Research Report KTC-94-8

FOLLOW-UP INSPECTION OF PREMATURE CRACKING ON THE NATIONAL TURNPIKE, JEFFERSON COUNTY

by

L. John Fleckenstein Engineering Geologist

and

David L. Allen Chief Research Engineer

Kentucky Transportation Center College of Engineering University of Kentucky Lexington, Kentucky

in cooperation with Transportation Cabinet Commonwealth of Kentucky

and

Federal Highway Administration U.S.Department of Transportation

The contents of this report reflect the views of the authors who are responsible for the facts and accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of the University of Kentucky, the Kentucky Transportation Cabinet, nor the Federal Highway Administration. This report does not constitute a standard, specification, or regulation. The inclusion of manufacturer names and trade names are for identification purposes and are not to be considered as endorsements.

March 1994

Technical Report Documentation Page

1. Report No. КТС-94-8	2. Government Accession	No.	3. Recipient's Catalog No	• • •
4. Title and Subtitle Follow-Up Investigation of Premature	Creaking on the		5. Report Date March 1994	- 6 ₀ -1
National Turnpike, Jefferson County	Clacking on the		6. Performing Organizatio	n Code
7. Author(s) L. J. Fleckenstein and David L. All	en		8. Performing Organization KTC-94-8	n Report No.6
9. Performing Organization Name and Address			10. Work Unit No. (TRAIS)
Kentucky Transporta College of Engineeri University of Kentuc Lexington, KY 4050	ng ky		11. Contract or Grant No. Federal Aid Research Task Project F841-1(39)	
12. Sponsoring Agency Name and Address Kentucky Transporta State Office Building			13. Type of Report and P 14. Sponsoring Agency C	
Frankfort, KY 4062	2	·		
15. Supplementary Notes Publication of this report with the U.S. Departmen				
16. Abstract This report discusses the invertex Turnpike, Jefferson County since to performance to date. A detailed distret The distress survey indicate inspection in 1990. An intense maintenance pro-	1989. A report was ess survey was perfo es that substantial p	issued in 1990, K rmed evaluating eac avement cracking h	TC-90-5. This report The stab throughout the stab throughout the stab throughout the stab throughout the stab stability of the stability of th	ort documents ne project site.
An intense maintenance pro	gram is recommende	;сц. 3		
17. Key Words Subgrade Stiffness Settlement Portland Cement Cracking Load Transfer Rock Stabiliza FWD Testing	ation	18. Distribution Statemen Unlimited	nt	
19. Security Classif. (of this report)	20. Security Classif. (of t	his page)	21. No. of Pages	22. Price
Unclassified	U	nclassified	39	

- di Xe

TABLE OF CONTENTS

and the second

· - ...

- 1 A S

INTRODUCTION	1
BRIEF OVERVIEW OF 1989-1990 FINDINGS	1
CONCLUSIONS	
RECOMMENDATIONS	3
APPENDIX A (CONCRETE SLAB DISTRESS)	6

INTRODUCTION

The National Turnpike, in Jefferson County (Project No. F 841-1(39)) was completed on May 3, 1989. At the recommendation of the Department of Highways' Geotechnical Branch, a considerable amount of subgrade stabilization was performed during construction. However, pavement cracking and settlement occurred throughout the project since project completion. On September 27, 1989, the Kentucky Transportation Cabinet and Federal Highway Administration requested the Kentucky Transportation Center staff to investigate the cause of the premature pavement failure and suggest possible remedial actions.

In March 1990, Research Report KTC-90-5, "Investigation of Premature Cracking on the National Turnpike, Jefferson County" was completed. In September and October 1993, KTC personnel conducted a follow-up inspection of the National Turnpike. This report includes a discussion of the findings that were documented in the original survey and presents the latest information.

Brief Overview of 1989-1990 Conditions

During the initial survey of the National Turnpike (1989-1990) five locations were identified where cracks in the slabs were associated with adjacent curb inlets and manholes. It was suspected that compaction around these inlets probably was not sufficient and did not provide adequate support for the slabs.

Additional distress was observed in two major areas where pavement damage appeared to be concentrated. The first area was from Station 84+50 to approximate Station 86+00, in the vicinity of Sinclair Street. The second major area of distress was from Station 108+20 to Station 113+20, near the south end of the project. Available cone penetrometer data indicated the subgrade was weaker in the vicinity of Sinclair Street.

Falling Weight Deflectometer data taken in 1990 showed the stiffness of the pavement structure was greater in the southbound lanes than in the northbound lanes. The southbound lanes were constructed largely on the old roadbed. It appeared the subgrade under the old structure had consolidated and was probably stiffer, and consequently, provided better support to the new structure.

