computer program called the Kentucky Watershed Model with OPSET, a self-calibrating version of the Stanford Watershed Model. OPSET is a computerized optimization procedure developed at the University of Kentucky [Liou, 1970] to determine an optimum set of parameter values by matching synthesized flows with recorded flows. The model was applied to twenty watersheds in Kentucky, and the parameter values selected by the model were correlated with the measurable physical characteristics of the watersheds [Ross, 1970].

Although OPSET is useful in determining the general effects of watershed changes on stream flows, it is not adequate for predicting the effect of channel improvements on downstream floods because the actual channel cross sections, which may vary continually in a watershed, were not considered. The model used a time-area histogram and a modified Muskingum method for flood routing, which was not reflective of channel changes. Therefore, a major effort was directed in this study to develop a kinematic method of channel routing, in which the actual channel cross sections were taken into account. As any changes in channel cross sections are reflected directly in the routed hydrographs, this modified version of OPSET, hereafter referred to as MOPSET, can be used for determining the effect of channel improvements on downstream floods.

In addition to the channel routing, other changes have been made to increase the versatility of MOPSET not only as a flood predicting model but also as a general model for hydrologic simulations.

In this report, only MOPSET will be presented. A modified version of the Kentucky Watershed Model, which provides more options but requires more experience to calibrate manually, will be described by Gaynor in his master's thesis.

CHAPTER II

REVIEW OF OPSET

2.1 History

Due to the prolific interest in the ecologic and economic aspects of watershed use, a flood of new hydrologic models programmed in digital electronic computers have appeared in recent years. One of the pioneering works is the Stanford Watershed Model developed by Crawford and Linsley [1966]. In spite of its great potential as a design tool, the Stanford Watershed Model has the limitations that the program was written in a seldom used language (BALGOL) and that the numerous input parameters are difficult to calibrate to achieve satisfactory results. At the University of Kentucky, James [1970] translated the model into Fortran IV and called his translated, revised and expanded version the Kentucky Watershed Model (KWM). At the same time, a self-calibrating watershed model, based on the KWM, was developed [Liou, 1970]. This model was named OPSET because its objective was to determine the optimum set of parameter values for the watershed.

An outstanding feature in OPSET as well as in KWM is the use of mnemonic names for all variables. With a computer program as bulky as OPSET, the use of mnemonics greatly helps the reader in understanding the program. The same feature is preserved in MOPSET, and a dictionary of all variables is presented in Appendix B.

In the following sections, the limitations of OPSET and the changes made in MOPSET are described. Readers not familiar with OPSET should refer to the report by Liou [1970].

2.2 Watershed Segmentation

One drawback of OPSET is that it does not allow watershed segmentation. Precipitation is assumed to be uniform, channel routing is performed for the watershed as a whole, and output is only available at the mouth of the watershed. All these severely limit its applicability only to small watersheds. When applying the Kentucky Watershed Model in

conjunction with a sediment model to a large watershed, David and Beer [1975] indicated that it was impossible to obtain representative hourly overland flow and pointed toward the need for watershed segmentation, which was not provided for in the Kentucky Watershed Model.

The use of only one segment in OPSET is probably due to the conception that a self-calibrating model is impractical for a multisegment basin. Linsley et al [1975] have indicated that if all segments have different parameters, the number of iterations increases exponentially as the number of segments and the computer time becomes too great. However, the segmentation of watershed is made possible in MOPSET because of the assumption that the parameter values at different segments be varied but kept at constant ratios. In the optimization process, the seven land phase parameters are specified as base values for the entire watershed. The individual segment values are read in as ratios to the base values. For a given set of base values, the individual values for each segment are computed from the given ratios, and a year of flow as well as the various moisture storages in each segment is simulated. The moisture storages are weighted by areas to calculate the average moisture storage of the entire watershed, thus allowing MOPSET to print the same moisture summary as OPSET.

The assumption of constant ratios in parameter values among different segments is a step forward in extending the applicability of the model to a large watershed. The base values of these land phase parameters are difficult to estimate, but the parameter ratios between two widely different segments can usually be estimated with more certainty. Gross [1970] related these parameter values to the measurable physical characteristics of the watershed, and his results can be used as a guide in determining these ratios.

The use of multiple segments in a watershed requires the independent calculation of both the direct runoff and the base flow in each segment and thus increases greatly the computer time. As only the monthly flows are matched in the optimizing procedure, it may be more economical to consider the watershed as a single segment by using the

weighted parameter values at the outset. Therefore, an option is provided in MOPSET such that either single or multiple segments can be used for optimization purposes. If a single segment is specified, the optimum set of parameter values is determined by considering the watershed as one segment, but the channel routing is performed on the basis of multiple segments.

2.3 Parameter Optimization

In OPSET, each computer run consists of three trips. Trip 1 optimizes the seven land phase parameters, trip 2 optimizes the four channel routing parameters, and trip 3 is the final run using the optimum set of parameters. Because the actual channel dimensions are specified in MOPSET, the optimization of channel parameters is no longer necessary, and only two trips are needed. The procedures for determining the two recession constants and optimizing the seven land phase parameters remain the same.

A major difficulty in the use of the Stanford or the Kentucky Watershed Model is the determination of the seven land phase parameters. The effects of these parameters on monthly flow are not clearly defined and may be highly overlapped. Consequently, satisfactory matching between synthesized and recorded monthly flows can be achieved by different combinations of these parameters. Based on the preset initial parameter values, OPSET will adjust the parameters according to a set of empirical rules until the sum of squares for the monthly deviation index is a minimum. However, the solution is not unique in that a different set of parameter values will be obtained if different initial values are assumed. It has also been found that the solution does not converge and may oscillate back and forth, and that the set of parameter values for the minimum sum of squares may not yield the most satisfactory solution. For these reasons, MOPSET was made more flexible by incorporating new features such that the user can assume any initial values, specify the number of adjustment cycles desired, and inspect the results before final selection of the parameter values being made.

In OPSET, the adjustment of LZC, SUZC, ETLF, BUZC, and SIAC is based on the monthly flow deviation index, MFDI, defined as

$$\text{MFDI} = \frac{\text{TMSTF} + 20}{\text{TMRTF} + 20} - 1 \quad (\text{when } \text{TMSTF} > \text{TMRTF}) \quad (1)$$

$$\text{MFDI} = 1 - \frac{\text{TMRTF} + 20}{\text{TMSTF} + 20} \quad (\text{when } \text{TMSTF} < \text{TMRTF}) \quad (2)$$

in which TMSTF is the synthesized monthly total flow volume and TMRTF is the recorded monthly total flow volume, both in Sfd. When the synthesized monthly flow volume exceeds the recorded, Eq. 1 is used, and the index is positive. When the synthesized monthly total is smaller, Eq. 2 is used, and the index is negative. The indices of a few arbitrarily selected months are used for adjusting each of the above parameters. For example, LZC is related to the overland flow months; SUZC to the two months between April and November with the greatest rainfall plus August and September, if they are base flow months; ETLF to summer months when the precipitation exceeds 2 in.; BUZC to either the three months of September, October and December or the three months of June, July and August, depending on the value of BUZC; and SIAC to either the first three summer months, when the rainfall is less than the potential evapotranspiration, or the first three winter months, when the rainfall is greater than the potential evapotranspiration, depending on the value of SIAC. If the adjustments based on the empirical rules cannot yield reasonable parameter values, alternate procedures will be evoked. These adjustment rules were developed from the sensitivity study of a few watersheds and may not work well in other watersheds. The adjustment of BMIR and BIVF was based on the matching between synthesized and recorded base flow and interflow during the first three days of each recession sequence.

Although the same optimization procedures are incorporated in MOPSET and both trips can be executed in the same computer run, it is suggested that the two trips be run separately for the following two reasons:

1. The automatic parameter-determination routines may not yield the most desirable solution. If found unsatisfactory, a new set of

initial parameter values should be assumed and each adjustment cycle inspected, so that the best set of parameter values can be selected.

2. One year of data is usually not enough for parameter determinations. Several years of records should be used and the results averaged to obtain the optimum set of parameters to be used for trip 2.

2.4 Channel Routing

In OPSET, each watershed is assumed to have only one segment, or one reach. By the use of a time-area histogram, it is possible to route the flow down a channel consisting of several reaches or branches. However, this method of routing is not only inaccurate but also difficult to perform, especially for large watersheds where a great number of branches exist.

In MOPSET, the watershed is divided into a number of segments. Each segment has one reach. The overland flow, interflow, and base flow from each segment are simulated and used as lateral flow distributed uniformly over the reach. Starting from the first day, the flow is routed from the uppermost reach to the lowest reach by a finite difference method. At the end of each day, the data for each reach are saved until all reaches have been routed for the same day. The procedure is then repeated for the next day, using the data stored in the previous day. Details of channel routing will be presented in Chapter III.

2.5 Storage Routing

Due to the numerous dams, weirs, and other hydraulic structures in Kentucky, it became evident that a storage routing routine would increase the power of the model. Therefore, a new subroutine STORRT was added. Only one structure may be routed for any one reach, and it must be located at the end of the reach. The subroutine takes the periodic flow, which has been channel routed, and adjusts it for the effects of storage and evaporation. This subroutine will be discussed in Chapter IV.

2.6 Orographic Influence

OPSET assumes that all rainfalls occur uniformly over the entire watershed. This assumption is probably quite reasonable for a small watershed but not for a large watershed. The model requires one recording precipitation gage and allows one storage gage to be used with a weighting factor for the purpose of adjusting the precipitation. In MOPSET, each segment can have its own rainfall pattern by estimating the precipitation from a given recording gage and any one storage gage representative of the segment. Thus, every segment may utilize a different storage gage. The capability for allowing the change in location and reading time of storage gages is preserved. The subroutine PRECHK, which was used in OPSET for checking precipitation-streamflow anomalies and adjusting precipitation where necessary, are also used in MOPSET when the number of watershed travel hours, NWSTH, is less than 12. the dated recording gage precipitation multipliers, DRGPM, of all segments are weighted by areas to obtain an average DRGPM to be used as an input to PRECHK.

Since snowmelt is relatively insignificant in Kentucky, the snowmelt subroutine and all related calculations were removed from MOPSET for economy.

CHAPTER III

CHANNEL ROUTING

3.1 Governing Equations

The two equations used for channel routing are the continuity equation and the discharge-flow area relation. The equation of continuity can be expressed as

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial x} = q \quad (3)$$

in which A = flow area; Q = discharge; q = rate of inflow per unit length of channel; t = time; and x = downslope distance along the channel. Note that both A and Q vary with x and t , and that in a given segment q is a function of t independent of x .

In general, the discharge can be expressed as a power function of the flow cross-sectional area, hereafter called end area, or

$$Q = \alpha A^m \quad (4)$$

in which α and m are coefficients whose values depend on the shape and roughness of the channel.

3.2 Linear Routing

Schaake [1971] suggests the use of a finite difference equation which converges to Eq. 3 as the step size decreases. Substituting Eq. 4 into Eq. 3 and assuming α and m as constants independent of x

$$\frac{\partial A}{\partial t} + \alpha mA^{m-1} \frac{\partial A}{\partial x} = q \quad (5)$$

In the finite difference method, both t and x are divided into finite increments, Δt and Δx . In MOPSET, Δx may vary from reach to reach but Δt is the same for all reaches (either 15 min or 1 hr).

The requirement that α and m be constant over the distance x implies that each reach can only have one set of α and m , or one discharge-end area rating curve. This severely limits the applicability of the model because actual channels may consist of a large number of different cross sections within a reach. For this reason, each reach is

further divided into a number of finite sections, each with a different length and a different set of α and m . The finite section is then further divided equally into one or more finite differences. The routing procedure described here is applied to one finite section.

Terms in Eq. 5 can be expressed in the following finite difference forms:

$$\frac{\partial A}{\partial t} = \frac{A(x + \Delta x, t + \Delta t) - A(x + \Delta x, t)}{\Delta t} \quad (6)$$

$$\frac{\partial A}{\partial x} = \frac{A(x + \Delta x, t + \Delta t) - A(x, t + \Delta t)}{\Delta x} \quad (7)$$

$$A^{m-1} = \left[\frac{A(x, t + \Delta t) + A(x + \Delta x, t)}{2} \right]^{m-1} \quad (8)$$

$$q = \frac{q(t + \Delta t) + q(t)}{2} \quad (9)$$

Substituting Eqs. 6 through 9 into Eq. 5

$$\begin{aligned} A(x + \Delta x, t + \Delta t) = & \left\{ \frac{q(t + \Delta t) + q(t)}{2} + \frac{A(x + \Delta x, t)}{\Delta t} \right. \\ & \left. + \frac{\alpha m A(x, t + \Delta t)}{\Delta x} \left[\frac{A(x, t + \Delta t) + A(x + \Delta x, t)}{2} \right]^{m-1} \right\} \\ & / \left\{ \frac{1}{\Delta t} + \frac{\alpha m}{\Delta x} \left[\frac{A(x, t + \Delta t) + A(x + \Delta x, t)}{2} \right]^{m-1} \right\} \quad (10) \end{aligned}$$

The solution obtained from Eq. 10 is called linear because all terms on the right hand side are known, and no iterations are needed. At the initial time, or $t = 0$, all end areas are assumed to be zero. The solution is approximate due to the use of Eq. 8. Theoretically, A^{m-1} should depend also on $A(x + \Delta x, t + \Delta t)$, but its inclusion will make the equation nonlinear and more difficult to solve. The linear solution is quite satisfactory for relatively flat channel slopes. When greater accuracy is desired, or channel slopes are steep and flows change rapidly, a nonlinear iterative method, as described in the next section, may be used.

Eq. 10 can be used directly to determine the end area at each finite difference point. The end area at the first, or the most upstream, point in a finite section can be determined from Eq. 4 based on the inflow from the previous finite section. Repeated applications of Eq. 8 down the stream will give the end area at the last, or the most downstream, point of the finite section. Knowing the end area, the discharge from the finite section can be determined from Eq. 4.

3.3 Nonlinear Routing

The nonlinear procedure was developed by Li et al. [1975]. The finite difference form of Eq. 3 can be represented by

$$\frac{A(x + \Delta x, t + \Delta t) - A(x + \Delta x, t)}{\Delta t} + \frac{Q(x + \Delta x, t + \Delta t) - Q(x, t + \Delta t)}{\Delta x} = \frac{q(t + \Delta t) + q(t)}{2} \quad (11)$$

From Eq. 4

$$Q(x + \Delta x, t + \Delta t) = \alpha[A(x + \Delta x, t + \Delta t)]^m \quad (12)$$

$$Q(x, t + \Delta t) = \alpha[A(x, t + \Delta t)]^m \quad (13)$$

Note that the same α and m are used in Eqs. 12 and 13. Substituting Eqs. 12 and 13 into Eq. 11 yields

$$\frac{\Delta x}{\Delta t} \cdot A(x + \Delta x, t + \Delta t) + \alpha[A(x + \Delta x, t + \Delta t)]^m = \frac{\Delta x}{\Delta t} \cdot A(x + \Delta x, t) + \alpha[A(x, t + \Delta t)]^m + \Delta x \left[\frac{q(t + \Delta t) + q(t)}{2} \right] \quad (14)$$

The right side of Eq. 14 consists of known quantities and can be represented by

$$\Omega = \frac{\Delta x}{\Delta t} \cdot A(x + \Delta x, t) + \alpha[A(x, t + \Delta t)]^m + \Delta x \left[\frac{q(t + \Delta t) + q(t)}{2} \right] \quad (15)$$

Letting $r = A(x + \Delta x, t + \Delta t)$ and $\theta = \Delta x / \Delta t$, Eq. 14 can be written as

$$f(r) = \theta r + \alpha r^m = \Omega \quad (16)$$

The Taylor series expansion of $f(r)$ is

$$f(r) = f(r^k) + (r - r^k) f'(r^k) + \frac{1}{2}(r - r^k)^2 f''(r^k) + \dots \quad (17)$$

where r^k is the value of r at the k th iteration, and $f'(r^k)$ and $f''(r^k)$ are the first and second derivatives of $f(r^k)$. Neglecting all higher order terms, it is possible to find a solution r^{k+1} such that

$$f(r^{k+1}) = f(r^k) + (r^{k+1} - r^k) f'(r^k) + \frac{1}{2}(r^{k+1} - r^k)^2 f''(r^k) = \Omega \quad (18)$$

Eq. 18 is a quadratic equation in terms of $(r^{k+1} - r^k)$ and the solution is

$$r^{k+1} = r^k - \frac{f'(r^k)}{f''(r^k)} \pm \left\{ \left[\frac{f'(r^k)}{f''(r^k)} \right]^2 - \frac{2[f(r^k) - \Omega]}{f''(r^k)} \right\}^{\frac{1}{2}} \quad (19)$$

where

$$f(r^k) = \Theta r^k + \alpha (r^k)^m \quad (20)$$

$$f'(r^k) = \Theta + \alpha m (r^k)^{m-1} \quad (21)$$

$$f''(r^k) = \alpha m(m-1) (r^k)^{m-2} \quad (22)$$

Eq. 19 is used to determine the value of r at $(k+1)$ th iteration from that at k th iteration. The process is repeated until

$$\left| \frac{f(r^{k+1})}{\Omega} - \Omega \right| \leq 0.01 \quad (23)$$

During the iteration, Ω is a constant and need not be changed. By using the linear solution as the first approximation, the procedure converges very rapidly, usually within three iterations. The iterative scheme is quite stable until Ω approaches zero. However, when $\Omega < 10^{-8}$, the end area is set to zero and the nonlinear procedure is bypassed for that particular point.

3.4 Direct Method for Determining Routing Parameters

In the linear or nonlinear channel routing, a prior knowledge of the two routing parameters, α and m , is needed for each finite section. The values of these parameters can be determined from Manning's formula, or

$$Q = \frac{1.49}{n} AR^{\frac{2}{3}} S^{\frac{1}{2}} \quad (24)$$

in which n = roughness coefficient; S = slope of channel; and R = hydraulic radius = Area/wetted perimeter. For a rectangular cross section of width, B , with the depth of water much smaller than B ,

$$R = A/B \quad (25)$$

Substituting Eq. 25 into 24 and comparing with Eq. 4

$$\alpha = \frac{1.49S^{\frac{1}{2}}}{nB^{\frac{2}{3}}} \quad (26)$$

$$m = 1.67 \quad (27)$$

Eqs. 26 and 27 can be used for rectangular cross sections when the water is shallow. For a triangular cross section with a width, w , at a depth of one ft, as shown in Figure 1a, the flow area for a depth, H , is

$$A = \frac{1}{2} wH^2 \quad (28)$$

or

$$H = \sqrt{\frac{2A}{w}} \quad (29)$$

If the side slopes are very flat, the wetted perimeter is equal to wH , and the hydraulic radius becomes

$$R = \frac{\frac{1}{2}wH^2}{wH} = \frac{1}{2}H \quad (30)$$

From Eqs. 29 and 30

$$R = \sqrt{\frac{A}{2w}} \quad (31)$$

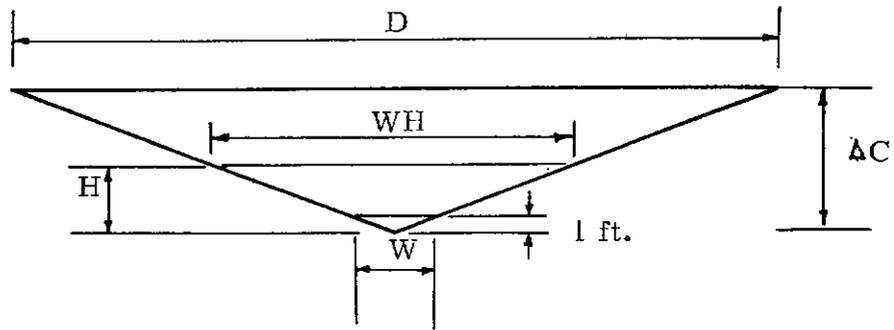
Substituting Eq. 31 into 24 and comparing with Eq. 4

$$\alpha = \frac{1.49S^{\frac{1}{2}}}{n(2w)^{\frac{1}{3}}} \quad (32)$$

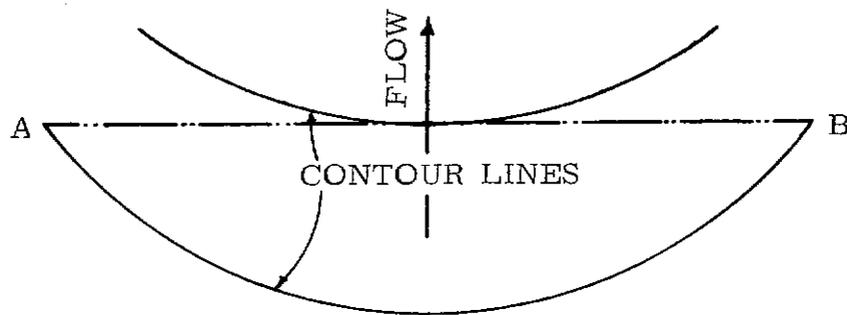
$$m = 1.33 \quad (33)$$

Eq. 32 can be used for determining the value of α from a topographic map, as shown in Figure 1.

Figure 1b is an exploded view of a contour map with a contour interval, ΔC . By measuring the length D on the map, which is the distance between A and B, the cross section of the channel can be determined, as shown in Figure 1a. Since $w = D/\Delta C$, from Eq. 32



(a) Section A-B



(b) PLAN VIEW

Figure 1. Determination of α and m from Topographic Map

$$\alpha = \frac{1.49S^{\frac{1}{2}}}{n\left(\frac{2D}{\Delta C}\right)^{\frac{1}{3}}} \quad (34)$$

Eqs. 26, 27, 33, and 34 are useful for determining α and m when the flow volume is small. When the flow volume is large, the channel cross section becomes more irregular, and Eq. 4 may be valid only over a limited range. Consequently, the discharge-end area method as described below should be used instead.

3.5 Discharge - End Area Method for Determining Routing Parameters

In this method, channel characteristics are represented by the discharge-end area relationship. This relationship at a particular cross section can be obtained from the water surface profile computer programs, such as the WSP2 program by the Soil Conservation Service [1976] and the HEC-2 program by the Corps of Engineers [1973]. Figure 2 shows a plot of $\log Q$ versus $\log A$ for a cross section in the North Fork Nolin River Watershed, as obtained from the WSP2 program.

If Eq. 4 is valid, the plot should result in a straight line. However, Figure 2 shows that the data do not lie on a straight line but can be approximated by three straight lines, each having a different α and m , where α is the vertical intercept at $A = 1$ sq ft and m is the slope of the straight line. Therefore, instead of one set of α and m , three sets must be specified together with the values of A separating each set. After the end area at each finite difference point has been determined from channel routing, the average end area is computed, and the proper set of α and m selected. If the average end area indicates that α and m should be changed from one set to the other, the average of the two sets will be used to prevent a sudden change and the resulting oscillations.

The above procedure requires the plotting of $\log Q$ versus $\log A$ and the determination of several sets of α and m for each finite section. If the number of finite sections is large, a substantial time will be

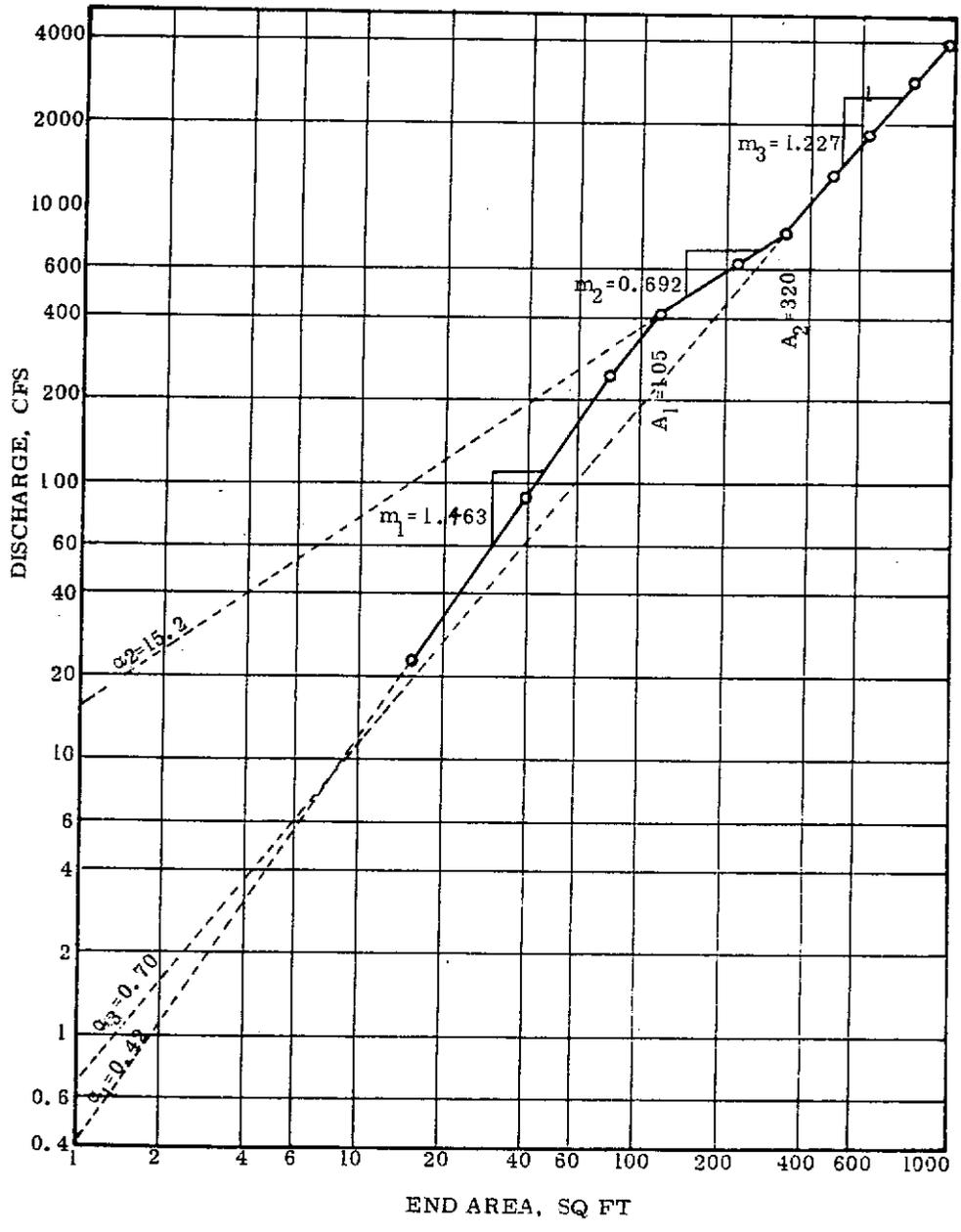


Figure 2. Determination of α and m from Discharge- End Area Relationships

needed for preparing the input data. To save the data preparation time, a subroutine ROUTAB was developed for determining α and m directly from the discharge-end area relationship, as will be described in the next chapter.

CHAPTER IV

PROGRAM DESCRIPTION

4.1 Main Program

The structure of MOPSET follows essentially that of OPSET except that a reach loop is imbedded between the day loop and the hour loop for the purpose of segmenting the watershed. It was attempted to make the model somewhat more modular in an effort to facilitate understanding. The channel routing and several related procedures were placed in sub-routines, thus shortening the main program. Figure 3 is a simplified flow chart showing the general structure of the model.

The purpose of the flow chart is to show the several major loops in operation. Some of the detailed procedures, which are important for the control of the program but are not shown in the flow chart, will be described as follows.

1. Although the flow chart shows trips 1 and 2 to be run at the same time, it is more desirable to run trip 1 first and inspect the data before proceeding to trip 2.
2. The gaged reach is not necessarily to be located at the mouth of the watershed. In trip 1, the synthesized flow is computed from the first reach to the gaged reach; but in trip 2, it is to the last reach.
3. Within the hour loop is a period loop. In trip 1, the period is 1 hour for the coarse adjustment cycle and 20 min for the fine adjustment cycle. In trip 2, the period is 15 min.
4. Since the optimization procedure requires a comparison between the synthesized and the recorded daily and monthly flows, flows are summed from the first reach to the gaged reach at the end of each day and each month.

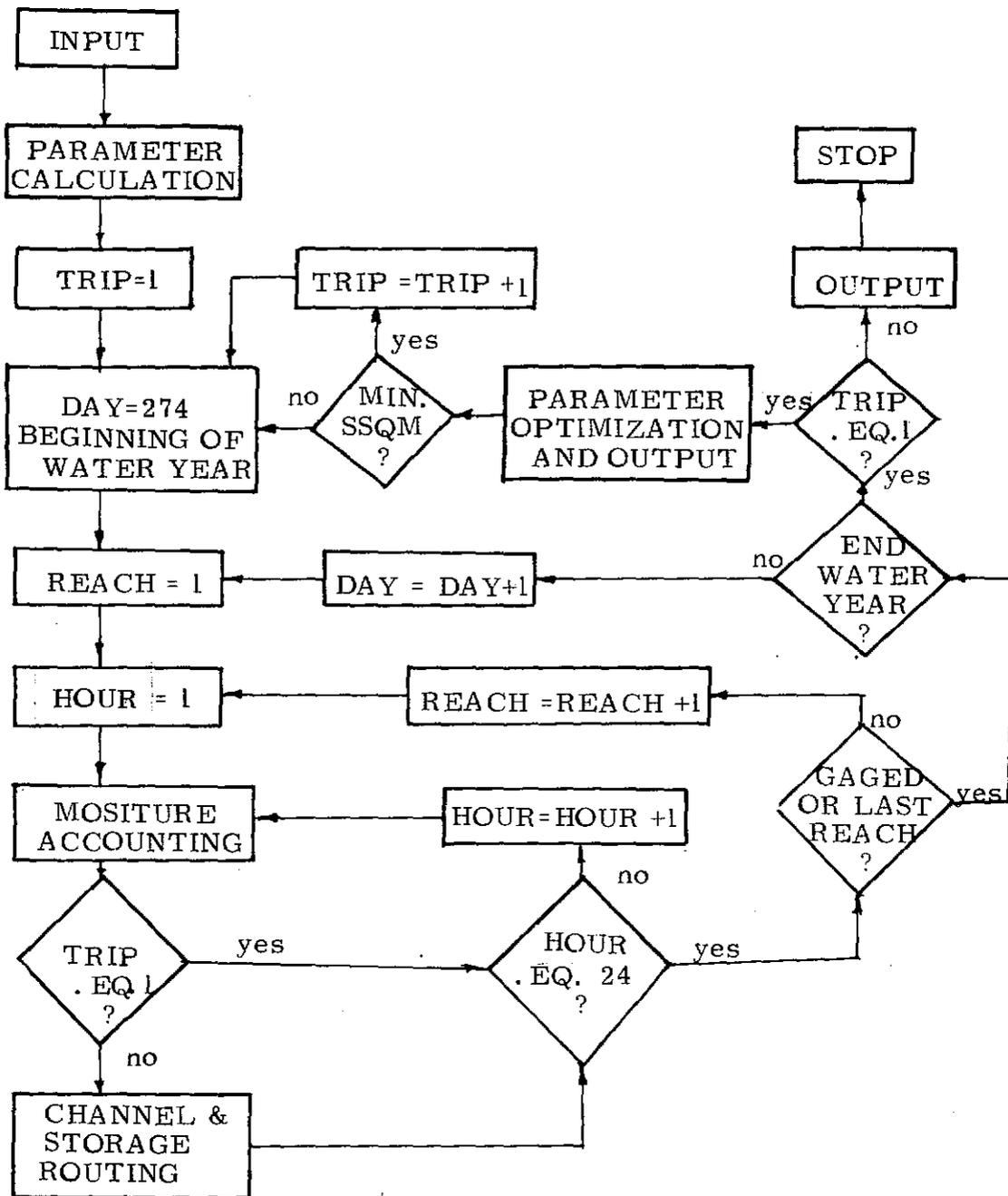


Figure 3. Simplified Flow Chart for MOPSET

5. An option is provided that during trip 1 the parameters used for different reaches are weighted over the areas, so the whole watershed is considered as one reach. This procedure may save considerable computer time without affecting significantly the accuracy of the results.

6. For the purpose of channel routing, each reach is divided into one or more finite sections. The lateral inflow per unit length of channel is the same for all finite sections in the same reach and equals to the total flow divided by the channel length in the reach. The outflow from different reaches can be added to form the inflow to the downstream reach. In other words, hydrographs can be added only at the junction of reach but not at the junction of finite sections in the same reach.

7. MOPSET used the same moisture accounting process as in the Stanford Watershed Model. The process was described by Crawford and Linsley [1966], Balk [1968], Ross [1970], and Ricca [1972]. The simulation procedures involving the seven land phase parameters are shown in Figure 4 as a flow chart. In actual simulations, each procedure can have several alternatives depending on the outcome of the results. However, these alternatives are not shown in the flow chart. The variables related directly to the land phase parameters are followed by a bracket in which the relevant parameters are shown.

4.2 Subroutines

Summary of Subroutines

MOPSET consists of a total of 18 subroutines, 4 of which were newly developed while the remaining 14 were borrowed from OPSET. Those borrowed from OPSET (including DAYNXT, DAYOUT, DAYSUM, EVPDAY, PRECHK, PREPRD, RECESS, SET2RC, SET1RC, SETBIV, SETBMI, SETFDI, SETFVP, and SETRBF) were described by Liou [1970] and will not be discussed here. Only the four new subroutines (including ROUTAB, RTGPAR, CHROUT, and STORRT) will be described. Table 1 is a summary of the subroutines used.

All subroutines are made self contained in that the input parameters are transferred from the main program to the subroutine by arguments.

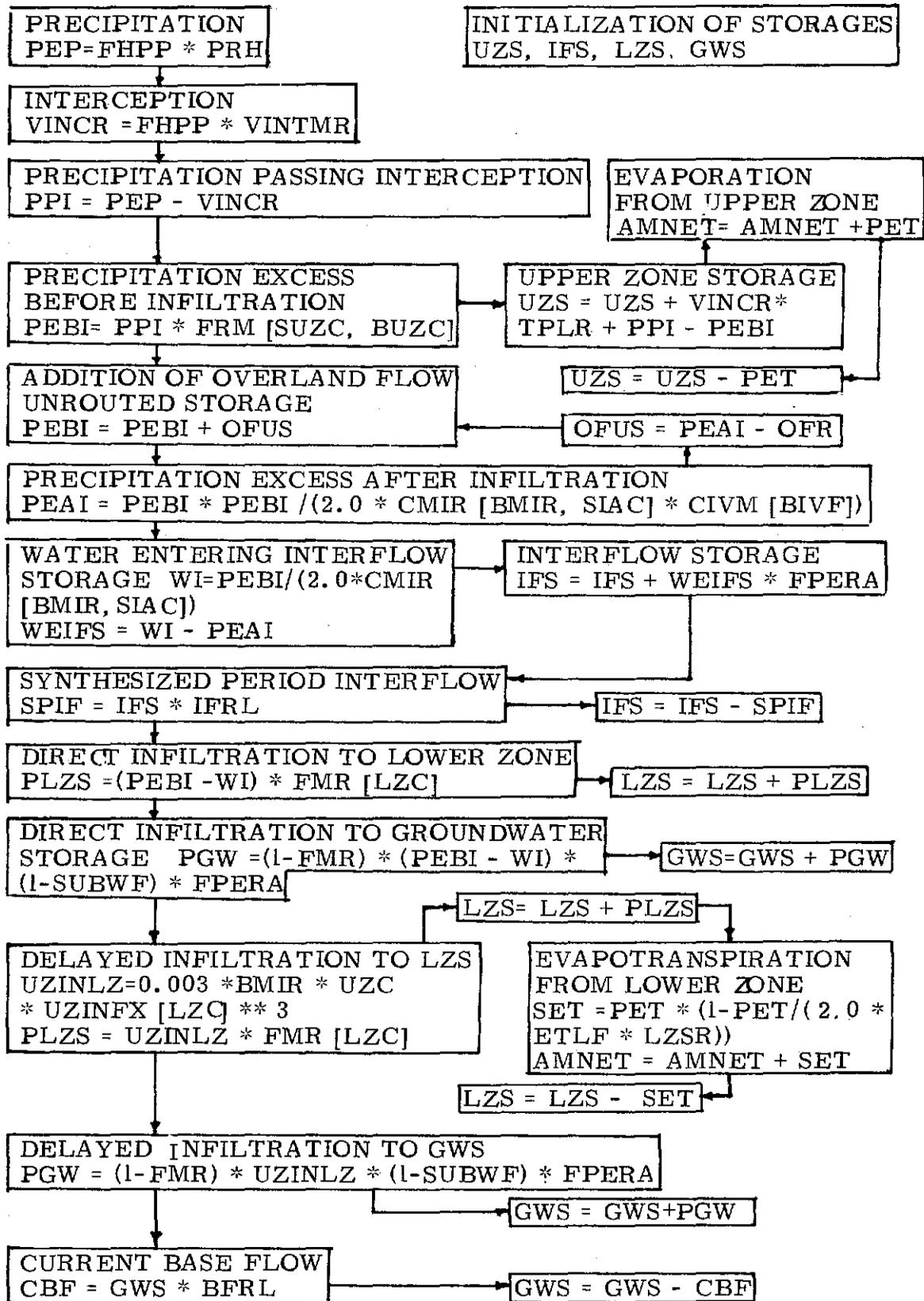


Figure 4. Flow Chart for Moisture Accounting Procedures

TABLE 1. SUMMARY OF SUBROUTINES

SUBROUTINE NAME	PURPOSE
<u>A. ACCOUNTING SUBROUTINES</u>	
DAYNXT	Determines next day of the year
DAYSUM	Sums daily values for monthly and yearly total
DAYOUT	Prints out daily values in tabular form
EVPDAY	Determines dated pan evaporation totals
PRECHK	Checks precipitation-streamflow anomalies
PREPRD	Divides hourly precipitation over 15 minute periods
<u>B. RECESSION CONSTANT SUBROUTINES</u>	
RECESS	Establishes recession sequences
SET2RC	Sets 2 recession constants
SET1RC	Sets 1 recession constant
<u>C. LAND PHASE PARAMETER SUBROUTINES</u>	
SETFVP	Sets new values of flow volume parameters
SETFDI	Sets values of flow deviation indices
SETBMI	Sets new values of basic maximum infiltration rate
SETRBF	Sets interflow and base flow at recession beginning
SETBIV	Sets new value of basic interflow volume factor
<u>D. ROUTING SUBROUTINES</u>	
ROUTAB	Establishes routing table
R.TGPAR	Determines routing parameters for each time period
CHROUT	Performs channel routing through each finite section
STORRT	Performs storage routing through each structure

TABLE 2. RELATIONSHIPS BETWEEN AVERAGE END AREA
AND ROUTING PARAMETERS

END AREA sq. ft	26.7	57.4	95.6	165.0	259.0	384.0	551.0	754.0	1020.0
α	0.46	0.42	0.79	21.2	640	1.58	1.89	0.21	0.77
m	1.45	1.47	1.32	0.63	0.85	1.09	1.06	1.41	1.21

This arrangement not only makes the subroutines more easily understandable but also facilitates the modification of the program, if needed. The user may like to replace some of the arguments by common statements in order to save the computer time.

Subroutine ROUTAB

This subroutine is called when the discharge-end area rating curve is read in for channel routing. A straight line is passed through every two successive points on the rating curve in logarithmic scales. The values of α and m , which are the vertical intercept and the slope, respectively, are computed and assigned to the average end area.

Table 2 shows the relationship between the average end area and the values of α and m for the data shown in Figure 2. There are a total of 10 data points, or nine sets of α and m . It can be seen that α and m do not change gradually but may jump back and forth. This is particularly true if any data point is erroneously out of line. Therefore, data on discharges and end areas should be carefully checked. Otherwise, the resulting hydrograph may oscillate up and down, and inaccurate results may be obtained. In view of the fact that channel routing is quite expensive, it is preferable to plot $\log Q$ versus $\log A$ and determine the several sets of α and m as input, thus any irregularity in the discharge-end area rating curve can be detected. Although subroutine ROUTAB was used for all three watersheds presented herein, it was found that the hydrographs sometimes oscillated up and down. These oscillations were completely eliminated when the several sets of α and m were determined from the logarithmic plot.

The subroutine consists of the following arguments:

DISCH (1000) - Values of discharges in the rating curve. Theoretically, the dimension required is only $NP + 1$, where NP is the number of points on the rating curve. However, a large dimension of 1000 is used because DISCH is also employed in subroutine STORRT, which requires a larger dimension. DISCH may be destroyed after the routing table is generated.

DAREA (150,50) - Values of end areas in the rating curve. The first subscript indicates the maximum number of finite sections, each with a different rating curve, and the second subscript indicates the maximum number of points on the rating curve. The original values of DAREA are destroyed and replaced with the average end area of two successive points on the rating curve.

KRKFS - Counter for finite section number. The number starts from the most upstream finite section of the first reach and ends at the most downstream finite section of the last reach.

NP - Number of points in the rating curve.

ALPH (150, 50) and EXPT (150, 50) - Values of α and m respectively. ALPH (KRKFS, 1) and EXPT (KRKFS, 1) are reserved exclusively for storing α and m for use in low volume flows, as will be described in subroutine RTGPAR. If these values are not read in, they are initialized as zero. The first point in the routing table is represented by ALPH (KRKFS, 2) and EXPT (KRKFS, 2).

Subroutine RTGPAR

This subroutine is called to determine the routing parameters, α and m , to be used at a given time increment. The average end area for a finite section during the previous time increment is computed, and the corresponding α and m are selected.

If several sets of α , m and the limiting end area are read in directly, the subroutine will compute the average end area for the finite section, check the average end area with the limiting areas, and determine the set of α and m to be used.

If the discharge-end area relationship is specified for a finite section, subroutine ROUTAB should be called first to generate a routing table, as shown in Table 2. If the average area for a finite section is smaller than the smallest end area in the routing table, α and m corresponding to the smallest area will be used. If the average end area is

greater than the largest area, α and m corresponding to the largest area will be used. A straight line interpolation is used for determining α and m when the average end area falls between two successive areas in the table.

In view of the fact that the discharge-end area rating curve obtained from the water surface profile is mostly based on high volume flows and may not be applicable to low volume flows, an option is provided that the user can specify α and m when the average end area is smaller than the first point on the rating curve. If the average end area is greater than the first point on the rating curve but less than the first point in the routing table, a straight line interpolation is used for determining α and m .

The subroutine has the following arguments:

XAREA (150, 21) - End area at each finite difference point. The first subscript indicates the maximum number of finite sections and the second subscript indicates the maximum number of finite difference points in a finite section. This is an input parameter for computing the average end area.

KFS - Counter for finite section number in each reach.

KRKFS - Counter for finite section number in the entire watershed.

ALPH (150, 50), EXPT (150, 50), and DAREA (150, 50) - Values in the routing table obtained by calling subroutine ROUTAB.

ALPHA and EXPM - α and m used for channel routing, which are the output of the subroutine.

NOFD (150) - Number of finite difference division in each finite section.

NSC (150) - Number of slope changes in the rating curve. If the three sets of α and m are read in directly, NSC = 3. If all data points on the rating curve are read in, NSC = 0.

Subroutine CHROUT

The procedures for channel routing, both linear and nonlinear as described in Chapter III, are programmed in this subroutine. In searching for the best routing method, the convex method used in the TR-20 program by the Soil Conservation Service [1965] was considered. However, it was ruled to be impractical and insufficient for use in MOPSET because no satisfactory method could be found for calculating the routing coefficient, nor did the procedure lend itself well to the moisture accounting by daily looping. The convex method requires that the entire rainfall hydrograph be known prior to routing; whereas MOPSET treats each periodic rainfall as it occurs in the simulation.

The subroutine has the following arguments:

XAREA (150,21) - End area at each finite difference point, which is the output desired.

ROLF1 (15) and ROLF2 (15) - Local inflow per linear ft of channel during the previous and the current time intervals, respectively. The subscript indicates the maximum number of reaches. ROLF1 and ROLF2 are computed by dividing the sum of direct runoff and base flow in each reach with the length of channel in that reach.

DELTAT - Routing time interval, or Δt .

DELTAX - Finite difference length, or Δx .

ALPHA and EXPM - routing parameters α and m .

N - Number of finite difference points.

KR - Counter for reach number.

KFS - Counter for finite section number in each reach.

INLR - Index for linear or nonlinear routing, 0 for linear routing and 1 for nonlinear routing.

Subroutine STORRT

This subroutine is used to account for the storage effect of any structure encountered in the watershed. It is used at the end of a reach with the simulated outflow from the reach as the inflow to the structure. The procedure is similar to that used in the TR-20 program by the Soil Conservation Service [1964, 1965]. The TR-20 program is concerned mainly with high volume flows, so the water in the structure is assumed to be full and the effect of evaporation on water level is neglected. Since MOPSET is designed to simulate flows throughout the year, the water in the structure at the beginning of the year is not necessarily to be full. By specifying the initial storage of the structure and considering the effect of evaporation, the storage and outflow can be simulated.

The basic equation for storage routing can be written as

$$\frac{(I_1 + I_2)}{2} \Delta t - \frac{(O_1 + O_2)}{2} \Delta t = S_2 - S_1 \quad (35)$$

in which I_1 = inflow at time t_1 ; I_2 = inflow at time t_2 ; O_1 = outflow at t_1 ; O_2 = outflow at t_2 ; S_1 = storage at t_1 ; S_2 = storage at t_2 ; and $\Delta t = t_2 - t_1$. The term involving known quantities is designated as

$$C_4 = (I_1 + I_2 - O_1)\Delta t + 2S_1. \quad (36)$$

The term involving unknown quantities is designated as

$$C_5 = O_2 \Delta t + 2S_2 \quad (37)$$

Note that $C_5 = C_4$ and Eq. 37 is a linear function of O_2 and S_2 . O_2 and S_2 are also related by the discharge-storage rating curve for the structure.

Therefore, the required values of O_2 and S_2 occur at the intersection of Eq. 37 and the rating curve, as indicated in Figure 5.

To find the intersection, the subroutine selects two "bracketing" points on the rating curve. These bracketing points are located by substituting two corresponding values of discharge and storage near the beginning of the rating curve into Eq. 37 and obtaining C_5 . If $C_5 < C_4$, the next set of discharge and storage is used and the same comparison is made between C_5 and C_4 . This is repeated until $C_5 > C_4$, thus the bracketing points are those which cause a change in sign for $C_5 - C_4$. A straight line is passed through these two bracketing points and its intersection with Eq. 37 yields the unknown O_2 and S_2 .

S_2 obtained at the end of one time period will be used as S_1 at the beginning of the next time period. Due to the effect of evaporation, S_2 must be reduced before being used for S_1 . The volume of water evaporated can be computed from the daily potential evaporation and the elevation-storage rating curve. The elevation and storage at the two bracketing points is used to convert the evaporation in in. to the storage in cu ft. If there is no outflow, or the structure is not filled, the slope of the line connecting the first two points on the elevation-storage rating curve is used for the conversion.

The subroutine has the following arguments:

TFCFS - Total inflow, I_2 , at t_2 .

KR - Reach number.

DELTA T - Time increment, Δt .

ELEV (1000), DISCH (1000) and STORAG (1000) - Values of elevation, discharge and storage in the structure rating curve. The subscript indicates the maximum number of points in the rating curves for all structures in the watershed. For example, if there are twenty structures, each rating curve can have 50 points.

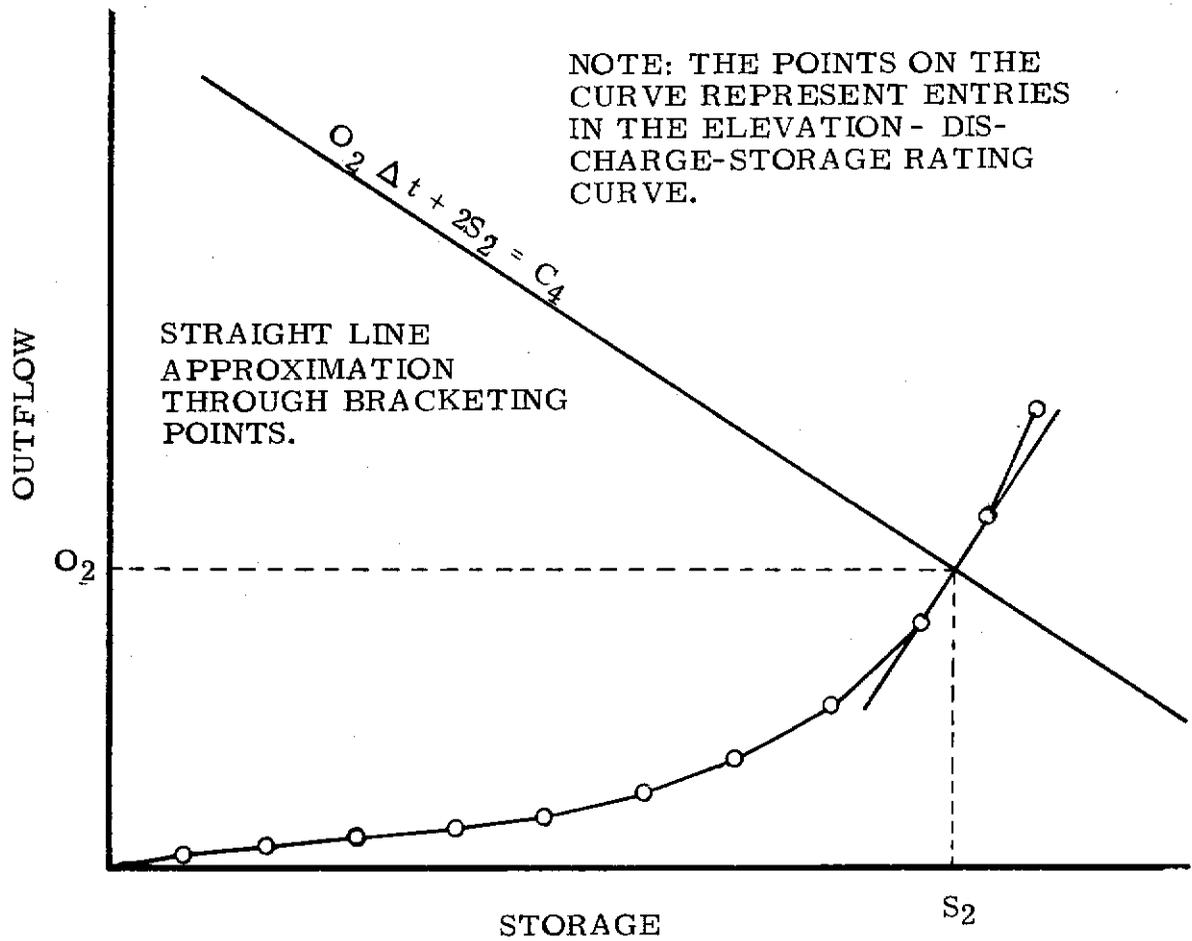


Figure 5. Solution of Storage Equation

IFST - Storage location of the first element in the rating curve.

ILST - Storage location of the last element in the rating curve.

INLST (15) - Inflow, I_1 , at t_1 . The subscript indicates the total number of reaches.

OUTLST (15) - Outflow, O_2 , at t_2 . O_2 is used as O_1 at the next time period.

STOLST (15) - Storage, S_2 , at t_2 . S_2 is used as S_1 at the next time period.

PET - Current daily potential evapotranspiration.

CHAPTER V

USER'S INFORMATION

5.1 Storage Requirement

The computer program was written in Fortran IV for an IBM 370 computer, model 165, which is used presently by the University of Kentucky. Because of the huge storage capacity of the computer, liberal storage locations were assigned to various variables. As a result, the program in its present form requires a storage of 360K. The user can reduce the required storage by following the suggestions described below:

1. If the total number of reaches is less than 15, the dimension of all variables having a subscript of 15 can be reduced to the number of reaches actually used.
2. If the number of data points in the discharge-end area rating curves is less than 50, all variables having a subscript of 50 can be reduced to a number equal to the number of data points plus one.
3. The dimension of ELEV (1000), DISCH (1000) and STORAG (1000) in the structure rating curves can be reduced to a number which is equal to the number of structures multiplied by the maximum number of data points in a rating curve.
4. If the maximum number of finite difference points in a finite section is fewer than 21, the second subscript in XAREA (150, 21) and EDAREA (150, 21) can be reduced to the number of points actually used.
5. If the total number of finite sections in the entire watershed is less than 150, the dimension of all variables having a subscript of 150 can be reduced to the number of finite sections actually used.

5.2 Data Preparation

Source of Information

MOPSET requires the same climatological and streamflow data as used in OPSET. Both Ross [1970] and Ricca [1974] suggest that a representative three to five year's record be available for calibration. Stream gage data and climatological data are published annually by the U.S. Geological Survey and the U.S. Weather Bureau respectively. Since the amount of required data is quite large, it is suggested that computer tapes containing these data be acquired so that the required data can be punched from the tapes.

Physical data of the watershed may be determined from topographic maps, aerial photos, and field surveys. The soil maps published by the Soil Conservation Service, which consist of soil boundaries superimposed on aerial photographs, are particularly useful. The available source of information and the procedures for obtaining physical data are discussed by Ross [1970].

Segmentation of Watershed

The first step in using MOPSET is to locate the stream gage on a topographic map and delineate the boundary of the watershed. The watershed is then divided into a number of reaches. The use of more reaches can approximate the watershed more closely but requires more computer times. Therefore, accuracy must be weighed carefully against economy in determining the number of reaches to be used.

The reaches are numbered from upstream to downstream, as shown in Figure 6. A basic principle in numbering is that the number of a reach must be greater than the number of the reaches, or branches, above it. Each reach is then divided into a number of finite sections, depending on the number of channel cross sections available. Each finite section is divided into a number of finite differences.

For more accurate results, it is desirable to consider each branch as a separate reach. However, economy may dictate that several small

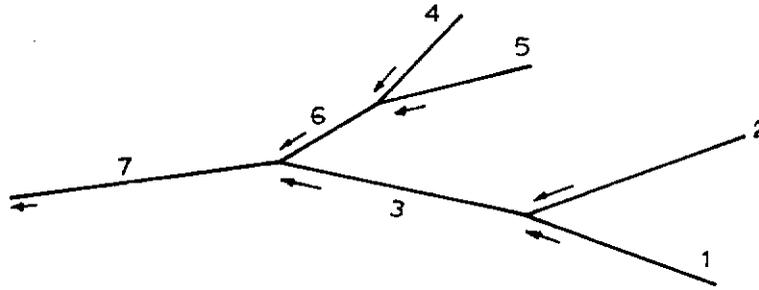


Figure 6. Numbering of Reaches

TABLE 3. ESTIMATES OF ETLF AND BUZC

Watershed Cover	Slope	ETLF	BUZC
Fallow	S	0.10	0.2
	M	0.15	0.4
	F	0.20	0.7
Cropland	S	0.13	0.8
	M	0.18	1.1
	F	0.23	1.6
Pasture	S	0.15	0.9
	M	0.20	1.3
	F	0.25	2.0
Woods	S	0.25	1.3
	M	0.30	1.7
	F	0.35	2.6

TABLE 4. ESTIMATES OF SIAC BY PERCENT OF WOODED AREA

Percent of Wooded Area	SIAC
0 - 20	0.15
21 - 40	0.30
41 - 60	0.50
61 - 100	0.70

branches be combined together and the channel in the largest branch be used for routing.

Determination of OFSL and OFSS

Two parameters most difficult to determine for each reach are the overland flow surface length, OFSL, and the overland flow surface slope, OFSS. Procedures were developed to measure these two parameters directly from a topographic map. By measuring with a chartometer the length of all discernible streams, as indicated by blue lines or V-shape contours, OFSL can be computed by

$$\text{OFSL} = \frac{\text{area of the reach}}{2 \times \text{total length of streams in the reach}} \quad (38)$$

DeWiest [1965] suggests a method for determining OFSS. A grid consisting of two sets of perpendicular lines is drawn on a transparent paper and superimposed on the topographic map. By measuring the length of each line within the boundary of the reach and counting the number of intersections with the contour lines, OFSS along each line can be determined by

$$\text{OFSS} = \frac{\text{Number of intersections} \times \text{contour interval}}{\text{length of line}} \quad (39)$$

The average of OFSS over the two sets of perpendicular lines gives the overland flow surface slope for the whole reach.

Estimation of parameter ratios.

When the watershed is divided into a number of reaches, it is necessary to specify the ratios of the seven land phase parameters to a set of base values. If the parameters for a given reach are used as the base values, all ratios for that reach will be assigned unity. The following information can be used to estimate the seven ratios.

1. LZC is related to the available water capacity, AWC, by

$$\text{LZC} = -0.7016 + 1.2404 \text{ AWC}. \quad (40)$$

The available water capacity of Kentucky soils are tabulated in Appendix D.

2. BMIR is related to the permeability of A-horizon, P_A , by

$$\text{BMIR} = 2.3595 P_A. \quad (41)$$

The permeability of Kentucky soils are also tabulated in Appendix D. Eqs. 40 and 41 were developed by Ross [1970].

3. In the absence of more reliable information, use of a ratio of 1 for SUZC.

4. ETLF is related to the watershed cover and the overland flow surface slope and can be estimated from Table 3. In the table, S indicates steep slope with OFSS > 0.2, M indicates moderate slope with OFSS between 0.05 and 0.2, and F indicates flat slope with OFSS < 0.05.

5. BUZC is also related to watershed cover and OFSS and can be estimated from Table 3.

6. SIAC is related to the percent of wooded area and can be estimated from Table 4.

7. Use a ratio of 1 for BIVF due to the lack of more reliable information.

Although the estimated parameter values based on the above information are quite crude and may not agree with the values determined from MOPSET, their use as a guide for determining the parameter ratios among different reaches will greatly facilitate the application of the model.

5.3 Required Input

Each input parameter will be explained in the order as it appears in the computer program.

Number of Cases and Title Card

NSYT - Total number of station years included in a computer run. Several years of data, not necessarily to be consecutive, may be run for

trip 1, so the optimum set of parameter values for each year can be determined and averaged. NSYT can also be used to indicate the total number of cases to be run, as long as all necessary data are read in for each case.

TITLE(20) - Title card with any number of characters punched in one card.

Control Options

Seven control options are used to control the program.

CONOPT(1) = 0 if daily evaporation values are to be read, 1 if evaporation data is in 10-day totals, and 2 if annual total evaporation is to be read.

CONOPT(2) = 0 if channel routing is to be done every 15 min, 1 if channel routing is to be done hourly.

CONOPT(3) = 0 if channel routing is linear, 1 if channel routing is nonlinear.

CONOPT(4) = 0 if initial parameter values are preset and not to be read, 1 if initial parameters are to be read. These parameters may be obtained from a previous run and used for trip 2 or the continuation of trip 1. They may also be any initial values arbitrarily assigned by the user.

CONOPT(5) = 0 if ALPH and EXPT for low volume flows are not to be read, and 1 if ALPH and EXPT for low volume flows are to be read.

CONOPT(6) = 0 if more than one reach is used for trip 1, and 1 if only one reach is used for trip 1. In the latter case, the parameters are averaged over all the reaches above the gage and used for trip 1.

CONOPT(7) = 0 if the fine adjustment cycle stops automatically when the sum of squares becomes greater. After checking the results,

if it is desired to run a few more fine adjustment cycles, assign CONOPT(4) to one and CONOPT(7) to the number of cycles desired.

Other Control Data

MNRC - Minimum number of rough cycles. Ross [1970] suggested the use of 12 rough cycles as the minimum. However, it was found that in many cases the use of fewer cycles would yield the same results. It is therefore suggested that 6 be used for MNRC. If the results are not satisfactory, the user can always rerun the program for a few more rough cycles, starting from the sixth cycle.

NFTR - Number of first trip to be run. Assign 1 if trip 1 is to be run, 2 if only trip 2 is to be run.

NLTR - Number of last trip to be run. Assign 1 if only trip 1 is to be run, and 2 if trip 2 is to be run.

NDCR - Day number for terminating channel routing. The day begins at 274. For example, assign 275 if only the first day of flow is to be routed, 305 if the first month of flow is to be routed, and any number greater than 366 if the entire year of flow is to be routed. The program will stop whenever DAY = NDCR. This feature is useful in cases where it is not necessary to obtain the hydrographs for the entire year.

NR - Total number of reaches.

NGR - Identifying number of the gaged reach. The stream gage can be located at the end of any reach, not necessarily to be at the mouth of the watershed.

NFS(KR) - Total number of finite sections in each reach, where KR ranges from 1 to NR.

NUBR(KR) - Total number of upstream branches above reach KR, or the number of hydrographs to be added. Assign 0 for the most upstream reach or reaches, to which inflow is zero.

IUH(KR, N) - Identifying number of upstream reaches, where KR ranges from 1 to NR and N from 1 to NUBR(KR). IUH is skipped if NUBR = 0.

NWSTH - Number of watershed travel hours, which can be estimated by

$$NWSTH = 0.00013 \left[\frac{L}{\sqrt{S}} \right]^{0.77} \quad (42)$$

in which L = the horizontal length in ft from the most distant point in the watershed to the stream gage, and S = the slope in ft per ft between these two points calculated by dividing the difference in elevation by L .

NOUT - Total number of reaches whose outflow is to be printed.

IROUT(I) - Reaches whose outflow is to be presented, where I ranges from 1 to NOUT.

RMPF - Requested minimum peak flow to be printed. If the flow at each routing time increment is desired, set RMPF to 0.0.

Channel Cross Sections

The channel cross section is represented either by a discharge-end area rating curve or sets of α and m . If NLTR = 1, no data on channel cross sections are read.

NP - Number of points in a channel rating curve. Assign 2 if α and m are read and a rating curve is not used.

NSC(KRKFS) - Number of slope changes in a rating curve, applicable only when α and m are to be read. Assign 0 if discharge and end area are to be read. KRKFS is an index for finite section.

NOFD(KRKFS) - Total number of finite difference divisions for the finite section identified as KRKFS. For short finite section, use 1 for NOFD.

FSLTH(KRKFS) - Length of finite section.

DISCH(I) and DAREA(KRKFS,I) - Values of discharge and end area in a rating curve, where I ranges from 1 to NP. These values are read only when NP \neq 2.

ALPH(KRKFS, 1) and EXPT(KRKFS, 1) - Values of α and m for low volume flows. These values are read only when NP \neq 2 and CONOPT(5) = 1.

ALPH(KRKFS, I), EXPT(KRKFS, I), DAREA(KRKFS, I) - Values of α , m, and the limiting end area, where I ranges from 1 to NSC(KRKFS). Assign a large value for DAREA when I = NSC(KRKFS).

Hydraulic Structures

The hydraulic structures can be numbered in any order so long as the data are referred to the reach in which the structure is located. If NLTR = 1, no data on structures are read.

NSTR - Total number of structures. Assign 0 if there is no structure in the watershed. If NSTR = 0, no structure data are needed.

NELV - Number of points, or elevations, in a structure rating curve.

ELEV(IE), DISCH(IE), and STORAG(IE) - Values of elevation, discharge, and storage in a rating curve, where IE is the storage location computed automatically by the program. Discharge for the first point in each rating curve is always 0.

IRSTR(KS) - Reach number in which the structure is located, where KS ranges from 1 to NSTR.

INLST(KR), OUTLST(KR), STOLST(KR) - Initial values of inflow, outflow, and storage, where KR ranges from 1 to NR. If the initial storage is less than the storage of the first point in the rating curve, OUTLST(KR) should be assigned 0.

Reach Parameters

RCHLTH(I) - Reach length, where I ranges from one to NR. The reach length can be measured in the field or from the topographic map by a chartometer.

SAREA(I) - Area of each segment, or reach. This can be measured from the topographic map by a planimeter.

OFSS(I) - Overland flow surface slope.

OFMN(I) - Overland flow Manning's coefficient, n. Table 5 gives the values for various types of surfaces [Chow, 1959].

OFSL(I) - Overland flow surface length.

OFMNIS(I) - Overland flow Manning's n for impervious surfaces.

FIMPA(I) - Fraction of area being impervious.

FWTRA(I) - Fraction of area being water.

RLZC(I), RBMIR(I), RSUZC(I), RETLF(I), RBUZC(I), RSIAC(I), and RBIVF(I) - Ratios of the seven land phase parameters to their respective base values. Methods for estimating these ratios are described in Section 5.2.

RGPMB(I) - Basic recording gage precipitation multiplier. If only a recording precipitation gage is used, RGPBM is the ratio of the average annual rainfall over the watershed to the annual rainfall recorded at the gage. If a storage gage is used in conjunction with the recording gage, RGPMB should be set to 1.0.

GWETF - Groundwater evapotranspiration factor, which is assumed to be constant for the entire watershed.

DIV - Division into basin, which is assumed to be constant for the entire watershed.

TABLE 5. MANNING'S ROUGHNESS COEFFICIENT FOR OVERLAND FLOW

Watershed Surface	Manning's n
Smooth Asphalt	.013
Concrete (trowel finish)	.013
Rough Asphalt	.016
Concrete (unfinished)	.017
Smooth Earth	.018
Firm Gravel	.020
Cemented Rubble Masonry	.025
Pasture (short grass)	.030
Pasture (high grass)	.035
Cultivated Area (row crops)	.035
Cultivated Area (field crops)	.040
Scattered Brush, Heavy Weeds	.050
Light Brush and Trees (winter)	.050
Light Brush and Trees (summer)	.060
Dense Brush (winter)	.070
Dense Brush (summer)	.100
Heavy Timber	.100

TABLE 6. INTERCEPTION VALUES FOR VARIOUS TYPES OF COVER

Watershed Cover	VINMRA(in.)
Grassland	0.10
Moderate Forest Cover	0.15
Heavy Forest Cover	0.20

VINMRA(I) - Maximum rate of vegetative interception. This value can be estimated by Table 6 [Crawford and Linsley, 1966].

SUBWFA(I) - Surface water flow out of basin. This parameter allows the user to account for any known subsurface loss such as sinkhole drainage.

Climatological Data

YR1 - The 2 digit integer specifying the first year of the water year.

YR2 - The 2 digit integer specifying the second year of the water year. YR2 is also used to determine whether the year has 366 days.

DPET(KRD) - Daily potential evapotranspiration, where KRD ranges from 274 to 365 and then from one to 273. If YR2 is a leap year, February 29 is read last as day 366. If CONOPT(1) = 0, daily values of DPET are read. If CONOPT(1) = 1, average evapotranspirations for each ten day period are read and the program assigns the same value to the remaining nine days. If CONOPT(1) = 2, EPAET and MNRD are read.

EPCM - Evaporation pan coefficient. This coefficient is read if CONOPT(1) = 0 or 1 to account for the difference between pan and lake evaporations.

EPAET and MNRD - Estimated potential annual evapotranspiration and mean number of rainy days, respectively. These two parameters are used in conjunction with subroutine EVPDAY to estimate the evaporation for each day of the year.

DRSF(DAY) - Daily recorded flow, where DAY ranges the same as KRD in DPET(KRD).

NSG - Total number of storage gages used. Each reach can have its own storage gage. If NSG = 0, all data related to storage gages are omitted.

NSGRD - Number of storage gage rainfall days.

ISGM (KG) - Index of storage gage movement during the year, where KG ranges from 1 to NSG. Assign 1 if the gage was moved, and 0 if not moved.

SGRT (KG) - Hour of day (24 hour clock) at which the storage gage was read.

SGRT2 (KG) - The new hour at which the storage gage was read. This parameter is read only if the storage gage was moved during the year.

SGMD (KG) - Storage gage moving day. This parameter is read only if the storage gage was moved during the year.

ISGRD - Storage gage rainfall day.

DRSGP (KG, ISGRD) - Daily recorded storage gage precipitation, where ISGRD ranges from 1 to NSGRD.

ISGA (KR) - Identity of the storage gage to be used for reach KR, where KR ranges from 1 to NR.

WSG (KR) - Storage gage weighting factor to be used for reach KR. This factor can be determined by dividing the distance between the centroid of the reach and the recording gage by the sum of the two distances, one from the centroid to the recording gage and the other from the centroid to the storage gage.

WSG2 (KR) - New storage gage weighting factor. This parameter is read only if the storage gage was moved during the year.

IWBG - 4 digit index number of Weather Bureau recording gage. All reaches must use the same recording gage.

YEAR - Last two-digits of current year.

MONTH - Current month, two digits.

DATE - Day of the month.

CN - 1 for A.M., 2 for P.M.

DRHP (DAY, HOUR) - Daily recorded hourly precipitation for calendar day, DAY, and 24 hour clock time, HOUR.

Continuation of Trip 1

Trip 1 can be continued at any adjustment cycle by simply reading in the following parameters, the values of which have been printed out in the previous run.

LZC, BMIR, SUZC, ETLF, BUZC, SIAC, BIVF - base values of the seven land phase parameters.

LZS - Initial value of lower zone storage. In MOPSET, it is assumed that the initial upper zone storage is always zero, and that the initial lower zone storage and the initial groundwater storage are the same for all reaches.

KRC - Counter of current adjustment cycle.

KBRC - Counter of rough cycles since best one. When CONOPT(7) = 0 and KBRC = 2, the program shifts from the rough to the fine adjustment cycle.

SSSQM - Current smallest estimate of the sum of squares of monthly flow deviations.

FTX - Fall trouble index. Initial value is 1.0. When the sum of the monthly deviation indices for November and December is greater than 2 , FTX is set to 0.9. When the sum is less than -2, FTX is set to 1.1.

LRC - Logical variable set TRUE for rough adjustment cycle.

LLZC, LBUZC, LBMIR, and LETLF - Logical variably set TRUE when exercising substitute approach for evaluating LZC, BUZC, BMIR, and ETLF, respectively. Initially, these variables are set FALSE.

Input of the above variables also allows the user to change the initial parameter values or the optimization procedures. For example, KRC can be set to 1, LRC to FALSE, and CONOPT(7) to the number of adjustment cycles desired. The user can then inspect the results of each fine adjustment cycle and select the land phase parameters which give the best results.

Separate Run for Trip 2

Because trip 2, which involves channel routing, is quite expensive, it is preferable to inspect the results of trip 1 and then read in the following parameters for trip 2.

LZC, BMIR, SUZC, ETLF, BUZC, SIAC, BIVF - Base values of the seven land phase parameters obtained from trip 1.

LZS - Initial value of lower zone storage.

IFRC and BFRC - Interflow and base flow recession constants determined from trip 1.

5.4 PRINTED OUTPUT

General Description

All input data, except climatological information, are printed so they can be checked for correctness. The climatological data are not printed because they are too voluminous. In fact, they can be read or punched from computer tapes, and the possibility for errors is minimum.

A table of recorded flows is printed showing the daily flow in cfs and the monthly and annual totals in sfd. Information in the table

can be compared with the synthesized flow to ascertain how satisfactory a match exists.

Trip 1

In trip 1, data related to the two recession constants are printed once, but those related to the optimization procedures are printed during each adjustment cycle. The following recession data are printed:

1. Flow sequences used to estimate recession constants.
2. BFRC and IFRC for each sequence.
3. Sequence not used for determining the average BFRC and IFRC because of their unreasonable values.
4. Number of base flow days and number of interflow days.
5. Amount of base flow, interflow and total flow at the first day of each recession sequence.
6. Rejected value of IFRC if it is smaller than 0.3.

The following optimization data are printed for each cycle:

1. Values of seven land phase parameters and two recession constants used for simulations.
2. Initial moisture storages, LZS and GWS.
3. Data used for continuation run, if needed, including KRC, KBRC, SSSQM, FTX, LRC, LLZC, LBUZC, LBMIR, and LETLF.
4. Summary table showing the monthly total in inches of overland flow, interflow, base flow, stream evaporation, precipitation, evapotranspiration, and the various storages and indices at the end of each month.

5. Monthly synthesized flows and annual total in sfd and the annual moisture balance error.

6. Monthly flow deviation indices, MFDI, and the sum of squares, SSQM.

7. New values of LZC, SUZC, ETLF, BUZC, and SIAC to be used for the next cycle and the index months upon which the new values are based.

8. New values of BMIR and BIVF to be used for the next cycle and a listing of synthesized and recorded base flow and interflow during the first three days of recession sequence.

Trip 2

In trip 2, the following data are printed:

1. Seven land phase parameters and two recession constants to be used for trip 2.

2. Initial moisture storages, LZS and GWS.

3. Hourly flows, maximum flow, and daily average of those days when the flow at any time exceeds RMPPF.

4. Daily flows and monthly total at the end of each month.

5. Table of synthesized flows at the end of the year showing the daily flow in cfs and the monthly and annual totals in sfd.

6. Summary table showing the monthly total in inches of overland flow, interflow, base flow, stream evaporation, precipitation, evapotranspiration, and the various storages and indices at the end of each month.

7. Monthly synthesized flows and annual total in sfd and the annual moisture balance error.

CHAPTER VI

DATA AND RESULTS

To illustrate the application of MOPSET, three watersheds of widely different characteristics were selected for analyses. They are the South Fork Beargrass Creek Watershed, the Floyds Fork Watershed, and the North Fork Nolin River Watershed. Due to the exhaust of the computer fund, each watershed was analyzed only for one year. Any error or inaccuracy in the recorded precipitation or streamflow data in that year will certainly affect the performance of the model. Therefore, the limited data presented in this chapter should not be used as a criterion to judge the adequacy of the model. However, this study did indicate that MOPSET could be applied to complex watersheds with variable channel characteristics. Further refinements are needed as more experience is gained in the application of the model.

6.1 South Fork Beargrass Creek Watershed

General Information

This watershed lies in the eastern portion of Louisville in Jefferson County. The U.S. Geological Survey has maintained a stream gaging station at Trevilian Way intermittently since 1939. The watershed has a drainage area of 17.2 sq mi. The downstream valley has very steep walls, the upstream section is hilly, and the majority of the basin is flat to gently rolling. The land use is primarily residential with some small vegetable farms. Much of the land is being held idle in anticipation of future urban development. A few sinkholes, ponds, and small wooded areas are scattered throughout the basin. The basin is transversed by numerous 4-lane state and interstate highways, city streets, and county roads. The soils in the watershed belong mostly to the Crider-Corydon association with level to sloping soils on broad ridges and steep, shallow soils over limestone on hillsides.

The watershed is located in the Louisville East and the Jeffersontown quadrangles of the 7.5 minute topographic map published in 1971 by the

U.S. Geological Survey. The map has a contour interval of 10 ft in rolling terrain and 5 ft in flat urban areas. Based on the topography indicated in the map, the watershed was divided into three reaches. Figure 7 shows a plan view of the the watershed. The reason that three reaches were used is because there are two major branches at the upstream. It is possible to combine the two upstream reaches and assume that all water in reach 2 be uniformly distributed along the stream in reach 1; however, this will result in a less accurate simulation. On the topographic map, each reach was measured for drainage area by a planimeter and for channel length by a chartometer. The overland flow surface length, OFSL, and the overland flow surface slope, OFSS, were determined by the method described in section 5.2. The percentage of urban area was measured, and the impervious area was assumed to be one-fifth of the urban area.

The soil survey maps of Jefferson County was issued in 1966 by the Soil Conservation Service [1966]. These maps, which indicate the boundary between different soils superimposed on aerial photographs, are very useful for determining the parameter ratios described in section 5.2. The area in each reach occupied by each major soil series was estimated. LZC and BMIR were computed for each series by Eqs. 40 and 41 and weighted over the area to obtain those for the reach. The areas in each reach covered by fallow, cropland, pasture and woods were estimated from the aerial photographs. By averaging the values listed in Table 3 for each watershed cover, ETLF and BUZC were also determined. SIAC was estimated from Table 4. The parameters for each reach were weighted by areas to obtain the base values. A division of the reach values by the base values provided the parameter ratios to be used as input.

The Louisville District of the Corps of Engineers has made an extensive study on the water surface profile in this watershed by the use of HEC-2 computer program. The profile was determined from the gage station up the stream through reaches 3 and 1, and no profile was run for reach 2. A total of 92 cross sections were used in reach 1 and 42 in reach 3. The output of the program included the discharge and end area for each of the 134 cross sections. The use of so many cross sections,

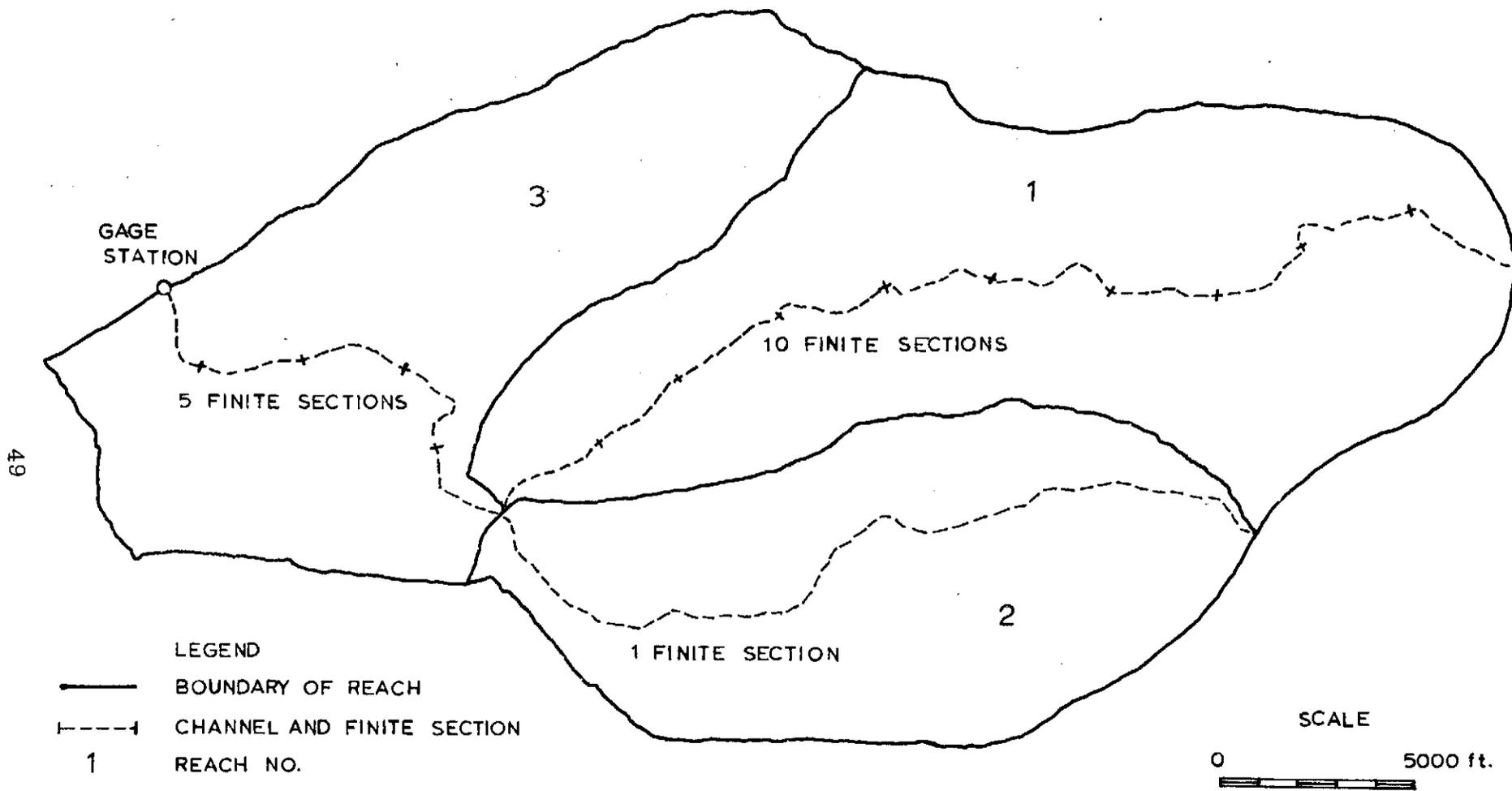


Figure 7. South Fork Beargrass Creek Watershed

some spaced as close as 5 ft apart, is neither economical nor necessary for MOPSET, in view of the many uncertainties involved in flood simulations. Therefore, the number of cross sections, one representing each finite section, was reduced to ten in reach 1 and five in reach 3. Each finite section consisted of only one finite difference division, i.e. the finite section was not further divided into finite differences. The discharge-end area rating curve for each cross section was the weighted average of all curves within the finite section. It will be shown that the use of fewer cross sections does not affect significantly the routed hydrograph. The α and m were determined at a number of points in reach 2 by the method described in section 3.4 and then averaged. Only one finite section was used in reach 2, which was divided into five finite differences. The α and m for low volume flows in reaches 1 and 3 were determined by the same method.

The climatological data for the 1971 water year was selected because they corresponded well with the information in the maps. One recording gage (No. 4954 at Louisville airport) and one storage gage (No. 0155 at Anchorage) were used. All the input data for this watershed are presented in Appendix C. The data shows that trips 1 and 2 are run at the same time. In reality, they were performed at two separate computer runs.

Comparison Between Synthesized and Recorded Flows

The synthesized flows presented herein were obtained by MOPSET, using the preset initial values for the land phase parameters. The sum of squares, SSQM, during the rough adjustment cycle was successively 166.466, 39.737, 9.434, 8.443, 9.061 and 9.538. Since the number of rough cycles had reached six, which was the minimum number specified, and since the SSQM had become greater twice, the program shifted to the fine adjustment cycle using the parameter values of the 4th rough cycle, which had the minimum SSQM. The SSQM during the fine adjustment cycle was successively 13.079, 12.517, and 12.605. Consequently, the program would shift to trip 2 using the parameter values of the 2nd fine cycle, should NLTR be set to 2.

Table 7 shows the optimized parameter values during the three fine adjustment cycles. Due to the insignificant amount of interflow, BIVF was set to 0. It can be seen that the parameter values

TABLE 7. PARAMETER VALUES DURING FINE ADJUSTMENT CYCLES FOR SOUTH FORK BEARGRASS CREEK WATERSHED

CYCLE NO.	LZC	BMIR	SUZC	ETLF	BUZC	SIAC	ANNUAL sfd
1	2.0000	8.2726	0.3000	0.0507	0.2864	0.1125	9775.5
2	3.9530	5.2540	0.1500	0.0568	0.2586	0.0562	10691.1
3	7.7819	3.8379	0.2019	0.1136	0.3393	0.0519	9598.9

TABLE 8. COMPARISON OF RECORDED AND SYNTHESIZED PEAKS FOR SOUTH FORK BEARGRASS CREEK WATERSHED

RECORDED PEAK			SYNTHESIZED PEAK		
DATE	TIME	CFS	DATE	TIME	CFS
12-22	0245	854	12-22	0200	491
2-22	0245	1,740	2-22	0100	3,168
5-6	1515	828	5-6	1100	910
5-25	0115	918	5-24	2400	1,886
6-15	1445	776	6-15	2100	1,896
6-21	1630	1,000	6-21	1500	718
7-18	2030	2,930	7-18	1700	4,263

had not converged when the program shifted to trip 2. It was suspected that this difficulty was due to the small number of rough cycles specified. However, after changing MNRC to 12, the same results were obtained because the SSQM at the fourth cycle was still the smallest, even though twelve rough cycles were run. The inability to converge certainly posed a serious problem in determining which set of parameter values were to be used. For example, cycle 2 had a SSQM of 12.517, which was not too much different from the 12.605 in cycle 3, then why should the optimum set of parameters be based on cycle 2 but not on cycle 3. In fact, the synthesized annual flow was 10,691 sfd in cycle 2 and 9,599 sfd in cycle 3; while the recorded flow was 9260 sfd, which checked more closely with cycle 3 than with cycle 2. Should the slight difference in SSQM outweigh the large difference in annual flow? It is therefore recommended that the optimum set of parameter values be based not on the SSQM alone. A major contribution of MOPSET is its ability to generate a variety of adjustment cycles, each with a different set of parameter values. It is up to the user to inspect the data and determine which set of parameter values are to be used for trip 2.

Figure 8 gives a comparison between synthesized and recorded monthly flows. Unless noted otherwise, all the flows presented in this report are referred to those at the gaging station, which is located at the mouth of the watershed. The synthesized flow was obtained from trip 1, using the parameter values of the second fine adjustment cycle. Also shown in the figure is the monthly precipitation and evapotranspiration. It can be seen that in some months the synthesized flow is greater than the recorded flow, while in other months the reverse is true. Even though the match is not considered very good, both flows have the same general trend and respond well with the climatological data. The figure also shows that the synthesized monthly flow before routing is not too much different from that after routing. The former used a 20 min loop and considered the watershed as one reach, while the latter used a 15 min loop and divided the watershed into three reaches. The channel routing was performed by the linear procedure with one hour looping.

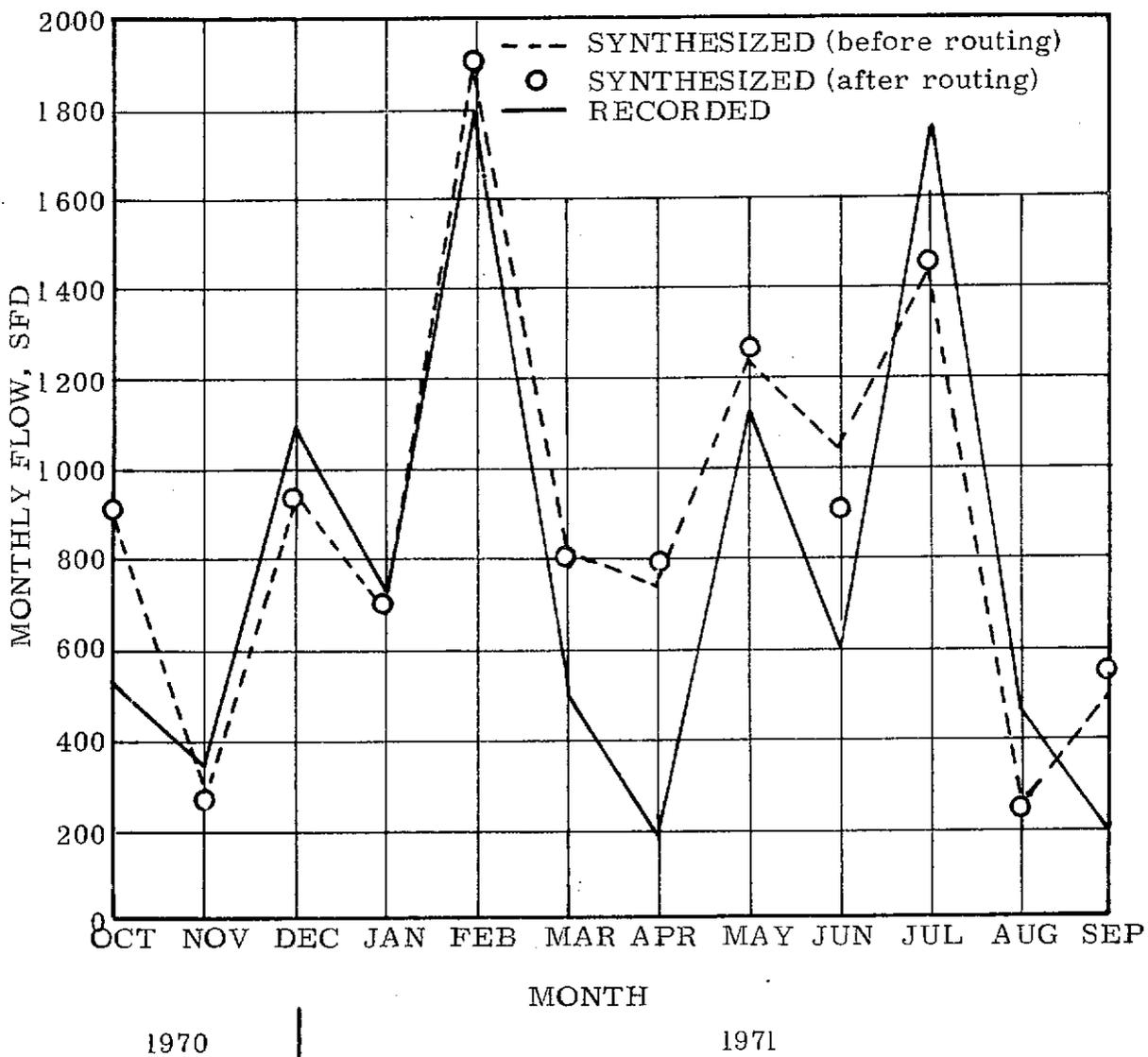
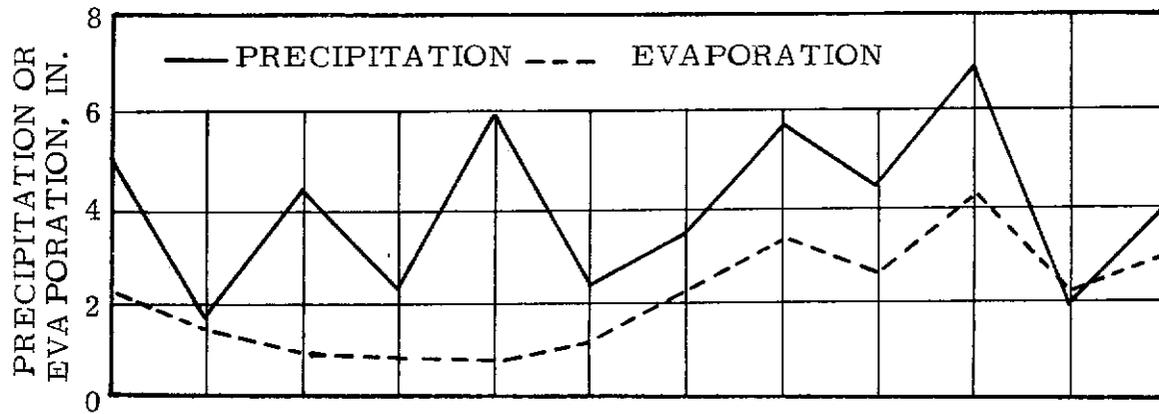


Figure 8. Comparison of Synthesized and Recorded Monthly Flows for South Fork Beargrass Creek Watershed

Figure 9 shows a comparison between the synthesized and recorded daily flows during February and July in 1971, the two months with the largest precipitation and runoff. The synthesized flow was obtained from trip 2, in which channel routing was performed. As can be seen, both flows check quite well. The computer time for trip 1 was 16 sec, and that for trip 2 with one year of channel routing was 60 sec, excluding the compiling time.

Table 8 gives a comparison of recorded and synthesized peak flows. Although the synthesized peaks are mostly much greater than the recorded peaks, the timings of both peaks are nearly the same.

Effect of Routing Procedures

Figure 10 shows the effect of number of cross sections used in channel routing on the synthesized hourly flow at the end of reaches 1 and 3. The solid curves represent the result of channel routing by the use of 16 finite sections, as shown in Figure 7, while the dashed curves use 135 finite sections. It can be seen that the results are not significantly different. The computer time for one month of channel routing was 6 sec for the former but 31 sec for the latter.

Figure 11 presents a comparison among three different routing procedures, viz. linear routing with 1 hour time increments, linear routing with 15 min time increments, and nonlinear routing with 15 min time increments. It can be seen that there is very little difference between 1 hr and 15 min routings, and that the nonlinear routing checks very well with the linear routing at all times except at the peak. The computer time for one month of channel routing is 6 sec for the linear 1 hr routing, 17 sec for the linear 15 min routing, and 38 sec for the nonlinear 15 min routing. In view of the good agreement between 1 hr and 15 min routings and the large increase in computer times for the nonlinear routing, only the linear 1 hr routing was used for the two other watersheds described in this chapter.

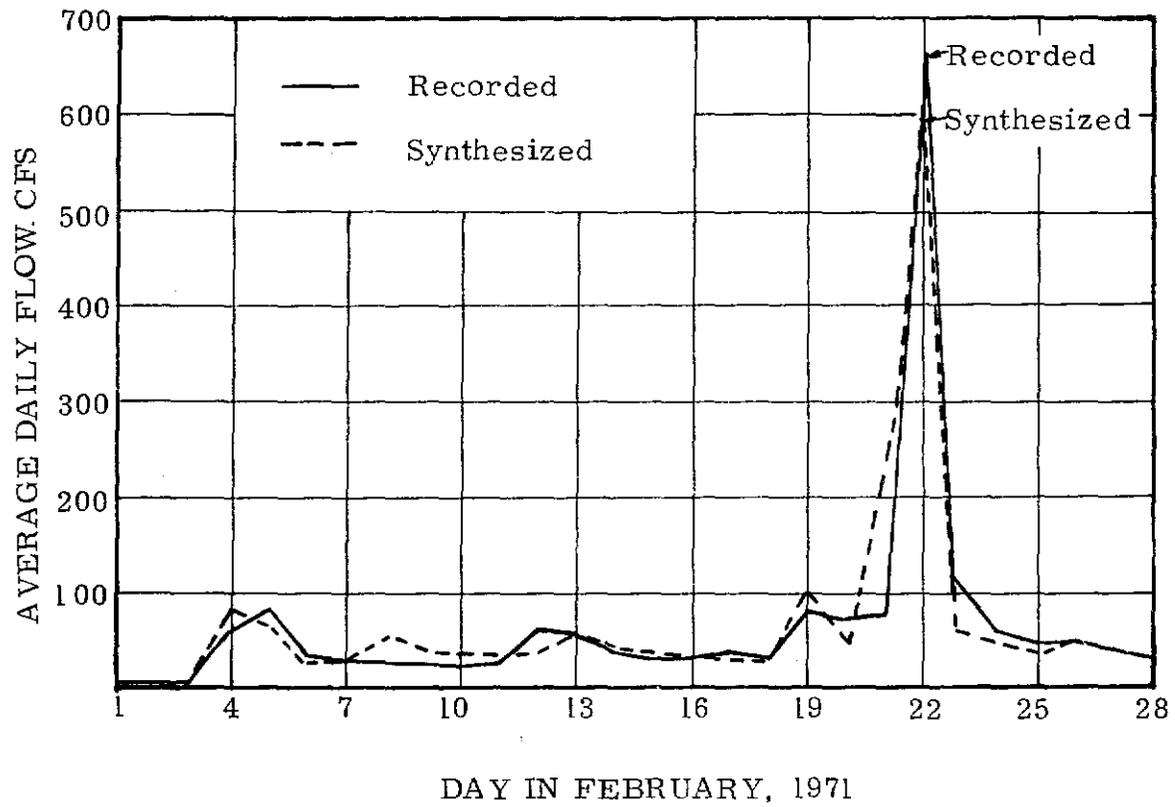
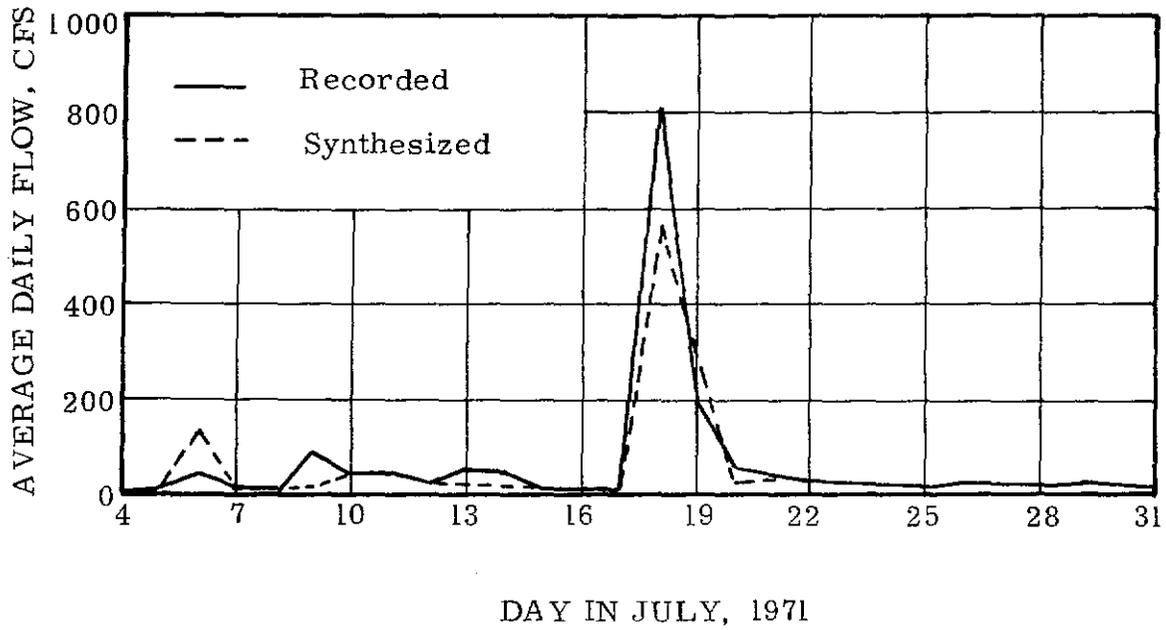


Figure 9. Comparison of synthesized and Recorded Daily Flows for South Fork Beargrass Creek Watershed

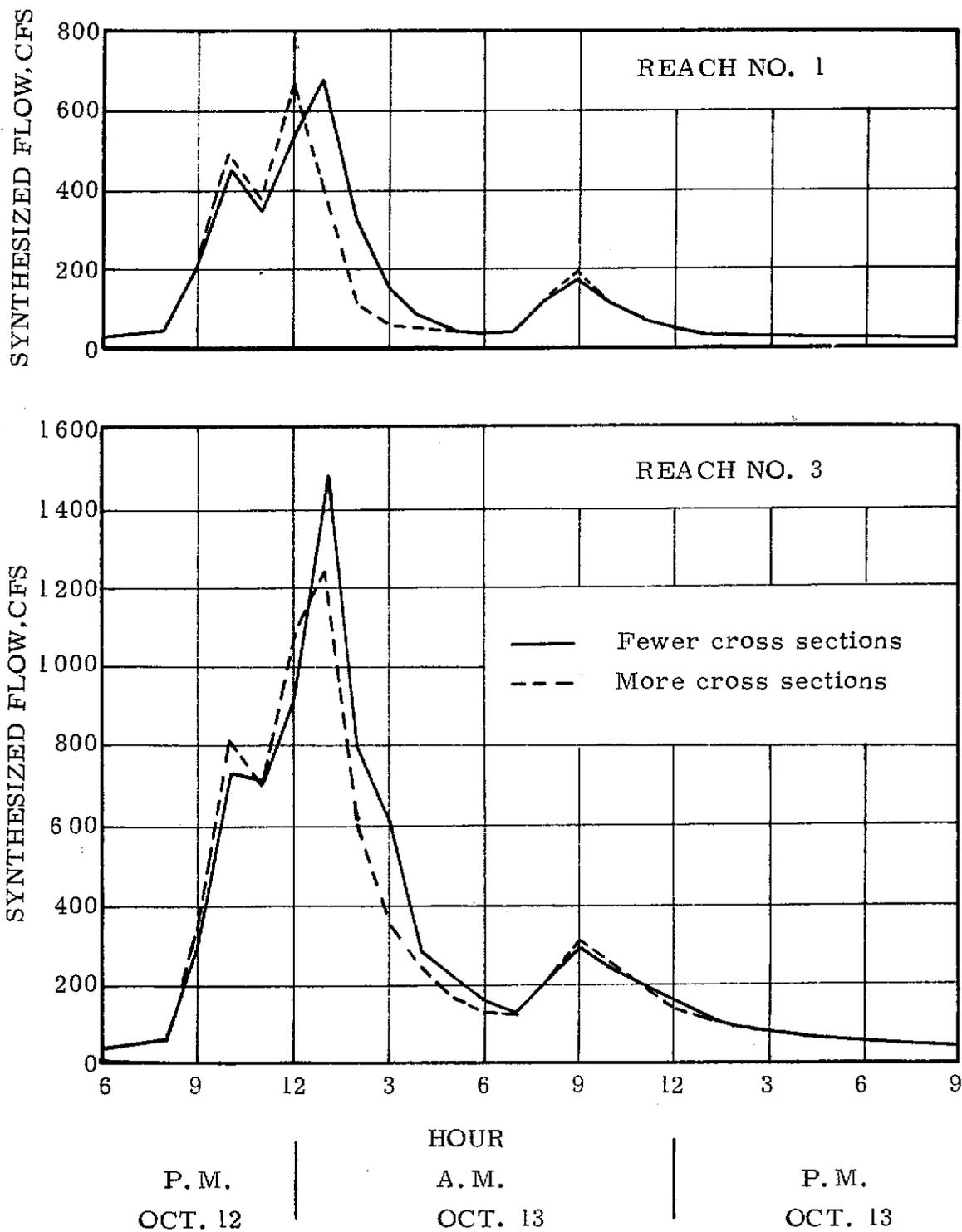


Figure 10. Effect of Number of Cross Sections on Synthesized Flow

6.2 Floyds Fork Watershed

General Information

This watershed lies in a four county area, including Jefferson, Shelby, Oldham, and Henry, with a drainage area of 138 sq mi. The U.S. Geological Survey has maintained a stream gaging station in the vicinity of Fisherville since 1944. The area is more hilly with an average overland flow surface slope of 6.1 percent, compared to the 2.1 percent for the Beargrass Creek. Figure 12 shows a plan view of the watershed. The gaging station is located at the mouth of the watershed.