Several dips in slab elevation were observed in the area of Sinclair Street and near the south end of the project.

A No. 3 stone was used in approximately 12-inch and 24-inch layers during construction to stabilize the subgrade. There appeared to be little or no difference in behavior between the 12-inch sections and the 24-inch sections. Considerable settlement was observed between Station 72+00 to 78+00 from 1989 to 1990. This area of the project had no stone layer. It appears that the addition of at least 12 inches of stone was beneficial.

The No. 3 stone did not meet the filter requirement for the DGA; therefore, it is possible that fines from the DGA may be filtering into the stone.

The subgrade soils consisted largely of silts with a large proportion of clay (25 to 35 percent). Repeated-load tests indicated that extremely small stresses in the saturated subgrade may produce significant deformations in the subgrade that could be reflected in settling of the slabs.

The northbound lanes were opened to traffic on July 8, 1988. In July and August of that year, the project area received abnormally high amounts of rainfall. The southbound lanes were opened to traffic on December 14, 1988. The previous month (November 1988) received well above the normal rainfall. It is suspected this excess moisture may have saturated and weakened the subgrade of the new pavement.

Water well data indicate the subgrade is probably in a state of near saturation for a large portion of the year.

Level elevations taken in 1989-1990 indicated that some shifting and settlement of the slabs had occurred since construction. The northbound lanes had settled more than the southbound lanes. Most settlement occurred from Station 72+00 to Station 78+00, area that has no stone layer. A second area where considerable settlement has occurred is from Station 85+00 to Station 95+00. This is in the area of Sinclair Street.

VISUAL SURVEY 1993

In August 1993, a detailed visual inspection was conducted of the study area. Distress which had occurred since the 1990 survey is logged on the distress data sheets in red (Appendix A). As indicated in Table 1, the southbound lanes had approximately 35 feet of additional cracking (13.4 % increase), the median had 25 feet of additional cracking (41.0 % increase), and the northbound side had approximately 360 feet of new cracking (82 % increase). A significant increase in spalling was also noted in the northbound lanes (Table 2).

A total of 79 slabs have been identified that will most likely need replacement. In 1990, 59 slabs were identified. Since this time, 20 additional slabs are showing signs of distress. Of the 59 specified in 1990, 13 of the 59 slabs are continuing to crack, and 16 adjacent slabs are also cracking. Significant faulting and cracking was observed at Station 82+12 and Station 75+04. Both of these stations are located adjacent to manholes (storm drains). The slabs needing to be replaced are identified by station numbers in Table 3.

RI data from 1989 to 1992 indicate that the northbound lanes have decreased at a more rapid rate on the southern end of the project from Fairdale Road to KY 841 (Milepost 2.669-3.011), and the southbound side has decreased at a more rapid rate on the north end of the project from KY 841 to KY 1065 (Milepost 3.011-3.661) (Table 4).

CONCLUSIONS

From the visual evaluation conducted in 1993, it is evident that the pavement is deteriorating at a more rapid rate on the northbound side due to the large increase in cracking since the 1990 survey.

Surface spalling also appears to be occurring at a more rapid rate in the northbound lanes.

Significant faulting and cracking were observed at Station 82+12 and Station 75+04. Both of these stations are located adjacent to manholes (storm drains). It appears that substantial settlement is still occurring around the some of the storm drains.

RECOMMENDATIONS

In 1990, several recommendations were made and those recommendations are reported here.

- 1. It is recommended that pavement distresses continue to be monitored every six months.
- 2. Longitudinal edge drains placed at the back of the curbs would help to intercept any surface water that infiltrated through the joints of the pavement and would otherwise become trapped at the interface between the DGA and the base of the slab. The edge drains would probably help to reduce pumping of the slab and the consequent loss of DGA fines. These edge drains are recommended. However, in all likelihood, this would alleviate only part of the problem. Edge drains would not significantly help the weak subgrade because the drains could not be placed sufficiently deep to effectively drain the subgrade.
- 3. An intensive and aggressive maintenance program is also recommended. Any future faulting at joints and/or new cracks should be maintained by grinding. Pavement undersealing could be used to help maintain grade. In some instances, as slabs and joints deteriorate, complete replacement is recommended. If a comprehensive maintenance program is implemented, the pavement may provide most of its original design life.
- 4. Significant faulting and cracking was observed at Station 82+12 and Station 75+04. Both of these stations are located adjacent to manholes (storm drains). These slabs need to be replaced. Table 3 also indicates several other slabs that need to be replaced.