Very little information is known about the watershed other than the 7.5 min topographic maps issued by the U.S. Geological Survey. The watershed is located in the following quadrangles: Jeffersontown, Fisherville, Crestwood, La Grange, Simpsonville, Ballardsville, Smithfield, Eminence, and New Castle. The maps have a contour interval of 10 ft. The watershed was divided into 5 reaches. The drainage area, OFSL, OFSS, and the channel length of each reach were measured from the map.

The Louisville District of the Corps of Engineers has run a water surface profile by the HEC-2 program in the Jefferson County area, which covers the entire reach 5 and the major part of reach 3. The discharge-end area rating curves of 37 cross sections obtained from the HEC-2 program were used in this study. The values of α and m for reaches 1, 2, and 4 as well as those for low volume flows in reaches 3 and 5 were measured from the topographic map as described in section 3.4. Due to the lack of information, the parameter ratios were assigned unity for each reach.

The rainfall data were obtained from one recording gage (NO. 4954 at Louisville airport) and two storage gages (NO. 4595 at La Grange and NO. 7324 at Shelbyville). Gage 4595 was used for reaches 1, 2 and 3, while gage 7324 was used for reaches 4 and 5. The program was run for the 1958 water year. An

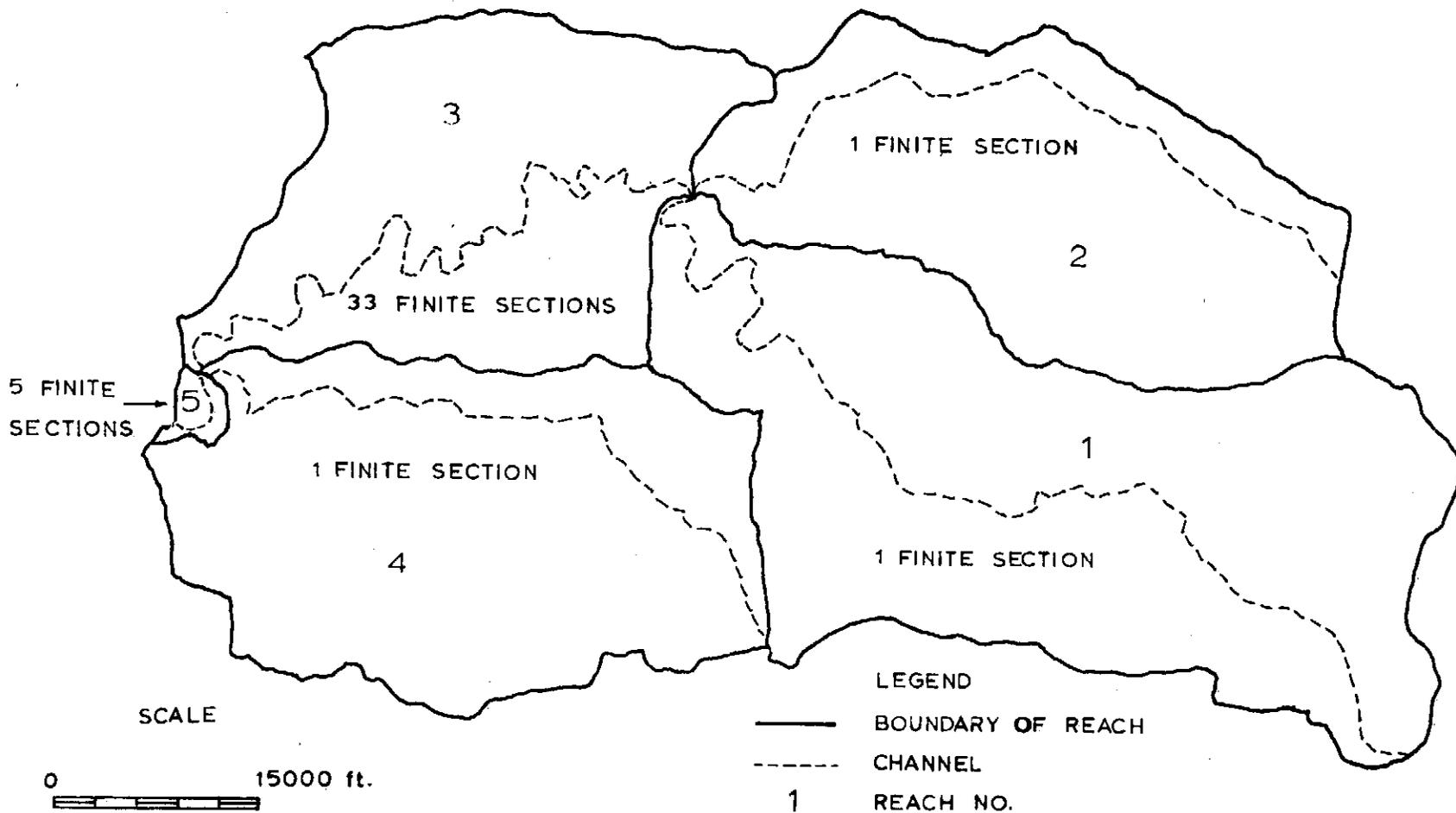


Figure 12. Floyds Fork Watershed

estimated potential annual evapotranspiration, EPAET, of 35.2 in. and a mean annual number of rainy days, MNRD, of 120 days were used for generating the daily potential evapotranspiration.

Comparison Between Synthesized and Recorded Flows

Trip 1 was run using the preset initial parameter values. The sum of squares, SSQM, during the rough adjustment cycle was successively 100.867, 12.755, 91.478, 9.528, 109.673, 8.165, 75.673, 8.161, 118.628, 8.140, 142.357, and 8.229. Because the minimum SSQM occurred at the 10th rough cycle and the result was worsened twice, the program then shifted to the fine adjustment cycle using the parameter values of the 10th rough cycle. The SSQM for the first fine cycle was 9.228, and that for the second cycle was 133.488. As the SSQM had become greater, the program stopped at the second cycle. This is undesirable because more cycles are needed to obtain better parameter values. It was found that the oscillation in SSQM was caused by the cyclic change in the values of SUZC, ETLF and SIAC. For example, the adjustment of SUZC was based on FSUZC, which is the sum of the monthly flow deviation indices of November, July, August and September. If FSUZC was positive, SUZC was increased by multiplying with $(1 + FSUZC)$ to decrease the flow of the index months. If FSUZC was negative, SUZC was decreased by dividing with $(1 - FSUZC)$. To prevent too much of a change, the absolute value of FSUZC was limited to a maximum of 1.0. If FSUZC exceeded 1.0, it was set equal to 1.0; if it was less than -1.0, it was set equal to -1.0. The sum of monthly flow deviation indices during the rough adjustment cycle was successively -11.777, +2.33, -13.302, + 0.768, -15.170, +0.388, -11.353, +1.882, -15.31, +1.766, -17.492, and +1.777, and the corresponding SUZC was 1.300, 0.650, 1.300, 0.650, 1.149, 0.575, 0.797, 0.399, 0.797, 0.399, 0.797 and 0.399. The same phenomenon also persisted in the adjustment of ETLF and SIAC. This oscillation was due to the large values of the monthly flow deviation indices. It is not known if the very large indices were the results of inaccurate data, because only data of the 1958 water year were used. It is suggested that, when oscillations occur, other initial values be tried. However, due to the limit of the computer fund, this was not done in this investigation.

Figure 13 shows a comparison between the synthesized and recorded monthly flows. The parameters of the first fine adjustment cycle was used to obtain the synthesized flow. It can be seen that there is a large difference between the synthesized and recorded flows for the month of November. The large amount of precipitation in November does not seem commensurate with the recorded flow. This may indicate that the precipitation recorded at the gages was not representative of what actually fell in the watershed. Due to this data difficulty, MOPSET would over adjust, thus resulting in larger monthly flow deviation indices and a greater sum of squares.

Figure 14 shows a comparison between the synthesized and recorded daily flows from January through April. Because the synthesized monthly flow during this period was smaller than the recorded flow, the synthesized daily flow was also smaller. Nevertheless, the general shape of the two hydrographs was quite similar. The maximum length of finite difference divisions used for channel routing was 10,000 ft. It was found that the result was not too much different when the maximum length was reduced to 5,000 ft.

Effects of Channel Improvements

The values of α and m for reaches 1, 2 and 4 were determined from the topographic map by assuming the channel as triangular with $m=1.33$. It will be interesting to find the effect of channel improvements on downstream floods by increasing the value of m for these reaches to 1.67, which is the case for a rectangular cross section. In both cases, the same values of α (i.e. 0.35 for reach 1, 0.44 for reach 2, and 0.52 for reach 4) were used.

Figure 15 shows the effect of channel improvements on the synthesized hydrograph at the mouth of the watershed. It can be seen that channel improvements increase the hydrograph peak and cause the peak flow to occur earlier, as would be expected.

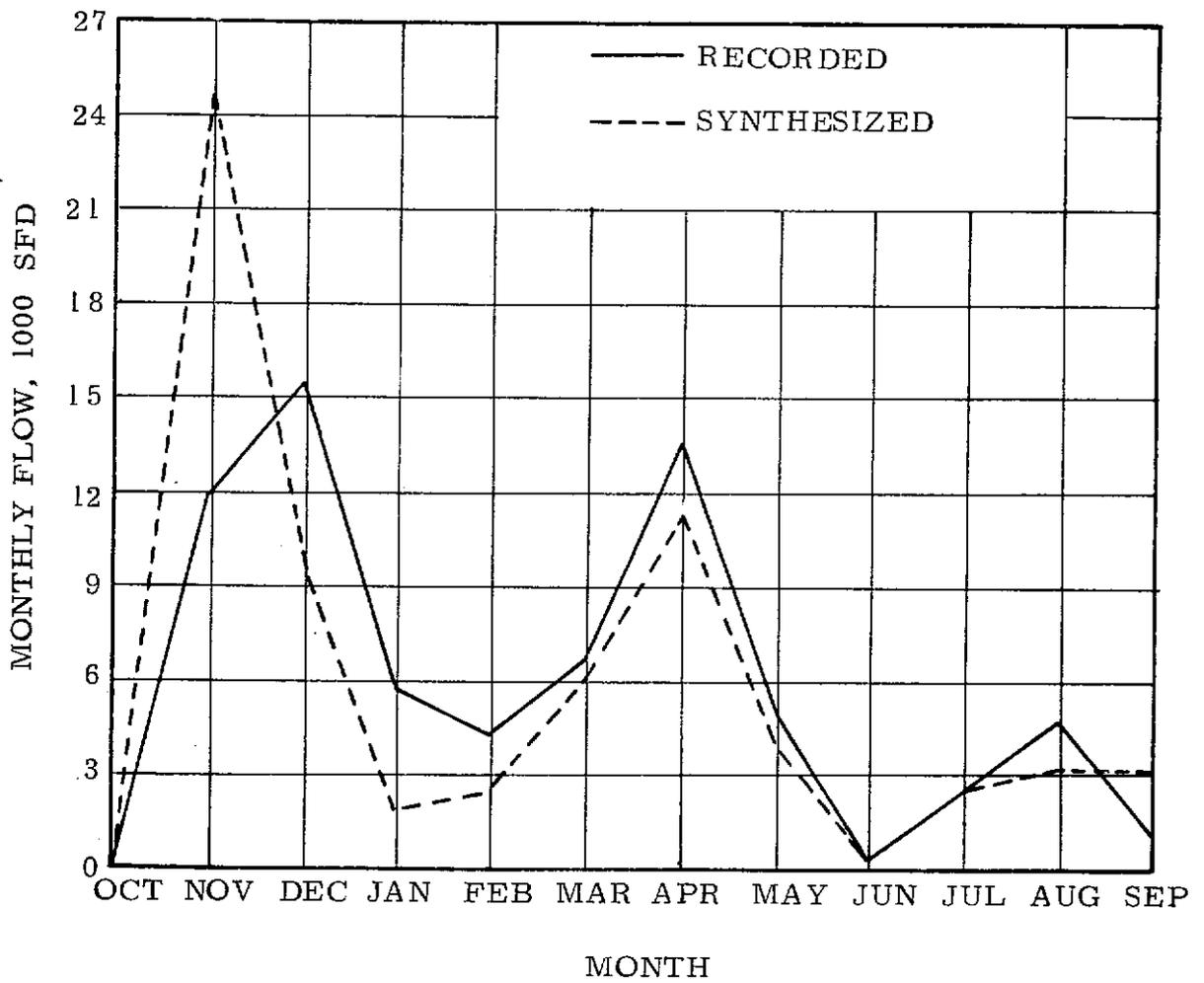
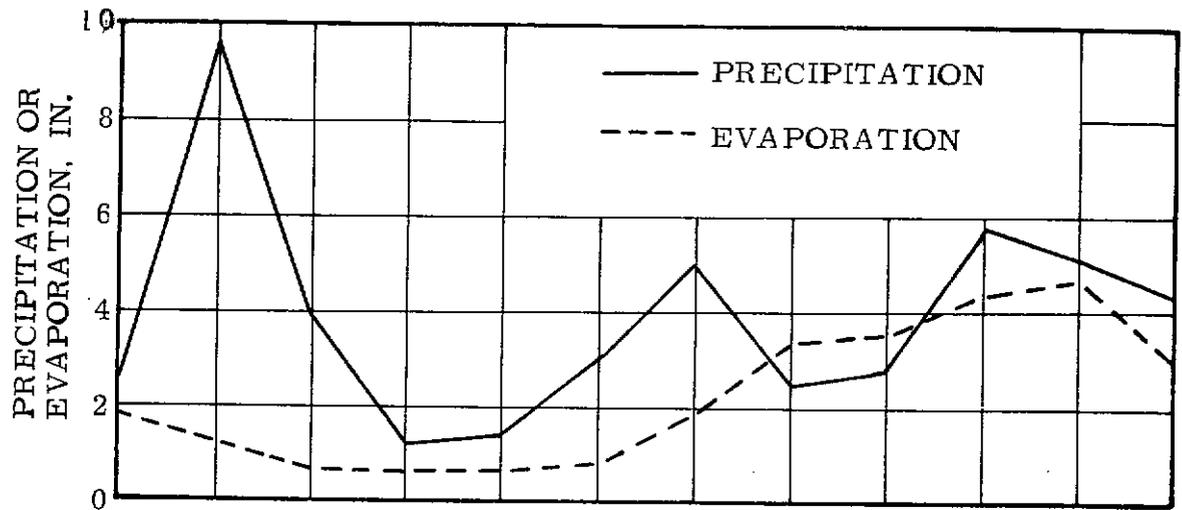


Figure 13. Comparison of Synthesized and Recorded Monthly Flows for Floyds Fork Watershed

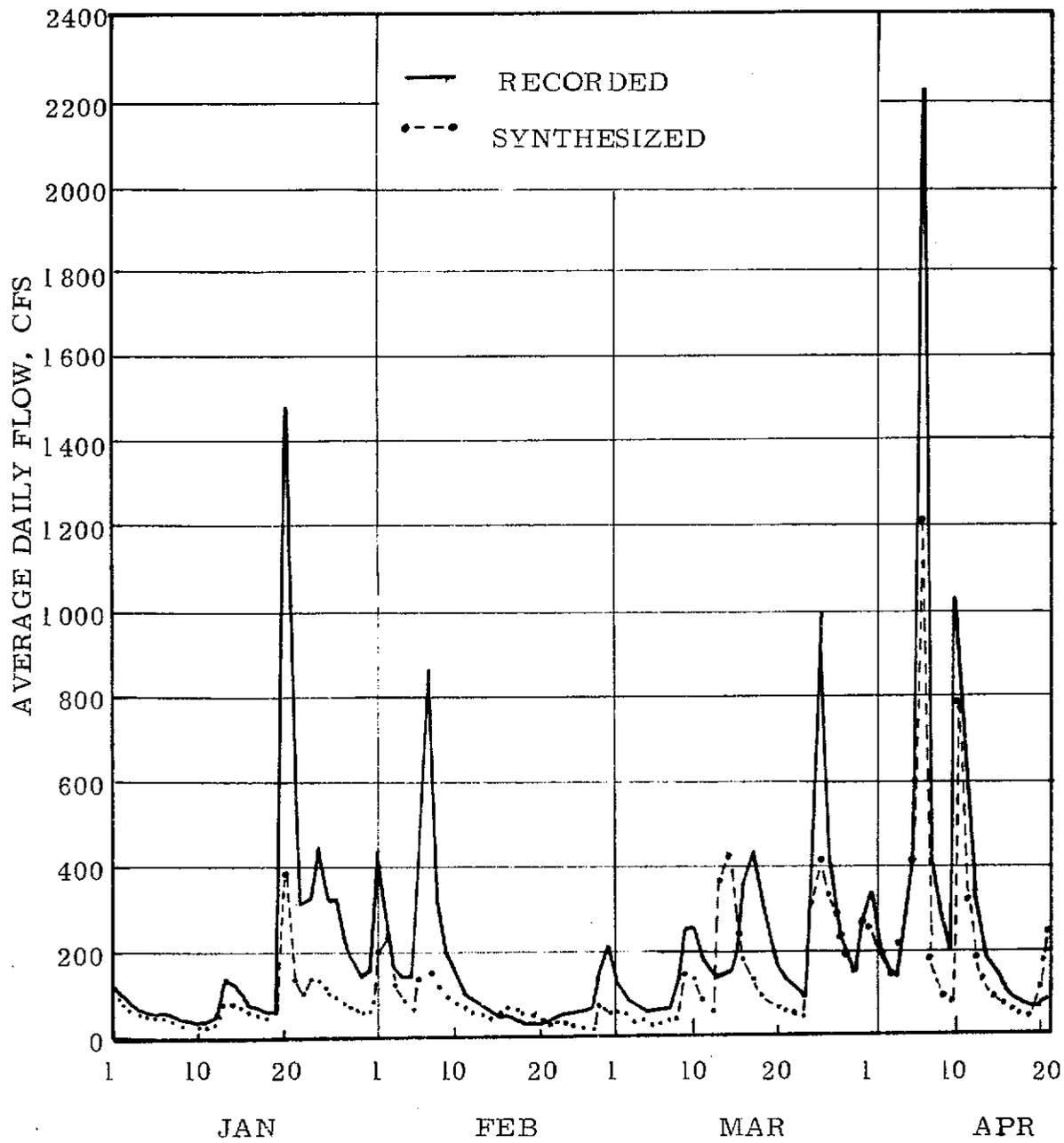


Figure 14. Comparison of Synthesized and Recorded Daily Flows for Floyd's Fork Watershed

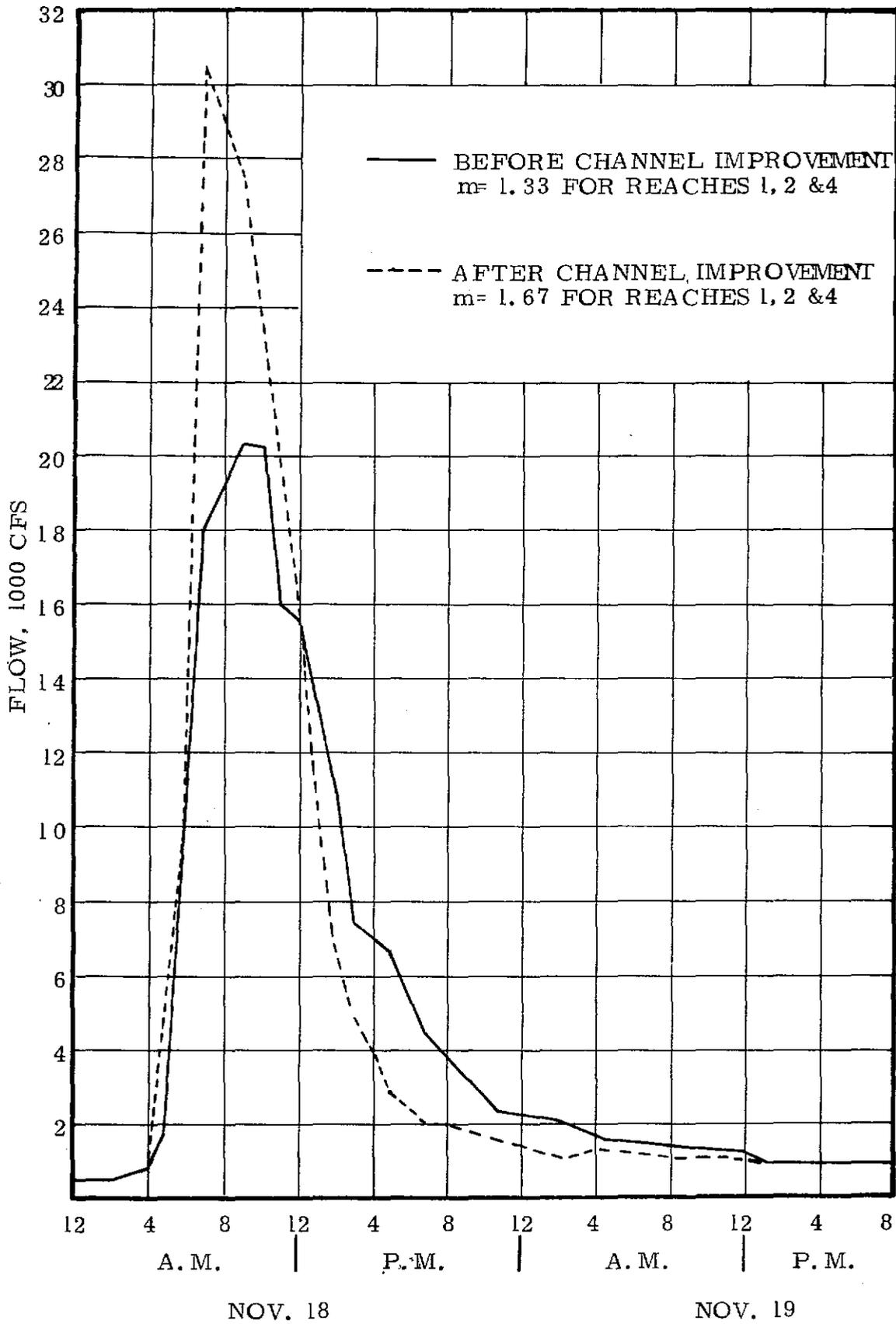


Figure 15. Effect of Channel Improvement on Downstream Floods

6.3 North Fork Nolin River Watershed

General Information

Figure 16 shows a plan view of the North Fork Nolin River Watershed located in Larue County. Two stream gaging stations have been maintained in this watershed. One gage, located at Hodgenville, has been recorded since 1941 and the other gage, located on McDougal Creek at the end of reach 1, has been recorded since 1953. In this study, only the gage at Hodgenville was employed for the optimization purpose. The watershed has a drainage area of 36.4 sq mi and is located in the Hodgenville, Magnolia, Hibernia, Howardstown and Nelsonville quadrangles of the 15 min topographic map with a contour interval of 20 ft. This watershed was selected because the Soil Conservation Service [1972] has made a study of this watershed and many of their data can be used as input to test the capability of MOPSET.

The watershed was divided into 15 reaches. Each reach was divided into one or more finite sections, as can be seen from Figure 16. Each finite section was divided into one or more finite differences. The maximum length of a finite difference division was about 5,000 ft. Each finite section had either a discharge-end area rating curve obtained from the WSP-2 computer program or a set of α and m measured from the topographic map. The total number of finite sections for the entire watershed was 38, of which 30 involved the use of discharge-end area rating curves. Because all the rating curves started from a small discharge, CONOPT(5) was set to 0, and no input of α and m for low volume flows was needed. Based on the information of soil type and watershed cover, the parameter ratios for each reach were estimated.

Also shown in Figure 16 is the location of six proposed flood control structures. The reason that these structures were included was due to the fact that their elevation-discharge-storage rating curves were reported by the Soil Conservation Service [1972]. The purposes

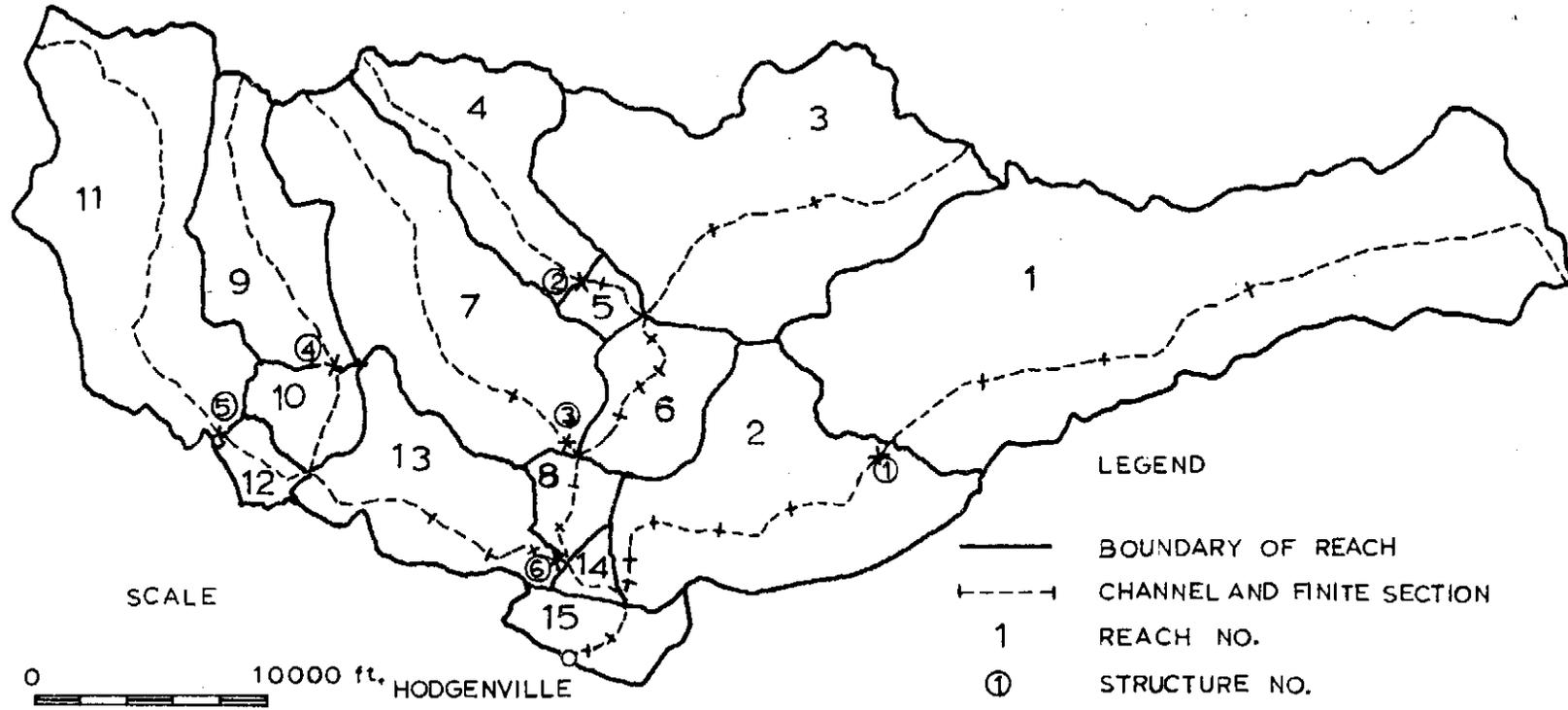


Figure 16. North Fork Nolin River Watershed

of including these structures were to check the capability of MOPSET in handling real data and also to illustrate the effect of structures on routed hydrographs.

The analyses were made for the 1965 water year. The rainfall data were obtained from a recording gage (NO. 3929 at Hodgenville), and no storage gage was used. The daily potential evapotranspiration was estimated by assuming 36 in. for EPAET and 116 days for MNRD.

Comparison Between Synthesized and Recorded Flows

Trip 1 was run using the preset initial parameter values. The SSQM during the rough adjustment cycle was successively 65.383, 4.134, 3.245, 3.278, 4.465, and 3.689, and that during the fine adjustment cycle was 1.706 and 3.288. A SSQM of 1.706 was much smaller than that obtained for the two previous watersheds. However, a closer inspection of the data revealed that the optimization procedure yielded a BMIR of 15.667 in./hr. As a result, more than three-fourths of the total flow was composed of the base flow, and the synthesized daily flow during the peak day was much less than the recorded flow. This further substantiates the previous contention that the optimum set of parameter values should not be based solely on SSQM. A number of fine adjustment cycles should be run and the best set of parameter values selected, even though the resulting SSQM may not be a minimum.

Figure 17 shows a comparison between the synthesized monthly flow obtained from trip 1 and the recorded flow. Two sets of parameter values were used. Synthesized flow NO. 1 was the result given by the first fine adjustment cycle with a SSQM of 1.706. Because BMIR was considered too large, the program was rerun by setting LBMIR to TRUE, so the alternate procedure for adjusting BMIR was evoked. The values of BMIR was reduced 10 percent each time, and synthesized flow NO. 2 was the result given by the 9th fine adjustment cycle with a SSMQ of 4.343. The parameter values used for these two synthesized flows, as well as synthesized flow NO. 3, are listed in

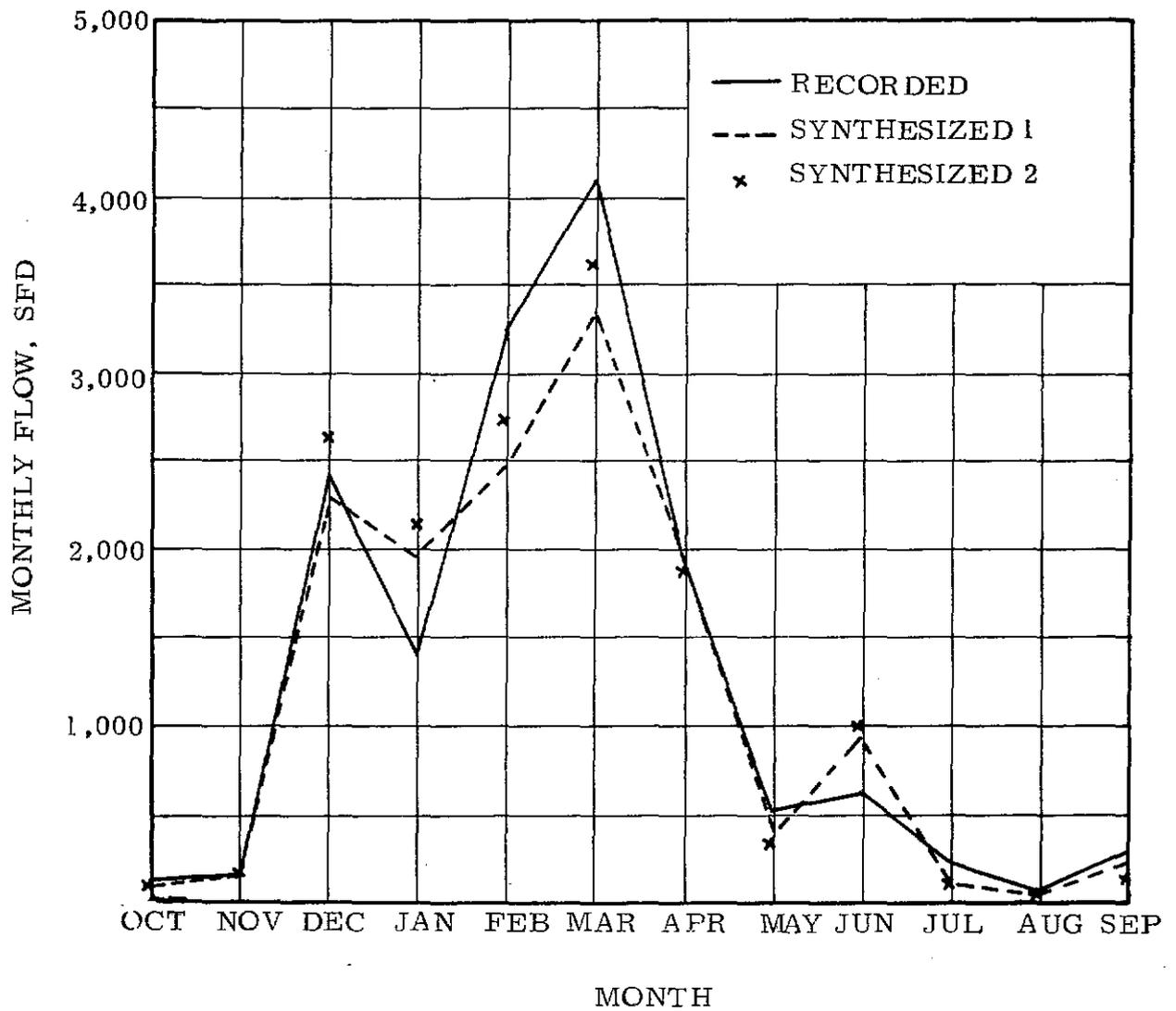
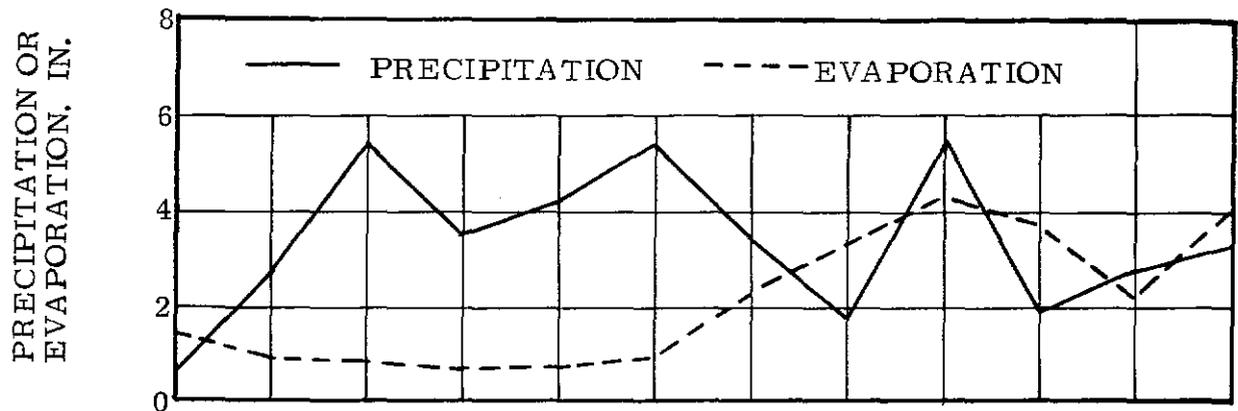


Figure 17. Comparison of Synthesized and Recorded Monthly Flows for North Fork Nolin River Watershed

Table 9. Even though flow NO. 2 has a larger SSQM, it checks better with the recorded flow not only in monthly variations but also in the total annual runoff. The recorded annual flow volume is 15,070 sfd, compared to 14,616 sfd for flow NO. 2 and 13,823 for flow NO. 1.

Figure 18 shows a comparison between the synthesized and recorded daily flows. The synthesized flow was obtained from trip 2 without the six structures. It can be seen that synthesized flow NO. 1 with a large BMIR does not match the recorded flow. The match is improved if synthesized flow NO. 2 with a much smaller BMIR is used. The large difference between the recorded and synthesized flows during December 11 and 12 may be the result of data errors, because it does not appear that the volume of recorded flow should be that large compared to the volume of precipitation. In order to compensate for any data difficulty, several years of data should be used.

Effect of Hydraulic Structures

Figure 19 shows the effect of flood control structures on daily flows. The location of the six structures are shown in Figure 16. It was arbitrarily assumed that the storages of all structures were only half full on October 1. To increase the flow peaks, synthesized flow NO. 3, having the parameter values listed in Table 9, were used. It can be seen that these structures have a significant effect in reducing the flow during peak days. The large difference of flow on December 4 was due to the fact that some of the structures were not completely filled. For example, the outflow from structure NO. 1, which has a storage capacity of 670 ac ft, started at 2 am, December 4, while the outflow from structure NO. 6 with a capacity of 401 ac ft did not start until 11 am on the same day. The significant difference in total flow during December was caused by this storage effect as well as the evaporation of water from the structure.

Figure 20 shows the effect of these six structures on the flood hydrograph. The structures decrease considerably the flood peak but increase the flow after the peak. These figures clearly demonstrate

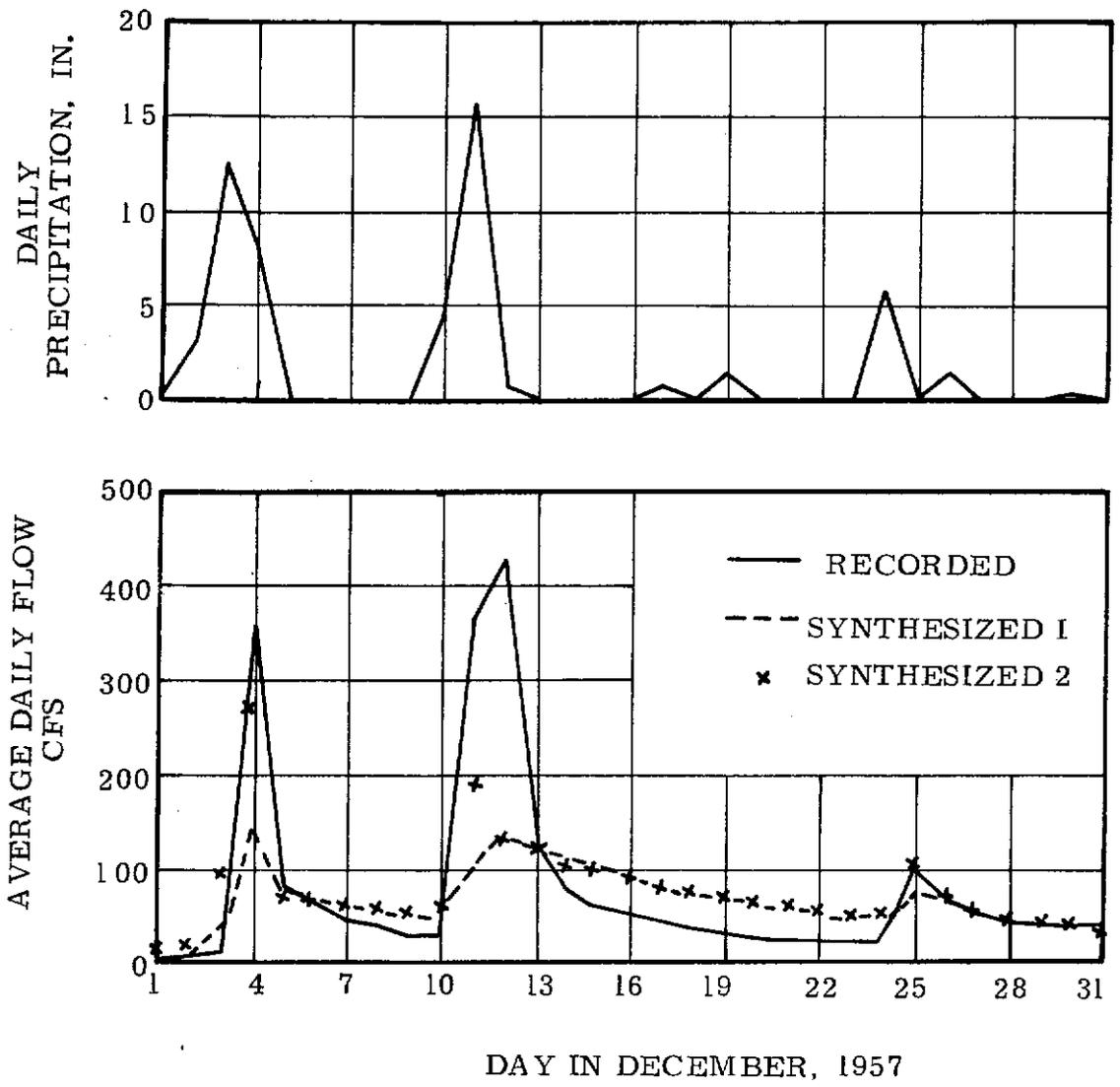


Figure 18. Comparison of Synthesized and Recorded Daily Flows for North Fork Nolin River Watershed

the effectiveness of using flood control structures to reduce downstream floods.

TABLE 9. PARAMETERS USED FOR SIMULATION OF NORTH FORK NOLIN RIVER WATERSHED

NQ	SSQM	LZC	BMIR	SUZC	ETLF	BUZC	SIAC	LZC	ANNUAL SED
1	1.706	4.251	15.667	0.764	0.010	0.474	0.388	3.839	13.823
2	4.343	3.149	6.744	0.744	0.120	0.294	0.018	2.933	14.616
3.	8.263	1.740	4.920	0.990	0.070	0.160	0.010	1.400	15.725

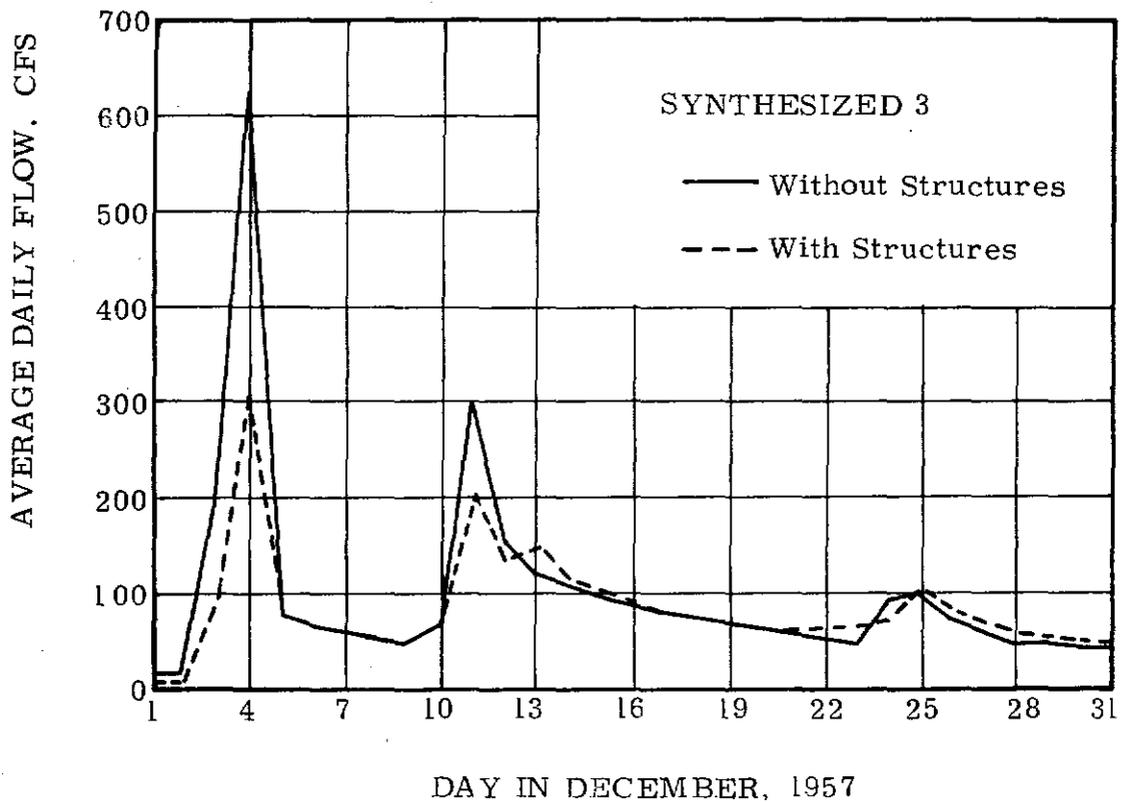


Figure 19. Effect of Flood Control Structures on Daily Flow

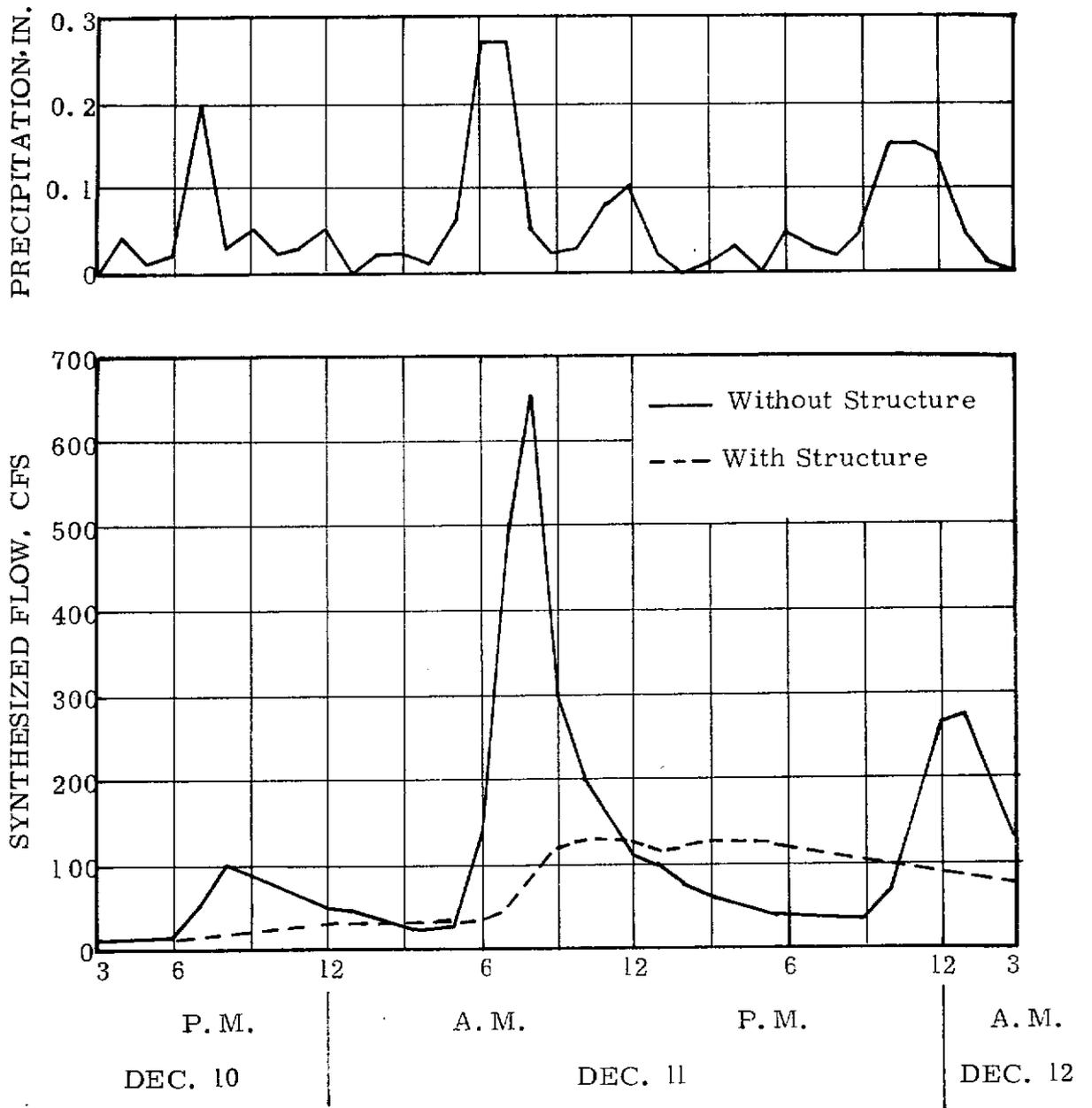


Figure 20. Effect of Flood Control Structures on Flood Hydrograph

CHAPTER VII

SUMMARY AND CONCLUSIONS

It is well known that the improvement of stream channels by straightening, lining, leveeing, or clearing and snagging generally increases the downstream peak flows. However, because of the unpredictable nature of hydrologic events, the effects of these improvements on downstream floods have not been evaluated quantitatively. The purpose of this research was to modify OPSET, a self-calibrating version of the Kentucky Watershed Model, and make it more suitable for evaluating the effect of channel improvements on downstream floods. This modified version of OPSET, which is given the name MOPSET, includes the following major changes:

1. A kinematic method of channel routing based on a finite difference scheme was used in place of the modified Muskingum method.

2. The watershed was divided into a number of segments, each subjected to different watershed characteristics and rainfall patterns.

3. The optimization of the seven land phase parameter values was achieved by assuming that the values in each segment bear fixed ratios to a set of base values, thus only the seven base values were optimized.

4. New options were provided that the user can specify the initial parameter values to be used for optimization and the number of rough or fine adjustment cycles desired, and that computations can be continued for additional cycles by simply reading in the data obtained from the previous cycle.

5. A storage routing subroutine was incorporated so that the model can be applied to large watersheds in which a number of reservoirs or flood control structures are located.

Procedures were developed for determining the routing parameters, α and m , either from a topographic map or from a discharge-end area rating curve. Methods were presented for estimating the parameter ratios to be used as input to the model. The program was well documented, together with a dictionary which defines every variable used in the main program as well as the subroutines.

The program was applied to three different watersheds, viz. the South Fork Beargrass Creek, the Floyds Fork, and the North Fork Nolin River. An inspection of the results obtained from these watersheds leads to the following conclusions:

1. The synthesized daily and monthly flows obtained from MOPSET checked favorably with the recorded flows in a qualitative sense. The large discrepancy in flow peaks at Floyd Fork Watershed was attributed to data errors because the recorded rainfall was simply not commensurate with the recorded streamflow. Therefore, several years of data must be used for calibration in order to balance the effects of positive and negative errors.

2. The adjustment rules used in MOPSET for optimizing the seven land phase parameter values were exactly the same as those in OPSET. It was found that the solution was not unique, because a slight difference in the assumed initial values yielded a set of optimized parameter values which were completely different. Furthermore, the parameters did not converge, and the values obtained from one adjustment cycle were quite different from the next cycle. This posed a serious problem in determining which cycle of values should be considered as optimum. It was also found that the cycle which resulted in a minimum sum of squares for the monthly flow deviation index might not give the most desirable solution. In the Nolin River Watershed, the optimum set of parameter values based on the minimum sum of squares produced such a large base flow and a small overland flow that the synthesized daily flow could not match the recorded peak. It is therefore suggested that the optimum set of

parameter values be based not on the minimum sum of squares but on a visual comparison between the synthesized and recorded flows. A major contribution of MOPSET is its ability to generate a variety of adjustment cycles, each with a different set of parameter values. It is up to the user to inspect the data and select the values to be used.

3. The use of one segment with an average set of parameter values weighted by areas and a 20 min looping yielded about the same monthly flow as when multiple segments and 15 min looping were used. Because most of the land phase parameters are optimized on the basis of the monthly flow, it is suggested that the entire watershed be considered as one segment in trip 1 but multiple segments in trip 2.

4. For the Beargrass Creek with a time of concentration of 4 hr, there existed very little difference in the simulated hydrograph whether a time increment of 1 hr or 15 min be used for channel routing. In view of the large amount of computer time required for nonlinear routing, it is suggested that linear and 1 hr routing be used unless local conditions indicate otherwise.

5. The finite difference method used in MOPSET for channel routing is unconditionally stable. Each finite difference division can be of any length. For the Floyds Fork Watershed, the results obtained from a maximum finite difference division of 10,000 ft were not too much different from those of 5,000 ft.

6. The computer time required for channel routing increases with the number of cross sections. The use of a large number of cross sections is unwarranted, because the results from the Beargrass Creek indicated that fewer cross sections could yield approximately the same results.

7. The values of α and m increase with the degree of channel improvements. Results from the Floyds Fork showed that channel

improvements increased the hydrograph peak downstream and caused the peak flow to occur earlier.

8. The use of flood control structures or reservoirs is an effective means of reducing downstream floods.

In this research, much emphasis was placed on the development of MOPSET. It is believed that if MOPSET is adequate to simulate stream flows, it can certainly be used to evaluate the effect of channel improvements on downstream floods, because the actual channel characteristics are employed in flood routing. Due to budget limitations and the exhaust of the computer fund, MOPSET has not been applied to watersheds where channel improvements have been documented. It is hoped that the publication of this report will promote its use, so that MOPSET can be constantly improved and updated to serve as an enviable tool for hydrologic simulations.

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APPENDIX A

LISTING OF MOPSET

C	MOPSET-- A MODIFIED VERSION OF OPSET, WHICH IS A SELF-CALIBRATING	MAIN0001
C	KENTUCKY WATERSHED MODEL WITH CAPABILITY OF ESTIMATING AN OPTIMUM	MAIN0002
C	SET OF WATERSHED PARAMETERS,UPDATED DECEMBER 1976	MAIN0003
	DIMENSION ADBF(15),ADIF(15),ALPH(150,50),AMBF(15),AMIF(15),	MAIN0004
1	AMNET(15),AMSE(15),ARHF(15),CONOPT(7),CVWINA(15),DARFA(150,50),	MAIN0005
2	DISCH(1000),DPET(366),DRGPM(15,366),DRHP(366,24),DRSF(366),	MAIN0006
3	DRSGP(5,366),DSSF(366),EDAREA(150,21),EMGWS(12),EMIFS(12),	MAIN0007
4	EMLZS(12),EMSIAM(12),EMUZC(12),EMUZS(12),EPCM(12),EQDF(15),	MAIN0008
5	EQDFIS(15),EXPT(150,50),FIMPA(15),FWTRA(15),GAFL(366),GWSA(15),	MAIN0009
6	HYDSTO(150,96),IFSA(15),IFST(15),ILST(15),INLST(15),IROUT(15),	MAIN0010
7	IRSTR(15),ISGA(15),ISGM(5),IUH(15,5),LZSA(15),MEDCY(12),MEDWY(12),	MAIN0011
8	NOFD(150),NUBR(15),OFMN(15),UFMNIS(15),OFRF(15),OFRFIS(15),	MAIN0012
9	OFSL(15),OFSS(15),OFUS(15),OFUSIS(15),OUTLST(15),PALPHA(150)	MAIN0013
	DIMENSION PEXPM(150),RBIVF(15),RBMIR(15),RBUZC(15),RCHLTH(15),	MAIN0014
1	RETLF(15),RGPM(15),RGPMB(15),RHF(15),RLZC(15),	MAIN0015
2	ROLF1(15),ROLF2(15),RSBBF(20),RSBD(20),RSBIF(20),RSIAC(15),	MAIN0016
3	RSUZC(15),SAREA(15),SBFRS(3,20),SGMD(5),SGRT(5),SGRT2(5),	MAIN0017
4	SIFRS(3,20),STOLST(15),STORAG(1000),SUBWFA(15),THSF(24),TITLE(20),	MAIN0018
5	TMBF(12),TMIF(12),TMNET(12),TMOF(12),TMPFT(12),TMPREC(12),	MAIN0019
6	TMRTF(12),TMSE(12),TMSTF(12),TMSTF1(12),TPLR(15),UZSA(15),	MAIN0020
7	VINCR(15),VINMRA(15),VWINA(15),WCFSA(15),WSG(15),WSG2(15),	MAIN0021
8	XAREA(150,21),XMPFT(12)	MAIN0022
	DIMENSION ADRGPM(366),CWCFSA(15),DRGPM1(366),ELEV(1000),FPERA(15),	MAIN0023
1	FSLTH(150),NFS(15),NFSEPR(15),NSC(150)	MAIN0024
	REAL IFPRC,IFRC,IFRL,IFS,IFSA,INLST,LZC,LZC1,LZRX,LZS,LZSA,LZSR,	MAIN0025
1	MNRD	MAIN0026
	LOGICAL LBMIR,LBUZC,LETLF,LLZC,LNPR,LRC,LSTR,SAVE	MAIN0027
	INTEGER CN, CONOPT, DATE, DAY, DPY, EHSgd, HOUR, HRF, HRL, PDAY,	MAIN0028

1	PRD, RHPD, RHPH, RSBD, SGMD, SGRT, SGRT2, TRIP, YEAR, YR1,	MAIN0029
2	YR2	MAIN0030
	INTEGER DPRD	MAIN0031
	DATA MEDCY/0,31,59,90,120,151,181,212,243,273,304,334/	MAIN0032
	DATA MEDWY/304,334,365,31,59,90,120,151,181,212,243,273/	MAIN0033
C	SPECIFY NUMBER OF STATION-YEARS INCLUDED IN COMPUTER RUN	MAIN0034
	NSYC=0	MAIN0035
	READ(5,10)NSYT	MAIN0036
10	FORMAT(16I5)	MAIN0037
20	NSYC= NSYC+1	MAIN0038
	SMF=0.0	MAIN0039
C	ASSUME 15 MINUTE ROUTING TIME INTERVAL	MAIN0040
	DELTAT=900.	MAIN0041
	NPPH=4	MAIN0042
C	READ TITLE TO COMPUTER RUN	MAIN0043
	READ (5,30) TITLE	MAIN0044
30	FORMAT(20A4)	MAIN0045
	WRITE(6,40)(TITLE(KTA),KTA=1,20)	MAIN0046
40	FORMAT(1H1, 25X,20A4)	MAIN0047
C	READ CONTROL OPTIONS	MAIN0048
	READ(5,10) (CONOPT(I),I=1,7)	MAIN0049
	IF(CONOPT(2) .NE. 1) GO TO 50	MAIN0050
C	USE ONE HOUR ROUTING TIME INTERVAL WHEN SPECIFIED	MAIN0051
	DELTAT=3600.	MAIN0052
	NPPH=1	MAIN0053
C	READ TRIP NUMBER AND CONTROL DATA FOR REACH AND CHANNEL	MAIN0054
50	READ(5,10)MNRC,NFTR,NLTR,NDCR	MAIN0055
	READ(5,10) NR,NGR,(NFS(KR),KR=1,NR)	MAIN0056
	WRITE(6,60)(CONOPT(I),I=1,7),MNRC,NFTR,NLTR,NR,NGR	MAIN0057
60	FORMAT(5X,10HCONOPT(1)=,I2,5X,10HCONOPT(2)=,I2,5X,10HCONOPT(3)=,I2,5X,10HCONOPT(4)=,I2,5X,10HCONOPT(5)=,I2,5X,10HCONOPT(6)=,I2,5X,10HCONOPT(7)=,I2,/,10X,5HMNRC=,I2,10X,5HNFTR=,I2,10X,5HNLTR=,I2,10X,5HNR=,I2,10X,5HNLR=,I2,10X,5HNGR=,I2)	MAIN0058
1	5X,10HCONOPT(4)=,I2,5X,10HCONOPT(5)=,I2,5X,10HCONOPT(6)=,I2,5X,10HCONOPT(7)=,I2,/,10X,5HMNRC=,I2,10X,5HNFTR=,I2,10X,5HNLTR=,I2,10X,5HNR=,I2,10X,5HNR=,I2,10X,5HNR=,I2,10X,5HNR=,I2)	MAIN0059
2	5X,10HCONOPT(7)=,I2,/,10X,5HMNRC=,I2,10X,5HNFTR=,I2,10X,5HNLTR=,I2,10X,5HNR=,I2,10X,5HNR=,I2,10X,5HNR=,I2)	MAIN0060
3	I2,12X,3HNR=,I2,11X,4HNGR=,I2)	MAIN0061
C	READ NUMBER OF BRANCHES IN UPSTREAM REACHES	MAIN0062
	READ(5,10) (NUBR(KR),KR=1,NR)	MAIN0063

DO 80 KR=1,NR	MAIN0064
NB=NUBR(KR)	MAIN0065
IF(NB.LE.0) GO TO 80	MAIN0066
C READ IDENTIFYING NUMBER OF UPSTREAM REACHES	MAIN0067
READ(5,10) (IUH(KR,N),N=1,NB)	MAIN0068
WRITE(6,70)KR,(IUH(KR,N),N=1,NB)	MAIN0069
70 FORMAT(5X,25HUPSTREAM REACHES OF REACH,I3,15H ARE AS FOLLOW:,5I5)	MAIN0070
80 CONTINUE	MAIN0071
C READ NUMBER OF WATERSHED TRAVEL HOURS AND OUTPUT REACHES	MAIN0072
READ(5,10) NWSTH,NOUT	MAIN0073
READ(5,10) (IROUT(I),I=1,NOUT)	MAIN0074
READ(5,90) RMPF	MAIN0075
90 FORMAT(8F10.2)	MAIN0076
WRITE(6,100)NWSTH,NOUT,(IROUT(I),I=1,NOUT)	MAIN0077
100 FORMAT(5X,6HNWSTH=,I5,5X,5HNOUT=,I2,55H REACHES AT WHICH FLOW IS	MAIN0078
1TO BE PRINTED ARE AS FOLLOW:,5I5)	MAIN0079
KNFS=0	MAIN0080
C SKIP READING CHANNEL AND STRUCTURE PARAMETERS IF TRIP 2 IS NOT RUN	MAIN0081
IF(NLTR .NE. 2) GO TO 290	MAIN0082
C READ CHANNEL PARAMETERS	MAIN0083
DO 230 KR=1,NR	MAIN0084
NFSEPR(KR)=KNFS	MAIN0085
NFSKR = NFS(KR)	MAIN0086
WRITE(6,110)KR,NFS(KR)	MAIN0087
110 FORMAT(/,5X,9HREACH NO.,I3,4H HAS,I3,16H FINITE SECTIONS)	MAIN0088
DO 220 KFS=1,NFSKR	MAIN0089
KNFS=KNFS+1	MAIN0090
KRKFS=NFSEPR(KR)+KFS	MAIN0091
ALPH(KRKFS,1)=0.0	MAIN0092
EXPT(KRKFS,1)=0.0	MAIN0093
C READ CONTROL DATA FOR EACH FINITE SECTION	MAIN0094
READ(5,120)NP,NSC(KRKFS),NOFD(KRKFS),FSLTH(KRKFS)	MAIN0095
120 FORMAT(3I5,F10.4)	MAIN0096
WRITE(6,130)KFS,NP,NSC(KRKFS),NOFD(KRKFS),FSLTH(KRKFS)	MAIN0097
130 FORMAT(5X,24HCHANNEL DATA FOR SECTION,I3,25H ARE TABULATED AS FOLL	MAIN0098

10W: , / , 5X, 3HNP=, I3, 5X, 4HNCS=, I2, 5X, 5HNOFD=, I3, 5X, 6HFSLTH=, F10.4)	MAIN0099
DAREA(KRKFS, NP+1)=0.0	MAIN0100
IF(NP .EQ. 2) GO TO 180	MAIN0101
C READ DISCHARGE-END AREA CURVE FOR EACH FINITE SECTION	MAIN0102
READ(5,140)(DISCH(I), DAREA(KRKFS, I), I=1, NP)	MAIN0103
140 FORMAT(8F10.2)	MAIN0104
IF(CONOPT(5).EQ.1) READ(5,190)ALPH(KRKFS, I), EXPT(KRKFS, I)	MAIN0105
DISCH(NP+1)=0.0	MAIN0106
WRITE(6,150)	MAIN0107
150 FORMAT (5X, 41HDISCHARGE AND END AREA RELATIONSHIPS ARE:)	MAIN0108
WRITE(6,160)(DISCH(I), DAREA(KRKFS, I), I=1, NP)	MAIN0109
160 FORMAT(8F15.2)	MAIN0110
WRITE(6,170)ALPH(KRKFS, I), EXPT(KRKFS, I)	MAIN0111
170 FORMAT(5X, 25HFOR LOW FLOW VOLUME, ALPH=, F10.3, 5X, 5HEXPT=, F10.3)	MAIN0112
C GENERATE DAREA VS ALPH AND DAREA VS EXPT FROM DISCHARGE AREA CURVE	MAIN0113
IF(NLTR.EQ.2) CALL ROUTAB(DISCH, DAREA, KRKFS, NP, ALPH, EXPT)	MAIN0114
GO TO 220	MAIN0115
180 NSCS=NSC(KRKFS)	MAIN0116
C IF DISCHARGE-END AREA CURVE IS NOT AVAILABLE, READ ALPH AND EXPT	MAIN0117
READ(5,190)(ALPH(KRKFS, I), EXPT(KRKFS, I), DAREA(KRKFS, I), I=1, NSCS)	MAIN0118
190 FORMAT(3F10.3)	MAIN0119
DAREA(KRKFS, NSCS+1)=0.0	MAIN0120
WRITE(6,200)NSCS	MAIN0121
200 FORMAT(5X, 27HALPH, EXPT, AND DAREA FOR THE, I2, 18H SLOPE CHAGES ARE:	MAIN0122
1)	MAIN0123
WRITE(6,210)(ALPH(KRKFS, I), EXPT(KRKFS, I), DAREA(KRKFS, I), I=1, NSCS)	MAIN0124
210 FORMAT(3(5X, F10.2))	MAIN0125
IF (NSC(KRKFS).LE.1) GO TO 220	MAIN0126
PALPHA(KRKFS) = ALPH(KRKFS, 1)	MAIN0127
PEXPM(KRKFS) = EXPT(KRKFS, 1)	MAIN0128
220 CONTINUE	MAIN0129
230 CONTINUE	MAIN0130
C READ STRUCTURE PARAMETERS	MAIN0131
READ(5,10) NSTR	MAIN0132
C SKIP READING STRUCTURE DATA IF NONE EXISTS	MAIN0133

IF(NSTR.EQ.0) GO TO 290	MAIN0134
IFIELD=1000/NSTR	MAIN0135
DU 280 KS=1,NSTR	MAIN0136
READ(5,10) NELV	MAIN0137
IEF=(KS-1)*IFIELD+1	MAIN0138
IEL=IEF+NELV-1	MAIN0139
C READ ELEVATION-DISCHARGE-STORAGE CURVE AND LOCATION OF STRUCTURES	MAIN0140
READ(5,240)(ELEV(IE),DISCH(IE),STORAG(IE),IE=IEF,IEL)	MAIN0141
240 FORMAT(9F8.2)	MAIN0142
READ(5,10) IRSTR(KS)	MAIN0143
KR=IRSTR(KS)	MAIN0144
C READ INITIAL CONDITIONS OF STRUCTURE	MAIN0145
READ(5,140) INLST(KR),OUTLST(KR),STOLST(KR)	MAIN0146
WRITE(6,250) KS,KR,NELV,INLST(KR),OUTLST(KR),STOLST(KR)	MAIN0147
250 FORMAT(/,5X,13HSTRUCTURE NO.,I2,30H IS LOCATED AT BOTTOM OF REACH,	MAIN0148
1 I3,/,5X,5HNELV=,I3,5X,6HINLST=,F10.2,5X,7HOUTLST=,F10.2,5X,7HSTOL	MAIN0149
2ST=,F10.2)	MAIN0150
WRITE(6,260)	MAIN0151
260 FORMAT(5X,50HELEVATION DISCHARGE AND STORAGE RELATIONSHIPS ARE:)	MAIN0152
WRITE(6,240) (ELEV(IE),DISCH(IE),STORAG(IE),IE=IEF,IEL)	MAIN0153
IFST(KR)=IEF	MAIN0154
ILST(KR)=IEL	MAIN0155
DO 270 IE=IEF,IEL	MAIN0156
270 STORAG(IE)=43560.*STORAG(IE)	MAIN0157
STOLST(KR)=43560.*STOLST(KR)	MAIN0158
280 CONTINUE	MAIN0159
C READ REACH PARAMETERS	MAIN0160
290 READ(5,300) (RCHLTH(I),SAREA(I),OFSS(I),OFMN(I),OFSL(I),	MAIN0161
10FMNIS(I),FIMPA(I),FWTRA(I),I=1,NR)	MAIN0162
300 FORMAT(8F10.4)	MAIN0163
WRITE(6,310)	MAIN0164
310 FORMAT(/,5X,3HNO.,9X,6HRCHLTH,10X,5HSAREA,11X,4HOFSS,11X,4HOFMN,	MAIN0165
1 11X,4HOFSL,9X,6HOFMNIS,10X,5HFIMPA,10X,5HFWTRA)	MAIN0166
WRITE(6,320) (I,RCHLTH(I),SAREA(I),OFSS(I),OFMN(I),OFSL(I),	MAIN0167
10FMNIS(I),FIMPA(I),FWTRA(I),I=1,NR)	MAIN0168

320	FORMAT(6X,I2,8F15.4)	MAIN0169
	READ(5,330) (RLZC(I),RBMIR(I),RSUZC(I),RETLF(I),RBUZC(I),RSIAC(I),	MAIN0170
	1RBIVF(I),I=1,NR)	MAIN0171
330	FORMAT(7F10.3)	MAIN0172
	WRITE(6,340)	MAIN0173
340	FORMAT(/,5X,3HNO.,11X,4HRLZC,10X,5HRBMIR,10X,5HRSUZC,10X,5HRETLF,	MAIN0174
	1 10X,5HRBUZC,10X,5HRSIAC,10X,5HRBIVF)	MAIN0175
	WRITE(6,350)(I,RLZC(I),RBMIR(I),RSUZC(I),RETLF(I),RBUZC(I),	MAIN0176
	1RSIAC(I),RBIVF(I),I=1,NR)	MAIN0177
350	FORMAT(6X,I2,7F15.3)	MAIN0178
C	SET INITIAL CONDITIONS	MAIN0179
	SGRT(1)=0.0	MAIN0180
	IFT = 1	MAIN0181
	LRC = .TRUE.	MAIN0182
	LLZC = .FALSE.	MAIN0183
	LBUZC = .FALSE.	MAIN0184
	LBMIR = .FALSE.	MAIN0185
	LETLF = .FALSE.	MAIN0186
	LNPR = .FALSE.	MAIN0187
	IF(CONOPT(2).EQ.0)LNPR=.TRUE.	MAIN0188
	ICYC=0	MAIN0189
	KRC = 1	MAIN0190
	KBRC = 0	MAIN0191
	KFFC = 0	MAIN0192
	SSSQM = 950.0	MAIN0193
C	READ OTHER PARAMETERS	MAIN0194
	READ(5,90) (RGPMB(I),I=1,NR)	MAIN0195
	READ(5,90) GWETF, DIV	MAIN0196
	READ(5,90) (VINMRA(I),SUBWFA(I),I=1,NR)	MAIN0197
	WRITE(6,360)RMPF,GWETF, DIV	MAIN0198
360	FORMAT(/,5X,5HRMPF=,F10.2,5X,6HGWETF=,F10.2,5X,4HDIV=,F10.2)	MAIN0199
	WRITE(6,370)	MAIN0200
370	FORMAT(/,5X,3HNO.,10X,5HRGPMB ,9X,6HVINMRA,9X,6HSUBWFA)	MAIN0201
	WRITE(6,380)(I,RGPMB(I),VINMRA(I),SUBWFA(I),I=1,NR)	MAIN0202
380	FORMAT(6X,I2,3F15.2)	MAIN0203

C	CALCULATE CONSTANTS SET BY REACH PARAMETERS	MAIN0204
	AREA=0.0	MAIN0205
	VWIN =0.0	MAIN0206
	WCFS=0.0	MAIN0207
	EPAET=0.0	MAIN0208
	DO 430 KR=1,NR	MAIN0209
	AREA=AREA+SAREA(KR)	MAIN0210
	FPERA(KR)=1.0-FIMPA(KR)-FWTRA(KR)	MAIN0211
	IF(FPERA(KR).GT.0.01) GO TO 390	MAIN0212
	TPLR(KR)=100.0	MAIN0213
	FPERA(KR)=0.01	MAIN0214
	GO TO 400	MAIN0215
390	TPLR(KR)=(1.0-FWTRA(KR))/FPERA(KR)	MAIN0216
400	VWINA(KR)=26.8888*SAREA(KR)	MAIN0217
	VWIN=VWIN+VWINA(KR)	MAIN0218
	WCFSA(KR)=VWINA(KR)*24.0	MAIN0219
	WCFS=WCFS+WCFSA(KR)	MAIN0220
	CWCFS(KR)=WCFS(KR)	MAIN0221
	CVWINA(KR)=VWINA(KR)	MAIN0222
	IF(NUBR(KR).EQ.0) GO TO 420	MAIN0223
	N=NUBR(KR)	MAIN0224
	DO 410 I=1,N	MAIN0225
	CWCFS(KR)=CWCFS(KR)+CWCFS(IUH(KR,I))	MAIN0226
	CVWINA(KR)=CVWINA(KR)+CVWINA(IUH(KR,I))	MAIN0227
410	CONTINUE	MAIN0228
420	ROLF1(KR)=0.0	MAIN0229
	ROLF2(KR)=0.0	MAIN0230
	RHF(KR)=0.0	MAIN0231
	ISGA(KR)=1	MAIN0232
	SSRT = SQRT(OFSS(KR))	MAIN0233
	OFRF(KR) = 1020.0*SSRT/(OFMN(KR)*OFSL(KR))	MAIN0234
	OFRFIS(KR) = 1020.0*SSRT/(OFMNIS(KR)*OFSL(KR))	MAIN0235
	EQDF(KR) = 0.00982*((OFMN(KR)*OFSL(KR)/SSRT)**0.6)	MAIN0236
	EQDFIS(KR) = 0.00982*((OFMNIS(KR)*OFSL(KR)/SSRT)**0.6)	MAIN0237
430	CONTINUE	MAIN0238

GAREA=CVWINA(NGR)/26.8888	MAIN0239
C STORE PARAMETERS OF REACH 1 FOR USE IN TRIP 2	MAIN0240
NGR1=NGR	MAIN0241
IF (NGR .EQ. 1 .OR. NFTR .EQ. 2 .OR. CONOPT(6) .EQ. 0) GO TO 460	MAIN0242
NR1=NR	MAIN0243
VWINA1=VWINA(1)	MAIN0244
VINMR1=VINMRA(1)	MAIN0245
SUBWF1=SUBWFA(1)	MAIN0246
FPERA1=FPERA(1)	MAIN0247
SAREA1=SAREA(1)	MAIN0248
FIMPA1=FIMPA(1)	MAIN0249
FWTRA1=FWTRA(1)	MAIN0250
TPLR1=TPLR(1)	MAIN0251
WCFSA1=WCFSA(1)	MAIN0252
CWCFSA1=CWCFSA(1)	MAIN0253
CVWIN1=CVWINA(1)	MAIN0254
OFRF1=OFRF(1)	MAIN0255
OFRF11=OFRFIS(1)	MAIN0256
EQDF1=EQDF(1)	MAIN0257
EQDF11=EQDFIS(1)	MAIN0258
RLZC1=RLZC(1)	MAIN0259
RBMR1=RBMR(1)	MAIN0260
RSUZC1=RSUZC(1)	MAIN0261
RETLF1=RETLF(1)	MAIN0262
RBUZC1=RBUZC(1)	MAIN0263
RSIAC1=RSIAC(1)	MAIN0264
RBIVF1=RBIVF(1)	MAIN0265
C COMPUTE AVERAGE PARAMETERS OF ALL REACHES ABOVE THE GAGE FOR TRIP 1	MAIN0266
440 FIMPA(1)=0.0	MAIN0267
FWTRA(1)=0.0	MAIN0268
TPLR(1)=0.0	MAIN0269
OFRF(1)=0.0	MAIN0270
OFRFIS(1)=0.0	MAIN0271
EQDF(1)=0.0	MAIN0272
EQDFIS(1)=0.0	MAIN0273

RLZC(1)=0.0	MAIN0274
RBMIR(1)=0.0	MAIN0275
RSUZC(1)=0.0	MAIN0276
RETLF(1)=0.0	MAIN0277
RBUZC(1)=0.0	MAIN0278
RSIAC(1)=0.0	MAIN0279
RBIVF(1)=0.0	MAIN0280
VINMRA(1)=0.0	MAIN0281
SUBWFA(1)=0.0	MAIN0282
FPERA(1)=0.0	MAIN0283
DD 450 KR=2,NGR	MAIN0284
FIMPA(1)=FIMPA(1)+FIMPA(KR)*SAREA(KR)	MAIN0285
FWTRA(1)=FWTRA(1)+FWTRA(KR)*SAREA(KR)	MAIN0286
TPLR(1)=TPLR(1)+TPLR(KR)*SAREA(KR)	MAIN0287
OFRF(1)=OFRF(1)+OFRF(KR)*SAREA(KR)	MAIN0288
OFRFIS(1)=OFRFIS(1)+OFRFIS(KR)*SAREA(KR)	MAIN0289
EQDF(1)=EQDF(1)+EQDF(KR)*SAREA(KR)	MAIN0290
EQDFIS(1)=EQDFIS(1)+EQDFIS(KR)*SAREA(KR)	MAIN0291
RLZC(1)=RLZC(1)+RLZC(KR)*SAREA(KR)	MAIN0292
RBMIR(1)=RBMIR(1)+RBMIR(KR)*SAREA(KR)	MAIN0293
RSUZC(1)=RSUZC(1)+RSUZC(KR)*SAREA(KR)	MAIN0294
RETLF(1)=RETLF(1)+RETLF(KR)*SAREA(KR)	MAIN0295
RBUZC(1)=RBUZC(1)+RBUZC(KR)*SAREA(KR)	MAIN0296
RSIAC(1)=RSIAC(1)+RSIAC(KR)*SAREA(KR)	MAIN0297
VINMRA(1)=VINMRA(1)+VINMRA(KR)*SAREA(KR)	MAIN0298
SUBWFA(1)=SUBWFA(1)+SUBWFA(KR)*SAREA(KR)	MAIN0299
FPERA(1)=FPERA(1)+FPERA(KR)*SAREA(KR)	MAIN0300
450 RBIVF(1)=RBIVF(1)+RBIVF(KR)*SAREA(KR)	MAIN0301
FIMPA(1)=(FIMPA(1)+FIMPA1*SAREA(1))/GAREA	MAIN0302
FWTRA(1)=(FWTRA(1)+FWTRA1*SAREA(1))/GAREA	MAIN0303
TPLR(1)=(TPLR(1)+TPLR1*SAREA(1))/GAREA	MAIN0304
OFRF(1)=(OFRF(1)+OFRF1*SAREA(1))/GAREA	MAIN0305
OFRFIS(1)=(OFRFIS(1)+OFRFIS1*SAREA(1))/GAREA	MAIN0306
EQDF(1)=(EQDF(1)+EQDF1*SAREA(1))/GAREA	MAIN0307
EQDFIS(1)=(EQDFIS(1)+EQDFIS1*SAREA(1))/GAREA	MAIN0308

RLZC(1)=(RLZC(1)+RLZC1*SAREA(1))/GAREA	MAIN0309
RBMIR(1)=(RBMIR(1)+RBMIR1*SAREA(1))/GAREA	MAIN0310
RSUZC(1)=(RSUZC(1)+RSUZC1*SAREA(1))/GAREA	MAIN0311
RETLF(1)=(RETLF(1)+RETLF1*SAREA(1))/GAREA	MAIN0312
RBUZC(1)=(RBUZC(1)+RBUZC1*SAREA(1))/GAREA	MAIN0313
RSIAC(1)=(RSIAC(1)+RSIAC1*SAREA(1))/GAREA	MAIN0314
RBIVF(1)=(RBIVF(1)+RBIVF1*SAREA(1))/GAREA	MAIN0315
VINMRA(1)=(VINMRA(1)+VINMRA1*SAREA(1))/GAREA	MAIN0316
SUBWFA(1)=(SUBWFA(1)+SUBWF1*SAREA(1))/GAREA	MAIN0317
FPERA(1)=(FPERA(1)+FPERA1*SAREA(1))/GAREA	MAIN0318
C READ WATER YEAR	MAIN0319
460 READ(5,10) YR1,YR2	MAIN0320
DPY = 365	MAIN0321
IF(MOD(YR2,4) .EQ. 0) DPY = 366	MAIN0322
C READ EVAPORATION DATA	MAIN0323
IF(CONOPT(1) .NE. 1) GO TO 500	MAIN0324
READ(5,470) (DPET(KRD),KRD=274,365,10), (DPET(KRD),KRD=1,273,10)	MAIN0325
470 FORMAT(1CF5.2)	MAIN0326
DO 490 IDAY2 = 1,9	MAIN0327
DO 480 IDAY1 = 274,360,10	MAIN0328
DAY = IDAY1 + IDAY2	MAIN0329
480 DPET(DAY) = DPET(IDAY1)	MAIN0330
DO 490 IDAY1 = 1,273,10	MAIN0331
DAY = IDAY1 + IDAY2	MAIN0332
IF(DAY .GT. 273) GO TO 490	MAIN0333
DPET(DAY) = DPET(IDAY1)	MAIN0334
490 CONTINUE	MAIN0335
DPET(366) = DPET(59)	MAIN0336
DPET(365) = DPET(363)	MAIN0337
DPET(364) = DPET(363)	MAIN0338
GO TO 510	MAIN0339
500 IF(CONOPT(1) .EQ. 2) GO TO 530	MAIN0340
DAY = 274	MAIN0341
READ(5,470) (DPET(KRD),KRD=274,365), (DPET(KRD),KRD=1,273)	MAIN0342
IF(DPY.EQ.366) READ(5,470) DPET(366)	MAIN0343

510	READ(5,470) EPCM	MAIN0344
	DO 520 DAY = 1,DPY	MAIN0345
520	EPAET = EPAET + DPET(DAY)	MAIN0346
	IF(EPCM(6) .NE. 1.0) EPAET = 0.7*EPAET	MAIN0347
	GO TO 540	MAIN0348
530	READ(5,90) EPAET,MNRD	MAIN0349
	EMAET = EPAET*(365.0 + MNRD)/404.0	MAIN0350
	CALL EVPDAY(DPET,EMAET)	MAIN0351
C	READ DAILY FLOW DATA	MAIN0352
540	DRSF(366) = 0.0	MAIN0353
	DAY=274	MAIN0354
	READ(5,550) (DRSF(DAY),DAY=274,365),(DRSF(DAY),DAY=1,273)	MAIN0355
550	FORMAT(10F8.2)	MAIN0356
	IF(DPY.EQ.366) READ(5,550) DRSF(366)	MAIN0357
	IF(DIV .EQ. 0.0) GO TO 580	MAIN0358
	DO 570 DAY = 1,DPY	MAIN0359
	IF(DRSF(DAY) .GT. DIV) GO TO 560	MAIN0360
	DRSF(DAY) = 0.0	MAIN0361
	GO TO 570	MAIN0362
560	DRSF(DAY) = DRSF(DAY) - DIV	MAIN0363
570	CONTINUE	MAIN0364
C	SUM DAILY FLOW AND PRINT FLOW TABLE	MAIN0365
580	CALL DAYSUM(DRSF,MEDCY,DPY,RATFV,TMRTF)	MAIN0366
	WRITE(6,590)	MAIN0367
590	FORMAT(1H0,42X,'RECORDED FLOWS')	MAIN0368
	CALL DAYOUT(DRSF,MEDWY,DPY)	MAIN0369
	WRITE(6,600) (TMRTF(KWD), KWD = 1,12),RATFV	MAIN0370
600	FORMAT(6X,'TOTAL',2X,12F8.1,2X,F10.1,2X,3H\$FD)	MAIN0371
C	INITIALIZE PRECIPITATION DATA ARRAYS	MAIN0372
	DO 620 DAY=1,366	MAIN0373
	DO 610 KR=1,NR	MAIN0374
610	DRGPM(KR,DAY)=RGPMB(KR)	MAIN0375
	DO 620 HOUR=1,24	MAIN0376
620	DRHP(DAY,HOUR)=0.0	MAIN0377
C	READ NUMBER OF STORAGE GAGES	MAIN0378

READ(5,10) NSG	MAIN0379
C SKIP READING STORAGE GAGE DATA IF NO STORAGE GAGE IS USED	MAIN0380
IF(NSG.EQ.0) GO TO 710	MAIN0381
DO 660 KG=1,NSG	MAIN0382
DO 630 DAY=1,366	MAIN0383
630 DRSGP(KG,DAY)=0.0	MAIN0384
C READ DATA FOR STORAGE GAGES	MAIN0385
READ(5,10) NSGRD,ISGM(KG),SGRT(KG)	MAIN0386
IF(ISGM(KG).EQ.0) GO TO 640	MAIN0387
READ(5,10) SGRT2(KG),SGMD(KG)	MAIN0388
640 READ(5,650) (ISGRD,DRSGP(KG,ISGRD),KRD=1,NSGRD)	MAIN0389
650 FORMAT(5(15,F7.2))	MAIN0390
660 CONTINUE	MAIN0391
WRITE(6,670)	MAIN0392
670 FORMAT(/,5X,9HREACH NO.,5X,16HSTORAGE GAGE NO.,5X,16HWEIGHTING FAC	MAIN0393
*TOR)	MAIN0394
C READ STORAGE GAGE NUMBER FOR EACH REACH AND ITS WEIGHTING FACTOR	MAIN0395
DO 700 KR=1,NR	MAIN0396
READ(5,680) ISGA(KR),WSG(KR)	MAIN0397
680 FORMAT(15,F10.2)	MAIN0398
WRITE(6,690)KR,ISGA(KR),WSG(KR)	MAIN0399
690 FORMAT(12X,I2,19X,I2,13X,F8.2)	MAIN0400
IF(ISGM(ISGA(KR)).EQ.0) GO TO 700	MAIN0401
READ(5,550) WSG2(KR)	MAIN0402
700 CONTINUE	MAIN0403
C READ RECORDING RAIN GAGE HOURLY TOTALS	MAIN0404
710 READ(5,720) IWBG,YEAR,MONTH,DATE,CN	MAIN0405
720 FORMAT(15,I4,14,I4,I3)	MAIN0406
C LAST CARD OF HOURLY PRECIPITATION HAS YEAR 98	MAIN0407
IF(YEAR .GE. 98) GO TO 750	MAIN0408
HRF = 12*(CN-1) + 1	MAIN0409
HRL = 12*(CN-1) + 12	MAIN0410
DAY = MEDCY(MONTH) + DATE	MAIN0411
READ(5,730) (DRHP(DAY,HOUR),HOUR=HRF,HRL)	MAIN0412
730 FORMAT(1X,12F6.2)	MAIN0413

IF(DPY .NE. 366 .OR. MONTH .NE. 2 .OR. DATE .NE. 29) GO TO 710	MAIN0414
DO 740 HOUR = HRF,HRL	MAIN0415
DRHP(366,HOUR) = DRHP(60,HOUR)	MAIN0416
740 DRHP(60,HOUR) = 0.0	MAIN0417
GO TO 710	MAIN0418
C CALCULATE PRECIPITATION WEIGHTING FACTORS	MAIN0419
750 IF(NSG.EQ.0) GO TO 830	MAIN0420
DO 820 KR=1,NR	MAIN0421
KG=ISGA(KR)	MAIN0422
DAY = 274	MAIN0423
PDAY = 274	MAIN0424
RDPT = 0.0	MAIN0425
760 IF(ISGM(KG).NE.1) GO TO 770	MAIN0426
IF(SGMD(KG).NE.DAY) GO TO 770	MAIN0427
WSG(KR)=WSG2(KR)	MAIN0428
SGRT(KG)=SGRT2(KG)	MAIN0429
770 EHS GD=SGRT(KG)	MAIN0430
IF(SGRT(KG).EQ.0)EHS GD=24	MAIN0431
EHS GDF = EHS GD	MAIN0432
DO 810 HOUR = 1,24	MAIN0433
RDPT = RDPT + DRHP(DAY,HOUR)	MAIN0434
IF (HOUR .NE. EHS GD) GO TO 810	MAIN0435
IF (RDPT .LE. 0.0) GO TO 780	MAIN0436
IF(SGRT(KG).EQ.0) PDAY=DAY	MAIN0437
DRGPM(KR,PDAY)=(DRSGP(KG,DAY)*WSG(KR)+RDPT*(1.0-WSG(KR)))/	MAIN0438
1 RDPT	MAIN0439
C REDUCE DATED POTENTIAL EVAPORATION TO ONE HALF FOR RAINY DAYS	MAIN0440
IF(CONOPT(1) .NE. 0 .AND. KR .EQ. 1) DPET(PDAY)=0.5*DPET(PDAY)	MAIN0441
IF(SGRT(KG).NE.0)PDAY=DAY	MAIN0442
RDPT = 0.0	MAIN0443
GO TO 810	MAIN0444
780 IF(DRSGP(KG,DAY).LE.0.0) GO TO 800	MAIN0445
IF(KR.NE.NR) GO TO 800	MAIN0446
DO 790 KHOUR = 1,EHS GD	MAIN0447
790 DRHP(DAY,KHOUR)=(WSG(KR)*DRSGP(KG,DAY))/EHS GDF	MAIN0448

800 IF(SGRT(KG).NE.0)PDAY=DAY	MAIN0449
810 CONTINUE	MAIN0450
CALL DAYNXT(DAY,DPY)	MAIN0451
IF(DAY.NE.274) GO TO 760	MAIN0452
820 CONTINUE	MAIN0453
C STORE DRGPM OF REACH 1 FOR USE IN TRIP 2	MAIN0454
830 IF (NGR.EQ.1 .OR. NFTR.EQ.2 .OR. NLTR.EQ.1 .OR. CONOPT(6).EQ.0)	MAIN0455
1 GO TO 850	MAIN0456
DO 840 DAY=1,366	MAIN0457
840 DRGPM1(DAY)=DRGPM(1,DAY)	MAIN0458
850 IF(NFTR.EQ.2 .OR. CONOPT(4).EQ.1) GO TO 860	MAIN0459
C SET INITIAL VALUES OF FLOW VOLUME PARAMETERS TO BE OPTIMIZED	MAIN0460
LZC = 12.0	MAIN0461
BMIR = 1.2	MAIN0462
SUZC = 1.3	MAIN0463
ETLF = 0.25	MAIN0464
BUZC = 1.50	MAIN0465
SIAC = 0.90	MAIN0466
BIVF = 0.90	MAIN0467
860 IF(NFTR .EQ. 2) GO TO 900	MAIN0468
IF(CONOPT(4).EQ.0) GO TO 880	MAIN0469
C READ DATA FROM PREVIOUS RUN AND CONTINUE TRIP 1	MAIN0470
READ(5,910) LZC,BMIR,SUZC,ETLF,BUZC,SIAC,BIVF,LZS	MAIN0471
READ(5,870) KRC,KBRC,SSSQM,FTX,LRC,LLZC,LBUZC,LBMIR,LETLF	MAIN0472
870 FORMAT(2I5,2F10.3,5L5)	MAIN0473
BLZC=LZC	MAIN0474
BBMIR=BMIR	MAIN0475
BSUZC=SUZC	MAIN0476
BETLF=ETLF	MAIN0477
BBUZC=BUZC	MAIN0478
BSIAC=SIAC	MAIN0479
BBYLZS=LZS	MAIN0480
KBRC=KBRC-1	MAIN0481
C ESTIMATE BASE AND INTERFLOW RECESSIION CONSTANTS	MAIN0482
880 CALL RECESS(DRSF,DPY,BFRC,IFRC,GAREA,RSBD,RSBIF,NRS,RSBBF)	MAIN0483

IF(IFRC .GE. 0.3) GO TO 900	MAIN0484
WRITE(6,890) IFRC	MAIN0485
890 FORMAT(/10X,'REJECTED IFRC =',F8.4)	MAIN0486
IFRC = 0.1	MAIN0487
BIVF = 0.0	MAIN0488
C READ FLOW VOLUME PARAMETERS AND RECESSON CONSTANTS IF ONLY TRIP 2 IS	MAIN0489
C RUN	MAIN0490
900 IF(NFTR.GE.2) READ(5,910) LZC,BMIR,SUZC,FTLF,BUZC,SIAC,BIVF,	MAIN0491
ILZS,IFRC,BFRC	MAIN0492
910 FORMAT(9F8.4)	MAIN0493
C COMPUTE AVERAGE DRGPM FOR ALL REACHES ABOVE THE GAGE	MAIN0494
DO 920 DAY=1,366	MAIN0495
ADRGPM(DAY)=0.0	MAIN0496
DO 920 KR=1,NGR	MAIN0497
920 ADRGPM(DAY)=ADRGPM(DAY)+DRGPM(KR,DAY)*SAREA(KR)/GAREA	MAIN0498
C ADJUST RAINFALL ANOMALIES	MAIN0499
930 IF(NWSTH.LT.12)CALL PRECHK(ADRGPM,DRHP,DRSF,CVWINA(NGR),SGRT(1),	MAIN0500
1 NWSTH)	MAIN0501
BFHRC = BFRC**(1.0/24.0)	MAIN0502
BFRL = -ALOG(BFHRC)	MAIN0503
TRIP = NFTR	MAIN0504
KHYD = 1	MAIN0505
C INITIALIZE DAILY HYDROGRAPH FOR OUTFLOW FROM EACH FINITE SECTION	MAIN0506
IF(TRIP .EQ. 1) GO TO 970	MAIN0507
DO 960 KR=1,NR	MAIN0508
NFSKR=NFS(KR)	MAIN0509
DO 960 KFS=1,NFSKR	MAIN0510
KRKFS=NFSEPR(KR)+KFS	MAIN0511
DO 940 DPRD=1,96	MAIN0512
940 HYDSTO(KRKFS,DPRD)=0.0	MAIN0513
C INITIALIZE END AREAS OF CHANNEL CROSS SECTIONS	MAIN0514
N=NOFD(KRKFS)+1	MAIN0515
DO 950 NX=1,N	MAIN0516
950 EDAREA(KRKFS,NX)=0.0	MAIN0517
960 CONTINUE	MAIN0518

C POINT OF RETURN FOR NEW TRIP	MAIN0519
970 IF(KRC .LE. 5) FTX = 1.0	MAIN0520
C RESTORE DATA OF REACH 1 FOR USE IN TRIP 2	MAIN0521
IF (NGR1.EQ.1 .OR. NFTR.EQ.2 .OR. TRIP.EQ.1 .OR. CONOPT(6).EQ.0)	MAIN0522
1 GO TO 990	MAIN0523
NR=NR1	MAIN0524
NGR=NGR1	MAIN0525
VWINA(1)=VWINA1	MAIN0526
WCFS1(1)=WCFS1	MAIN0527
VWINA(1)=VWINA1	MAIN0528
CVWINA(1)=CVWIN1	MAIN0529
VINMRA(1)=VINMRA1	MAIN0530
SUBWFA(1)=SUBWFA1	MAIN0531
FPERA(1)=FPERA1	MAIN0532
SAREA(1)=SAREA1	MAIN0533
FIMPA(1)=FIMPA1	MAIN0534
FWTRA(1)=FWTRA1	MAIN0535
TPLR(1)=TPLR1	MAIN0536
WCFS1(1)=WCFS1	MAIN0537
CWCFS1(1)=CWCFS1	MAIN0538
CVWINA(1)=CVWIN1	MAIN0539
OFRF(1)=OFRF1	MAIN0540
OFRF1S(1)=OFRF1S	MAIN0541
EQDF(1)=EQDF1	MAIN0542
EQDF1S(1)=EQDF1S	MAIN0543
RLZC(1)=RLZC1	MAIN0544
RSUZC(1)=RSUZC1	MAIN0545
RETLF(1)=RETLF1	MAIN0546
RBUZC(1)=RBUZC1	MAIN0547
RSIAC(1)=RSIAC1	MAIN0548
RBIVF(1)=RBIVF1	MAIN0549
DO 980 DAY=1,366	MAIN0550
980 DRGPM(1,DAY)=DRGPM1(DAY)	MAIN0551
990 ICYC= ICYC+1	MAIN0552
IF(TRIP.EQ.2 .OR. CONOPT(6).EQ.0) GO TO 1000	MAIN0553

SAREA(1)=GAREA	MAIN0554
NR=1	MAIN0555
VWINA(1)=CVWINA(NGR)	MAIN0556
WCFSA(1)=CWCFSA(NGR)	MAIN0557
CWCFSA(1)=CWCFSA(NGR)	MAIN0558
CVWINA(1)=CVWINA(NGR)	MAIN0559
NGR=1	MAIN0560
1000 DO 1010 DAY=1,366	MAIN0561
1010 GAFL(DAY)=0.0	MAIN0562
KBRC=KBRC+1	MAIN0563
C STORE FLOW VOLUME PARAMETERS REPRESENTING THE ENTIRE WATERSHED	MAIN0564
LZC1=LZC	MAIN0565
BMIR1=BMIR	MAIN0566
SUZC1=SUZC	MAIN0567
ETLF1=ETLF	MAIN0568
BUZC1=BUZC	MAIN0569
SIAC1=SIAC	MAIN0570
BIVF1=BIVF	MAIN0571
IF(DPY .EQ. 366) MEDWY(5) = 366	MAIN0572
C SET TIME INCREMENT FOR SIMULATION	MAIN0573
PPH = 1.0	MAIN0574
IF(.NOT. LRC) PPH = 3.0	MAIN0575
IF(TRIP .NE. 1) PPH = 4.0	MAIN0576
IPPH = PPH	MAIN0577
FHPP = 1.0/PPH	MAIN0578
IFPRC = IFRC**(FHPP/24.0)	MAIN0579
IFRL = -ALOG(IFPRC)	MAIN0580
C INITIALIZE PARAMETERS	MAIN0581
HOURL=0	MAIN0582
HSE = 0.0	MAIN0583
PEAI = 0.0	MAIN0584
SPIF = 0.0	MAIN0585
DO 1020 KR=1,NR	MAIN0586
IFSA(KR)=0.0	MAIN0587
OFUS(KR)=0.0	MAIN0588

DFUSIS(KR)=0.0	MAIN0589
AMIF(KR)=0.0	MAIN0590
AMNET(KR)=0.0	MAIN0591
AMBF(KR) = 0.0	MAIN0592
AMSE(KR) = 0.0	MAIN0593
1020 CONTINUE	MAIN0594
AMPET = 0.0	MAIN0595
AMPREC = 0.0	MAIN0596
DO 1030 MONTH=1,12	MAIN0597
TMPREC(MONTH) = 0.0	MAIN0598
TMBF(MONTH) = 0.0	MAIN0599
TMIF(MONTH) = 0.0	MAIN0600
TMSE(MONTH) = 0.0	MAIN0601
TMPET(MONTH) = 0.0	MAIN0602
TMNET(MONTH) = 0.0	MAIN0603
1030 CONTINUE	MAIN0604
KRS = 1	MAIN0605
KDRS = 400	MAIN0606
UZS = 0.0	MAIN0607
IFS = 0.0	MAIN0608
IF(NFTR .EQ. 2) GO TO 1060	MAIN0609
IF(KRC .NE. 1) GO TO 1040	MAIN0610
C ADJUST BEGINNING YEAR LOWER ZONE STORAGE	MAIN0611
BYLZS = 6.00	MAIN0612
IF(CONOPT(4) .EQ. 0) LZS=BYLZS	MAIN0613
GO TO 1060	MAIN0614
1040 IF(ICYC .EQ. 1) GO TO 1060	MAIN0615
IF(EMLZS(11) .LT. LZS) LZS= EMLZS(11)	MAIN0616
LZS = LZS*LZC/PLZC	MAIN0617
IF(LLZC) LZS = LZC - (LZC-LZS)*(SATFV/RATEFV)	MAIN0618
IF(ABS(FTX - 1.0) .LT. 0.02) GO TO 1050	MAIN0619
LZS = FTX*BBYLZS*LZC/BLZC	MAIN0620
IF(LRC .AND. LZC-LZS .LT. 2.0) LZC = LZS + 2.0	MAIN0621
1050 IF(TRIP .EQ. 2 .OR. KFFC .EQ. 1) LZS = BBYLZS	MAIN0622
KFFC = 0	MAIN0623

C DETERMINE BEGINNING YEAR GROUNDWATER STORAGE	MAIN0624
1060 OCT1BF = 0.05*TMRTF(1)	MAIN0625
IF(DRSF(274) .LT. 0.05*TMRTF(1)) OCT1BF = DRSF(274)	MAIN0626
IF(DRSF(276) .LT. OCT1BF*BFRC**2) OCT1BF = DRSF(276)/BFRC**2	MAIN0627
BYGWS = OCT1BF/(WCFS*BFRL*SQRT(BFRC))	MAIN0628
GWS = BYGWS	MAIN0629
BYLZS = LZS	MAIN0630
WRITE(6,1070) TRIP,LZC,BMIR,SUZC,ETLF,BUZC,SIAC,BIVF,BFRC,IFRC	MAIN0631
1070 FORMAT(1H1,3X,'TRIAL RUN NUMBER',I3/5X,'PARAMETER VALUES'/10X,	MAIN0632
1 5HLZC =,3X,F8.4,2X,6HBMIR =,2X,F8.4,2X,6HSUZC =,2X,F8.4,2X,	MAIN0633
2 6HETLF =,2X,F8.4,2X,6HBUZC =,2X,F8.4,2X,6HSIAC =,2X,F8.4/10X,	MAIN0634
3 6HBIVF =,2X,F8.4,2X,6HBFRC =,2X,F8.4,2X,6HIFRC =,2X,F8.4)	MAIN0635
WRITE(6,1080) LZS,GWS	MAIN0636
1080 FORMAT(/5X,'INITIAL MOISTURE STORAGEES, LZS =',F9.4,5X,'GWS =',	MAIN0637
1 F9.4)	MAIN0638
IF(TRIP .EQ. 1) WRITE(6,1090) KRC,KBRC,SSSQM,FTX,LRC,LLZC,LBUZC,	MAIN0639
1 LBMIR,LETLF	MAIN0640
1090 FORMAT(5X,'DATA USED FOR CONTINUATION RUN: KRC =',I5,5X,'KBRC =',	MAIN0641
2 I5,5X,'SSSQM =',F10.3,5X,'FTX =',F10.3,5X,'LRC =',L3,/,36X,	MAIN0642
3 'LLZC =',L3,6X,'LBUZC =',L3,7X,'LBMIR =',L3,10X,'LETLF =',L3)	MAIN0643
AETX = 24.0*EPAE1/365.0	MAIN0644
AEX96 = 1.2*AETX	MAIN0645
AEX90 = 0.3*AETX	MAIN0646
PAEX90=AEX90	MAIN0647
PAEX96=AEX96	MAIN0648
SIAM = 1.2**SIAC	MAIN0649
DO 1100 KR=1,NR	MAIN0650
UZSA(KR)=UZS	MAIN0651
LZSA(KR) = LZS	MAIN0652
GWSA(KR)=GWS	MAIN0653
VINCR(KR)=FHPP*VINMRA(KR)	MAIN0654
OFUS(KR) = 0.0	MAIN0655
OFUSIS(KR) = 0.0	MAIN0656
1100 CONTINUE	MAIN0657
MONTH = 1	MAIN0658

MDAY = 273	MAIN0659
IF (TRIP .EQ. 1) GO TO 1130	MAIN0660
WRITE(6,1110) YR1,YR2	MAIN0661
1110 FORMAT(03X,61HOPTIMIZATION OF MODEL INPUT PARAMETERS BASED ON WATER	MAIN0662
1R YEAR 19,12,14-,12)	MAIN0663
WRITE(6,1120)	MAIN0664
1120 FORMAT(8H OCTOBER)	MAIN0665
C BEGIN DAY LOOP	MAIN0666
1130 DAY = 274	MAIN0667
LDAY=DAY	MAIN0668
1140 NNJ=1	MAIN0669
C CHECK THE SPECIFIED DAY NUMBER AND DETERMINE IF TRIP 2 SHOULD STOP	MAIN0670
IF (TRIP .EQ. 2 .AND. DAY .EQ. NDCR) GO TO 2460	MAIN0671
KR=0	MAIN0672
C BEGIN REACH LOOP	MAIN0673
1150 KR=KR+1	MAIN0674
IF (TRIP .EQ. 1) GO TO 1200	MAIN0675
C CHECK WHETHER FLOW AT REACH IS TO BE PRINTED	MAIN0676
SAVE=.FALSE.	MAIN0677
DO 1160 I=1,NOUT	MAIN0678
J=I	MAIN0679
IF (KR.EQ.IROUT(I)) GO TO 1170	MAIN0680
1160 CONTINUE	MAIN0681
GO TO 1180	MAIN0682
1170 SAVE=.TRUE.	MAIN0683
KOUT=J	MAIN0684
C CHECK WHETHER A STRUCTURE EXISTS AT THE BOTTOM OF REACH	MAIN0685
1180 LSTR=.FALSE.	MAIN0686
IF (INSTR.EQ.0) GO TO 1200	MAIN0687
DO 1190 I=1,NSTR	MAIN0688
IF (KR.NE.IRSTR(I)) GO TO 1190	MAIN0689
LSTR=.TRUE.	MAIN0690
GO TO 1200	MAIN0691
1190 CONTINUE	MAIN0692
1200 CONTINUE	MAIN0693

C	COMPUTE FLOW VOLUME PARAMETERS FOR EACH PEACH BASED ON GIVEN RATIOS	MAIN0694
	SUBWF=SUBWFA(KR)	MAIN0695
	LZC=RLZC(KR)*LZC1	MAIN0696
	BMIR=RBMIR(KR)*BMIR1	MAIN0697
	SUZC=RSUZC(KR)*SUZC1	MAIN0698
	ETLF=RETLF(KR)*ETLF1	MAIN0699
	BUZC=RBUZC(KR)*BUZC1	MAIN0700
	SIAC=RSIAC(KR)*SIAC1	MAIN0701
	BIVF=RBIVF(KR)*BIVF1	MAIN0702
	AEX90=PAEX90	MAIN0703
	AEX96=PAEX96	MAIN0704
	IF(TRIP .EQ. 2) GO TO 1210	MAIN0705
C	INITIALIZE FIRST THREE DAYS OF RECESSION SEQUENCE FOR BASE AND	MAIN0706
C	INTERFLOW	MAIN0707
	IF(KR.NE.1) GO TO 1210	MAIN0708
	KDRS=KDRS+1	MAIN0709
	IF(KDRS.LE.3) SBFRS(KDRS,KRS-1)=0.0	MAIN0710
	IF(KDRS.LE.3) SIFRS(KDRS,KRS-1)= 0.0	MAIN0711
	IF(RSBD(KRS) .NE. DAY) GO TO 1210	MAIN0712
	KDRS = 1	MAIN0713
	KRS = KRS + 1	MAIN0714
	SBFRS(KDRS,KRS-1)=0.0	MAIN0715
	SIFRS(KDRS,KRS-1)=0.0	MAIN0716
1210	ADIF(KR)= 0.0	MAIN0717
	ADBF(KR) = 0.0	MAIN0718
	TDSF = 0.0	MAIN0719
	PET = DPET(DAY)	MAIN0720
	IF(CONOPT(1) .NE. 2) PET = PET*EPCM(MONTH)	MAIN0721
	PETU = PET	MAIN0722
	TFMAX = 0.0	MAIN0723
	TDFP24=0.0	MAIN0724
	IF(TRIP .EQ. 1) GO TO 1230	MAIN0725
C	ASSIGN CHANNEL FLOW END AREAS AT BEGINNING OF EACH DAY	MAIN0726
	NFSKR=NFS(KR)	MAIN0727
	DO 1220 KFS=1,NFSKR	MAIN0728

KRKFS=NFSEPR(KR)+KFS	MAIN0729
N=NOFD(KRKFS)+1	MAIN0730
DO 1220 NX=1,N	MAIN0731
XAREA(KFS,NX)=EDAREA(KRKFS,NX)	MAIN0732
1220 CONTINUE	MAIN0733
C BEGIN HOUR LOOP	MAIN0734
1230 DO 1610 HOUR = 1,24	MAIN0735
CBF=GWSA(KR)*BFRL	MAIN0736
URHF=0.0	MAIN0737
IF(NSG.EQ.0 .AND. DRHP(DAY,HOUR).NE.0.0 .AND. PET.EQ.PETU .AND.	MAIN0738
1 CONOPT(1).NE.0) PET=0.5*PET	MAIN0739
C COMPUTE HOURLY PRECIPITATION FOR EACH REACH	MAIN0740
IF(DAY .EQ. 274) RGPM(KR)= DRGPM(KR,DAY)	MAIN0741
IF(HOUR.EQ.SGRT(ISGA(KR))+1) RGPM(KR)= DRGPM(KR,DAY)	MAIN0742
IF(HOUR.EQ.9) HSE = (FWTRA(KR)*PET)/12.0	MAIN0743
IF(HOUR .EQ. 21) HSE = 0.0	MAIN0744
PRH= RGPM(KR)*DRHP(DAY,HOUR)	MAIN0745
IF(KR.LE.NGR)AMPREC=AMPREC+PRH*SAREA(KR)	MAIN0746
ARHF(KR)=0.0	MAIN0747
C 15 MIN ACCOUNTING AND ROUTING LOOP (60 MINUTES USED FOR ROUGH	MAIN0748
C ADJUSTMENT, AND 20 MINUTES FOR FINE ADJUSTMENT IN TRIP 1)	MAIN0749
DO 1550 PRD=1,IPPH	MAIN0750
DPRD=IPPH*(HOUR-1)+PRD	MAIN0751
PEP=FHPP*PRH	MAIN0752
PEBI = 0.0	MAIN0753
PPI = 0.0	MAIN0754
OFR = 0.0	MAIN0755
OFRIS = 0.0	MAIN0756
WI = 0.0	MAIN0757
WEIFS = 0.0	MAIN0758
C USE NONUNIFORM RAIN DISTRIBUTION WHEN NEEDED	MAIN0759
IF(TRIP.EQ.2.AND.LNPR) CALL PREPRD(RGPM(KR),DRHP,DAY,HOUR,DPY,PRD,	MAIN0760
1 PEP,PRH)	MAIN0761
IF(PEP .GT. 0.0) GO TO 1240	MAIN0762
IF(OFUS(KR).GT.0.0) GO TO 1260	MAIN0763

IF(IFSA(KR).GT.0.0) GO TO 1350	MAIN0764
IF(TRIP .EQ. 1) GO TO 1540	MAIN0765
IF(RHF(KR).GT. 0.0) GO TO 1370	MAIN0766
IF(CONOPT(2) .EQ. 1 .AND. PRD .NE. 4) GO TO 1540	MAIN0767
TFCFS=0.0	MAIN0768
IF(CBF .GT. HSE) TFCFS=(CBF-HSE)*WCFS(KR)	MAIN0769
GO TO 1510	MAIN0770
C RAINFALL UPPER ZONE INTERACTION	MAIN0771
1240 IF(PEP.GE.VINCR(KR))GO TO 1250	MAIN0772
UZSA(KR) = UZSA(KR)+PEP*TPLR(KR)	MAIN0773
VINCR(KR)=VINCR(KR)-PEP	MAIN0774
PPI=0.0	MAIN0775
PEBI = 0.0	MAIN0776
IF(OFUS(KR).GT.0.0) GO TO 1260	MAIN0777
GO TO 1350	MAIN0778
1250 PPI = PEP-VINCR(KR)	MAIN0779
UZSA(KR) = UZSA(KR)+VINCR(KR)*TPLR(KR)	MAIN0780
VINCR(KR)=0.0	MAIN0781
LZSR=LZSA(KR)/LZC	MAIN0782
UZC = SUZC*AEX90 + BUZC*EXP(-2.7*LZSR)	MAIN0783
IF(UZC .LT. 0.25) UZC = 0.25	MAIN0784
UZRX=2.0*ABS(UZSA(KR)/UZC-1.0) + 1.0	MAIN0785
FMR=(1.0/(1.0+UZRX))*UZRX	MAIN0786
IF(UZSA(KR).GT.UZC) FMR=1.0-FMR	MAIN0787
PEBI=PPI*FMR	MAIN0788
UZSA(KR)=UZSA(KR)+PPI-PEBI	MAIN0789
C LOWER ZONE AND GROUNDWATER INFILTRATION	MAIN0790
1260 LZSR=LZSA(KR)/LZC	MAIN0791
EID = 4.0*LZSR	MAIN0792
IF(LZSR .LE. 1.0) GO TO 1270	MAIN0793
EID = 4.0 + 2.0*(LZSR - 1.0)	MAIN0794
IF(LZSR .LE. 2.0) GO TO 1270	MAIN0795
EID = 6.0	MAIN0796
1270 PEBI=PEBI+OFUS(KR)	MAIN0797
CMIR = FHPP*STAM*BMIR/(2.0**EID)	MAIN0798

	CIVM = BIVF*2.0**LZSR	MAIN0799
	IF(CIVM .LT. 1.0) CIVM = 1.0	MAIN0800
	PEAI = PEBI*PEBI/(2.0*CMIR*CIVM)	MAIN0801
	WI = PEBI*PEBI/(2.0*CMIR)	MAIN0802
	IF (PEBI .GE. CMIR) WI = PEBI-0.5*CMIR	MAIN0803
	IF (PEBI .GE. CMIR*CIVM) PEA1 = PEBI-0.5*CMIR*CIVM	MAIN0804
	WEIFS = WI - PEA1	MAIN0805
C	OVERLAND RUNOFF	MAIN0806
	IF((PEAI-OFUS(KR)).GT.0.0) GO TO 1280	MAIN0807
	EQD= (OFUS(KR)+PEAI)/2.0	MAIN0808
	GO TO 1290	MAIN0809
1280	EQD=EQDF(KR)*((PEAI-OFUS(KR))**0.6)	MAIN0810
1290	IF((OFUS(KR)+PEAI).GT.(2.0*EQD)) EQD=0.5*(OFUS(KR)+PEAI)	MAIN0811
	IF((OFUS(KR)+PEAI).LE.0.001) GO TO 1300	MAIN0812
	OFR = FHPP*OFRF(KR)*(((OFUS(KR)+PEAI)*0.5)**1.67)*((1.0+0.6*	MAIN0813
	1 ((OFUS(KR)+PEAI)/(2.0*EQD))**3.0)**1.67)	MAIN0814
	IF(OFR .GT. (0.75*PEAI)) OFR = 0.75*PEAI	MAIN0815
1300	IF(FIMPA(KR).EQ.0) GO TO 1330	MAIN0816
C	OVERLAND RUNOFF ON IMPERVIOUS SURFACE	MAIN0817
	PEIS=PPI+OFUSIS(KR)	MAIN0818
	IF((PEIS-OFUSIS(KR)).GT.0.0) GO TO 1310	MAIN0819
	EQDIS=(OFUSIS(KR)+PEIS)/2.0	MAIN0820
	GO TO 1320	MAIN0821
1310	EQDIS=EQDFIS(KR)*((PEIS-OFUSIS(KR))**0.6)	MAIN0822
1320	IF((OFUSIS(KR)+PEIS).GT.(2.0*EQDIS))EQDIS=0.5*(OFUSIS(KR)+PEIS)	MAIN0823
	IF((OFUSIS(KR)+PEIS).LE.0.01) GO TO 1330	MAIN0824
	OFRIS = FHPP*OFRFIS(KR)*(((OFUSIS(KR)+PEIS)*0.5)**1.67)*((1.0+0.6*	MAIN0825
	1 ((OFUSIS(KR)+PEIS)/(2.0*EQDIS))**3.0)**1.67)	MAIN0826
	IF(OFRIS .GT. PEIS) OFRIS = PEIS	MAIN0827
1330	OFUSIS(KR) = PEIS-OFRIS	MAIN0828
	OFUS(KR)=PEAI-OFR	MAIN0829
	IF(OFUS(KR).GE.0.001) GO TO 1340	MAIN0830
	LZSA(KR)=LZSA(KR)+OFUS(KR)	MAIN0831
	OFUS(KR)=0.0	MAIN0832
	OFRIS=OFRIS+OFUSIS(KR)	MAIN0833

DFUSIS(KR)=0.0	MAIN0834
C PERCOLATION TO GROUNDWATER	MAIN0835
1340 LZR _X =1.5*ABS(LZSA(KR)/LZC-1.0)+1.0	MAIN0836
FMR = (1.0/(1.0 + LZR _X))*LZR _X	MAIN0837
IF(LZSA(KR).LT.LZC)FMR=1.0-FMR*(LZSA(KR)/LZC)	MAIN0838
PLZS = FMR*(PEBI-WI)	MAIN0839
PGW = (1.0 -FMR)*(PEBI - WI)*(1.0 - SUBWF)*FPERA(KR)	MAIN0840
GWSA(KR)=GWSA(KR)+PGW	MAIN0841
LZSA(KR)=LZSA(KR)+PLZS	MAIN0842
IFSA(KR)=IFSA(KR)+WEIFS*FPERA(KR)	MAIN0843
1350 SPIF=IFRL*IFSA(KR)	MAIN0844
AMIF(KR)=AMIF(KR)+SPIF	MAIN0845
ADIF(KR)=ADIF(KR)+SPIF	MAIN0846
IFSA(KR)=IFSA(KR)-SPIF	MAIN0847
IF(IFSA(KR).GE.0.0001)GO TO 1360	MAIN0848
LZSA(KR)=LZSA(KR)+IFSA(KR)	MAIN0849
IFSA(KR)=0.0	MAIN0850
1360 SPDR=FPERA(KR)*OFR+PPI*FWTRA(KR)+FIMPA(KR)*OFRTS+SPIF	MAIN0851
IF(TRIP .EQ. 2) GO TO 1380	MAIN0852
ARHF(KR)=ARHF(KR)+SPDR*WCPSA(KR)	MAIN0853
GO TO 1540	MAIN0854
1370 SPDR=0.0	MAIN0855
C ROUTING	MAIN0856
1380 IF(CONOPT(2) .NE. 1) GO TO 1390	MAIN0857
C HOURLY ROUTING	MAIN0858
URHF=URHF+SPDR	MAIN0859
IF(PRD.NE.4)GO TO 1540	MAIN0860
C SPDR MUST BE IN INCHES PER QUARTER HOUR	MAIN0861
SPDR = URHF/4.	MAIN0862
1390 IF(NUBR(KR).LE.0) GO TO 1410	MAIN0863
C IF THE CURRENT REACH IS THE FIRST REACH AFTER A JUNCTION, THEN	MAIN0864
C ADD THE PREVIOUS JUNCTION'S HYDROGRAPHS TO OBTAIN INFLOW TO REACH	MAIN0865
NSUM=IUH(KR,1)	MAIN0866
IF(NUBR(KR).EQ.1) GO TO 1410	MAIN0867
NB=NUBR(KR)	MAIN0868

DO 1400 NH=2,NB	MAIN0869
IH=IUH(KR,NH)	MAIN0870
1400 HYDSTO(NFSEPR(NSUM)+NFS(NSUM),DPRD)=HYDSTO(NFSEPR(NSUM)+NFS(NSUM),	MAIN0871
1 DPRD)+HYDSTO(NFSEPR(IH)+NFS(IH),DPRD)	MAIN0872
C CALCULATE LOCAL INFLOW PER LINEAR FT ALONG THE REACH	MAIN0873
1410 ROLF2(KR)=(SPDR*4.*WCFS(A(KR)))/RCHLTH(KR)	MAIN0874
DO 1500 KFS=1,NFSKR	MAIN0875
KRKFS=NFSEPR(KR)+KFS	MAIN0876
DELTA X=FSLTH(KRKFS)/NOFD(KRKFS)	MAIN0877
C DETERMINE ALPHA AND EXPM TO BE USED FOR ROUTING	MAIN0878
CALL RTGPAR(XAREA,KFS,KRKFS,ALPH,EXPT,DAREA,ALPHA,EXPM,NOFD,NSC)	MAIN0879
IF(NSC(KRKFS) .LE. 1) GO TO 1420	MAIN0880
ALPHA1=ALPHA	MAIN0881
EXPM1=EXPM	MAIN0882
IF(PALPHA(KRKFS) .EQ. ALPHA) GO TO 1420	MAIN0883
ALPHA=(PALPHA(KRKFS)+ALPHA)/2.0	MAIN0884
EXPM=(PEXPM(KRKFS)+EXPM)/2.0	MAIN0885
PALPHA(KRKFS) = ALPHA1	MAIN0886
PEXPM(KRKFS) = EXPM1	MAIN0887
1420 IF(NUBR(KR) .LE. 0 .AND. KFS .EQ. 1) GO TO 1450	MAIN0888
IF(KFS .EQ. 1) GO TO 1430	MAIN0889
NLR=KR	MAIN0890
NLS = KFS-1	MAIN0891
GO TO 1440	MAIN0892
1430 NLR = NSUM	MAIN0893
NLS = NFS(NSUM)	MAIN0894
1440 IF(HYDSTO(NFSEPR(NLR)+NLS,DPRD)/ALPHA .LE. 0.) GO TO 1450	MAIN0895
C CALCULATE XAREA AT HEAD OF REACH FROM INFLOW HYDROGRAPH	MAIN0896
XAREA(KFS,1)=(HYDSTO(NFSEPR(NLR)+NLS,DPRD)/ALPHA)**(1./EXPM)	MAIN0897
GO TO 1460	MAIN0898
1450 XAREA(KFS,1)=0.0	MAIN0899
C FINITE DIFFERENCE ROUTING THROUGH STREAM REACH	MAIN0900
1460 N=NOFD(KRKFS)+1	MAIN0901
IF(EXPM .GE. 0.0) GO TO 1480	MAIN0902
WRITE(6,1470) EXPM,KR,KFS	MAIN0903

1470	FORMAT(5X,6HEXPM= ,E10.3,2X,13HAT REACH NO. ,12,2X,12HSECTION NO. ,12)	MAIN0904
	GO TO 2460	MAIN0906
1480	CALL CHROUT(XAREA,ROLF1,ROLF2,DELTAT,DELTAX,ALPHA,EXPM,N,KR,KFS,1 CONOPT(3))	MAIN0907
C	CALCULATE CURRENT ROUTED OUTFLOW FROM THE REACH	MAIN0908
	RHF(KR)=ALPHA*XAREA(KFS,N)**EXPM	MAIN0909
C	CALCULATE TOTAL OUTFLOW	MAIN0910
	IF(KFS .NE. NFSKR) GO TO 1490	MAIN0911
	TFCFS=RHF(KR)+(CBF-HSE)*WCFS(KR)	MAIN0912
	IF(TFCFS .LT. 0.0) TFCFS=0.0	MAIN0913
	GO TO 1500	MAIN0914
1490	HYDSTO(KRKFS,DPRD)=RHF(KR)	MAIN0915
1500	CONTINUE	MAIN0916
1510	IF(.NOT. LSTR) GO TO 1520	MAIN0917
	CALL STORRT(TFCFS,KR,DELTAT,ELEV,DISCH,STORAG,IFST(KR),1 ILST(KR),INLST,OUTLST,STOLST,PET)	MAIN0918
	TFCFS=OUTLST(KR)	MAIN0919
C	STORE FLOW FOR INFLOW TO NEXT LOWER REACH	MAIN0920
1520	HYDSTO(NFSEPR(KR)+NFSKR,DPRD)=TFCFS	MAIN0921
C	CHECK TO SEE IF DAY'S FLOWS SHOULD BE PRINTED	MAIN0922
	IF(TFCFS .LE. TFMAX) GO TO 1530	MAIN0923
	PRDF = PRD	MAIN0924
	TDFP24 = HOUR	MAIN0925
	IF(PRD .LE. 3) TDFP24 = (TDFP24 - 1.0) + 0.15*PRDF	MAIN0926
	TFMAX = TFCFS	MAIN0927
1530	ARHF(KR)=ARHF(KR)+TFCFS/NPPH	MAIN0928
	ROLF1(KR)=ROLF2(KR)	MAIN0929
1540	IF(VINCR(KR).LT.FHPP+VINMRA(KR)) VINCR(KR)=VINCR(KR)+DPET(DAY)/1 (24.0/FHPP)	MAIN0930
1550	CONTINUE	MAIN0931
C	END OF 15 MINUTE LOOP	MAIN0932
C	ADDING GROUNDWATER FLOW	MAIN0933
	GWSA(KR)=GWSA(KR)-CBF	MAIN0934
	AMBF(KR)=AMBF(KR)+CBF	MAIN0935
		MAIN0936
		MAIN0937
		MAIN0938

IF(TRIP .EQ. 1) ARHF(KR)=ARHF(KR)+CBF*WCFS(A(KR))	MAIN0939
C EVAPORATION FROM STREAM SURFACE	MAIN0940
IF(HSE*WCFS(A(KR)) .GT. ARHF(KR)) HSE=ARHF(KR)/WCFS(A(KR))	MAIN0941
IF(CBF.GT.HSE)ADBF(KR)=ADBF(KR)+CBF-HSE	MAIN0942
AMSE(KR)=AMSE(KR)+HSE	MAIN0943
THSF(HOUR)=ARHF(KR)	MAIN0944
IF(TRIP .EQ. 1) THSF(HOUR)=ARHF(KR)-(HSE*WCFS(A(KR)))	MAIN0945
IF(TFMAX .LE. 0.0) TFMAX = THSF(HOUR)	MAIN0946
TDSF = TDSF + THSF(HOUR)	MAIN0947
C DRAINING OF UPPER ZONE STORAGE	MAIN0948
UZINFX=(UZSA(KR)/UZC)-(LZSA(KR)/LZC)	MAIN0949
IF(UZINFX .LE. 0.0) GO TO 1560	MAIN0950
LZSR=LZSA(KR)/LZC	MAIN0951
UZINLZ = 0.003*BMR*UZC*UZINFX**3.0	MAIN0952
IF(UZINLZ.GT.UZSA(KR))UZINLZ=UZSA(KR)	MAIN0953
UZSA(KR)=UZSA(KR)-UZINLZ	MAIN0954
LZRX = 1.5*ABS(LZSR - 1.0) + 1.0	MAIN0955
FMR = (1.0/(1.0 + LZRX))**LZRX	MAIN0956
IF(LZSA(KR).LT.LZC)FMR=1.0-FMR*LZSR	MAIN0957
PGW = (1.0-FMR)*UZINLZ*(1.0 - SUBWF)*FPEFA(KR)	MAIN0958
PLZS =FMR*UZINLZ	MAIN0959
LZSA(KR)=LZSA(KR)+PLZS	MAIN0960
GWSA(KR)=GWSA(KR)+PGW	MAIN0961
C 4 PM ADJUSTMENTS OF VARIOUS VALUES	MAIN0962
1560 IF(HOUR .NE. 16) GO TO 1610	MAIN0963
PAEX90=AEX90	MAIN0964
PAEX96=AEX96	MAIN0965
AEX90 = 0.9*(AEX90 + PET)	MAIN0966
AEX96 = 0.96*(AEX96 + PET)	MAIN0967
IF(KR.NE.NR) GO TO 1570	MAIN0968
PAEX90=AEX90	MAIN0969
PAEX96=AEX96	MAIN0970
C INFILTRATION CORRECTION	MAIN0971
1570 SIAM = (AEX96/AETX)**SIAC	MAIN0972
IF(SIAM .LT. 0.33) SIAM = 0.33	MAIN0973

	IF(PET .EQ. 0.0) GO TO 1610	MAIN0974
C	EVAP-TRANS LOSS FROM GROUNDWATER	MAIN0975
	GWET=GWSA(KR)*GWETF*PET*FPERA(KR)	MAIN0976
	GWSA(KR)=GWSA(KR)-GWET	MAIN0977
	IF(KR.EQ.1)AMPET=AMPET+PET	MAIN0978
	IF(PET.GE.UZSA(KR))GO TO 1580	MAIN0979
	UZSA(KR)=UZSA(KR)-PET	MAIN0980
	AMNET(KR)=AMNET(KR)+PET	MAIN0981
	GO TO 1610	MAIN0982
1580	PET=PET-UZSA(KR)	MAIN0983
	AMNET(KR)=AMNET(KR)+UZSA(KR)	MAIN0984
	UZSA(KR)=0.0	MAIN0985
	LZSR=LZSA(KR)/LZC	MAIN0986
	IF(PET .GE. ETLF*LZSR) GO TO 1590	MAIN0987
	SET = PET*(1.0 - PET/(2.0*ETLF*LZSR))	MAIN0988
	GO TO 1600	MAIN0989
1590	SET = 0.5*ETLF*LZSR	MAIN0990
1600	LZSA(KR)=LZSA(KR)-SET	MAIN0991
	AMNET(KR)=AMNET(KR)+SET	MAIN0992
1610	CONTINUE	MAIN0993
C	END OF HOUR LOOP	MAIN0994
	IF(TRIP .EQ. 1) GO TO 1630	MAIN0995
C	STORE END AREA OF EACH FINITE DIFFERENCE SECTION FOR USE IN NEXT DAY	MAIN0996
	DO 1620 KFS=1,NFSKR	MAIN0997
	KRKFS=NFSEPR(KR)+KFS	MAIN0998
	N=NUFD(KRKFS)+1	MAIN0999
	DO 1620 NX=1,N	MAIN1000
	EDAREA(KRKFS,NX)=XAREA(KFS,NX)	MAIN1001
1620	CONTINUE	MAIN1002
1630	DSSF(DAY)=TDSF/24.0	MAIN1003
	IF(TRIP.EQ.1.AND.KR.LE.NGR) GAFL(DAY)=GAFL(DAY)+DSSF(DAY)	MAIN1004
	IF(TRIP .EQ. 1) GO TO 1730	MAIN1005
	IF(KR.NE.NGR)GO TO 1640	MAIN1006
C	DETERMINE SYNTHESIZED DAILY AND MONTHLY FLOW AT GAGED REACH	MAIN1007
	GAFL(DAY)=DSSF(DAY)	MAIN1008

SMF=SMF+DSSF(DAY)	MAIN1009
1640 IF(.NOT.SAVE .OR. TFMAX.LE. RMPF) GO TO 1730	MAIN1010
C PRINT HOURLY,DAILY AND PEAK FLOW FOR DAY WHEN PEAK FLOW EXCEEDS RMPF	MAIN1011
WRITE(6,1650)KR,RMPF	MAIN1012
1650 FORMAT(1H/,'*** FLOW FROM REACH = ',I2,' (REQUESTED MINIMUM PEAK	MAIN1013
1= ',F5.1,'CFS) ***')	MAIN1014
IF(DAY .EQ. 366) MDAY = 337	MAIN1015
DATE = MOD(DAY,MDAY)	MAIN1016
WRITE(6,1660) DATE, (THSF(HOUR),HOUR=1,12)	MAIN1017
1660 FORMAT(1H/,1X/,1X,14,2X,2HAM,1X,6F8.1,3X,6F8.1)	MAIN1018
WRITE(6,1670) (THSF(HOUR),HOUR=13,24), DSSF(DAY)	MAIN1019
1670 FORMAT(1HJ,6X,2HPM,1X,6F8.1,3X,7F8.1)	MAIN1020
IF(TDFP24 .LT. 12.0) GO TO 1690	MAIN1021
TDFP12 = TDFP24 - 12.0	MAIN1022
WRITE(6,1680) TFMAX, TDFP12	MAIN1023
1680 FORMAT(1H/,10X,8HMAXIMUM=,F8.1,2X,6HC.F.S.,5X,4HTIME,3X,F5.2,2X,	MAIN1024
1 4HP.M.)	MAIN1025
GO TO 1710	MAIN1026
1690 WRITE(6,1700) TFMAX,TDFP24	MAIN1027
1700 FORMAT(1H/,10X,8HMAXIMUM=,F8.1,2X,6HC.F.S.,5X,4HTIME,3X,F5.2,2X,	MAIN1028
1 4HA.M.)	MAIN1029
1710 WRITE(6,1720)	MAIN1030
1720 FORMAT(1H/,120('_''))	MAIN1031
1730 IF(TRIP .EQ. 1 .AND. .NOT. LRC .AND. KDRS .LE. 3 .AND. IFRC .GT.	MAIN1032
1 0.1) SIFRS(KDRS,KRS-1) = SIFRS(KDRS,KRS-1)+ADIF(KR)*VWINA(KR)	MAIN1033
IF(TRIP.EQ.1.AND.KDRS.LE.3) SBFRS(KDRS,KRS-1)=SBFRS(KDRS,KRS-1)+	MAIN1034
1 ADBF(KR)*VWINA(KR)	MAIN1035
C MONTHLY SUMMARY STORAGE	MAIN1036
IF(DAY .NE. MEDWY(MONTH)) GO TO 2080	MAIN1037
IF(KR.GT.NGR)GO TO 1840	MAIN1038
IF(KR.NE.NGR)GO TO 1820	MAIN1039
IF(TRIP .EQ. 1) GO TO 1810	MAIN1040
C PRINT SYNTHESIZED DAILY FLOW AND MONTHLY TOTAL	MAIN1041
IF(MONTH-1) 1740,1740,1750	MAIN1042
1740 MEDWYP=MEDWY(12)	MAIN1043

	GO TO 1760	MAIN1044
1750	MEDWYP=MEDWY(MONTH-1)	MAIN1045
	IF(MONTH.EQ.4) MEDWYP=0	MAIN1046
	IF(MONTH.EQ.5) MEDWY(5)=59	MAIN1047
1760	NDPM=MEDWY(MONTH)-MEDWYP	MAIN1048
	WRITE(6,1770)	MAIN1049
1770	FORMAT(/,5X,49HDAILY SYNTHESIZED FLOWS ARE TABULATED AS FOLLOWS:)	MAIN1050
	WRITE(6,1780)(I,DSSF(MEDWYP+I),I=1,NDPM)	MAIN1051
1780	FORMAT(8(15,F10.3))	MAIN1052
	IF(MONTH.EQ.5.AND.DPY.EQ.366) WRITE(6,1790) DSSF(366)	MAIN1053
1790	FORMAT(2X,3H366,F10.3)	MAIN1054
	WRITE(6,1800)MONTH,SMF	MAIN1055
1800	FORMAT(5X,6HMONTH=,13,5X,20HSUM OF MONTHLY FLOW=,F10.3)	MAIN1056
	SMF=0.0	MAIN1057
1810	TMPET(MONTH) = AMPET	MAIN1058
	AMPET = 0.0	MAIN1059
1820	IF(KR.GT.NGR) GO TO 1840	MAIN1060
	TMPREC(MONTH)=TMPREC(MONTH)+AMPREC	MAIN1061
	AMPREC=0.0	MAIN1062
	TMBF(MONTH)=TMBF(MONTH)+AMBF(KR)*SAREA(KR)	MAIN1063
	AMBF(KR)=0.0	MAIN1064
	TMIF(MONTH)=TMIF(MONTH)+AMIF(KR)*SAREA(KR)	MAIN1065
	AMIF(KR)=0.0	MAIN1066
	TMSE(MONTH)=TMSE(MONTH)+AMSE(KR)*SAREA(KR)	MAIN1067
	AMSE(KR)=0.0	MAIN1068
	TMNET(MONTH)=TMNET(MONTH)+AMNET(KR)*SAREA(KR)	MAIN1069
	AMNET(KR)=0.0	MAIN1070
	UZC=SUZC*AEX90+BUZC*EXP(-2.7*LZSA(KR)/LZC)	MAIN1071
	IF(UZC.LT.0.25) UZC = 0.25	MAIN1072
	IF(KR.NE.1) GO TO 1830	MAIN1073
	EMGWS(MONTH)=GWSA(KR)*SAREA(KR)	MAIN1074
	EMUZC(MONTH)=UZC*SAREA(KR)	MAIN1075
	EMUZS(MONTH)=UZSA(KR)*SAREA(KR)	MAIN1076
	EMSIAM(MONTH)=SIAM*SAREA(KR)	MAIN1077
	EMLZS(MONTH)=LZSA(KR)*SAREA(KR)	MAIN1078

	EMIFS (MONTH)=IFSA(KR)*SAREA(KR)	MAIN1079
	GO TO 1840	MAIN1080
1830	EMUZC(MONTH)=EMUZC(MONTH)+UZC*SAREA(KR)	MAIN1081
	EMUZS(MONTH)=EMUZS(MONTH)+UZSA(KR)*SAREA(KR)	MAIN1082
	EMSIAM(MONTH)=EMSIAM(MONTH)+SIAM*SAREA(KR)	MAIN1083
	EMLZS(MONTH)=EMLZS(MONTH)+LZSA(KR)*SAREA(KR)	MAIN1084
	EMIFS(MONTH)=EMIFS(MONTH)+IFSA(KR)*SARFA(KR)	MAIN1085
	EMGWS(MONTH)=EMGWS(MONTH)+GWSA(KR)*SAREA(KR)	MAIN1086
1840	IF(KR.NE.NR) GO TO 1150	MAIN1087
C	END OF REACH LOOP	MAIN1088
	IF(MONTH.EQ.5) MEDWY(5) = 59	MAIN1089
	MDAY = MEDWY(MONTH)	MAIN1090
	IF(TRIP.EQ.1) GO TO 2070	MAIN1091
	GO TO (1850,1870,1890,1910,1930,1950,1970,1990,2010,2030,2050,	MAIN1092
1	2070),MONTH	MAIN1093
1850	WRITE(6,1860)	MAIN1094
1860	FORMAT(1H/,8HNOVEMBER)	MAIN1095
	GO TO 2070	MAIN1096
1870	WRITE(6,1880)	MAIN1097
1880	FORMAT(1H/,8HDECEMBER)	MAIN1098
	GO TO 2070	MAIN1099
1890	WRITE(6,1900)	MAIN1100
1900	FORMAT(1H/,7HJANUARY)	MAIN1101
	GO TO 2070	MAIN1102
1910	WRITE(6,1920)	MAIN1103
1920	FORMAT(1H/,8HFEBRUARY)	MAIN1104
	GO TO 2070	MAIN1105
1930	WRITE(6,1940)	MAIN1106
1940	FORMAT(1H/,5HMARCH)	MAIN1107
	GO TO 2070	MAIN1108
1950	WRITE(6,1960)	MAIN1109
1960	FORMAT(1H/,5HAPRIL)	MAIN1110
	GO TO 2070	MAIN1111
1970	WRITE(6,1980)	MAIN1112
1980	FORMAT(1H/,3HMAY)	MAIN1113

GO TO 2070	MAIN1114
1990 WRITE(6,2000)	MAIN1115
2000 FORMAT(1H/,4HJUNE)	MAIN1116
GO TO 2070	MAIN1117
2010 WRITE(6,2020)	MAIN1118
2020 FORMAT(1H/,4HJULY)	MAIN1119
GO TO 2070	MAIN1120
2030 WRITE(6,2040)	MAIN1121
2040 FORMAT(1H/,6HAUGUST)	MAIN1122
GO TO 2070	MAIN1123
2050 WRITE(6,2060)	MAIN1124
2060 FORMAT(1H/,9HSEPTEMBER)	MAIN1125
2070 IF(KR.EQ.NR) MONTH=MONTH+1	MAIN1126
LDAY=DAY	MAIN1127
C END OF DAY LOOP	MAIN1128
2080 IF(KR.NE.NR) GO TO 1150	MAIN1129
CALL DAYNXT(DAY,DPY)	MAIN1130
IF(DAY.NE.274) GO TO 1140	MAIN1131
DO 2090 MONTH=1,12	MAIN1132
TMPREC(MONTH)=TMPREC(MONTH)/GAREA	MAIN1133
TMBF(MONTH)=TMBF(MONTH)/GAREA	MAIN1134
TMIF(MONTH)=TMIF(MONTH)/GAREA	MAIN1135
TMSE(MONTH)=TMSE(MONTH)/GAREA	MAIN1136
TMNET(MONTH)=TMNET(MONTH)/GAREA	MAIN1137
EMGWS(MONTH)=EMGWS(MONTH)/GAREA	MAIN1138
EMUZC(MONTH)=EMUZC(MONTH)/GAREA	MAIN1139
EMUZS(MONTH)=EMUZS(MONTH)/GAREA	MAIN1140
EMSIAM(MONTH)=EMSIAM(MONTH)/GAREA	MAIN1141
EMLZS(MONTH)=EMLZS(MONTH)/GAREA	MAIN1142
EMIFS(MONTH)=EMIFS(MONTH)/GAREA	MAIN1143
2090 CONTINUE	MAIN1144
IF(TRIP.NE.1) WRITE(6,2100) (TITLE(KTA), KTA=1,20,1)	MAIN1145
2100 FORMAT(1H1,25X,20A4)	MAIN1146
C ANNUAL SUMMARY	MAIN1147
APREC = 0.0	MAIN1148

	ABFV = 0.0	MAIN1149
	ASEV = 0.0	MAIN1150
	ANET = 0.0	MAIN1151
	APET = 0.0	MAIN1152
	AIFV = 0.0	MAIN1153
	DO 2110 MONTH = 1,12	MAIN1154
	APREC = APREC + TMPREC(MONTH)	MAIN1155
	ABFV = ABFV + TMBF(MONTH)	MAIN1156
	ASEV = ASEV + TMSE(MONTH)	MAIN1157
	ANET = ANET + TMNET(MONTH)	MAIN1158
	APET = APET + TMPET(MONTH)	MAIN1159
2110	AIFV = AIFV + TMIF(MONTH)	MAIN1160
	WRITE(6,2120)	MAIN1161
2120	FORMAT(1H///44X,23HSYNTHESIZED FLOWS)	MAIN1162
	IF(TRIP .EQ. 1) WRITE(6,2130)	MAIN1163
2130	FORMAT(/5X,*SUMMARY WHILE OPTIMIZING VOLUME VARIABLES*)	MAIN1164
	CALL DAYSUM(GAFL,MEDCY,DPY,SATFV,TMSTF)	MAIN1165
	IF(TRIP .EQ. 1) GO TO 2140	MAIN1166
	CALL DAYOUT(GAFL,MEDWY,DPY)	MAIN1167
2140	WRITE(6,2150) (TMSTF(KWD), KWD=1,12),SATFV	MAIN1168
2150	FORMAT(1X, 9HSYNTHETIC,3X,12F8.1,2X,F10.1,2X,3HSFD)	MAIN1169
	DO 2160 MONTH = 1,12	MAIN1170
2160	TMSTFI(MONTH) = TMSTF(MONTH)/CVWINA(NGR)	MAIN1171
	SATFVI = SATFV/CVWINA(NGR)	MAIN1172
	WRITE(6,2170) (TMSTFI(KWD),KWD=1,12),SATFVI	MAIN1173
2170	FORMAT(1X,5HTOTAL,8X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN1174
	DO 2180 MONTH = 1,12	MAIN1175
	TMOF(MONTH) = TMSTFI(MONTH) - TMIF(MONTH) - TMBF(MONTH) +	MAIN1176
	1 TMSE(MONTH)	MAIN1177
2180	IF(TMOF(MONTH) .LT. 0.0) TMOF(MONTH) = 0.0	MAIN1178
	AOFV = SATFVI - AIFV - ABFV + ASEV	MAIN1179
	IF(AOFV .LT.0.0) AOFV = 0.0	MAIN1180
	WRITE(6,2190) (TMOF(KWD), KWD=1,12), AOFV	MAIN1181
2190	FORMAT(1X,8HOVERLAND ,5X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN1182
	WRITE(6,2200) (TMIF(KWD), KWD=1,12),AIFV	MAIN1183

2200	FORMAT(1X,9HINTERFLOW,4X,12F8.3,4X,F7.3,2X,6HINCHFS)	MAIN1184
	WRITE(6,2210) (TMBF(KWD), KWD=1,12),ABFV	MAIN1185
2210	FORMAT(1X,4HBASE,9X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN1186
	WRITE(6,2220) (TMSE(KWD), KWD=1,12), ASEV	MAIN1187
2220	FORMAT(1X,9HSTRM EVAP,4X,12F8.3,4X,F7.3,2X,6HINCHES)	MAIN1188
	WRITE(6,2230) (TMPREC(KWD), KWD=1,12),APREC	MAIN1189
2230	FORMAT(1X,6HPRECIP,7X,12F8.2,3X,F8.2,2X,6HINCHES)	MAIN1190
	WRITE(6,2240) (TMNET(KWD), KWD=1,12),ANET	MAIN1191
2240	FORMAT(1X,12HEVP/TRAN-NET,2X,12F8.3,3X,F7.3,2X,6HINCHES)	MAIN1192
	WRITE(6,2250) (TMPET(KWD), KWD=1,12),APET	MAIN1193
2250	FORMAT(3X,10H-POTENTIAL,2X,12F8.3,3X,F7.3,2X,6HINCHES)	MAIN1194
	WRITE(6,2260) (EMUZS(KWD), KWD=1,12)	MAIN1195
2260	FORMAT(1X,12HSTORAGES-UZS,2X,12F8.3,12X,6HINCHES)	MAIN1196
	WRITE(6,2270) (EMLZS(KWD), KWD=1,12)	MAIN1197
2270	FORMAT(10X,3HLZS,2X,12F8.3,12X,6HINCHES)	MAIN1198
	WRITE(6,2280) (EMIFS(KWD), KWD=1,12)	MAIN1199
2280	FORMAT(10X,3HIFS,2X,12F8.3,12X,6HINCHES)	MAIN1200
	WRITE(6,2290) (EMGWS(KWD), KWD=1,12)	MAIN1201
2290	FORMAT(10X,3HGWS,2X,12F8.3,12X,6HINCHES)	MAIN1202
	WRITE(6,2300) (EMUZC(KWD), KWD=1,12)	MAIN1203
2300	FORMAT(1X,12HINDICES- UZC,2X,12F8.3)	MAIN1204
	WRITE(6,2310) (EMSIAM(KWD), KWD=1,12)	MAIN1205
2310	FORMAT(9X,4HSIAM,2X,12F8.3)	MAIN1206
	LZS=EMLZS(12)	MAIN1207
	UZS=EMUZS(12)	MAIN1208
	IFS=EMIFS(12)	MAIN1209
	GWS=EMGWS(12)	MAIN1210
	AMBER=(LZS-BYLZS)*FPERA(NGR)+(UZS+IFS+GWS-BYGWS)*(1.0-FWTRA(NGR))+	MAIN1211
	1 SATFVI+ANET*FPERA(NGR)+ASEV-APREC	MAIN1212
	WRITE(6,2320) AMBER	MAIN1213
2320	FORMAT(1H/,7HBALANCE,5X,F10.4,2X,6HINCHES)	MAIN1214
C	ESTABLISH WHETHER MONTH IS PREDOMINATELY BASE FLOW OR DIRECT RUNOFF	MAIN1215
	NOFM = 0	MAIN1216
	MONTH1 = 1	MAIN1217
	IF(FTX .LT. 0.95) MONTH1 = 4	MAIN1218

LZC=LZC1	MAIN1219
BMIR=BMIR1	MAIN1220
SUZC=SUZC1	MAIN1221
ETLF=ETLF1	MAIN1222
BUZC=BUZC1	MAIN1223
SIAC=SIAC1	MAIN1224
BIVF=BIVF1	MAIN1225
DO 2340 MONTH = 1,12	MAIN1226
XMPFT(MONTH) = 0.0	MAIN1227
IF(MONTH .LT. MONTH1) GO TO 2340	MAIN1228
IF(TMSTFI(MONTH) .GT. 0.001) GO TO 2330	MAIN1229
XMPFT(MONTH) = 1.0	MAIN1230
GO TO 2340	MAIN1231
2330 IF(TMBF(MONTH)/TMSTFI(MONTH) .GT. 0.5) XMPFT(MONTH) = 1.0	MAIN1232
IF(TMOF(MONTH)/TMSTFI(MONTH) .LT. 0.5) GO TO 2340	MAIN1233
NOFM = NOFM + 1	MAIN1234
XMPFT(MONTH) = 2.0	MAIN1235
2340 CONTINUE	MAIN1236
C NATURE OF TRIPS	MAIN1237
C TRIP 1 OPTIMIZE VOLUME VARIABLES WHILE BYPASSING ROUTING	MAIN1238
C TRIP 2 FINAL RUN WITH OPTIMIZED VALUES	MAIN1239
IF(TRIP .EQ. 1) GO TO 2350	MAIN1240
GO TO 2460	MAIN1241
C SYSTEMATIC ADJUSTMENT OF VOLUME VARIABLES CONVERGING ON OPTIMUM VALUES	MAIN1242
2350 KRC = KRC + 1	MAIN1243
PLZC = LZC	MAIN1244
PBMIR = BMIR	MAIN1245
PSUZC = SUZC	MAIN1246
PETLF = ETLF	MAIN1247
PBUZC = BUZC	MAIN1248
PSIAC = SIAC	MAIN1249
C ADJUST FIVE VOLUME VARIABLES: LZC,SUZC,ETLF,BUZC,SIAC	MAIN1250
CALL SETFVP(LZC,SUZC,ETLF,BUZC,SIAC,TMSTF, TMRTF, TMPREC, TMPET,	MAIN1251
1 EMLZS,SSQM,LRC,XMPFT,FTX,NOFM,LBUZC,LETLF,LLZC,APREC,APET)	MAIN1252
C ADJUST INTERFLOW VOLUME CONSTANT DURING FINE ADJUSTMENT PHASE	MAIN1253

FNWSTH=FNWSTH	MAIN1254
IF(.NOT. LRC .AND. IFRC .GT. 0.1) CALL SETBIV(BIVF,NRS,IFRC,RSBIF,	MAIN1255
1 SIFRS,FNWSTH)	MAIN1256
C ADJUST INFILTRATION RATE CONSTANT: BMIR	MAIN1257
IF(.NOT. LBMIR) GO TO 2360	MAIN1258
BMIR = 0.9*BMIR	MAIN1259
GO TO 2370	MAIN1260
2360 IF(ABS(FTX-1.0) .GT. 0.02 .AND. KRC .GT. 5) IFT = 2	MAIN1261
CALL SETBMI(BMIR,NRS,BFRC,RSBBF,SBFRS,FNWSTH,IFT)	MAIN1262
2370 IF((KRC .GT. 6) .AND. (LZC .GT. 29.0)) LLZC = .TRUE.	MAIN1263
IF((KRC .GT. 6) .AND. (ETLF .GT. 0.59)) LETLF = .TRUE.	MAIN1264
IF((KRC .GT. 6) .AND. (BUZC .GT. 3.9)) LBUZC = .TRUE.	MAIN1265
IF(.NOT. LLZC) GO TO 2390	MAIN1266
LZC = PLZC*SATFV/RATFV	MAIN1267
WRITE(6,2380) LZC	MAIN1268
2380 FORMAT(/2X,'LZC WAS CHANGED TO',F6.2,' BASED ON ANNUAL RUNOFF VOLUMAIN1269	MAIN1270
1ME')	MAIN1271
2390 IF(KRC .LT. 6 .OR. BMIR .LT. 20.0) GO TO 2400	MAIN1272
LBMIR = .TRUE.	MAIN1273
BMIR = 20.0	MAIN1274
2400 IF (CONOPT(7) .EQ. 0) GO TO 2410	MAIN1275
IF(KRC .EQ. CONOPT(7)+1) GO TO 2460	MAIN1276
GO TO 970	MAIN1277
2410 IF(SSSQM .LE. SSQM .AND. ((KRC .GT. MNRC .AND. KBRC .GE. 2) .OR.	MAIN1278
1 (.NOT. LRC))) GO TO 2440	MAIN1279
IF(SSSQM .LE. SSQM) GO TO 970	MAIN1280
BLZC = PLZC	MAIN1281
6BMIR = PBMIR	MAIN1282
BSUZC = PSUZC	MAIN1283
BETLF = PETLF	MAIN1284
BBUZC = PBUZC	MAIN1285
BSIAC = PSIAC	MAIN1286
SSSQM = SSQM	MAIN1287
BBYLZS = BYLZS	MAIN1288
KBRC = 0	

	IF(SSQM .LT. 0.15 .AND. LRC) GO TO 2420	MAIN1289
	GO TO 970	MAIN1290
2420	LRC = .FALSE.	MAIN1291
	WRITE(6,2430)	MAIN1292
2430	FORMAT(/5X,'SHIFT TO FINE ADJUSTMENT BEGINNING AT BEST ROUGH ADJUSTMENT POINT')	MAIN1293
	SSSQM = 1000.0	MAIN1294
	GO TO 2450	MAIN1295
2440	CONTINUE	MAIN1296
	IF(LRC) GO TO 2420	MAIN1297
	IF(TRIP .EQ. NLTR) GO TO 2460	MAIN1298
	TRIP = TRIP + 1	MAIN1299
2450	LZC = BLZC	MAIN1300
	BMIR = BBMIR	MAIN1301
	SUZC = BSUZC	MAIN1302
	ETLF = BETLF	MAIN1303
	BUZC = BBUZC	MAIN1304
	SIAC = BSIAC	MAIN1305
	KFFC = 1	MAIN1306
	GO TO 970	MAIN1307
2460	CONTINUE	MAIN1308
	IF(NSYC .LT. NSYT) GO TO 20	MAIN1309
	STOP	MAIN1310
	END	MAIN1311
		MAIN1312

	SUBROUTINE ROUTAB(DISCH,DAREA,KRKFS,NP,ALPH,EXPT)	RTAB0001
C	THIS SUBROUTINE PRODUCES A TABLE OF ROUTING PARAMETERS FOR EACH	RTAB0002
C	FINITE SECTION IN EACH REACH OF THE WATERSHED FROM THE DISCHARGE-END	RTAB0003
C	AREA RELATIONSHIPS	RTAB0004
	DIMENSION DISCH(1000),DAREA(150,50),ALPH(150,50),EXPT(150,50)	RTAB0005
	REAL LDIS1,LDIS2,LDAR1,LDAR2	RTAB0006
	AREA1=DAREA(KRKFS,1)	RTAB0007
	DO 10 I=2,NP	RTAB0008

AREA2= DAREA(KRKFS,I)	RTAB0009
LDIS2=ALOG10(DISCH(I))	RTAB0010
LDIS1=ALOG10(DISCH(I-1))	RTAB0011
LDAR2=ALOG10(AREA2)	RTAB0012
LDAR1=ALOG10(AREA1)	RTAB0013
EXPT(KRKFS,I) = (LDIS2-LDIS1)/(LDAR2-LDAR1)	RTAB0014
ALPH(KRKFS,I) = 10.** (LDIS2-EXPT(KRKFS,I)*LDAR2)	RTAB0015
DAREA(KRKFS,I)= (AREA1+AREA2)/2.	RTAB0016
AREA1=AREA2	RTAB0017
10 CONTINUE	RTAB0018
RETURN	RTAB0019
END	RTAB0020

SUBROUTINE RTGPAR(XAREA,KFS,KRKFS,ALPH,EXPT,DAREA,ALPHA,EXPM,	RPAR0001
1 NOFD,NSC)	RPAR0002
C THIS SUBROUTINE READS THE PROPER TABLE PRODUCED BY SUBROUTINE ROUTAB,	RPAR0003
C AND RETURNS THE CURRENT VALUES OF ALPHA AND THE EXPONENT M TO BE	RPAR0004
C USED IN THE STREAM ROUTING FOR ONE TIME INCREMENT.	RPAR0005
DIMENSION XAREA(150,21),ALPH(150,50),EXPT(150,50),DAREA(150,50),	RPAR0006
1 NOFD(150),NSC(150)	RPAR0007
AVXAR=0.0	RPAR0008
NL=NOFD(KRKFS)+1	RPAR0009
DO 10 N=2,NL	RPAR0010
10 AVXAR = AVXAR+XAREA(KFS,N)	RPAR0011
AVXAR=AVXAR+0.5*(XAREA(KFS,1)-XAREA(KFS,NL))	RPAR0012
AVXAR= AVXAR/NOFD(KRKFS)	RPAR0013
DO 20 J=1,50	RPAR0014
I=J	RPAR0015
IF(AVXAR .LE. DAREA(KRKFS,J)) GO TO 40	RPAR0016
IF(DAREA(KRKFS,J) .LE. 0.) GO TO 30	RPAR0017
20 CONTINUE	RPAR0018
30 ALPHA= ALPH(KRKFS,I-1)	RPAR0019
EXPM= EXPT(KRKFS,I-1)	RPAR0020

RETURN	RPAR0021
40 IF(I.LE.2 .AND. ALPH(KRKFS,1) .EQ. 0.0) GO TO 60	RPAR0022
IF(I .EQ. 1 .OR. NSC(KRKFS) .NE. 0) GO TO 50	RPAR0023
C INTERPOLATE BETWEEN VALUES	RPAR0024
RAT=(AVXAR-DAREA(KRKFS,I-1))/(DAREA(KRKFS,I)-DAREA(KRKFS,I-1))	RPAR0025
ALPHA=ALPH(KRKFS,I-1)+(ALPH(KRKFS,I)-ALPH(KRKFS,I-1))*RAT	RPAR0026
EXPM=EXPT(KRKFS,I-1)+(EXPT(KRKFS,I)-EXPT(KRKFS,I-1))*RAT	RPAR0027
RETURN	RPAR0028
50 ALPHA=ALPH(KRKFS,I)	RPAR0029
EXPM=EXPT(KRKFS,I)	RPAR0030
RETURN	RPAR0031
60 ALPHA=ALPH(KRKFS,2)	RPAR0032
EXPM=EXPT(KRKFS,2)	RPAR0033
RETURN	RPAR0034
END	RPAR0035

SUBROUTINE CHR0UT(XAREA,ROLF1,ROLF2,DELTAT,DELTAX,ALPHA,EXPM,N,KR,CHRTO001 1 KFS,INLR)	CHRTO002
C THIS SUBROUTINE ROUTES STREAMFLOW THROUGH THE REACHES OF THE	CHRTO003
C WATERSHED USING THE KINEMATIC METHODS.	CHRTO004
DIMENSION XAREA(150,21),ROLF1(15),ROLF2(15)	CHRTO005
C LINEAR SOLUTION	CHRTO006
DO 70 NX=2,N	CHRTO007
PXAREA = XAREA(KFS,NX)	CHRTO008
AX=0.5*(XAREA(KFS,NX-1)+PXAREA)	CHRTO009
IF(AX .GT. 1.0E-20) GO TO 10	CHRTO010
XAREA(KFS,NX)=DELTAT*(ROLF1(KR)+ROLF2(KR))/2.+PXAREA	CHRTO011
GO TO 20	CHRTO012
10 CONTINUE	CHRTO013
XAREA(KFS,NX) = (ROLF1(KR)+ROLF2(KR))/2.+PXAREA/DELTAT+XAREA(KFS,	CHRTO014
1 NX-1)/DELTAX*ALPHA*EXPM*AX**(EXPM-1)	CHRTO015
XAREA(KFS,NX) = XAREA(KFS,NX)/((1./DELTAT)+(1./DELTAX)*ALPHA*EXPM*CHRTO016	CHRTO016
1 AX**(EXPM-1))	CHRTO017

20	IF(XAREA(KFS,NX) .LT. 1.0E-20) XAREA(KFS,NX)=0.0	CHRT0018
	IF(INLR.NE.1) GO TO 70	CHRT0019
C	ITERATION FOR NON-LINEAR SOLUTION USING LINEAR SOLUTION AS FIRST	CHRT0020
C	ESTIMATE	CHRT0021
	IF(XAREA(KFS,NX) .LE. 0.) GO TO 70	CHRT0022
	THETA = DELTAX/DELTAT	CHRT0023
	CONST = THETA*PXAREA+DELTAX*(RDLF1(KR)+RDLF2(KR))/2.0	CHRT0024
	OMEGA = CONST+ALPHA*XAREA(KFS,NX-1)**EXPM	CHRT0025
	IF(OMEGA.GT.0.001/1000) GO TO 30	CHRT0026
	XAREA(KFS,NX) = 0.	CHRT0027
	GO TO 70	CHRT0028
30	CONTINUE	CHRT0029
	NI=0	CHRT0030
40	FUNC = THETA*XAREA(KFS,NX)+ALPHA*XAREA(KFS,NX)**EXPM	CHRT0031
	NI=NI+1	CHRT0032
	IF(NI.LT.10.OR.OMEGA.GT.1.0) GO TO 50	CHRT0033
	XAREA(KFS,NX) = 0.	CHRT0034
	GO TO 70	CHRT0035
50	CONTINUE	CHRT0036
	FUNC1 = THETA+ALPHA*EXPM*XAREA(KFS,NX)**(EXPM-1)	CHRT0037
	FUNC2 = ALPHA*EXPM*(EXPM-1)*XAREA(KFS,NX)**(EXPM-2)	CHRT0038
	IF(FUNC2.EQ.0.0) GO TO 70	CHRT0039
	XAREA1 = XAREA(KFS,NX)-FUNC1/FUNC2+((FUNC1/FUNC2)**2-2.*(FUNC-	CHRT0040
1	OMEGA)/FUNC2)**0.5	CHRT0041
	XAREA2 = XAREA(KFS,NX)-FUNC1/FUNC2-((FUNC1/FUNC2)**2-2.*(FUNC-	CHRT0042
1	OMEGA)/FUNC2)**0.5	CHRT0043
	IF(XAREA2.LT.0) XAREA2=XAREA1	CHRT0044
	FUNC1 = THETA*XAREA1+ALPHA*XAREA1**EXPM	CHRT0045
	FUNC2 = THETA*XAREA2+ALPHA*XAREA2**EXPM	CHRT0046
	DEV1=ABS(FUNC1-OMEGA)	CHRT0047
	DEV2=ABS(FUNC2-OMEGA)	CHRT0048
	XAREA(KFS,NX) = XAREA1	CHRT0049
	FUNC=FUNC1	CHRT0050
	IF(DEV2.GE.DEV1) GO TO 60	CHRT0051
	XAREA(KFS,NX) = XAREA2	CHRT0052

	FUNC=FUNC2	CHRT0053
60	CONTINUE	CHRT0054
	IF(NI.GT.20) GO TO 80	CHRT0055
	IF(ABS((FUNC-OMEGA)/OMEGA).GT.0.01) GO TO 40	CHRT0056
70	CONTINUE	CHRT0057
	RETURN	CHRT0058
80	WRITE(6,90)NI	CHRT0059
90	FORMAT(' ',' EXECUTION TERMINATING DUE TO EXCESSIVE ITERATIONS IN INONLINEAR ROUTING DETERMINATION. NI = ',I2)	CHRT0060 CHRT0061
	WRITE(6,100)	CHRT0062
100	FORMAT(' ',' CONDITIONS AT TERMINATION: ') WRITE(6,110)KR,KFS,NX,XAREA(KFS,NX),PXAREA	CHRT0063 CHRT0064
110	FORMAT(' ',' CURRENT REACH = ',I2,5X, ' NUMBER OF FINITE SECTIONS = ', 2 I2,/, ' NUMBER OF ENDAREA = ',I3,/, ' ENDAREA = ',F12.4, ' PREVIOUS EN 2DAREA = ',E12.4)	CHRT0065 CHRT0066 CHRT0067
	WRITE(6,120)ROLF2(KR),ROLF1(KR),DELTAT,DELTAX,ALPHA,EXPM	CHRT0068
120	FORMAT(' ',' LOCAL INFLOW = ',E12.4, ' PREVIOUS LOCAL INFLOW = ', 1 E12.4,/, ' DELTAT = ',F5.0, ' DELTAX = ',F5.0,/, ' ALPHA = ',F8.4, 2 ' EXPONENT M = ',F8.4)	CHRT0069 CHRT0070 CHRT0071
	WRITE(6,130) CONST,OMEGA,FUNC,FUNC1,FUNC2	CHRT0072
130	FORMAT('CONST',E12.4, 'OMEGA',E12.4, 'FUNC',E12.4, 'FUNC1',E12.4, 1 'FUNC2',E12.4)	CHRT0073 CHRT0074
	STOP	CHRT0075
	END	CHRT0076
	SUBROUTINE STORRT(TFCFS,KK,DELTAT,ELEV,DISCH,STORAG,IFST,1LST, 1 INLST,OUTLST,STOLST,PET)	STOR0001 STOR0002
C	THIS SUBROUTINE ADJUSTS THE CHANNEL ROUTED FLOWS FOR ANY REACH	STOR0003
C	TO ACCOUNT FOR THE EFFECTS OF ANY EXISTING STRUCTURE.	STOR0004
	DIMENSION DISCH(1000),STORAG(1000),INLST(15),OUTLST(15),STOLST(15)	STOR0005
1	,ELEV(1000)	STOR0006
	REAL INLST	STOR0007
	IF(STOLST(KR) .GE. STORAG(IFST)) GO TO 10	STOR0008

```

J=IFST+1
OUTLST(KR)=0.0
STOLST(KR)=STOLST(KR)+(INLST(KR)+TFCFS)*DELTAT/2.0
GO TO 40
10 C4 = (INLST(KR)+TFCFS-OUTLST(KR))*DELTAT+2.*STOLST(KR)
J = IFST
20 J = J+1
IF(J .GT. ILST-1) GO TO 30
C5 = DISCH(J)*DELTAT+2.*STORAG(J)
IF(C5-C4) 20,30,30
C SOLUTION OF SIMULTANEDUS EQUATIONS
30 A2 = DISCH(J)-DISCH(J-1)
B2 = STORAG(J-1)-STORAG(J)
C2 = DISCH(J-1)*STORAG(J)-STORAG(J-1)*DISCH(J)
AZ = 2.*B2-A2*DELTAT
OUTLST(KR) = (-C4*A2-2.0*C2)/AZ
STOLST(KR) = (DELTAT*C2+C4*B2)/AZ
40 SLOPE = (STORAG(J)-STORAG(J-1))/(ELEV(J)-ELEV(J-1))
STOLST(KR) = STOLST(KR)-PET*DELTAT*SLOPE/1036800.
INLST(KR)=TFCFS
RETURN
END

```

```

STOR0009
STOR0010
STOR0011
STOR0012
STOR0013
STOR0014
STOR0015
STOR0016
STOR0017
STOR0018
STOR0019
STOR0020
STOR0021
STOR0022
STOR0023
STOR0024
STOR0025
STOR0026
STOR0027
STOR0028
STOR0029
STOR0030

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SUBROUTINE DAYNXT(DAY,DPY)
C DETERMINES NUMBER OF NEXT DAY OF THE YEAR
INTEGER DAY,DPY
DAY = DAY + 1
IF(DAY .EQ. 366) DAY = 1
IF(DAY .EQ. 60 .AND. DPY .EQ. 366) DAY = 366
IF(DAY .EQ. 367) DAY = 60
RETURN
END

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DYNX0001
DYNX0002
DYNX0003
DYNX0004
DYNX0005
DYNX0006
DYNX0007
DYNX0008
DYNX0009

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SUBROUTINE DAYSUM(DRSF,MEDCY,DPY,ATFV,TMTFWY)	DYSM0001
C SUMS DAILY VALUES TO GET MONTHLY AND ANNUAL TOTALS	DYSM0002
DIMENSION DRSF(366),EMATF(13),MEDCY(12),TMTFCY(12),TMTFWY(12)	DYSM0003
INTEGER DAY,DPY	DYSM0004
C SUM ANNUAL AND CUMULATIVE MONTHLY FLOWS	DYSM0005
EMATF(1) = 0.0	DYSM0006
ATF = 0.0	DYSM0007
DO 101 DAY = 1,365	DYSM0008
ATF = ATF + DRSF(DAY)	DYSM0009
DO 100 KMO = 2,12	DYSM0010
100 IF(DAY .EQ. MEDCY(KMO)) EMATF(KMO) = ATF	DYSM0011
101 CONTINUE	DYSM0012
EMATF(13) = ATF	DYSM0013
ATFV = ATF + DRSF(366)	DYSM0014
C CALCULATE MONTHLY FLOWS	DYSM0015
DO 102 KMO = 1,12	DYSM0016
102 TMTFCY(KMO) = EMATF(KMO + 1) - EMATF(KMO)	DYSM0017
TMTFCY(2) = TMTFCY(2) + DRSF(366)	DYSM0018
C CONVERT MONTHLY FLOWS TO A WATER YEAR ORDER	DYSM0019
DO 103 KMO = 1,9	DYSM0020
103 TMTFWY(KMO+3) = TMTFCY(KMO)	DYSM0021
DO 104 KMO = 10,12	DYSM0022
104 TMTFWY(KMO-9) = TMTFCY(KMO)	DYSM0023
RETURN	DYSM0024
END	DYSM0025
SUBROUTINE DAYOUT(VDCY,MEDWY,DPY)	DYOT0001
C PRINTS TABLE OF DAILY VALUES	DYOT0002
DIMENSION MEDWY(12),VDCY(366),VDMO(12)	DYOT0003
INTEGER DATE,DAY,DPY	DYOT0004
100 WRITE(6,1)	DYOT0005

1	FORMAT(7X,3HDAY,7X,3HOCT,5X,3HNOV,5X,3HDEC,5X,3HJAN,5X,3HFEB,5X,	DYDT0006
1	3HMAR,5X,3HAPR,5X,3HMAY,5X,3HJUN,5X,3HJUL,5X,3HAUG,5X,4HSEPT)	DYDT0007
	MEDWY(3) = 0	DYDT0008
	DO 104 DATE = 1,28,1	DYDT0009
	IF(MOD(DATE,5) .NE. 1) GO TO 102	DYDT0010
	DO 101 KMO = 1,12	DYDT0011
	DAY = MEDWY(KMO) + DATE	DYDT0012
101	VDMO(KMO) = VDCY(DAY)	DYDT0013
	WRITE(6,2) DATE,VDMO(12),(VDMO(KMO), KMO=1,11)	DYDT0014
2	FORMAT(1H0,3X,16,3X,12F8.1)	DYDT0015
	GO TO 104	DYDT0016
102	DO 103 KMO = 1,12	DYDT0017
	DAY = MEDWY(KMO) + DATE	DYDT0018
103	VDMO(KMO) = VDCY(DAY)	DYDT0019
	WRITE(6,3) DATE,VDMO(12),(VDMO(KMO), KMO = 1,11)	DYDT0020
3	FORMAT(1X,3X,16,3X,12F8.1)	DYDT0021
104	CONTINUE	DYDT0022
	IF(DPY .NE. 366) GO TO 106	DYDT0023
	DATE = 29	DYDT0024
	VDCY(60) = VDCY(366)	DYDT0025
	DO 105 KMO = 1,12	DYDT0026
	DAY = MEDWY(KMO) + DATE	DYDT0027
105	VDMO(KMO) = VDCY(DAY)	DYDT0028
	WRITE(6,3) DATE,VDMO(12),(VDMO(KMO), KMO=1,11)	DYDT0029
	GO TO 107	DYDT0030
106	CONTINUE	DYDT0031
	WRITE(6,4) VDCY(302),VDCY(333),VDCY(363),VDCY(29),VDCY(88),	DYDT0032
1	VDCY(119),VDCY(149),VDCY(180),VDCY(210),VDCY(241),VDCY(272)	DYDT0033
4	FORMAT(1X,7X,2H29,3X,4F8.1,8X,7F8.1)	DYDT0034
107	CONTINUE	DYDT0035
108	WRITE(6,5) VDCY(303),VDCY(334),VDCY(364),VDCY(30),VDCY(89),	DYDT0036
1	VDCY(120),VDCY(150),VDCY(181),VDCY(211),VDCY(242),VDCY(273)	DYDT0037
5	FORMAT(1X,7X,2H30,3X,4F8.1,8X,7F8.1)	DYDT0038
	WRITE(6,6) VDCY(304),VDCY(365),VDCY(31),VDCY(90),VDCY(151),	DYDT0039
1	VDCY(212),VDCY(243)	DYDT0040

```
6 FORMAT(1H/,7X,2H31,3X,F8.1,8X,2F8.1,8X,F8.1,8X,F8.1,8X,2F8.1)
MEDWY(3) = 365
RETURN
END
```

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DYOT0041
DYOT0042
DYOT0043
DYOT0044
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```
      SUBROUTINE EVPDAY(DPET, EMAET)
C  DETERMINES DATED PAN EVAPORATION TOTALS
      DIMENSION DPET(366)
      INTEGER DAY
      DO 100 DAY = 1,5
100  DPET(DAY) = 0.00060*EMAET
      DPET( 6) = 0.00059*EMAET
      DPET( 7) = DPET( 6)
      DO 101 DAY = 8,10
101  DPET(DAY) = 0.00058*EMAET
      DO 102 DAY = 11,16
102  DPET(DAY) = 0.00057*EMAET
      DPET( 17) = DPET( 8)
      DO 103 DAY = 18,20
103  DPET(DAY) = DPET( 6)
      DO 104 DAY = 21,32
104  DPET(DAY) = DPET( 1)
      DPET( 33) = 0.00061*EMAET
      DO 105 DAY = 34,38
105  DPET(DAY) = 0.00062*EMAET
      DPET( 39) = 0.00063*EMAET
      DPET( 40) = DPET( 39)
      DPET( 41) = 0.00064*EMAET
      DPET( 42) = 0.00065*EMAET
      DPET( 43) = 0.00066*EMAET
      DO 106 DAY = 44,50
106  DPET(DAY) = 0.00067*EMAET
      DO 107 DAY = 51,55
```

```
EVDY0001
EVDY0002
EVDY0003
EVDY0004
EVDY0005
EVDY0006
EVDY0007
EVDY0008
EVDY0009
EVDY0010
EVDY0011
EVDY0012
EVDY0013
EVDY0014
EVDY0015
EVDY0016
EVDY0017
EVDY0018
EVDY0019
EVDY0020
EVDY0021
EVDY0022
EVDY0023
EVDY0024
EVDY0025
EVDY0026
EVDY0027
EVDY0028
```

107 DPET(DAY) = 0.00068*EMAET
 DPET(56) = 0.00069*EMAET
 DO 108 DAY = 57,61
 108 DPET(DAY) = 0.00070*EMAET
 DPET(62) = 0.00071*EMAET
 DPET(63) = 0.00072*EMAET
 DPET(64) = DPET(63)
 DPET(65) = 0.00073*EMAET
 DPET(66) = 0.00074*EMAET
 DPET(67) = 0.00075*EMAET
 DPET(68) = 0.00076*EMAET
 DPET(69) = 0.00077*EMAET
 DPET(70) = DPET(69)
 DPET(71) = 0.00078*EMAET
 DPET(72) = DPET(71)
 DPET(73) = 0.00079*EMAET
 DPET(74) = DPET(73)
 DPET(75) = 0.00080*EMAET
 DPET(76) = 0.00081*EMAET
 DPET(77) = 0.00082*EMAET
 DPET(78) = 0.00084*EMAET
 DPET(79) = 0.00086*EMAET
 DPET(80) = 0.00088*EMAET
 DPET(81) = 0.00090*EMAET
 DPET(82) = 0.00092*EMAET
 DPET(83) = 0.00094*EMAET
 DPET(84) = 0.00097*EMAET
 DPET(85) = 0.00099*EMAET
 DPET(86) = 0.00102*EMAET
 DPET(87) = 0.00106*EMAET
 DPET(88) = 0.00109*EMAET
 DPET(89) = 0.00113*EMAET
 DPET(90) = 0.00118*EMAET
 DPET(91) = 0.00122*EMAET
 DPET(92) = 0.00128*EMAET

EVDY0029
 EVDY0030
 EVDY0031
 EVDY0032
 EVDY0033
 EVDY0034
 EVDY0035
 EVDY0036
 EVDY0037
 EVDY0038
 EVDY0039
 EVDY0040
 EVDY0041
 EVDY0042
 EVDY0043
 EVDY0044
 EVDY0045
 EVDY0046
 EVDY0047
 EVDY0048
 EVDY0049
 EVDY0050
 EVDY0051
 EVDY0052
 EVDY0053
 EVDY0054
 EVDY0055
 EVDY0056
 EVDY0057
 EVDY0058
 EVDY0059
 EVDY0060
 EVDY0061
 EVDY0062
 EVDY0063

DPET(93) = 0.00132*EMAET
DPET(94) = 0.00137*EMAET
DPET(95) = 0.00142*EMAET
DPET(96) = 0.00147*EMAET
DPET(97) = 0.00151*EMAET
DPET(98) = 0.00157*EMAET
DPET(99) = 0.00163*EMAET
DPET(100) = 0.00168*EMAET
DPET(101) = 0.00173*EMAET
DPET(102) = 0.00178*EMAET
DPET(103) = 0.00185*EMAET
DPET(104) = 0.00193*EMAET
DPET(105) = 0.00201*EMAET
DPET(106) = 0.00208*EMAET
DPET(107) = 0.00214*EMAET
DPET(108) = 0.00221*EMAET
DPET(109) = 0.00227*EMAET
DPET(110) = 0.00234*EMAET
DPET(111) = 0.00241*EMAET
DPET(112) = 0.00249*EMAET
DPET(113) = 0.00256*EMAET
DPET(114) = 0.00262*EMAET
DPET(115) = 0.00268*EMAET
DPET(116) = 0.00276*EMAET
DPET(117) = 0.00281*EMAET
DPET(118) = 0.00287*EMAET
DPET(119) = 0.00293*EMAET
DPET(120) = 0.00299*EMAET
DPET(121) = 0.00305*EMAET
DPET(122) = 0.00310*EMAET
DPET(123) = 0.00317*EMAET
DPET(124) = 0.00322*EMAET
DPET(125) = 0.00328*EMAET
DPET(126) = 0.00333*EMAET
DPET(127) = 0.00338*EMAET

EVDY0064
EVDY0065
EVDY0066
EVDY0067
EVDY0068
EVDY0069
EVDY0070
EVDY0071
EVDY0072
EVDY0073
EVDY0074
EVDY0075
EVDY0076
EVDY0077
EVDY0078
EVDY0079
EVDY0080
EVDY0081
EVDY0082
EVDY0083
EVDY0084
EVDY0085
EVDY0086
EVDY0087
EVDY0088
EVDY0089
EVDY0090
EVDY0091
EVDY0092
EVDY0093
EVDY0094
EVDY0095
EVDY0096
EVDY0097
EVDY0098

DPET(128) = 0.00344*EMAET
DPET(129) = 0.00348*EMAET
DPET(130) = 0.00354*EMAET
DPET(131) = 0.00359*EMAET
DPET(132) = 0.00365*EMAET
DPET(133) = 0.00370*EMAET
DPET(134) = 0.00374*EMAET
DPET(135) = 0.00378*EMAET
DPET(136) = 0.00382*EMAET
DPET(137) = 0.00387*EMAET
DPET(138) = 0.00391*EMAET
DPET(139) = 0.00394*EMAET
DPET(140) = 0.00399*EMAET
DPET(141) = 0.00402*EMAET
DPET(142) = 0.00407*EMAET
DPET(143) = 0.00411*EMAET
DPET(144) = 0.00417*EMAET
DPET(145) = 0.00420*EMAET
DPET(146) = 0.00426*EMAET
DPET(147) = 0.00430*EMAET
DPET(148) = 0.00436*EMAET
DPET(149) = 0.00440*EMAET
DPET(150) = 0.00446*EMAET
DPET(151) = 0.00450*EMAET
DPET(152) = 0.00455*EMAET
DPET(153) = 0.00460*EMAET
DPET(154) = 0.00466*EMAET
DPET(155) = 0.00470*EMAET
DPET(156) = 0.00473*EMAET
DPET(157) = 0.00478*EMAET
DPET(158) = 0.00482*EMAET
DPET(159) = 0.00487*EMAET
DPET(160) = 0.00491*EMAET
DPET(161) = 0.00495*EMAET
DPET(162) = 0.00500*EMAET

EVDY0099
EVDY0100
EVDY0101
EVDY0102
EVDY0103
EVDY0104
EVDY0105
EVDY0106
EVDY0107
EVDY0108
EVDY0109
EVDY0110
EVDY0111
EVDY0112
EVDY0113
EVDY0114
EVDY0115
EVDY0116
EVDY0117
EVDY0118
EVDY0119
EVDY0120
EVDY0121
EVDY0122
EVDY0123
EVDY0124
EVDY0125
EVDY0126
EVDY0127
EVDY0128
EVDY0129
EVDY0130
EVDY0131
EVDY0132
EVDY0133

DPET(163) = 0.00504*EMAET
DPET(164) = 0.00508*EMAET
DPET(165) = 0.00510*EMAET
DPET(166) = 0.00512*EMAET
DPET(167) = 0.00514*EMAET
DPET(168) = 0.00515*EMAET
DPET(169) = 0.00517*EMAET
DPET(170) = 0.00519*EMAET
DPET(171) = 0.00520*EMAET
DPET(172) = 0.00521*EMAET
DPET(173) = DPET(172)
DPET(174) = DPET(172)
DPET(175) = 0.00522*EMAET
DPET(176) = 0.00523*EMAET
DPET(177) = 0.00524*EMAET
DPET(178) = 0.00525*EMAET
DPET(179) = 0.00527*EMAET
DPET(180) = 0.00528*EMAET
DPET(181) = DPET(180)
DPET(182) = 0.00529*EMAET
DPET(183) = 0.00530*EMAET
DPET(184) = DPET(183)
DPET(185) = 0.00531*EMAET
DPET(186) = 0.00532*EMAET
DPET(187) = 0.00533*EMAET
DPET(188) = 0.00534*EMAET
DPET(189) = DPET(188)
DPET(190) = 0.00535*EMAET
DPET(191) = 0.00536*EMAET
DPET(192) = 0.00537*EMAET
DPET(193) = 0.00538*EMAET
DPET(194) = DPET(193)
DPET(195) = 0.00539*EMAET
DPET(196) = 0.00540*EMAET
DPET(197) = DPET(196)

EVDY0134
EVDY0135
EVDY0136
EVDY0137
EVDY0138
EVDY0139
EVDY0140
EVDY0141
EVDY0142
EVDY0143
EVDY0144
EVDY0145
EVDY0146
EVDY0147
EVDY0148
EVDY0149
EVDY0150
EVDY0151
EVDY0152
EVDY0153
EVDY0154
EVDY0155
EVDY0156
EVDY0157
EVDY0158
EVDY0159
EVDY0160
EVDY0161
EVDY0162
EVDY0163
EVDY0164
EVDY0165
EVDY0166
EVDY0167
EVDY0168

DPET(198) = 0.00541*EMAET	EVDY0169
DPET(199) = 0.00542*EMAET	EVDY0170
DPET(200) = 0.00543*EMAET	EVDY0171
DPET(201) = 0.00545*EMAET	EVDY0172
DPET(202) = 0.00546*EMAET	EVDY0173
DPET(203) = 0.00547*EMAET	EVDY0174
DPET(204) = 0.00548*EMAET	EVDY0175
DPET(205) = 0.00549*EMAET	EVDY0176
DPET(206) = 0.00550*EMAET	EVDY0177
DPET(207) = 0.00551*EMAET	EVDY0178
DPET(208) = 0.00552*EMAET	EVDY0179
DPET(209) = 0.00553*EMAET	EVDY0180
DPET(210) = 0.00555*EMAET	EVDY0181
DPET(211) = 0.00557*EMAET	EVDY0182
DPET(212) = 0.00558*EMAET	EVDY0183
DPET(213) = 0.00560*EMAET	EVDY0184
DPET(214) = DPET(213)	EVDY0185
DPET(215) = 0.00561*EMAET	EVDY0186
DPET(216) = 0.00562*EMAET	EVDY0187
DPET(217) = 0.00563*EMAET	EVDY0188
DPET(218) = 0.00565*EMAET	EVDY0189
DPET(219) = 0.00567*EMAET	EVDY0190
DPET(220) = DPET(219)	EVDY0191
DO 109 DAY = 221,226	EVDY0192
109 DPET(DAY) = 0.00568*EMAET	EVDY0193
DO 110 DAY = 227,229	EVDY0194
110 DPET(DAY) = DPET(219)	EVDY0195
DPET(230) = 0.00566*EMAET	EVDY0196
DPET(231) = 0.00564*EMAET	EVDY0197
DPET(232) = DPET(217)	EVDY0198
DPET(233) = DPET(216)	EVDY0199
DPET(234) = DPET(213)	EVDY0200
DPET(235) = 0.00559*EMAET	EVDY0201
DPET(236) = DPET(211)	EVDY0202
DPET(237) = DPET(210)	EVDY0203

DPET(238) = DPET(209)
DPET(239) = DPET(206)
DPET(240) = DPET(203)
DPET(241) = DPET(199)
DPET(242) = DPET(193)
DPET(243) = DPET(190)
DPET(244) = DPET(185)
DPET(245) = DPET(179)
DPET(246) = DPET(175)
DPET(247) = DPET(169)
DPET(248) = 0.00511*EMAET
DPET(249) = DPET(163)
DPET(250) = 0.00497*EMAET
DPET(251) = 0.00490*EMAET
DPET(252) = DPET(158)
DPET(253) = 0.00476*EMAET
DPET(254) = 0.00468*EMAET
DPET(255) = 0.00461*EMAET
DPET(256) = 0.00454*EMAET
DPET(257) = DPET(150)
DPET(258) = 0.00437*EMAET
DPET(259) = 0.00427*EMAET
DPET(260) = 0.00418*EMAET
DPET(261) = DPET(142)
DPET(262) = 0.00397*EMAET
DPET(263) = DPET(137)
DPET(264) = 0.00377*EMAET
DPET(265) = 0.00367*EMAET
DPET(266) = 0.00356*EMAET
DPET(267) = 0.00347*EMAET
DPET(268) = 0.00337*EMAET
DPET(269) = 0.00329*EMAET
DPET(270) = DPET(124)
DPET(271) = 0.00315*EMAET
DPET(272) = 0.00308*EMAET

EVDY0204
EVDY0205
EVDY0206
EVDY0207
EVDY0208
EVDY0209
EVDY0210
EVDY0211
EVDY0212
EVDY0213
EVDY0214
EVDY0215
EVDY0216
EVDY0217
EVDY0218
EVDY0219
EVDY0220
EVDY0221
EVDY0222
EVDY0223
EVDY0224
EVDY0225
EVDY0226
EVDY0227
EVDY0228
EVDY0229
EVDY0230
EVDY0231
EVDY0232
EVDY0233
EVDY0234
EVDY0235
EVDY0236
EVDY0237
EVDY0238

DPET(273) = 0.00303*EMAET
DPET(274) = 0.00300*EMAET
DPET(275) = 0.00298*EMAET
DPET(276) = 0.00294*EMAET
DPET(277) = 0.00290*EMAET
DPET(278) = 0.00286*EMAET
DPET(279) = 0.00283*EMAET
DPET(280) = 0.00279*EMAET
DPET(281) = DPET(116)
DPET(282) = 0.00271*EMAET
DPET(283) = DPET(115)
DPET(284) = DPET(114)
DPET(285) = 0.00259*EMAET
DPET(286) = 0.00254*EMAET
DPET(287) = 0.00252*EMAET
DPET(288) = 0.00247*EMAET
DPET(289) = 0.00244*EMAET
DPET(290) = 0.00239*EMAET
DPET(291) = DPET(110)
DPET(292) = 0.00230*EMAET
DPET(293) = 0.00225*EMAET
DPET(294) = 0.00222*EMAET
DPET(295) = 0.00217*EMAET
DPET(296) = 0.00213*EMAET
DPET(297) = 0.00210*EMAET
DPET(298) = 0.00206*EMAET
DPET(299) = 0.00200*EMAET
DPET(300) = 0.00197*EMAET
DPET(301) = 0.00194*EMAET
DPET(302) = 0.00189*EMAET
DPET(303) = 0.00186*EMAET
DPET(304) = 0.00183*EMAET
DPET(305) = 0.00180*EMAET
DPET(306) = 0.00177*EMAET
DPET(307) = 0.00174*EMAET

EVDY0239
EVDY0240
EVDY0241
EVDY0242
EVDY0243
EVDY0244
EVDY0245
EVDY0246
EVDY0247
EVDY0248
EVDY0249
EVDY0250
EVDY0251
EVDY0252
EVDY0253
EVDY0254
EVDY0255
EVDY0256
EVDY0257
EVDY0258
EVDY0259
EVDY0260
EVDY0261
EVDY0262
EVDY0263
EVDY0264
EVDY0265
EVDY0266
EVDY0267
EVDY0268
EVDY0269
EVDY0270
EVDY0271
EVDY0272
EVDY0273

DPET(308) = 0.00172*EMAET
DPET(309) = DPET(100)
DPET(310) = DPET(99)
DPET(311) = 0.00160*EMAET
DPET(312) = 0.00156*EMAET
DPET(313) = 0.00152*EMAET
DPET(314) = 0.00149*EMAET
DPET(315) = 0.00146*EMAET
DPET(316) = DPET(95)
DPET(317) = 0.00138*EMAET
DPET(318) = 0.00135*EMAET
DPET(319) = 0.00131*EMAET
DPET(320) = 0.00127*EMAET
DPET(321) = 0.00124*EMAET
DPET(322) = 0.00120*EMAET
DPET(323) = DPET(90)
DPET(324) = 0.00116*EMAET
DPET(325) = DPET(89)
DPET(326) = 0.00110*EMAET
DPET(327) = 0.00107*EMAET
DPET(328) = 0.00104*EMAET
DPET(329) = DPET(86)
DPET(330) = 0.00100*EMAET
DPET(331) = 0.00098*EMAET
DPET(332) = 0.00097*EMAET
DPET(333) = 0.00095*EMAET
DPET(334) = 0.00093*EMAET
DPET(335) = DPET(81)
DPET(336) = DPET(80)
DPET(337) = 0.00087*EMAET
DPET(338) = DPET(79)
DPET(339) = DPET(78)
DPET(340) = DPET(77)
DPET(341) = DPET(75)
DPET(342) = DPET(73)

EVDY0274
EVDY0275
EVDY0276
EVDY0277
EVDY0278
EVDY0279
EVDY0280
EVDY0281
EVDY0282
EVDY0283
EVDY0284
EVDY0285
EVDY0286
EVDY0287
EVDY0288
EVDY0289
EVDY0290
EVDY0291
EVDY0292
EVDY0293
EVDY0294
EVDY0295
EVDY0296
EVDY0297
EVDY0298
EVDY0299
EVDY0300
EVDY0301
EVDY0302
EVDY0303
EVDY0304
EVDY0305
EVDY0306
EVDY0307
EVDY0308

DPET(343) = DPET(71)	EVDY0309
DPET(344) = DPET(71)	EVDY0310
DPET(345) = DPET(69)	EVDY0311
DPET(346) = DPET(68)	EVDY0312
DPET(347) = DPET(66)	EVDY0313
DPET(348) = DPET(63)	EVDY0314
DPET(349) = DPET(62)	EVDY0315
DPET(350) = DPET(57)	EVDY0316
DPET(351) = DPET(57)	EVDY0317
DPET(352) = DPET(56)	EVDY0318
DO 111 DAY = 353,355	EVDY0319
111 DPET(DAY) = DPET(51)	EVDY0320
DPET(356) = DPET(44)	EVDY0321
DPET(357) = DPET(44)	EVDY0322
DPET(358) = DPET(42)	EVDY0323
DPET(359) = DPET(41)	EVDY0324
DPET(360) = DPET(39)	EVDY0325
DPET(361) = DPET(34)	EVDY0326
DPET(362) = DPET(33)	EVDY0327
DO 112 DAY = 363,365	EVDY0328
112 DPET(DAY) = DPET(1)	EVDY0329
DPET(366) = DPET(57)	EVDY0330
RETURN	EVDY0331
END	EVDY0332

SUBROUTINE PRECHK(DRGPM,DRHP,DRSF,VWIN,SGRT,NATRH)	PRCK0001
C CHECKS PRECIPITATION-STREAMFLOW ANOMALIES AND ADJUSTS PRECIPITATION	PRCK0002
C WHERE NECESSARY	PRCK0003
DIMENSION DRGPM(366),DRHP(366,24),DRSF(366)	PRCK0004
INTEGER DAY,HOUR,SGRT	PRCK0005
AHP = 0.0	PRCK0006
NRHA = 24 - NATRH	PRCK0007
RGPM = DRGPM(90)	PRCK0008

DAY = 90	PRCK0009
RMWR = 1.25	PRCK0010
100 DAY = DAY + 1	PRCK0011
IF(DAY .GT. 200 .OR. VWIN .GT. 750.0) RMWR = 2.00	PRCK0012
RFRISE = (DRSF(DAY) - DRSF(DAY-1))/VWIN	PRCK0013
GO 101 HOUR = 1,24	PRCK0014
IF(HOUR .EQ. SGRT+1) RGPM = DRGPM(DAY)	PRCK0015
AHP = AHP + DRHP(DAY,HOUR)*RGPM	PRCK0016
IF(HOUR .NE. NRHA) GO TO 101	PRCK0017
RWRAIN = AHP	PRCK0018
AHP = 0.0	PRCK0019
101 CONTINUE	PRCK0020
IF(RFRISE .GT. RWRAIN .AND. RFRISE .GT. 0.1) GO TO 102	PRCK0021
IF((RWRAIN .GT. RMWR .AND. RFRISE .LT. 0.02*RWRAIN) .OR. (RWRAIN	PRCK0022
1 .GT. 3.00 .AND. RFRISE .LT. 0.05*RWRAIN)) GO TO 104	PRCK0023
GO TO 108	PRCK0024
102 IF(RWRAIN .GT. 0.05) GO TO 103	PRCK0025
RAA = RFRISE*2.0 - RWRAIN + 1.0	PRCK0026
DRHP(DAY,12) = RAA	PRCK0027
WRITE(6,1) DAY, RAA	PRCK0028
1 FORMAT(/10X, 'FOR DAY',I4,1X, 'RAIN ADDED OF',F7.2)	PRCK0029
GO TO 108	PRCK0030
103 RAM = 2.0*RFRISE/RWRAIN	PRCK0031
GO TO 105	PRCK0032
104 RAM = 10.0*RFRISE/RWRAIN	PRCK0033
105 IF(RAM .LT. 0.0) GO TO 108	PRCK0034
WRITE(6,2) DAY,RAM,RWRAIN	PRCK0035
2 FORMAT(/5X, 'FOR DAY',I4,1X, 'RAIN ADJUSTMENT MULTIPLIER IS',F8.4,	PRCK0036
1 1X, 'RECORDED RAIN IS',F7.2)	PRCK0037
DO 106 HOUR = 1,NRHA	PRCK0038
106 DRHP(DAY,HOUR) = DRHP(DAY,HOUR)*RAM	PRCK0039
IF(NATRH .EQ. 0) GO TO 108	PRCK0040
NFRHA = NRHA + 1	PRCK0041
DO 107 HOUR = NFRHA,24	PRCK0042
107 DRHP(DAY-1,HOUR) = DRHP(DAY-1,HOUR)*RAM	PRCK0043

108 IF(DAY .NE. 273) GO TO 100
RETURN
END

PRCK0044
PRCK0045
PRCK0046

SUBROUTINE PREPRD(RGPM,DRHP,DAY,HOUR,DPY,PRD,PEP,PRH)
C DIVIDES HOURLY PRECIPITATION TOTALS AMONG PERIODS FOR SMALL BASINS
DIMENSION DRHP(366,24), PE4P(4)
INTEGER DAY,DPY,HOUR,PRD
PEP = 0.0
IF(PRH .EQ. 0.0) RETURN
IF(PRD .EQ. 1) GO TO 100
PEP = PE4P(PRD)
RETURN
100 LHOOR = HOUR - 1
LDAY = DAY
IF(LHOOR .GE. 1) GO TO 101
LHOOR = 24
LDAY = DAY - 1
IF(LDAY .EQ. 0) LDAY = 365
IF(LDAY .EQ. 365) LDAY = 59
IF(LDAY .EQ. 59 .AND. DPY .EQ. 366) LDAY = 366
101 PRLH = RGPM*DRHP(LDAY,LHOOR)
NHOOR = HOUR + 1
NDAY = DAY
IF(NHOOR .LE. 24) GO TO 102
NHOOR = 1
CALL DAYNXT(NDAY,DPY)
102 PRNH = RGPM*DRHP(NDAY,NHOOR)
IF(PRH .GT. PRLH .AND. PRH .GT. PRNH) GO TO 103
GO TO 104
103 PE4P(1) = 0.10
PE4P(2) = 0.28
PE4P(3) = 0.46

PREP0001
PREP0002
PREP0003
PREP0004
PREP0005
PREP0006
PREP0007
PREP0008
PREP0009
PREP0010
PREP0011
PREP0012
PREP0013
PREP0014
PREP0015
PREP0016
PREP0017
PREP0018
PREP0019
PREP0020
PREP0021
PREP0022
PREP0023
PREP0024
PREP0025
PREP0026
PREP0027
PREP0028
PREP0029

PE4P(4) = 0.16	PREP0030
GO TO 108	PREP0031
104 IF(PRH .LT. PRLH .AND. PRH .LT. PRNH) GO TO 105	PREP0032
GO TO 106	PREP0033
105 PE4P(1) = 0.28	PREP0034
PE4P(2) = 0.10	PREP0035
PE4P(3) = 0.16	PREP0036
PE4P(4) = 0.46	PREP0037
GO TO 108	PREP0038
106 IF(PRNH .GE. PRLH) GO TO 107	PREP0039
PE4P(1) = 0.46	PREP0040
PE4P(2) = 0.16	PREP0041
PE4P(3) = 0.28	PREP0042
PE4P(4) = 0.10	PREP0043
GO TO 108	PREP0044
107 PE4P(1) = 0.10	PREP0045
PE4P(2) = 0.28	PREP0046
PE4P(3) = 0.16	PREP0047
PE4P(4) = 0.46	PREP0048
108 UD 109 KPRD = 1,4	PREP0049
109 PE4P(KPRD) = PE4P(KPRD)*PRH	PREP0050
PEP = PE4P(1)	PREP0051
RETURN	PREP0052
END	PREP0053

SUBROUTINE RECESS(DRSF,DPY,EFRC,IFRC,AREA,RSBD,RSBIF,NRS,RSBBF)	RCSS0001
C ESTABLISHES RECESSION SEQUENCES	RCSS0002
DIMENSION DRSF(366),LBFO(20),NDRS(20),RSBBF(20),RSBD(20),	RCSS0003
1 RSEFRC(20),RSBIF(20),RSIFRC(20),RSTF(50,20)	RCSS0004
LOGICAL LBFO	RCSS0005
INTEGER DAY,DPY,RSBD,RSL	RCSS0006
REAL IFRC	RCSS0007
MRSL = 9	RCSS0008

BFRC = 0.9	RCSS0009
IFRC = 0.05	RCSS0010
FRERS = 0.1*SQRT(AREA)	RCSS0011
100 DO 101 KSD = 1,50	RCSS0012
DO 101 KRS = 1,20	RCSS0013
101 RSTF(KSD,KRS) = 0.0	RCSS0014
KRS = 0	RCSS0015
DAY = 274	RCSS0016
C BEGIN NFW SEQUENCE	RCSS0017
102 IF(KRS .GE. 20) GO TO 109	RCSS0018
KRS = KRS + 1	RCSS0019
KSD = 1	RCSS0020
RSF1 = DRSF(DAY)	RCSS0021
CALL DAYNXT(DAY,DPY)	RCSS0022
IF(DAY .EQ. 274) GO TO 107	RCSS0023
RSF2 = DRSF(DAY)	RCSS0024
RSBD(KRS) = DAY	RCSS0025
IF(RSF2 .LT. RSF1+FRERS .AND. (RSF2 .GT. 0.4*AREA .OR. RSF2 .GT.	RCSS0026
1 10.0)) GO TO 103	RCSS0027
KRS = KRS - 1	RCSS0028
GO TO 102	RCSS0029
103 RSTF(1,KRS) = RSF2	RCSS0030
RSFM = RSF2	RCSS0031
104 KSD = KSD + 1	RCSS0032
CALL DAYNXT(DAY,DPY)	RCSS0033
IF(DAY .EQ. 274) GO TO 107	RCSS0034
RSFN = DRSF(DAY)	RCSS0035
IF(RSFN .LT. (RSFM + FRERS) .AND. RSFN .GT. 0.0) GO TO 106	RCSS0036
IF(KSD .GE. MRSL) GO TO 102	RCSS0037
NDRS(KRS) = 0	RCSS0038
DO 105 KSD = 1,MRSL	RCSS0039
105 RSTF(KSD,KRS) = 0.0	RCSS0040
KRS = KRS - 1	RCSS0041
GO TO 102	RCSS0042
106 IF(RSFN .LT. RSFM) RSFM = RSFN	RCSS0043

RSTF(KSD,KRS) = RSFN	RCSS0044
NDRS(KRS) = KSD	RCSS0045
IF(KSD .GE. 50) GO TO 102	RCSS0046
GO TO 104	RCSS0047
107 IF(KSD .GE. MRSL) GO TO 109	RCSS0048
NTRS = KRS - 1	RCSS0049
DO 108 KSD = 1,MRSL	RCSS0050
108 RSTF(KSD,KRS) = 0.0	RCSS0051
GO TO 110	RCSS0052
109 NTRS = KRS	RCSS0053
110 CONTINUE	RCSS0054
IF(NTRS .GF. 3) GO TO 111	RCSS0055
IF(MRSL .LT. 7) RETURN	RCSS0056
MRSL = 6	RCSS0057
GO TO 100	RCSS0058
C WRITE OUT ESTABLISHED ARRAY OF FLOW SEQUENCES	RCSS0059
111 WRITE(6,1)	RCSS0060
1 FORMAT(/5X,'FLOW SEQUENCES USED TO ESTIMATE RECESSION CONSTANTS')	RCSS0061
DO 113 KRS = 1,NTRS	RCSS0062
NDRSC = NDRS(KRS)	RCSS0063
DO 112 KSD = 2,NDRSC	RCSS0064
112 RSTF(KSD-1,KRS) = RSTF(KSD,KRS)	RCSS0065
NDRS(KRS) = NDRS(KRS) - 1	RCSS0066
NDRSC = NDRSC - 1	RCSS0067
WRITE(6,2) KRS,(RSTF(KSD,KRS),KSD=1,NDRSC)	RCSS0068
2 FORMAT(/10X,I2,5(10F8.1/12X))	RCSS0069
113 CONTINUE	RCSS0070
C DETERMINE RECESSION CONSTANTS FROM EACH SEQUENCE	RCSS0071
114 DO 116 KRS = 1,NTRS	RCSS0072
IF((RSTF(1,KRS) .LT. 0.4*AREA) .AND. (RSTF(2,KRS) .GT. 0.8*	RCSS0073
1 (RSTF(1,KRS)))) GO TO 115	RCSS0074
LBFO(KRS) = .FALSE.	RCSS0075
CALL SET2RC(RSTF,KRS,NDRS(KRS),RSIFRC(KRS),RSBFRC(KRS),LBFO(KRS))	RCSS0076
IF(LBFO(KRS) .OR. RSBFRC(KRS) .GT. 1.2 .OR. RSBFRC(KRS) .LT. 0.6	RCSS0077
1 .OR. RSIFRC(KRS) .GT. 0.8 .OR. RSIFRC(KRS) .LT. -0.4) GO TO 115	RCSS0078

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GO TO 116
115 LBFD(KRS) = .TRUE.
    CALL SETIRC(RSTF,KRS,NDRS(KRS),RSBFRC(KRS))
116 CONTINUE
C  CALCULATE WEIGHTED AVERAGE RECESSION CONSTANTS
    BFRC = 0.0
    IFRC = 0.0
    ABFSL = 0.0
    AIFSL = 0.0
    DO 118 KRS = 1,NTRS
    IF(RSBFRC(KRS) .GT. 1.2 .OR. RSBFRC(KRS) .LT. 0.7) GO TO 117
    RSL = NDRS(KRS)
    BFRC = BFRC + RSBFRC(KRS)*RSL
    ABFSL = ABFSL + RSL
    IF(LBFD(KRS)) GO TO 118
    IF(RSL .GE. 20.0) RSL = 20.0
    IFRC = IFRC + RSIFRC(KRS)*RSL
    AIFSL = AIFSL + RSL
    GO TO 116
117 WRITE(6,3) KRS
    3 FORMAT(10X,'SEQUENCE',13,1X,'OMITTED IN AVERAGING')
118 CONTINUE
    WRITE(6,4) ABFSL,AIFSL
    4 FORMAT(10X,'BASE FLOW DAYS =',F5.0,2X,'INTERFLOW DAYS =',F5.0)
    BFRC = BFRC/ABFSL
    IFRC = IFRC/AIFSL
    IF(BFRC .GT. 0.99) BFRC = 0.99
    IF(BFRC .LT. 0.70) BFRC = 0.70
    KSQ = 0
    DO 119 KRS = 1,NTRS
    IF(LBFD(KRS)) GO TO 119
    CALL SETRBF(RSTF,NDRS,KRS,BFRC,IFRC,CRSBIF,CRSBBF)
    IF(CRSBIF .GT. 95000.0 .OR. CRSBBF .LT. 0.0) GO TO 119
    IF(CRSBIF .LT. 0.0) CRSBIF = 0.0
    KSQ = KSQ + 1

```

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RCSS0079
RCSS0080
RCSS0081
RCSS0082
RCSS0083
RCSS0084
RCSS0085
RCSS0086
RCSS0087
RCSS0088
RCSS0089
RCSS0090
RCSS0091
RCSS0092
RCSS0093
RCSS0094
RCSS0095
RCSS0096
RCSS0097
RCSS0098
RCSS0099
RCSS0100
RCSS0101
RCSS0102
RCSS0103
RCSS0104
RCSS0105
RCSS0106
RCSS0107
RCSS0108
RCSS0109
RCSS0110
RCSS0111
RCSS0112
RCSS0113

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RSBD(KSQ) = RSBD(KRS)	RCSS0114
RSBIF(KSQ) = CRSBIF	RCSS0115
RSBBF(KSQ) = CRSBBF	RCSS0116
119 CONTINUE	RCSS0117
NRS = KSQ	RCSS0118
DO 120 KSQ = 1,NRS	RCSS0119
DAY = RSBD(KSQ)	RCSS0120
CALL DAYNXT(DAY,DPY)	RCSS0121
120 RSBD(KSQ) = DAY	RCSS0122
DO 121 KSQ = 1,NRS	RCSS0123
CRSBTF = RSBIF(KSQ) + RSBBF(KSQ)	RCSS0124
121 WRITE(6,5) KSQ,RSBD(KSQ),RSBIF(KSQ),RSBBF(KSQ),CRSBTF,	RCSS0125
5 FORMAT(/10X,'REVISED FLOW SEQUENCE',I3,1X,1X,'BEGINS ON DAY',I4,	RCSS0126
1 1X,'AT INTERFLOW =',F7.2,1X,'CFS, BASE FLOW =',F7.2,1X,'CFS,	RCSS0127
1 TOTAL =',F7.2,1X,'CFS')	RCSS0128
RETURN	RCSS0129
END	RCSS0130

SUBROUTINE SETZRC(RSTF,KRS,NDRSC,IFRC,BFRC,LBFO)	ST2R0001
C SETS BEST VALUES FOR TWO RECESSION CONSTANTS	ST2R0002
DIMENSION RSTF(50,20)	ST2R0003
LOGICAL LBFO	ST2R0004
REAL IFRC	ST2R0005
REAL*8 RA1,RA2,RA3,RA4,RA5,CRSTF(50),RA6,DBFRC,DIFRC,RA,RR,RD	ST2R0006
DO 100 KSD = 1,NDRSC	ST2R0007
100 CRSTF(KSD) = RSTF(KSD,KRS)	ST2R0008
NDRSC2 = NDRSC - 2	ST2R0009
RA1 = 0.0	ST2R0010
RA2 = 0.0	ST2R0011
RA3 = 0.0	ST2R0012
DO 101 KSD = 1,NDRSC2	ST2R0013
RA1 = RA1 + CRSTF(KSD)**2	ST2R0014
RA2 = RA2 + CRSTF(KSD)*CRSTF(KSD+1)	ST2R0015

101	RA3 = RA3 + CRSTF(KSD)*CRSTF(KSD+2)	ST2R0016
	RA4 = RA1 + CRSTF(NDRSC-1)**2 - CRSTF(1)**2	ST2R0017
	RA5 = RA2 + CRSTF(NDRSC-1)*CRSTF(NDRSC) - CRSTF(1)*CRSTF(2)	ST2R0018
	RA6 = RA4*RA1 - RA2**2	ST2R0019
	IF(RA6 .EQ. 0.0) GO TO 102	ST2R0020
	RA5 = RA5/RA6	ST2R0021
	RA3 = RA3/RA6	ST2R0022
	RA = RA1*RA5 - RA2*RA3	ST2R0023
	RB = RA4*RA3 - RA2*RA5	ST2R0024
	KD = RA**2 + 4.0*RB	ST2R0025
	IF(RD .LT. 0.0) GO TO 102	ST2R0026
	DBFRC = (RA + KD**0.5)/2.0	ST2R0027
	DIFRC = RA - DBFRC	ST2R0028
	BFRC = DBFRC	ST2R0029
	IFRC = DIFRC	ST2R0030
	WRITE(6,1) KRS,BFRC,IFRC	ST2R0031
	1 FORMAT(15X,'KRS =',I3,5X,'BFRC =',F8.4,5X,'IFRC =',F8.4)	ST2R0032
	GO TO 103	ST2R0033
102	LBFO = .TRUE.	ST2R0034
	WRITE(6,2) KRS	ST2R0035
	2 FORMAT(/15X,'IMAGINARY VALUES ENCOUNTERED IN SET2RC, SEQUENCE =',	ST2R0036
	1 I3)	ST2R0037
103	RETURN	ST2R0038
	END	ST2R0039

	SUBROUTINE SETIRC(RSTF,KRS,NDRSC,BFRC)	ST1R0001
C	SETS BEST VALUE FOR ONE REFESSION CONSTANT	ST1R0002
	DIMENSION RSTF(50,20)	ST1R0003
	RA1 = 0.0	ST1R0004
	KA2 = 0.0	ST1R0005
	NDRSC1 = NDRSC - 1	ST1R0006
	DO 100 KSD = 1,NDRSC1	ST1R0007
	KA1 = RA1 + RSTF(KSD,KRS)**2	ST1R0008

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100 RA2 = RA2 + RSTF(KSD,KRS)*RSTF(KSD+1,KRS)
MFRC = RA2/RA1
WRITE(6,1) KRS,BFRC
1 FORMAT(15X,'KRS =',I3,5X,'BFRC =',F8.4)
RETURN
END

```

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STIR0009
STIR0010
STIR0011
STIR0012
STIR0013
STIR0014

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SUBROUTINE SETFVP(LZC,SUZC,ETLF,BUZZ,SIAC,TMSTF,TMRTF,TMPREC,
1 TMPET,EMLZS,SSQM,LRC,XMPFT,FTX,NOFM,LBUZZ,LETLF,LLZC,APREC,APET)
C SETS BEST VALUES OF FLOW VOLUME PARAMETERS
DIMENSION EMLZS(12),MFDP(12),MXA(12),TMPET(12),TMPREC(12),
1 TMRTF(12),TMSTF(12),XMPFT(12)
LOGICAL LBUZZ,LETLF,LLZC,LRC
REAL LZC,MFDP
CALL SETFDI(MFDP,TMSTF,TMRTF,SSQM)
IF((MFDP(2) + MFDP(3)) .GT. 2.0 .AND. FTX .LT. 1.05) FTX = 0.9
IF((MFDP(2) + MFDP(3)) .LT. -2.0 .AND. FTX .GT. 0.95) FTX = 1.1
C ADJUSTMENT OF LZC BASED ON MONTHS WHERE OVER HALF OF TOTAL
C SYNTHESIZED RUNOFF IS OVERLAND FLOW, MINIMUM OF TWO MONTHS
C WITH GREATEST RUNOFF USED
PLZC = LZC
FNOFM = NOFM
IF(FNOFM .GT. 2) GO TO 103
M1R = 2
M2R = 1
IF(TMRTF(2) .GT. TMRTF(1)) GO TO 100
M1R = 1
M2R = 2
100 DO 102 MONTH = 3,12
IF(TMRTF(MONTH) .LT. TMRTF(M2R)) GO TO 102
IF(TMRTF(MONTH) .GT. TMRTF(M1R)) GO TO 101
M2R = MONTH
GO TO 102

```

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STFV0001
STFV0002
STFV0003
STFV0004
STFV0005
STFV0006
STFV0007
STFV0008
STFV0009
STFV0010
STFV0011
STFV0012
STFV0013
STFV0014
STFV0015
STFV0016
STFV0017
STFV0018
STFV0019
STFV0020
STFV0021
STFV0022
STFV0023
STFV0024
STFV0025
STFV0026

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101 M2R = M1R	STFV0027
M1R = MONTH	STFV0028
102 CONTINUE	STFV0029
IF(LLZC) GO TO 106	STFV0030
FLZC = (MFDP(M1R) + MFDP(M2R))/2.0	STFV0031
GO TO 105	STFV0032
103 SOFMD = 0.0	STFV0033
KM1 = 0	STFV0034
DO 104 MONTH = 1,12	STFV0035
IF(XMPFT(MONTH) .LT. 1.5) GO TO 104	STFV0036
SOFMD = SOFMD + MFDP(MONTH)	STFV0037
KM1 = KM1 + 1	STFV0038
MXA(KM1) = MONTH	STFV0039
104 CONTINUE	STFV0040
FLZC = SOFMD/(FNQFM*0.75)	STFV0041
105 IF(FLZC .GT. 1.0) FLZC = 1.0	STFV0042
IF(FLZC .LT. -1.0) FLZC = -1.0	STFV0043
IF(FLZC .GT. 0.0) LZC = (FLZC + 1.0)*LZC	STFV0044
IF(FLZC .LE. 0.0) LZC = LZC/(1.0 + ABS(FLZC))	STFV0045
IF(NQFM .LE. 2) WRITE(6,1) LZC,M1R,M2R	STFV0046
1 FORMAT(/5X,'LZC WAS CHANGED TO',F6.2,' BASED ON MONTHS',2I3)	STFV0047
IF(NQFM .GT. 2) WRITE(6,2) LZC,(MXA(KWD), KWD = 1,NQFM)	STFV0048
2 FORMAT(/5X,'LZC WAS CHANGED TO',F6.2,' BASED ON MONTHS',12I3)	STFV0049
IF(LZC .LT. 2.0 .AND. LRC) LZC = 2.0	STFV0050
IF(LZC .GT. 30.0 .AND. LRC) LZC = 30.0	STFV0051
C SELECTION OF MONTHS BEGINNING WET AND BEGINNING DRY SEASONS	STFV0052
106 MBWS = 0	STFV0053
MBDS = 0	STFV0054
DO 109 MONTH = 2,10	STFV0055
IF(TMPET(MONTH) .GT. TMPREC(MONTH)) GO TO 108	STFV0056
IF(MBWS .NE. 0) GO TO 107	STFV0057
MBWS = MONTH	STFV0058
GO TO 109	STFV0059
107 MBDS = MONTH + 1	STFV0060
GO TO 109	STFV0061

108	IF(MBDS .NE. 0) GO TO 110	STFV0062
109	CONTINUE	STFV0063
110	MBDS = MBDS + 1	STFV0064
C	ADJUSTMENT OF SUZC BASED ON TWO WETTEST SUMMER MONTHS AND LAST TWO	STFV0065
C	BASE FLOW MONTHS	STFV0066
	M11 = 0	STFV0067
	M12 = 0	STFV0068
	PRM1 = 0.0	STFV0069
	M1SP = 0	STFV0070
	DO 112 MNX = 7,14	STFV0071
	MONTH = MNX	STFV0072
	IF(MNX .GT. 12) MONTH = MNX - 12	STFV0073
	IF(TMPREC(MONTH) .LE. PRM1) GO TO 111	STFV0074
	M2SP = M1SP	STFV0075
	PRM2 = PRM1	STFV0076
	M1SP = MONTH	STFV0077
	PRM1 = TMPREC(MONTH)	STFV0078
	GO TO 112	STFV0079
111	IF(TMPREC(MONTH) .LE. PRM2) GO TO 112	STFV0080
	M2SP = MONTH	STFV0081
	PRM2 = TMPREC(MONTH)	STFV0082
112	CONTINUE	STFV0083
	FSUZC = MFDP(M1SP) + MFDP(M2SP)	STFV0084
	IF(ABS(XMPFT(12) - 1.0) .GT. 0.2) GO TO 113	STFV0085
	FSUZC = FSUZC + MFDP(12)	STFV0086
	M12 = 12	STFV0087
113	IF(ABS(XMPFT(11) - 1.0) .GT. 0.2) GO TO 114	STFV0088
	FSUZC = FSUZC + MFDP(11)	STFV0089
	M11 = 11	STFV0090
114	IF(FSUZC .GT. 1.0) FSUZC = 1.0	STFV0091
	IF(FSUZC .LT. -1.0) FSUZC = -1.0	STFV0092
	IF(FSUZC .GT. 0.0) SUZC = (FSUZC + 1.0)*SUZC	STFV0093
	IF(FSUZC .LE. 0.0) SUZC = SUZC/(1.0 + ABS(FSUZC))	STFV0094
	WRITE(6,3) SUZC,M1SP,M2SP,M11,M12	STFV0095
	3 FORMAT(4X,'SUZC WAS CHANGED TO',F6.2,' BASED ON MONTHS',4I3)	STFV0096

	IF(SUZC .LT. 0.3 .AND. LRC) SUZC = 0.3	STFV0097
	IF(SUZC .GT. 3.0 .AND. LRC) SUZC = 3.0	STFV0098
C	ADJUSTMENT OF ETLF BASED ON SUMMER MONTHS OF RAINFALL EXCEEDING TWO	STFV0099
C	INCHES OR NEED TO PREVENT MOISTURE BUILDUP	STFV0100
	IF(EMLZS(12) .LT. PLZC .OR. EMLZS(11) .LT. PLZC .OR. APREC .GT.	STFV0101
	1 1.5*APET) GO TO 115	STFV0102
	FETLF = 1.0	STFV0103
	MXA(1) = 13	STFV0104
	KWSM = 1	STFV0105
	GO TO 120	STFV0106
115	SWSMD = 0.0	STFV0107
	KWSM = 0	STFV0108
	DO 116 MONTH = 1,12	STFV0109
	IF((MONTH .GT. MBWS .OR. MONTH .GT. 2) .AND. (MONTH .LT. MRDS	STFV0110
	1 .AND. MONTH .LT. 9)) GO TO 116	STFV0111
	IF(TMPREC(MONTH) .LT. 2.0) GO TO 116	STFV0112
	SWSMD = SWSMD + MFDP(MONTH)	STFV0113
	KWSM = KWSM + 1	STFV0114
	MXA(KWSM) = MONTH	STFV0115
116	CONTINUE	STFV0116
	IF(KWSM .GE. 1) GO TO 117	STFV0117
	MXA(1) = M1R	STFV0118
	KWSM = 1	STFV0119
	FETLF = 5.0*MFDP(M1R)	STFV0120
	GO TO 120	STFV0121
117	WSM = KWSM	STFV0122
	IF(.NOT. LETLF .OR. KWSM .EQ. 1) GO TO 119	STFV0123
	LMFDP = 0.0	STFV0124
	DO 118 MONTH = 1,KWSM	STFV0125
	KM1 = MXA(MONTH)	STFV0126
	IF(MFDP(KM1) .LT. EMFDP) GO TO 118	STFV0127
	EMFDP = MFDP(KM1)	STFV0128
	KM2 = MONTH	STFV0129
118	CONTINUE	STFV0130
	MXA(KM2) = 0	STFV0131

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      SWSMD = SWSMD - EMFDP
      WSM = WSM - 1.0
119 FETLF = 1.2*SWSMD/WSM
120 IF(FETLF .GT. 1.0) FETLF = 1.0
      IF(FETLF .LT. -1.0) FETLF = -1.0
      IF(FETLF .GT. 0.0) ETLF = (FETLF + 1.0)*ETLF
      IF(FETLF .LT. 0.0) ETLF = ETLF/(1.0 + ABS(FETLF))
      WRITE(6,4) ETLF,(MXA(KWD), KWD = 1,KWSM)
      4 FORMAT(4X,'ETLF WAS CHANGED TO',F6.2,' BASED ON MONTHS',12I3)
      IF(ETLF .LT. 0.05 .AND. LRC) ETLF = 0.05
      IF(ETLF .GT. 0.6 .AND. LRC) ETLF = 0.6
C   ADJUSTMENT OF BUZC BASED ON SEPTEMBER, NOVEMBER, AND DECEMBER
      KM1 = 12
      KM2 = 2
      KM3 = 3
      FBUZC = 0.4*(MFDP(12) + MFDP(2) + MFDP(3))
      IF(.NOT. LBUZC) GO TO 121
      FBUZC = 0.4*(MFDP(9) + MFDP(10) + MFDP(11))
      KM1 = 9
      KM2 = 10
      KM3 = 11
121 IF(FBUZC .GT. 1.0) FBUZC = 1.0
      IF(FBUZC .LT. -1.0) FBUZC = -1.0
      IF(FBUZC .GT. 0.0) BUZC = (FBUZC + 1.0)*BUZC
      IF(FBUZC .LE. 0.0) BUZC = BUZC/(1.0 + ABS(FBUZC))
      WRITE(6,5) BUZC,KM1,KM2,KM3
      5 FORMAT(4X,'BUZC WAS CHANGED TO',F6.2,' BASED ON MONTHS',3I3)
      IF(BUZC .LT. 0.2 .AND. LRC) BUZC = 0.2
      IF(BUZC .GT. 4.0 .AND. LRC) BUZC = 4.0
C   ADJUSTMENT OF SIAC BASED ON THREE FIRST MOISTURE EXCESS AND THREE
C   FIRST MOISTURE DEFICIENT MONTHS
      KM1 = MBDS
      KM2 = MBDS + 1
      KM3 = MBDS - 1
      KM4 = 0

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STFV0132
STFV0133
STFV0134
STFV0135
STFV0136
STFV0137
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STFV0153
STFV0154
STFV0155
STFV0156
STFV0157
STFV0158
STFV0159
STFV0160
STFV0161
STFV0162
STFV0163
STFV0164
STFV0165
STFV0166

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KM5 = 0
KM6 = 0
WFDX = 0.0
IF(SIAC .GT. 1.0) GO TO 122
WFDX = (MFDP(MBWS) + MFDP(MBWS+1) + MFDP(MBWS+2))/3.0
IF(SIAC .GT. 0.6) WFDX = WFDX*(1.0 - SIAC)/0.4
KM4 = MBWS
KM5 = MBWS + 1
KM6 = MBWS + 2
122 SFDX = (MFDP(MBDS) + MFDP(MBDS+1) + MFDP(MBDS-1))/3.0
FSIAC = 1.5*(SFDX - WFDX)
IF(FSIAC .GT. 1.0) FSIAC = 1.0
IF(FSIAC .LE. -1.0) FSIAC = -1.0
IF(SIAC .LT. 0.02) SIAC = 0.02
IF(FSIAC .GT. 0.0) SIAC = (FSIAC + 1.0)*SIAC
IF(FSIAC .LE. 0.0) SIAC = SIAC/(1.0 + ABS(FSIAC))
WRITE(6,6) SIAC,KM4,KM5,KM6,KM3,KM1,KM2
6 FORMAT(4X,'SIAC WAS CHANGED TO',F6.2,' BASED ON MONTHS',6I3)
IF(SIAC .LT. 0.02 .AND. LRC) SIAC = 0.00
IF(SIAC .GT. 4.0 .AND. LRC) SIAC = 4.0
RETURN
END

```

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STFV0167
STFV0168
STFV0169
STFV0170
STFV0171
STFV0172
STFV0173
STFV0174
STFV0175
STFV0176
STFV0177
STFV0178
STFV0179
STFV0180
STFV0181
STFV0182
STFV0183
STFV0184
STFV0185
STFV0186
STFV0187
STFV0188

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SUBROUTINE SETFDI(MFDP, TMSTF, TMRTF, SSQM)
C SETS VALUES OF FLOW DEVIATION INDICES
DIMENSION MFDP(12), TMRTF(12), TMSTF(12)
REAL MFDP
DO 101 MONTH = 1, 12
IF(MONTH .LE. 2) SSQM = 0.0
SMFX = TMSTF(MONTH) + 20.0
RMFX = TMRTF(MONTH) + 20.0
MFDP(MONTH) = SMFX/RMFX - 1.0
IF(MFDP(MONTH) .GT. 8.0) MFDP(MONTH) = 8.0

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```

STFD0001
STFD0002
STFD0003
STFD0004
STFD0005
STFD0006
STFD0007
STFD0008
STFD0009
STFD0010

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```

      IF(MFDP(MONTH) .LT. 0.0) MFDP(MONTH) = 1.0 - RMFX/SMFX
      IF(MFDP(MONTH) .LT. -8.0) MFDP(MONTH) = -8.0
100 SSQM = SSQM + MFDP(MONTH)*MFDP(MONTH)
101 CONTINUE
      WRITE(6,1) (MFDP(MONTH), MONTH=1,12), SSQM
      1 FORMAT(/2X,'MONTHLY DEVIATIONS',/16X,12(F7.3,1X),'SSQM =',F7.3)
      RETURN
      END

```

```

STFD0011
STFD0012
STFD0013
STFD0014
STFD0015
STFD0016
STFD0017
STFD0018

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```

      SUBROUTINE SETBMI(BMIR,NRS,BFRC,RSBBF,SBFRS,FNCTRH,IFT)
C   SETS BEST VALUE OF BASIC MAXIMUM INFILTRATION RATE WITHIN WATERSHED
      DIMENSION RSBBF(20),SBFRS(3,20)
      ARSTR = 0.0
      DO 101 KRS = IFT,NRS
      RBF = RSBBF(KRS)/BFRC
      DO 100 KDY = 1,3
      KBF = RBF*BFRC
      SBF = SBFRS(KDY,KRS)*BFRC**(FNCTRH/48.0)
      RSTR = SBF/RBF
      IF(RSTR .GT. 3.0) RSTR = 3.0
      ARSTR = ARSTR + RSTR
      WRITE(6,1) KRS,KDY,SBF,RBF
      1 FORMAT(10X,'KRS =',I3,2X,'KDY =',I2,2X,'SBF =',F7.1,5X,'RBF =',
      1 F7.1)
100 CONTINUE
101 CONTINUE
      TBRD = NRS*3
      ARSTR = ARSTR/TBRD
      ARSTR = ARSTR**1.3
      PBMIR = BMIR
      BMIR = PBMIR/ARSTR
      WRITE(6,2) PBMIR,BMIR
      2 FORMAT(5X,'BMIR CHANGED FROM',F6.2,2X,'TO',F6.2//)

```

```

STBM0001
STBM0002
STBM0003
STBM0004
STBM0005
STBM0006
STBM0007
STBM0008
STBM0009
STBM0010
STBM0011
STBM0012
STBM0013
STBM0014
STBM0015
STBM0016
STBM0017
STBM0018
STBM0019
STBM0020
STBM0021
STBM0022
STBM0023
STBM0024

```

RETURN
END

STBM0025
STBM0026

 SUBROUTINE SETRBF(KSTF,NDRS,KRS,BFRC,IFRC,CRSBIF,CRSBBF)
C SETS VALUES OF INTERFLOW AND BASE FLOW AT RECESSION BEGINNING
 DIMENSION RSTF(50,20),NDRS(20)
 REAL*8 RA1,RA2,RA3,RA4,RA5,RA6
 REAL IFRC
 RA1 = 0.0
 RA2 = 0.0
 RA3 = 0.0
 RA4 = 0.0
 RA5 = 0.0
 MNDRS = 12
 IF(NDRS(KRS) .LT. 12) MNDRS = NDRS(KRS)
 IF(IFRC .GE. 0.3) GO TO 101
 CRSBIF = 0.0
 DO 100 KSD = 1,MNDRS
 RA1 = RA1 + BFRC**(2*KSD)
100 RA4 = RA4 + RSTF(KSD,KRS)*(BFRC**KSD)
 CRSOBF = RA4/RA1
 CRSBBF = CRSOBF*BFRC
 RETURN
101 CRSBIF = 100000.0
 DO 102 KSD = 1,MNDRS
 RA1 = RA1 + BFRC**(2*KSD)
 RA2 = RA2 + IFRC**(2*KSD)
 RA3 = RA3 + (BFRC*IFRC)**KSD
 RA4 = RA4 + RSTF(KSD,KRS)*(BFRC**KSD)
 RA5 = RA5 + RSTF(KSD,KRS)*(IFRC**KSD)
102 CONTINUE
 RA6 = RA1*RA2 - RA3**2
 IF(RA6 .EQ. 0.0) RETURN

STRB0001
STRB0002
STRB0003
STRB0004
STRB0005
STRB0006
STRB0007
STRB0008
STRB0009
STRB0010
STRB0011
STRB0012
STRB0013
STRB0014
STRB0015
STRB0016
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STRB0019
STRB0020
STRB0021
STRB0022
STRB0023
STRB0024
STRB0025
STRB0026
STRB0027
STRB0028
STRB0029
STRB0030

```

CRSOIF = -(RA3/RA6)*RA4 + (RA1/RA6)*RA5
CRSBIF = CRSOIF*IFRC
CRSOBF = (RA2/RA6)*RA4 - (RA3/RA6)*RA5
CRSBBF = CRSOBF*BFRC
RETURN
END

```

```

STRB0031
STRB0032
STRB0033
STRB0034
STRB0035
STRB0036

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```

SUBROUTINE SETBIV(BIVF,NRS,IFRC,RSBIF,SIFRS,FNCTRH)
C SETS BEST VALUE OF BASIC INTERFLOW VOLUME FACTOR
DIMENSION RSBIF(20),SIFRS(3,20)
REAL IFRC
ARSTR = 0.0
DO 101 KRS = 1,NRS
RIF = RSBIF(KRS)/IFRC
DO 100 KDY = 1,3
RIF = RIF*IFRC
SIF = SIFRS(KDY,KRS)*IFRC** (FNCTRH/48.0)
RSTR = 0.0
IF(RIF .GT. 0.0) RSTR = SIF/RIF
IF(RSTR .GT. 3.0 .OR. (SIF .GT. 0.0 .AND. RIF .EQ. 0.0))RSTR=3.0
ARSTR = ARSTR + RSTR
WRITE(6,1) KRS,KDY,SIF,RIF
1 FORMAT(10X,'KRS =',I3,2X,'KDY =',I2,2X,'SIF =',F7.1,5X,'RIF =',
1 F7.1)
100 CONTINUE
101 CONTINUE
TIRD = NRS*3
PBIVF = BIVF
BIVF = 0.40
IF(ARSTR .GT. 0.0) BIVF = ((PBIVF - 0.40)*TIRD)/ARSTR + 0.40
WRITE(6,2) PBIVF,BIVF
2 FORMAT(5X,'BIVF CHANGED FROM',F6.2,2X,'TO',F6.2//)
RETURN
END

```

```

STBV0001
STBV0002
STBV0003
STBV0004
STBV0005
STBV0006
STBV0007
STBV0008
STBV0009
STBV0010
STBV0011
STBV0012
STBV0013
STBV0014
STBV0015
STBV0016
STBV0017
STBV0018
STBV0019
STBV0020
STBV0021
STBV0022
STBV0023
STBV0024
STBV0025
STBV0026
STBV0027

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APPENDIX B

DICTIONARY OF VARIABLES USED IN MOPSET

- ITEM 1 - VARIABLE NAME
- ITEM 2 - WHETHER VARIABLE IS REAL, INTEGER, OR LOGICAL
- ITEM 3 - VARIABLE DIMENSIONS
- ITEM 4 - UNITS
- ITEM 5 - DEFINITION OF THE VARIABLE

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	1	2	3	4	5
ARFSL	R		1	DAY	ACCUMULATED BASE FLOW SEQUENCE LENGTH
ABFV	R		1	IN	ANNUAL BASE FLOW VOLUME
ADBFL	R		15	IN	ACCUMULATED DAILY BASE FLOW
ADIF	R		15	IN	ACCUMULATED DAILY INTERFLOW
ADRGPM	R		366	-	AVERAGE DATED RECORDING GAGE PRECIPITATION MULTIPLIER ABOVE GAGED REACH
AETX	R		1	IN	ANNUAL EVAPOTRANSPIRATION INDEX
AEX90	R		1	IN	ANTECEDENT EVAPORATION INDEX, DECAY RATE = 0.9
AEX96	R		1	IN	ANTECEDENT EVAPORATION INDEX, DECAY RATE = 0.96
AHP	R		1	IN	ACCUMULATED HOURLY PRECIPITATION
AIFSL	R		1	DAY	ACCUMULATED INTERFLOW SEQUENCE LENGTH
AIFV	R		1	IN	ANNUAL INTERFLOW VOLUME
ALPH	R	150,50	-		TABLE OF ALPHA VALUES
ALPHA	R		1	-	CURRENT ROUTING PARAMETER ALPHA
ALPHA1	R		1	-	TEMPORARY STORAGE OF ALPHA

AMBER	R	1	IN	ANNUAL MOISTURE BALANCE ERROR
AMBF	R	15	IN	ACCUMULATED MONTHLY BASE FLOW
AMIF	R	15	IN	ACCUMULATED MONTHLY INTERFLOW
AMNET	R	15	IN	ACCUMULATED MONTHLY NET EVAPOTRANSPIRATION
AMPET	R	1	IN	ACCUMULATED MONTHLY POTENTIAL EVAPOTRANSPIRATION
AMPREC	R	1	IN	ACCUMULATED MONTHLY PRECIPITATION
AMSE	R	15	IN	ACCUMULATED MONTHLY STREAM EVAPORATION
ANET	R	1	IN	ANNUAL NET EVAPOTRANSPIRATION
ADVF	R	1	IN	ANNUAL OVERLAND FLOW VOLUME
APET	R	1	IN	ANNUAL POTENTIAL EVAPOTRANSPIRATION
APREC	R	1	IN	ANNUAL PRECIPITATION
AREA	R	1	SQ MI	AREA OF WATERSHED
AREA1	R	1	SQ FT	TEMPORARY STORAGE FOR 1ST END AREA DURING ROUTING
AREA2	R	1	SQ FT	TEMPORARY STORAGE FOR 2ND END AREA DURING ROUTING
ARHF	R	15	CFS	ACCUMULATED ROUTED HYDROGRAPH FLOW
ARSTR	R	1	-	ACCUMULATED RATIO OF SYNTHESIZED TO RECORDED FLOWS
ASEV	R	1	IN	ANNUAL STREAM EVAPORATION VOLUME
ATF	R	1	SFD	ACCUMULATED TOTAL FLOW
ATFV	R	1	SFD	ANNUAL TOTAL FLOW VOLUME
AVXAR	R	1	SQ FT	AVERAGE FLOW END AREA FOR CURRENT REACH
AX	R	1	SQ FT	AVERAGE END AREA AT TWO SUCCESSIVE ROUTING TIMES
AZ	R	1	CU FT	DUMMY VARIABLE USED IN STORAGE ROUTING
A2	R	1	CFS	DIFFERENCE IN DISCHARGE BETWEEN TWO SUCCESSIVE POINTS IN STORAGE RATING TABLE
BBMIR	R	1	IN/HR	CURRENT BEST ESTIMATE OF BASIC MAXIMUM INFILTRATION RATE
BBUJC	R	1	-	CURRENT BEST ESTIMATE OF BASIC UPPER ZONE STORAGE CAPACITY FACTOR
BBYLZS	R	1	IN	CURRENT BEST ESTIMATE OF BEGINNING OF YEAR LOWER ZONE STORAGE
BETLF	R	1	-	CURRENT BEST ESTIMATE OF EVAPOTRANSPIRATION LOSS FACTOR
BFHRC	R	1	-	BASE FLOW HOURLY RECESSON CONSTANT
BFRC	R	1	-	BASE FLOW RECESSON CONSTANT
BFRL	R	1	-	BASE FLOW RECESSON LOGARITHM
BIVF	R	1	-	BASIC INTERFLOW VOLUME FACTOR
BIVF1	R	1	-	STORAGE FOR AVERAGE BIVF

BLZC	R	1	IN	CURRENT BEST ESTIMATE OF LOWER ZONE STORAGE CAPACITY
BMIR	R	1	IN/HR	BASIC MAXIMUM INFILTRATION RATE WITHIN WATERSHED
BMIR1	R	1	IN/HR	STORAGE FOR AVERAGE BMIR
BSIAC	R	1	-	CURRENT BEST ESTIMATE OF SEASONAL INFILTRATION ADJUSTMENT FACTOR
BSUZC	R	1	-	CURRENT BEST ESTIMATE OF SEASONAL UPPER ZONE STORAGE CAPACITY FACTOR
BUZC	R	1	-	BASIC UPPER ZONE STORAGE CAPACITY FACTOR
BUZC1	R	1	-	STORAGE FOR AVERAGE BUZC
BYGWS	R	1	IN	BEGINNING OF YEAR GROUNDWATER STORAGE
BYLZS	R	1	IN	BEGINNING OF YEAR LOWER ZONE STORAGE
B2	R	1	CF	DIFFERENCE IN STORAGE BETWEEN TWO SUCCESSIVE POINTS
CBF	R	1	IN/HR	CURRENT BASE FLOW
CIVM	R	1	-	CURRENT INTERFLOW VOLUME MULTIPLIER
CMIR	R	1	IN	CURRENT MAXIMUM INFILTRATION RATE DURING PERIOD
CN	I	1	-	1 = A. M., 2 = P. M.
CONOPT	I	7	-	CONTROL OPTION
CONST	R	1	-	NONLINEAR ROUTING ITERATION CONSTANT
CRSBBF	R	1	CFS	CURRENT RECESSION SEQUENCE BEGINNING BASE FLOW
CRSBIF	R	1	CFS	CURRENT RECESSION SEQUENCE BEGINNING INTERFLOW
CRSBTF	R	1	CFS	CURRENT RECESSION SEQUENCE BEGINNING TOTAL FLOW
CRSTF	R	50	CFS	CURRENT RECESSION SEQUENCE TOTAL FLOWS
CRSOBF	R	1	CFS	CURRENT RECESSION SEQUENCE BASE FLOW ON DAY ZERO
CRSOIF	R	1	CFS	CURRENT RECESSION SEQUENCE INTERFLOW ON DAY ZERO
CVWINA	R	15	SFD	CUMULATIVE VWINA FOR PREVIOUS REACHES
CVWIN1	R	1	SFD	TEMPORARY STORAGE OF CVWINA FOR REACH 1
CWCFSA	R	15	CFS	CUMULATIVE WCFS A FOR PREVIOUS REACHES
CWCFS1	R	1	CFS	TEMPORARY STORAGE OF CWCFS A FOR REACH 1
C2	R	1	-	DUMMY VARIABLE USED IN STORAGE ROUTING
C4	R	1	CUFT	FUNCTION OF KNOWN QUANTITIES IN STORAGE ROUTING EQUATION
C5	R	1	CUFT	FUNCTION OF UNKNOWN QUANTITIES IN STORAGE ROUTING EQUATION
DAREA	R	150,50	SQFT	TABLE OF END AREAS FOR CHANNEL ROUTING
DATE	I	1	-	CURRENT DAY OF THE MONTH

DAY	I	1	-	CURRENT DAY OF THE YEAR
DBFRC	R	1	-	DOUBLE PRECISION BFRC
DELTAT	R	1	SEC	ROUTING TIME INTERVAL
DELTA	R	1	FT	FINITE DIFFERENCE LENGTH FOR CURRENT REACH
DEV1	R	1	-	DEVIATION BETWEEN 1ST TRIAL VALUE AND EXPECTED SOLUTION
DEV2	R	1	-	DEVIATION BETWEEN 2ND TRIAL VALUE AND EXPECTED SOLUTION
DIFRC	R	1	-	DOUBLE PRECISION IFRC
DISCH	R	1000	CFS	TABLE OF DISCHARGES
DIV	R	1	CFS	DIVERSION INTO BASIN, MEAN DAILY FLOW
DPET	R	366	IN	DATED POTENTIAL EVAPOTRANSPIRATION
OPRD	I	1	-	CURRENT DAILY 15-MINUTE PERIOD
DPY	I	1	-	DAYS PER YEAR
DRGPM	R	15,366	-	DATED RECORDING GAGE PRECIPITATION MULTIPLIER
DRGPM1	R	366	-	TEMPORARY STORAGE OF DRGPM FOR REACH 1
DRHP	R	366,24	IN	DATED RECORDED HOURLY PRECIPITATION
DRSF	R	366	CFS	DATED RECORDED STREAMFLOW
DRSGP	R	5,366	IN	DATED RECORDED STORAGE GAGE PRECIPITATION
DSSF	R	366	CFS	DATED SYNTHESIZED STREAMFLOW
EDAREA	R	150,21	SQ FT	END OF DAY FLOW END AREA
EHS GD	I	1	-	ENDING HOUR OF STORAGE GAGE DAY
EHS GDF	R	1	-	ENDING HOUR OF STORAGE GAGE DAY - FLOATING POINT
EID	R	1	-	EXPONENT OF INFILTRATION RATE DECAY WITH INCREASED SOIL MOISTURE CONTENT
ELEV	R	1000	FT	ELEVATION OF STORAGE ROUTING TABLE
EMAET	R	1	IN	ESTIMATED MAXIMUM ANNUAL EVAPOTRANSPIRATION
EMATF	R	13	SFD	END OF MONTH ACCUMULATED TOTAL FLOWS
EMFDP	R	1	-	EXTREME MONTHLY FLOW DEVIATION PARAMETER
EMGWS	R	12	IN	END OF MONTH GROUNDWATER STORAGE
EMIFS	R	12	IN	END OF MONTH INTERFLOW STORAGE
EMLZS	R	12	IN	END OF MONTH LOWER ZONE STORAGE
EMS IAM	R	12	-	END OF MONTH SEASONAL INFILTRATION ADJUSTMENT MULTIPLIER
EMUZC	R	12	IN	END OF MONTH UPPER ZONE STORAGE CAPACITY
EMUZS	R	12	IN	END OF MONTH UPPER ZONE STORAGE
EPAET	R	1	IN	ESTIMATED POTENTIAL ANNUAL EVAPOTRANSPIRATION
EPCM	R	12	-	EVAPORATION PAN COEFFICIENT FOR MONTH

EQD	R	1	IN	EQUILIBRIUM DEPTH OF OVERLAND FLOW
EQDF	R	15	-	EQUILIBRIUM DEPTH FACTOR FOR OVERLAND FLOW
EQDF1	R	1	-	TEMPORARY STORAGE OF EQDF FOR REACH 1
EQDFIS	R	15	-	EQUILIBRIUM DEPTH FACTOR FOR OVERLAND FLOW, IMPERVIOUS SURFACES
EQDFI1	R	1	-	TEMPORARY STORAGE OF EQDFIS FOR REACH 1
EQDIS	R	1	IN	EQUILIBRIUM DEPTH OF OVERLAND FLOW ON IMPERVIOUS SURFACES
ETLF	R	1	-	EVAPOTRANSPIRATION LOSS FACTOR
ETLF1	R	1	-	STORAGE FOR AVERAGE ETLF
EXPM	R	1	-	CURRENT ROUTING EXPONENT - M
EXPT	R	150,50	-	TABLE OF M EXPONENTS
FBUZC	R	1	-	ADJUSTMENT FACTOR FOR BUZC
FETLF	R	1	-	ADJUSTMENT FACTOR FOR ETLF
FHPP	R	1	-	FRACTIONAL HOUR PER PERIOD
FIMPA	R	15	-	FRACTION OF SUBAREA BEING IMPERVIOUS
FIMPA1	R	1	-	TEMPORARY STORAGE OF FIMPA FOR REACH 1
FLZC	R	1	-	ADJUSTMENT FACTOR FOR LZC
FMR	R	1	-	FRACTION OF MOISTURE NOT RETAINED IN UPPER ZONE
FNCTRH	R	1	HR	FLOATING POINT NUMBER OF CURRENT TIME ROUTING HOUR, WHICH IS EQUIVALENT TO FNWSTH IN MAIN PROGRAM
FNOFM	R	1	-	FLOATING POINT NUMBER OF OVERLAND FLOW MONTHS
FNWSTH	R	1	HR	FLOATING POINT NUMBER OF WATERSHED TRAVEL HOUR
FPERA	R	15	-	FRACTION OF SUBAREA BEING PERVIOUS
FPERA1	R	1	-	TEMPORARY STORAGE OF FPERA FOR REACH 1
FRERS	R	1	CFS	FLOW RISE ENDING RECESSION SEQUENCE
FSIAC	R	1	-	ADJUSTMENT FACTOR FOR SIAC
FSLTH	I	150	FT	LENGTH OF FINITE SECTION
FSUZC	R	1	-	ADJUSTMENT FACTOR FOR SUZC
FTX	R	1	-	FALL TROUBLE INDEX
FUNC	R	1	-	FUNCTION USED IN NONLINEAR ROUTING
FUNC1	R	1	-	FIRST DERIVATIVE OF FUNCTION USED IN NONLINEAR ROUTING
FUNC2	R	1	-	2ND DERIVATIVE OF FUNCTION USED IN NONLINEAR ROUTING
FWTR	R	1	-	FRACTION OF WATERSHED BEING WATER
FWTRA	R	15	-	FRACTION OF SUBAREA BEING WATER
FWTRA1	R	1	-	TEMPORARY STORAGE OF FWTRA FOR REACH 1

GAFI	R	366	CFS	DAILY SYNTHESIZED STREAM FLOW FROM GAGED REACH
GAREA	R	1	SQ MI	DRAINAGE AREA ABOVE THE GAGE
GWET	R	1	IN	CURRENT HOURLY GROUNDWATER EVAPOTRANSPIRATION
GWETF	R	1	-	GROUNDWATER EVAPOTRANSPIRATION FACTOR
GWS	R	1	IN	CURRENT GROUNDWATER STORAGE
GWSA	R	15	IN	GWS FOR SUBAREA
HOUR	I	1	-	CURRENT HOUR OF THE DAY
HOURL	I	1	-	LAST HOUR
HRF	I	1	-	FIRST HOUR OF LOOP
HRL	I	1	-	LAST HOUR OF LOOP
HSE	R	1	IN	CURRENT HOURLY STREAM EVAPORATION
HYDSTO	R	150,96	CFS	DAILY STORAGE FOR OUTFLOW HYDROGRAPHS TO BE USED AS INFLOW HYDROGRAPHS TO THE NEXT LOWER REACH
I	I	1	-	DO-LOOP COUNTER
ICYC	I	1	-	IDENTIFICATION OF CYCLE NUMBER IN TRIP 1
IDAY1	I	1	-	INDEX TO 10-DAY PERIOD
IDAY2	I	1	-	INDEX WITHIN 10-DAY PERIOD
IE	I	1	-	STORAGE ELEMENT COUNTER
IEF	I	1	-	FIRST ELEMENT IN STRUCTURE RATING CURVE
IEL	I	1	-	LAST ELEMENT IN STRUCTURE RATING CURVE
IFIELD	I	1	-	LENGTH OF AVAILABLE ARRAY FOR STORAGE ROUTING
IFPRC	R	1	-	INTERFLOW PERIOD RECESSON CONSTANT
IFRC	R	1	-	INTERFLOW RECESSON CONSTANT
IFRL	R	1	-	INTERFLOW RECESSON LOGARITHM
IFS	R	1	IN	INTERFLOW STORAGE
IFSA	R	15	IN	IFS FOR SUBAREA
IFST	I	15	-	FIRST ELEMENT IN STRUCTURE RATING CURVE STORAGE ARRAY
IFT	I	1	-	INDICATOR OF FALL TROUBLE (SKIP FIRST RECESSON IN EVALUATION OF BMIR)
IH	I	1	-	UPSTREAM HYDROGRAPH COUNTER
ILST	I	15	-	LAST ELEMENT IN STRUCTURE RATING CURVE STORAGE ARRAY
INLR	I	1	-	INDEX EQUAL TO 1 FOR NONLINEAR ROUTING
INLST	R	15	CFS	INITIAL STRUCTURE INFLOW
IP	I	1	-	RATING CURVE POINT COUNTER
IPPH	I	1	-	INTEGER PERIODS PER HOUR

IROUT	I	15	-	REACHES WHOSE OUTFLOW IS TO BE PRINTED
IRSTR	I	15	-	IDENTITY OF REACH CONTAINING STRUCTURE
ISGA	I	15	-	STORAGE GAGE DESIGNATION
ISGM	I	5	-	INDEX OF STORAGE GAGE MOVEMENT DURING THE YEAR
ISGRD	I	1	-	CURRENT STORAGE GAGE RAINFALL DAY
IUH	I	15,5	-	IDENTITY OF UPSTREAM HYDROGRAPHS TO BE ADDED
IWBG	I	1	-	INDEX NUMBER OF WEATHER BUREAU PRECIPITATION GAGE
J	I	1	-	DO-LOOP COUNTER
KBRC	I	1	-	COUNTER OF ROUGH CYCLES SINCE BEST ONE
KDRS	I	1	-	COUNTER OF CURRENT DAY IN RECESSION SEQUENCE
KDY	I	1	-	COUNTER FOR DAY
KFFC	I	1	-	COUNTER EQUALLING ONE ON FIRST FINE ADJUSTMENT CYCLE
KFS	I	1	-	COUNTER FOR FINITE SECTION
KG	I	1	-	STORAGE GAGE COUNTER
KHOUR	I	1	-	COUNTER FOR HOUR OF DAY
KHYD	I	1	-	COUNTER SPECIFYING CURRENT HYDROGRAPH
KMO	I	1	-	COUNTER INDEXING MONTH OF THE YEAR
KMI-6	I	1	-	MONTH COUNTERS
KNFS	I	1	-	COUNTER FOR FINITE SECTION FROM HEAD OF 1ST REACH
KOUT	I	1	-	COUNTER FOR REACH OUTPUT
KPRD	I	1	-	COUNTER FOR PERIOD
KR	I	1	-	REACH COUNTER
KRC	I	1	-	COUNTER OF CURRENT ADJUSTMENT CYCLE
KRD	I	1	-	COUNTER FOR READING DATA ARRAYS
KRKFS	I	1	-	ONE DIMENSIONAL ARRAY FOR (KR,KFS)
KRS	I	1	-	COUNTER FOR RECESSION SEQUENCE NUMBER
KS	I	1	-	STRUCTURE COUNTER
KSD	I	1	-	COUNTER FOR RECESSION SEQUENCE DAYS
KSQ	I	1	-	COUNTER FOR REVISED SEQUENCES
KTA	I	1	-	COUNTER FOR TITLE ARRAY
KWD	I	1	-	COUNTER FOR WRITING DATA ARRAYS
KWSM	I	1	-	COUNTER OF WET SUMMER MONTHS
LBFO	L	20	-	LOGICAL VARIABLE SET TRUE WHERE BASE FLOW ONLY ENCOUNTERED
LBMIR	L	1	-	LOGICAL VARIABLE SET TRUE WHEN EXERCISING SUBSTITUTE

LBUZC	L	1	-	APPROACH FOR EVALUATING BMIR LOGICAL VARIABLE SET TRUE WHEN EXERCISING SUBSTITUTE APPROACH FOR EVALUATING BUZC
LDAR1	R	1	-	LOG OF END AREA FOR POINT 1
LDAR2	R	1	-	LOG OF END AREA FOR POINT 2
LDAY	I	1	-	LAST DAY OF YEAR
LDIS1	R	1	-	LOG OF DISCHARGE FOR POINT 1
LDIS2	R	1	-	LOG OF DISCHARGE FOR POINT 2
LETLF	L	1	-	LOGICAL VARIABLE SET TRUE WHEN EXERCISING SUBSTITUTE APPROACH FOR EVALUATING ETLF
LHOUR	I	1	-	LAST HOUR OF DAY
LLZC	L	1	-	LOGICAL VARIABLE SET TRUE WHEN EXERCISING SUBSTITUTE APPROACH FOR EVALUATING LZC
LNPR	L	1	-	LOGICAL VARIABLE SET TRUE FOR NONEQUAL PERIOD RAINFALL
LRC	L	1	-	LOGICAL VARIABLE SET TRUE DURING ROUGH ADJUSTMENT CYCLES
LSTR	L	1	-	SET TRUE WHEN CURRENT REACH CONTAINS STRUCTURE
LZC	R	1	IN	LOWER ZONE STORAGE CAPACITY
LZC1	R	1	IN	STORAGE FOR AVERAGE LZC
LZR _X	R	1	-	LOWER ZONE MOISTURE RETENTION INDEX
LZS	R	1	IN	CURRENT LOWER ZONE STORAGE
LZSA	R	15	IN	LZS FOR SUBAREA
LZSR	R	1	-	CURRENT LOWER ZONE STORAGE RATIO (LZS/LZC)
MBDS	I	1	-	MONTH BEGINNING DRY SEASON
MBWS	I	1	-	MONTH BEGINNING WET SEASON
MDAY	I	1	-	DAY OF YEAR OF LAST DAY OF PREVIOUS MONTH
MEDCY	I	12	-	MONTH END DATES - CALENDAR YEAR
MEDWY	I	12	-	MONTH END DATES - WATER YEAR
MEDWYP	I	1	-	MEDWY OF PREVIOUS MONTH
MFDP	R	12	-	MONTHLY FLOW DEVIATION PARAMETER
MNDRS	I	1	-	MAXIMUM NUMBER OF DAYS IN RECESSION SEQUENCE
MNRC	I	1	-	MINIMUM NUMBER OF ROUGH CYCLES
MNRD	R	1	-	MEAN ANNUAL NUMBER OF RAINY DAYS
MN _X	I	1	-	MONTH INDEX
MONTH	I	1	-	CURRENT MONTH OF THE YEAR
MONTH1	I	1	-	COUNTER FOR BEGINNING MONTH

MRSL	I	1 DAY	MINIMUM RECESSION SEQUENCE LENGTH
MXA	I	12 -	MONTH INDEX ARRAY (SPECIFYING MONTHS USED IN PARAMETER ADJUSTMENT)
MIR	I	1 -	MONTH WITH MOST RUNOFF
M1SP	I	1 -	MONTH WITH MOST SUMMER PRECIPITATION
M11	I	1 -	SET AT 11 IF AUGUST IS A BASE FLOW MONTH
M12	I	1 -	SET AT 12 IF SEPTEMBER IS A BASE FLOW MONTH
M2R	I	1 -	MONTH WITH SECOND MOST RUNOFF
M2SP	I	1 -	MONTH WITH SECOND MOST SUMMER PRECIPITATION
N	I	1 -	DO-LOOP COUNTER
NATRH	I	1 HR	NUMBER OF ANTICIPATED TIME ROUTING HOURS, WHICH IS EQUIVALENT TO NWSTH IN MAIN PROGRAM
NB	I	1 -	NUMBER OF CURRENT UPSTREAM BRANCHES
NDAY	I	1 -	NEXT DAY OF YEAR
NDCR	I	1 -	DAY NUMBER FOR TERMINATING CHANNEL ROUTING
NOPM	I	1 DAY	NUMBER OF DAYS PER MONTH
NDRS	I	20 DAY	NUMBER OF DAYS IN RECESSION SEQUENCE
NDRSC	I	1 DAY	NUMBER OF DAYS IN CURRENT RECESSION SEQUENCE
NDRSC1	I	1 DAY	NUMBER OF DAYS IN CURRENT RECESSION SEQUENCE LESS 1
NDRSC2	I	1 DAY	NUMBER OF DAYS IN CURRENT RECESSION SEQUENCE LESS 2
NELV	I	1 -	NUMBER OF ELEVATIONS IN STRUCTURE RATING CURVE
NFRHA	I	1 -	NUMBER OF FIRST RAINFALL HOUR ADJUSTED, PREVIOUS DAY
NFS	I	150 -	NUMBER OF FINITE SECTIONS IN EACH REACH
NFSKR	I	1 -	NUMBER OF FINITE SECTION FOR REACH KR
NFSEPR	I	15 -	NUMBER OF FINITE SECTIONS AT THE END OF PREVIOUS REACH
NFTR	I	1 -	NUMBER OF FIRST TRIP TO BE RUN FOR A GIVEN STATION YEAR
NGR	R	1 -	IDENTIFYING NUMBER OF GAGED REACH
NGR1	I	1 -	TEMPORARY STORAGE FOR NGR
NH	I	1 -	NUMBER OF HYDROGRAPHS
NHOUR	I	1 -	NEXT HOUR OF DAY
NI	I	1 -	COUNTER FOR NUMBER OF ITERATIONS IN NONLINEAR ROUTING
NL	I	1 -	NUMBER OF FINITE DIFFERENCE PLUS 1
NLR	I	1 -	NUMBER OF LAST REACH
NLS	I	1 -	NUMBER OF LAST SECTION
NLTR	I	1 -	NUMBER OF LAST TRIP TO BE RUN FOR A GIVEN STATION YEAR

NNJ	I	1	-	ARRAY NUMBER OF NEXT JUNCTION TO BE CALCULATED
NOFD	I	150	-	NUMBER OF FINITE DIFFERENCE DIVISION FOR EACH FINITE SECTION
NOFM	I	1	-	NUMBER OF OVERLAND FLOW MONTHS
NOUT	I	1	-	NUMBER OF REACHES WHOSE OUTFLOWS ARE TO BE PRINTED
NP	I	1	-	NUMBER OF POINTS IN RATING CURVE
NPPH	I	1	-	NUMBER OF PERIODS PER HOUR OF ROUTING
NR	I	1	-	NUMBER OF REACHES IN WATERSHED LAYOUT
NRHA	I	1	-	NUMBER OF RAINFALL HOURS ADJUSTED, CURRENT DAY
NRS	I	1	-	NUMBER OF RECESSION SEQUENCES
NR1	I	1	-	TEMPORARY STORAGE FOR NR
NSC	I	150	-	NUMBER OF SLOPE CHANGE IN RATING CURVE
NSCS	I	1	-	NSC FOR (KR,KFS)
NSG	I	1	-	NUMBER OF STORAGE GAGES
NSGRD	I	1	-	NUMBER OF STORAGE GAGE RAINFALL DAYS
NSTR	I	1	-	NUMBER OF STRUCTURES
NSUM	I	1	-	NUMBER OF SUM HYDROGRAPH
NSYC	I	1	-	NUMBER OF STATION YEAR, CURRENT ONE BEING RUN
NSYT	I	1	-	NUMBER OF STATION YEARS, TOTAL INCLUDED IN A GIVEN JOB
NTRS	I	1	-	NUMBER OF TENTATIVE RECESSION SEQUENCES
NUBR	I	15	-	NUMBER OR UPSTREAM HYDROGRAPHS TO BE ADDED
NWSTH	I	1	HR	NUMBER OF WATERSHED TRAVEL HOURS
NX	I	1	-	NUMBER OF CURRENT XAREA BEING CALCULATED
OCT1BF	R	1	-	OCTOBER FIRST BASE FLOW
OFMN	R	15	-	OVERLAND FLOW MANNING'S N
OFMNIS	R	15	-	OVERLAND FLOW MANNING'S N , IMPERVIOUS SURFACES
OFR	R	1	IN	CURRENT OVERLAND FLOW RUNOFF
OFRF	R	15	-	OVERLAND FLOW ROUTING FACTOR
OFRF1	R	1	-	TEMPORARY STORAGE OF OFRF FOR REACH 1
OFRFIS	R	15	-	OVERLAND FLOW ROUTING FACTOR, IMPERVIOUS SURFACES
OFRF11	R	1	-	TEMPORARY STORAGE OF OFRFIS FOR REACH 1
OFRIS	R	1	IN	CURRENT OVERLAND FLOW RUNOFF, IMPERVIOUS SURFACES
OFSL	R	15	FT	OVERLAND FLOW SURFACE LENGTH
OFSS	R	15	-	OVERLAND FLOW SURFACE SLOPE
OFUS	R	15	IN	CURRENT OVERLAND FLOW UNROUTED STORAGE

OFUSIS	R	15	IN	CURRENT OVERLAND FLOW UNROUTED STORAGE, IMPERVIOUS SURFACES
OMEGA	R	1	-	NONLINEAR ROUTING ITERATION VARIABLE
OUTLST	R	15	CFS	INITIAL OUTFLOW FROM STRUCTURE
PAEX90	R	1	IN	PRESENT VALUE OF AEX90
PAEX96	R	1	IN	PRESENT VALUE OF AEX96
PALPHA	R	150	-	VALUE OF ALPHA AT PREVIOUS TIME
PBIVF	R	1	-	PREVIOUS VALUE OF BIVF
PBMIR	R	1	IN/HR	PREVIOUS VALUE OF BMIR
PBUZC	R	1	-	PREVIOUS ESTIMATE OF BASIC UPPER ZONE STORAGE CAPACITY FACTOR
PDAY	I	1	-	PREVIOUS DAY OF THE YEAR
PEAI	R	1	IN	PRECIPITATION EXCESS AFTER INFILTRATION
PEBI	R	1	IN	PRECIPITATION EXCESS BEFORE INFILTRATION
PEIS	R	1	IN	PRECIPITATION EXCESS ON IMPERVIOUS SURFACES
PEP	R	1	IN	PRECIPITATION ESTIMATED FOR PERIOD
PET	R	1	IN	CURRENT DAILY POTENTIAL EVAPOTRANSPIRATION
PETLF	R	1	-	PREVIOUS ESTIMATE OF EVAPOTRANSPIRATION LOSS FACTOR
PETU	R	1	IN	UNADJUSTED CURRENT DAILY POTENTIAL EVAPOTRANSPIRATION
PEXPM	R	150	-	VALUE OF EXPM AT PREVIOUS TIME
PE4P	R	4	IN	PRECIPITATION ESTIMATES FOR 4 PERIODS
PGW	R	1	IN	PERCOLATION TO GROUND WATER
PLZC	R	1	IN	PREVIOUS ESTIMATE OF LZC
PLZS	R	1	IN	PERCOLATION TO LOWER ZONE STORAGE
PPH	R	1	-	PERIODS PER HOUR
PPI	R	1	IN	PRECIPITATION PASSING INTERCEPTION
PRD	I	1	-	CURRENT PERIOD OF THE HOUR
PRDF	R	1	-	CURRENT PERIOD OF THE HOUR-FLOATING POINT
PRH	R	1	IN	PRECIPITATION RECORDED FOR HOUR
PRLH	R	1	IN	PRECIPITATION RECORDED FOR LAST HOUR
PRM1	R	1	IN	PRECIPITATION DURING WETTEST MONTH
PRM2	R	1	IN	PRECIPITATION DURING SECOND WETTEST MONTH
PRNH	R	1	IN	PRECIPITATION RECORDED FOR NEXT HOUR
PSIAC	R	1	-	PREVIOUS ESTIMATE OF SEASONAL INFILTRATION ADJUSTMENT
PSUZC	R	1	-	PREVIOUS ESTIMATE OF SEASONAL UPPER ZONE STORAGE CAPACITY FACTOR

PXAREA	R	1	SQ FT	PREVIOUS FLOW END AREA AT CURRENT SECTION
RA	R	1	-	RECESSION ALPHA
RAA	R	1	IN	RAINFALL ADJUSTMENT ADDITION
RAM	R	1	-	RAINFALL ADJUSTMENT MULTIPLIER
RAT	R	1	-	INTERPOLATING RATIO
RATEV	R	1	SFD	RECORDED ANNUAL TOTAL FLOW VOLUME
RA1-6	R	1	-	REGRESSION ACCUMULATORS
RB	R	1	-	RECESSION BETA
RBF	R	1	CFS	RECORDED BASE FLOW
KBIVF	R	15	-	RATIO OF BIVF FOR SUBAREA TO AVERAGE BIVF
RBIVF1	R	1	-	TEMPORARY STORAGE OF RBIVF FOR REACH 1
RBMIR	R	15	-	RATIO OF BMIR FOR SUBAREA TO AVERAGE BMIR
RBMIR1	R	1	-	TEMPORARY STORAGE OF RBMIR FOR REACH 1
RBUZC	R	15	-	RATIO OF BUZC FOR SUBAREA TO AVERAGE PUZC
RBUZC1	R	1	-	TEMPORARY STORAGE OF RBUZC FOR REACH 1
RCHLTH	R	15	FT	REACH LENGTH
RD	R	1	-	RECESSION DISCRIMINANT
RDPT	R	1	IN	RECORDED DAILY PRECIPITATION TOTAL
RETLF	R	15	-	RATIO OF ETLF FOR SUBAREA TO AVERAGE ETLF
RETLF1	R	1	-	TEMPORARY STORAGE OF RETLF FOR REACH 1
RFRISE	K	1	IN	RECORDED FLOW RISE
RGPM	R	15	-	RECORDING GAGE PRECIPITATION MULTIPLIER
RGPMB	R	15	-	RECORDING GAGE PRECIPITATION MULTIPLIER - BASIC
RHF	R	1	CFS	ROUTED HYDRAGRAPH FLOW
RIF	R	1	CFS	RECORDED INTERFLOW
RLZC	R	15	-	RATIO OF LZC FOR SUBAREA TO AVERAGE LZC
RLZC1	R	1	-	TEMPORARY STORAGE OF RLZC FOR REACH 1
RMFX	R	1	-	RECORDED MONTHLY FLOW INDEX
RMPE	R	1	CFS	REQUESTED MINIMUM DAILY PEAK FLOW TO BE PRINTED
RMWR	R	1	IN	RAINFALL MAXIMUM WITHOUT RUNOFF
ROLF1	R	15	CFS/FT	RUNOFF (LOCAL INFLOW) PER LINEAR FOOT OF STREAM FOR THE PREVIOUS TIME INTERVAL
ROLF2	R	15	CFS/FT	RUN OFF (LOCAL INFLOW) PER LINEAR FOOT OF STREAM FOR THE CURRENT TIME INTERVAL
RSBBF	R	20	CFS	ESTIMATED BASE FLOW AT BEGINNING OF RECESSION SEQUENCE
RSBD	I	20	-	RECESSION SEQUENCE BEGINNING DAY

RSBFRC	R	20	-	RECESSION SEQUENCE BASE FLOW RECESSION CONSTANT
RSBIF	R	20	CFS	ESTIMATED INTERFLOW AT BEGINNING OF RECESSION SEQUENCE
RSFM	R	1	CFS	RECESSION SEQUENCE FLOW MINIMUM
RSFN	R	1	CFS	RECORDED STREAMFLOW ON NEW DAY
RSF1	R	1	CFS	RECORDED STREAMFLOW ON DAY 1
RSF2	R	1	CFS	RECORDED STREAMFLOW ON DAY 2
RSIAC	R	15	-	RATIO OF SIAC FOR SUBAREA TO AVERAGE SIAC
RSIAC1	R	1	-	TEMPORARY STORAGE OF RSIAC FOR REACH 1
RSIFRC	R	20	-	RECESSION SEQUENCE INTERFLOW RECESSION CONSTANT
RSL	I	1	DAY	CURRENT RECESSION SEQUENCE LENGTH
RSTF	R	50,20	CFS	RECESSION SEQUENCE TOTAL FLOWS
RSTR	R	1	-	RATIO OF SYNTHESIZED TO RECORDED FLOW
RSUZC	R	15	-	RATIO OF SUZC FOR SUBAREA TO AVERAGE SUZC
RSUZC1	R	1	-	TEMPORARY STORAGE OF RSUZC FOR REACH 1
RWRAIN	R	1	IN	RECORDED WATERSHED RAINFALL
SAREA	R	15	SQ MI	SUBAREA AREA IN SQUARE MILES
SAREA1	R	1	SQ MI	TEMPORARY STORAGE OF SAREA FOR REACH 1
SATFV	R	1	SFD	SYNTHESIZED ANNUAL TOTAL FLOW VOLUME
SATFVI	R	1	IN	SYNTHESIZED ANNUAL TOTAL FLOW VOLUME IN INCHES
SAVE	L	1	-	SET TRUE WHEN CURRENT REACH OUTFLOW IS SUBJECT TO PRINT
SBF	R	1	CFS	SYNTHESIZED BASE FLOW
SBFRS	R	3,20	CFS	SYNTHESIZED BASE FLOW DURING THE FIRST THREE DAYS OF EACH RECESSION SEQUENCE
SET	R	1	IN	CURRENT HOURLY SOIL EVAPOTRANSPIRATION
SFDX	R	1	-	SUMMER FLOW DEVIATION INDEX
SGMD	I	5	-	STORAGE GAGE MOVING DAY (WHEN IT IS MOVED DURING WATER YEAR)
SGRT	I	5	-	STORAGE GAGE READING TIME
SGRT2	I	5	-	SECOND STORAGE GAGE READING TIME
SIAC	R	1	-	SEASONAL INFILTRATION ADJUSTMENT CONSTANT
SIAC1	R	1	-	STORAGE FOR AVERAGE SIAC
SIAM	R	1	-	SEASONAL INFILTRATION ADJUSTMENT MULTIPLIER
SIF	R	1	CFS	SYNTHESIZED INTERFLOW
SIFRS	R	3,20	CFS	SYNTHESIZED INTERFLOW DURING THE FIRST THREE DAYS OF EACH RECESSION SEQUENCE
SLOPE	R	1	-	SLOPE OF STORAGE OVER ELEVATION

SMF	R	1	SFD	SUM OF MONTHLY SYNTHESIZED FLOW
SMFX	R	1	-	SYNTHESIZED MONTHLY FLOW INDEX
SOFMD	R	1	-	SUM OF OVERLAND FLOW MONTH DEVIATIONS
SSQM	R	1	-	SUM OF THE SQUARES OF THE MONTHLY FLOW DEVIATIONS
SSRT	R	1	-	SQUARE ROOT OF OVERLAND FLOW SURFACE SLOPE
SSSQM	R	1	-	CURRENT SMALLEST ESTIMATE OF SSQM
STOLST	R	15	CU FT	INITIAL STORAGE IN STRUCTURE
STORAG	R	1000	AC FT	TABLE OF STRUCTURE STORAGES FROM RATING CURVE
SUBWF	R	1	-	SUBSURFACE WATER FLOW OUT OF BASIN
SUBWFA	R	15	IN	SUBWF FROM SUBAREA
SUBWF1	R	1	IN	TEMPORARY STORAGE OF SUBWFA FOR REACH 1
SUZC	R	1	-	SEASONAL UPPER ZONE STORAGE CAPACITY FACTOR
SUZC1	R	1	-	STORAGE FOR AVERAGE SUZC
SWSMD	R	1	-	SUM OF WET SUMMER MONTH DEVIATIONS
TBRD	R	1	-	TOTAL BASE FLOW RECESSION DAYS
TDFP12	R	1	-	TIME OF DAILY FLOOD PEAK, 12-HOUR CLOCK
TDFP24	R	1	-	TIME OF DAILY FLOOD PEAK, 24-HOUR CLOCK
TDSF	R	1	CFS	TOTAL DAILY STREAMFLOW
TFCFS	R	1	CFS	CURRENT TOTAL FLOW
TFMAX	R	1	CFS	MAXIMUM TOTAL FLOW DURING CURRENT DAY
THETA	R	1	FT/SEC	DELTA X / DELTA T
THSF	R	24	CFS	TOTAL HOURLY STREAMFLOW
TIKD	R	1	-	TOTAL INTERFLOW RECESSION DAYS
TITLE	A	20	-	TITLE OF CURRENT STATION YEAR (STREAM GAGE LOCATION AND DATE)
TMBF	R	12	IN	TOTALS OF MONTHLY BASE FLOW
TMIF	R	12	IN	TOTALS OF MONTHLY INTERFLOW
TMNET	R	12	IN	TOTALS OF MONTHLY NET EVAPOTRANSPIRATION
TMOF	R	12	IN	TOTALS OF MONTHLY OVERLAND FLOW
TMPEF	R	12	IN	TOTALS OF MONTHLY POTENTIAL EVAPOTRANSPIRATION
TMPRFC	R	12	IN	TOTALS OF MONTHLY PRECIPITATION
TMKTF	R	12	SFD	TOTALS OF MONTHLY RECORDED TOTAL FLOW
TMSE	R	12	IN	TOTALS OF MONTHLY STREAM EVAPORATION
TMSTF	R	12	SFD	TOTALS OF MONTHLY SYNTHESIZED TOTAL FLOW
TMSTFI	R	12	IN	TOTALS OF MONTHLY SYNTHESIZED TOTAL FLOW IN INCHES
IMTFCY	R	12	SFD	TOTALS OF MONTHLY TOTAL FLOW BY CALENDAR YEAR

TMTFWY	R	12	SFD	TOTALS OF MONTHLY TOTAL FLOW BY WATER YEAR
TOFR	R	1	IN	CURRENT TOTAL OVERLAND FLOW RUNOFF
TPLR	R	15	-	TOTAL TO PERVIOUS LAND RATIO
TPLR1	R	1	-	TEMPORARY STORAGE OF TPLR FOR REACH 1
TRIP	I	1	-	COUNTER SPECIFYING PROGRAM PORTIONS
URHF	R	1	IN	CURRENT UNROUTED HYDROGRAPH FLOW
UZC	R	1	IN	UPPER ZONE STORAGE CAPACITY
UZINLZ	R	1	IN/HR	CURRENT UPPER ZONE INFILTRATION TO LOWER ZONE
UZKX	R	1	-	UPPER ZONE MOISTURE RETENTION INDEX
UZS	R	1	IN	CURRENT UPPER ZONE STORAGE
UZSA	R	15	IN	UZS FOR SUBAREA
VDCY	R	365	-	VALUE DATED BY CALENDAR DAY
VDMO	R	12	-	VALUE DATED BY MONTH DAY
VINCR	R	15	IN	VEGETATIVE INTERCEPTION - CURRENT RATE PER PERIOD
VINMRA	R	15	IN/HR	VEGETATIVE INTERCEPTION - MAXIMUM RATE
VINMRA1	R	1	IN/HR	TEMPORARY STORAGE OF VINMRA FOR REACH 1
VWIN	R	1	SFD	VOLUME OF AN INCH OF RUNOFF FROM WATERSHED
VWINA	R	15	SFD	VWIN FOR SUBAREA
VWINA1	R	1	SFD	TEMPORARY STORAGE OF VWINA FOR REACH 1
WCFS	R	1	CFS	WATERSHED CFS EQUALLING ONE INCH PER HOUR
WCFSA	R	15	CFS	WCFS FOR SUBAREA
WCFSA1	R	1	CFS	TEMPORARY STORAGE OF WCFSA FOR REACH 1
WEIFS	R	1	IN	WATER ENTERING INTERFLOW STORAGE
WFDX	R	1	-	WINTER FLOW DEVIATION INDEX
WI	R	1	IN	WATER REMAINING AFTER INFILTRATION INCLUDING INTERFLOW
WSG	R	15	-	WEIGHTING FACTOR FOR STORAGE RAIN GAGE
WSG2	R	15	-	SECOND WEIGHTING FACTOR FOR STORAGE RAIN GAGE
WSM	R	1	-	NUMBER OF WET SUMMER MONTHS
XAREA	R	150,21	SQ FT	END AREA OF WATER DURING ROUTING
XAREA1	R	1	SQ FT	1ST SOLUTION OF END AREA DURING NONLINEAR ROUTING
XAREA2	R	1	SQ FT	2ND SOLUTION OF END AREA DURING NONLINEAR ROUTING
XMPFT	R	12	-	INDEX OF MONTHLY PREDOMINATE FLOW TYPE
YEAR	I	1	-	LAST TWO DIGITS OF CURRENT YEAR
YR1	I	1	-	LAST TWO DIGITS OF FIRST CALENDAR YEAR IN WATER YEAR
YR2	I	1	-	LAST TWO DIGITS OF SECOND CALENDAR YEAR IN WATER YEAR

APPENDIX C

INPUT DATA FOR SOUTH FORK BEARGRASS CREEK AT LOUISVILLE, KY.
1970 WATER YEAR

* NUMBER OF CASES AND TITLE CARD *

1
SOUTH FORK BEARGRASS CREEK

* CONTROL OPTIONS *

2 1 0 0 1 1 0

* OTHER CONTROL DATA *

6 1 2 400
3 3 10 1 5
0 0 2
1 2
4 2
1 3
50.00

* CHANNEL CROSS SECTIONS *

5 0 1 2300.0000

3.2	1.27	33.	32.42	66.	44.02	132.	78.39
264.	128.23						
1.39	1.33						
5 0	1 3000.0000						
5.	40.27	57.5	66.22	102.	73.19	204.	96.52
408.	126.72						
0.57	1.67						
5 0	1 3000.0000						
11.	6.11	106.67	37.31	213.6	80.24	429.	148.53
858.	270.68						
1.14	1.33						
5 0	1 3000.0000						
24.6	18.69	246.	61.81	492.	116.09	984.	232.24
1968.	519.82						
0.47	1.67						
5 0	1 3000.0000						
29.5	18.26	296.2	133.06	592.5	231.05	1185.	465.42
2370.	978.65						
0.32	1.67						
5 0	1 3000.0000						
30.	15.44	300.	75.45	600.	125.96	1200.	229.47
2400.	449.58						
0.3	1.67						
5 0	1 3000.0000						
35.6	26.3	356.6	96.51	713.3	153.63	1426.6	285.99
2520.	544.28						
1.5	1.33						
5 0	1 3000.0000						
40.4	27.5	415.	145.96	830.	238.55	1660.	485.66
3320.	1038.69						
0.3	1.67						
5 0	1 3000.0000						
46.	29.9	460.	623.87	920.	714.24	2828.	1039.27
3680.	2153.66						
0.22	1.67						

5	0	1	3000.0000					
65.		40.43	650.	202.26	1301.82	357.08	2596.	917.86
5207.		2374.64						
0.66		1.33						
2	1	522400.						
0.46		1.33	999999.					
5	0	1	2800.0000					
79.		52.01	790.	234.47	1580.	402.44	3160.	1051.03
6320.		2005.65						
0.1		1.67						
5	0	1	2800.0000					
79.		41.50	790.	204.66	1580.	370.64	3160.	759.87
6320.		1773.33						
0.3		1.67						
5	0	1	2800.0000					
84.		51.89	840.	269.44	1980.	465.13	3360.	932.30
6720.		1825.93						
0.11		1.67						
5	0	1	2800.0000					
84.		54.65	840.	253.55	1680.	427.51	3360.	943.14
6720.		2225.86						
0.25		1.67						
5	0	1	3200.					
86.		39.95	860.	204.43	1720.	525.24	3440.	2171.87
6880.		5696.58						
0.8		1.33						

* HYDRAULIC STRUCTURES *								

0								

* REACH PARAMETERS *								

29300.		7.2	0.023	0.035	1119.4890	0.015	0.068	0.005
22400.		4.66	0.017	0.035	918.3501	0.015	0.034	0.005

14500.	5.25	0.023	0.035	803.02	0.015	0.0082	0.005
1.03	0.96	1.	1.	1.	1.	1.	1.
0.97	1.05	1.	1.	1.	1.	1.	1.
0.99	1.02	1.	1.	1.	1.	1.	1.
1.	1.	1.					
0.	0.						
0.12	0.00	0.11	0.00	0.10	0.00		

 * CLIMATOLOGICAL DATA *

70 71
 46.21 105.

RECORDED DAILY STREAM FLOW

3.20	2.20	1.80	1.60	1.60	1.50	1.30	4.40	18.00	17.00
3.00	48.00	146.00	48.00	29.00	21.00	15.00	12.00	7.80	40.00
22.00	11.00	8.00	6.20	5.40	4.90	6.60	17.00	11.00	8.00
5.20	8.00	7.00	6.00	5.00	4.30	3.80	3.20	3.50	4.00
4.70	3.90	3.30	2.90	21.00	19.00	11.00	9.30	7.80	7.00
94.00	24.00	20.00	15.00	12.00	11.00	9.30	7.80	5.50	5.10
4.80	4.20	3.40	5.30	7.00	4.10	3.60	3.30	3.00	2.90
2.70	11.00	55.00	17.00	13.00	11.00	62.00	38.00	29.00	24.00
19.00	153.00	321.00	77.00	48.00	39.00	32.00	27.00	23.00	19.00
17.00	15.00	13.00	12.00	16.00	91.00	30.00	23.00	21.00	19.00
16.00	14.00	12.00	11.00	112.00	71.00	41.00	32.00	27.00	23.00
20.00	18.00	14.00	14.00	12.00	11.00	10.00	7.80	6.30	7.80
7.50	8.60	8.40	6.80	4.40	4.10	53.00	79.00	33.00	26.00
23.00	22.00	20.00	22.00	59.00	58.00	36.00	29.00	29.00	36.00
29.00	91.00	69.00	72.00	664.00	113.00	59.00	45.00	49.00	38.00
30.00	25.00	21.00	23.00	18.00	15.00	34.00	26.00	20.00	17.00
43.00	30.00	23.00	20.00	17.00	58.00	23.00	19.00	17.00	29.00
19.00	0.42	0.29	0.08	0.04	0.02	0.01	0.14	0.03	0.03
0.0	0.0	0.0	0.0	0.07	0.46	0.07	0.04	0.07	0.02
0.0	0.0	0.06	57.00	1.40	20.00	4.10	4.10	3.70	3.30
2.70	2.40	3.10	4.20	2.80	2.30	1.90	2.00	26.00	22.00

7.80	4.20	2.60	12.00	2.60	2.00	3.00	278.00	132.00	50.00
34.00	26.00	24.00	27.00	62.00	32.00	24.00	19.00	15.00	12.00
15.00	9.30	7.00	5.30	4.80	17.00	180.00	21.00	41.00	22.00
16.00	14.00	11.00	9.30	12.00	6.30	5.00	4.20	2.90	2.20
2.70	1.70	1.60	1.60	6.30	1.70	33.00	170.00	29.00	12.00
7.00	4.80	3.90	179.00	33.00	16.00	9.30	6.30	4.80	4.10
3.90	3.90	4.10	24.00	6.30	3.40	2.70	5.30	46.00	16.00
8.50	87.00	43.00	48.00	24.00	52.00	42.00	13.00	7.80	5.30
840.00	198.00	50.00	36.00	27.00	21.00	17.00	18.00	22.00	18.00
14.00	25.00	17.00	10.00	3.50	11.00	103.00	113.00	46.00	31.00
23.00	17.00	12.00	11.00	7.00	5.10	4.40	3.20	2.70	2.20
1.80	19.00	3.50	1.80	1.60	2.90	1.80	1.70	1.60	12.00
1.60	1.40	1.30	1.30	13.00	1.70	1.80	1.20	1.10	0.97
80.00	4.20	2.50	1.70	1.10	7.00	5.30	1.30	0.71	0.57
0.54	0.53	0.51	0.53	58.00	3.40	2.60	1.70	1.40	2.10
7.00	3.40	1.40	1.40	1.20					

STRDAGE GAGE

1

100	0	17							
281	0.52	283	0.27	286	2.55	287	0.24	288	0.26
293	1.00	294	0.05	298	0.05	301	0.15	302	0.33
306	0.24	313	0.07	314	0.18	318	0.45	319	0.17
324	0.65	338	0.35	345	0.30	346	0.47	350	0.74
351	0.23	355	0.95	356	1.08	357	0.06	365	0.05
4	1.15	14	0.44	23	0.07	28	0.03	30	0.25
34	0.29	35	0.52	36	0.26	38	0.01	39	0.54
43	0.17	44	0.05	48	0.06	51	0.54	53	2.68
57	0.38	62	0.26	65	0.41	66	0.04	69	0.58
74	0.72	78	0.34	85	0.13	91	0.28	92	0.32
96	0.38	104	0.17	113	0.20	118	0.87	122	0.26
125	0.02	126	1.30	127	0.99	128	0.07	131	0.11
132	0.22	133	0.58	140	0.28	144	0.06	145	0.77
147	0.57	153	0.25	163	0.13	164	0.12	165	0.42
166	1.05	167	2.20	172	1.46	173	0.17	182	0.17

186	0.14	187	0.97	191	0.04	192	1.02	193	0.32
195	0.24	200	2.43	205	0.10	207	0.90	210	0.61
211	0.16	214	0.30	215	0.28	216	0.62	231	0.16
234	0.03	238	0.51	243	0.31	245	0.58	249	0.48
254	1.03	260	0.07	263	0.90	269	0.19	270	0.05
1	0.51								
2	0.42								
3	0.27								

RECORDING GAGE

4954	70	10	8	1								
0.0	0.01	0.07	0.05	0.03	0.01	0.01	0.02	0.04	0.01	0.0	0.0	
4954	70	10	8	2								
0.03	0.07	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
4954	70	10	9	2								
0.01	0.0	0.0	0.08	0.02	0.14	0.0	0.09	0.03	0.0	0.0	0.0	
4954	70	10	12	1								
0.01	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.01	
4954	70	10	12	2								
0.0	0.0	0.0	0.0	0.60	0.13	0.04	0.12	0.35	0.20	0.02	0.45	
4954	70	10	13	1								
0.03	0.0	0.0	0.04	0.0	0.0	0.17	0.18	0.02	0.03	0.01	0.0	
4954	70	10	13	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.01	
4954	70	10	14	1								
0.05	0.04	0.10	0.13	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
4954	70	10	14	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.02	0.0	0.0	0.0	0.0	0.0	
4954	70	10	19	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.03	0.05	
4954	70	10	20	1								
0.07	0.13	0.13	0.09	0.02	0.02	0.05	0.0	0.0	0.03	0.01	0.03	
4954	70	10	20	2								
0.02	0.01	0.02	0.0	0.0	0.0	0.0	0.01	0.03	0.0	0.0	0.0	
4954	70	10	28	1								

0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.03	0.04
4954	70	10	28	2							
0.07	0.01	0.03	0.03	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.01
4954	70	10	29	1							
0.02	0.02	0.01	0.08	0.05	0.10	0.01	0.0	0.01	0.0	0.0	0.01
4954	70	11	2	1							
0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.07	0.08	0.04	0.0	0.0
4954	70	11	9	2							
0.0	0.0	0.05	0.01	0.03	0.03	0.0	0.0	0.0	0.0	0.06	0.03
4954	70	11	10	1							
0.01	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	70	11	14	1							
0.0	0.0	0.0	0.0	0.0	0.03	0.03	0.04	0.06	0.09	0.05	0.02
4954	70	11	14	2							
0.05	0.03	0.03	0.01	0.04	0.08	0.05	0.02	0.01	0.01	0.01	0.0
4954	70	11	19	2							
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.02	0.01	0.08	0.18
4954	70	11	20	1							
0.08	0.17	0.03	0.0	0.0	0.0	0.0	0.01	0.0	0.0	0.0	0.0
4954	70	11	22	1							
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.02	0.04	0.0	0.0
4954	70	11	23	1							
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.02	0.0	0.0	0.0
4954	70	11	29	2							
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.0	0.0	0.0	0.0
4954	70	12	3	1							
0.0	0.0	0.09	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	70	12	3	2							
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.14	0.0	0.0
4954	70	12	11	2							
0.01	0.13	0.18	0.0	0.02	0.0	0.0	0.0	0.0	0.02	0.03	0.14
4954	70	12	12	1							
0.16	0.06	0.09	0.02	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	70	12	16	1							
0.0	0.05	0.10	0.10	0.10	0.04	0.07	0.08	0.01	0.04	0.02	0.01

0.22	0.03	0.03	0.01	0.01	0.02	0.04	0.01	0.0	0.0	0.0	0.0	0.0
4954	71	2 4	2									
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.27	0.05
4954	71	2 5	1									
0.02	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	2 7	1									
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.0	0.0	0.0	0.0
4954	71	2 7	2									
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.02	0.01
4954	71	2 8	1									
0.01	0.05	0.03	0.04	0.06	0.03	0.04	0.02	0.08	0.07	0.10	0.08	0.08
4954	71	2 8	2									
0.05	0.04	0.03	0.0	0.01	0.01	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	2 12	1									
0.01	0.0	0.01	0.0	0.01	0.0	0.0	0.02	0.0	0.01	0.02	0.01	0.01
4954	71	2 12	2									
0.0	0.01	0.0	0.09	0.06	0.09	0.08	0.10	0.10	0.09	0.07	0.03	0.03
4954	71	2 13	1									
0.04	0.03	0.02	0.03	0.01	0.01	0.0	0.0	0.02	0.01	0.01	0.01	0.0
4954	71	2 17	1									
0.0	0.0	0.0	0.0	0.0	0.03	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	2 19	2									
0.0	0.0	0.0	0.02	0.03	0.10	0.30	0.07	0.0	0.0	0.0	0.0	0.0
4954	71	2 21	2									
0.0	0.0	0.0	0.0	0.0	0.0	0.08	0.22	0.20	0.21	0.37	0.50	0.50
4954	71	2 22	1									
0.32	0.26	0.14	0.20	0.03	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	2 26	1									
0.0	0.0	0.02	0.08	0.01	0.03	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	2 26	2									
0.0	0.08	0.08	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	3 2	2									
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.02
4954	71	3 3	1									
0.05	0.05	0.01	0.0	0.0	0.0	0.03	0.08	0.04	0.01	0.02	0.03	0.03

4954	71	3	3	2								
0.0	0.01	0.02	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	3	6	1								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.11
4954	71	3	6	2								
0.13	0.01	0.01	0.03	0.0	0.03	0.01	0.02	0.0	0.0	0.0	0.0	0.0
4954	71	3	10	1								
0.0	0.0	0.01	0.03	0.04	0.06	0.08	0.07	0.06	0.06	0.03	0.03	
4954	71	3	10	2								
0.02	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	3	15	1								
0.0	0.0	0.0	0.0	0.47	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	3	18	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.08	0.06	0.03	
4954	71	3	19	1								
0.04	0.01	0.02	0.02	0.01	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	3	19	2								
0.01	0.0	0.0	0.0	0.01	0.0	0.01	0.01	0.01	0.01	0.01	0.01	0.0
4954	71	3	20	1								
0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	3	25	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.01	0.03	0.0	0.01	0.0	
4954	71	3	26	1								
0.0	0.0	0.0	0.0	0.0	0.02	0.0	0.01	0.0	0.0	0.0	0.0	0.0
4954	71	4	1	1								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.02	0.03	0.0	
4954	71	4	1	2								
0.10	0.10	0.04	0.05	0.06	0.03	0.01	0.15	0.05	0.02	0.0	0.0	
4954	71	4	6	1								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.04	0.02	0.04	0.09	
4954	71	4	6	2								
0.01	0.05	0.01	0.03	0.03	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	4	13	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.07	0.08	0.0	0.0	0.0	0.0
4954	71	4	21	2								

4954	71	5	24	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.02	0.58	0.79	0.05	
4954	71	5	27	1								
0.0	0.0	0.0	0.02	0.22	0.12	0.05	0.01	0.01	0.14	0.03	0.0	
4954	71	6	1	2								
0.0	0.0	0.0	0.0	0.02	0.01	0.0	0.01	0.01	0.0	0.0	0.0	
4954	71	6	8	1								
0.0	0.0	0.0	0.0	0.02	0.03	0.02	0.01	0.0	0.0	0.0	0.0	
4954	71	6	12	1								
0.0	0.0	0.0	0.0	0.0	0.03	0.07	0.0	0.0	0.0	0.0	0.0	
4954	71	6	14	1								
0.14	0.06	0.02	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
4954	71	6	14	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.50	0.20	0.15	0.01	0.0	0.0	
4954	71	6	15	1								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.17	
4954	71	6	15	2								
0.0	0.0	0.0	0.0	0.02	0.0	0.04	0.10	0.01	0.0	0.0	0.0	
4954	71	6	21	2								
0.36	0.25	0.06	0.03	0.0	0.0	0.0	0.0	0.0	0.0	0.02	0.01	
4954	71	6	22	1								
0.01	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
4954	71	6	30	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.07	0.14	0.03	
4954	71	7	5	1								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.02	0.08	
4954	71	7	5	2								
0.02	0.02	0.0	0.0	0.0	0.0	0.0	0.0	0.20	0.0	0.0	0.0	
4954	71	7	6	1								
0.0	0.0	0.0	0.0	0.0	0.68	0.18	0.01	0.0	0.0	0.0	0.0	
4954	71	7	9	2								
0.0	0.0	0.05	0.29	0.01	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
4954	71	7	10	2								
0.0	0.0	0.0	0.0	0.0	0.02	0.20	0.36	0.02	0.01	0.0	0.0	
4954	71	7	11	2								

4954	71	8	22	1								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.03	0.01
4954	71	8	25	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.07
4954	71	8	26	1								
0.03	0.0	0.09	0.12	0.02	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	9	1	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.01	0.01	0.0
4954	71	9	2	2								
0.0	0.20	0.0	0.01	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	9	6	1								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.14
4954	71	9	6	2								
0.86	0.15	0.07	0.06	0.05	0.02	0.0	0.0	0.01	0.0	0.0	0.0	0.0
4954	71	9	11	1								
0.0	0.0	0.0	0.0	0.0	0.22	0.46	0.27	0.04	0.03	0.05	0.0	0.0
4954	71	9	11	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.66
4954	71	9	16	1								
0.0	0.08	0.01	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4954	71	9	20	1								
0.0	0.0	0.16	0.16	0.04	0.12	0.38	0.09	0.0	0.0	0.0	0.0	0.0
4954	71	9	25	2								
0.0	0.0	0.0	0.0	0.0	0.0	0.18	0.0	0.03	0.0	0.0	0.0	0.0
4954	71	9	26	1								
0.0	0.0	0.0	0.0	0.08	0.01	0.0	0.0	0.06	0.0	0.0	0.0	0.0
4954	98											

APPENDIX D

PERMEABILITY AND AVAILABILITY WATER
CAPACITY OF KENTUCKY SOILS

Soil Series	Perm. in./hr	AWC in.	Soil Series	Perm. in./hr	AWC in.
Adler	2.53	7.54	Crevasse	10.00	4.22
Allegheny	2.93	7.17	Crider	1.34	11.25
Armour	0.91	10.26	Cruze	2.11	8.27
Ashton	2.53	10.25	Cuba	1.31	13.39
Ashwood	0.50	2.97	Culleoka	2.07	5.68
Atkins	3.12	9.11	Cumberland	1.31	8.47
			Cynthiana	1.32	3.17
Barbourville	5.50	5.99			
Baxter	2.08	6.87	Danridge	1.32	3.02
Beasley	1.74	7.75	Dekalb	3.79	3.70
Bedford	1.19	11.18	Dekoven	1.32	8.40
Beknap	1.32	9.50	Dewey	1.32	8.06
Berks	3.47	3.22	Dickson	1.84	10.04
Beulah	7.50	3.90	Donahue	4.15	4.37
Bewleyville	3.75	5.84	Donerail	1.32	8.73
Birds	1.48	11.66	Dowellton	1.32	8.76
Blago	3.75	8.19	Dubbs	1.65	8.94
Bondine	4.34	2.64	Dundee	1.65	8.88
Bonnie	1.30	12.13	Dunning	1.09	7.82
Brandon	1.40	7.41			
Brashear	0.91	8.56	Eden	0.88	5.14
Braxton	1.36	7.11	Egan	1.40	8.12
Brookside	1.30	8.52	Elk	1.47	9.19
Bruno	7.77	3.95	Enders	1.40	6.24
Burgin	0.13	7.22	Ennsi	3.50	5.40
			Epley	1.30	9.74
Calloway	1.43	10.19	Etowah	2.12	9.86
Caneyville	2.22	5.26			
Captina	1.62	8.65	Fairmount	1.20	2.57
Chagrin	1.30	6.90	Falaya	1.16	11.37
Chavies	4.15	9.10	Faywood	1.32	6.12
Christian	2.77	7.09	Fleming	1.88	5.32
Clarksville	2.76	5.09	Forestdale	0.50	7.75
Clifty	2.02	5.55	Frankstown	3.78	4.72
Clymer	4.15	5.29	Frederick	1.30	16.36
Colbert	0.86	6.61	Fredonia	1.10	4.54
Collins	1.95	10.20	Fronfort	1.31	4.64
Colyer	2.26	1.92			
Commerce	1.08	9.72	Garmon	1.31	3.47
Cookeville	3.75	13.53	Gilpin	1.35	4.71
Corydon	1.36	4.04	Ginat	1.34	8.20
Cotaco	4.12	7.15	Grenada	1.42	10.10
Cranston	4.15	8.75	Guthrie	1.98	8.39

Soil Series	Perm. in./hr	AWC in.	Soil Series	Perm. in./hr	AWC in.
Hagerstown	2.88	9.63	Newark	2.03	8.99
Hamblen	1.32	10.28	Nicholson	2.53	7.70
Hampshire	0.95	9.66	Nolickucky	3.54	11.58
Hartsells	4.12	4.54	Nolin	1.30	12.98
Hayter	2.45	9.55			
Heitt	1.32	6.18	Otway	1.01	3.38
Henry	0.50	13.54	Otwell	1.32	8.49
Henshaw	1.32	10.45			
Holston	2.45	8.47	Patton	1.33	11.16
Humphreys	3.88	5.46	Pembroke	1.73	10.69
Huntington	3.35	7.98	Philo	2.86	8.49
			Pickwick	1.30	13.97
Jacob	0.42	6.71	Pope	4.70	7.28
Jefferson	4.53	5.72	Purdy	1.22	8.73
Johnsburg	1.94	9.64			
			Ramsey	5.68	1.57
Karnak	0.13	7.35	Rarden	1.97	5.45
			Renox	2.26	7.63
Lakin	6.90	4.07	Righley	4.15	5.87
Landisburg	2.61	7.96	Robertsville	1.43	8.90
Lanton	0.66	8.85	Robinsonville	3.22	6.82
Latham	1.31	5.42	Rockcastle	0.99	4.44
Lawrence	1.39	9.93	Roellen	1.32	7.61
Lindside	1.54	9.26	Russellville	1.37	11.81
Linker	4.00	6.40			
Litz	2.04	4.71	Sadler	1.30	9.93
Loradale	2.04	8.94	Sango	2.13	8.60
Loring	1.42	9.96	Salvisa	2.77	3.91
Loudon	1.32	8.55	Sciotoville	1.34	8.16
Lowell	1.74	6.21	Sees	1.65	7.86
			Sequatchie	3.95	6.44
Markland	1.34	6.75	Sharkey	0.39	8.59
Maury	3.93	11.51	Shelbyville	2.88	8.20
McAfee	3.79	3.68	Shelocta	1.31	8.06
McGary	1.36	7.51	Shrouts	1.03	3.91
Melvin	1.37	9.74	Skidmore	4.15	4.31
Memphis	1.35	10.91	Staser	3.14	6.77
Mercer	2.04	8.79	Steff	1.30	7.79
Monogahela	2.77	8.40	Steinsburg	4.15	2.21
Montevallo	7.50	1.92	St endal	4.11	7.76
Montgomery	0.42	7.42			
Morehead	1.32	10.97	Taft	1.60	8.62
Morganfield	1.32	9.24	Talbott	1.66	6.96
Mountview	1.92	9.44	Torklin	1.31	6.52
Mullins	1.87	8.90	Tate	3.47	7.68
Muse	1.90	8.42	Tilsit	1.27	9.15
Muskingum	5.23	4.76	Trappist	2.07	5.45
			Trimble	1.31	11.74
Needmore	1.82	5.72	Tunica	0.13	7.35

Soil Series	Perm. in./hr	AWC in.
Tyler	2.68	8.92
Uniontown	1.32	9.77
Upshur	0.13	6.54
Vicksburg	2.45	10.06
Wakeland	1.43	11.47
Waverly	1.42	9.84
Waynesboro	1.30	14.85
Weibach	1.04	8.44
Weikert	1.32	1.58
Wellston	1.64	7.33
Westmoreland	3.63	3.24
Wheeling	1.34	8.35
Whitley	1.31	8.91
Whitewell	4.10	8.28
Wilbour	1.32	9.50
Wolftever	1.75	7.46
Woolper	1.29	7.76
Zaleski	1.40	8.05
Zanesville	1.34	10.54
Zipp	0.50	6.02