TABLE 1. SLAB C	RACKING IN LINEAF	R FEET BETWEEN (1	989-1990) AN	4D (1990-199	3)]
(89-90) SOUTHBOUND	(90-93) Southbound	PERCENT INCREASE SOUTHBOUND	(89-90) MEDIAN	(90-93) MEDIAN	PERCENT INCREASE MEDIAN	(89-90) Northbound	(90-93) Northbound	PERCENT INCREASE NORTHBOUND
261	296	13.4	61	86	41.0	441	801	81.6

··· *

1003 1 1

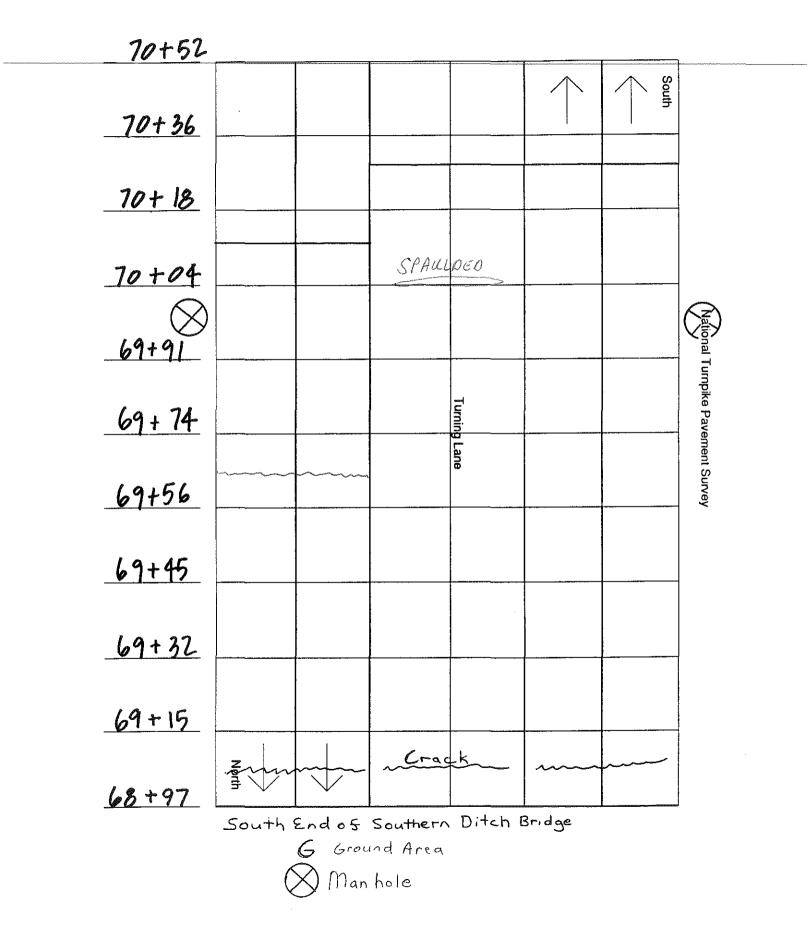
TABLE 2. NUMBER	R OF SPALLED ARE	AS BETWEE	N (1989-1990)) AND (1990-1993)	
(89-90) SOUTHBOUND	(90-93) SOUTHBOUND	(89-90) MEDIAN	(90-93) MEDIAN	(89-90) Northbound	(90-93) Northbound
1	3	0	1	3	13

TABLE 3, RECOMMEND	ED SLABS TO BE REPLACED		<u></u>	
YEAR DEFINED	STATION	DIRECTION	LANE	CRACKING SINCE LAST SURVEY
1993	69+56	Northbound	Both Lanes	New Crack
1989-1990	74+96	Northbound	Outside	Additional Cracking
1993	75+04	Northbound	Outside	New Crack
1993	75+58	Northbound	Outside	New Crack
1989-1990	78+14	Northbound	Both Lanes	
1993	78+32	Northbound	Both Lanes	New Crack
1989-1990	79+95	Northbound	Both Lanes	
1993	81+94	Northbound	Outside Lane	New Crack
1989-1990	82+12	Northbound	Outside Lane	Additional Cracking
1989-1990	84+79	Southbound	Both Lanes, Plus Turning Lanes	
1989-1990	84+98	Southbound	Outside Lane	
1989-1990	85+58	Southbound	Both Lanes	
1989-1990	91+46	Southbound	Outside Lane	
1993	92+07	Northbound	Both Lanes	New Crack
1993	95+60	Northbound	Inside Lanes	New Crack
1993	102+52	Southbound	Both Lanes, Plus Turning Lanes	New Crack
1989-1990	103+07	Southbound	Outside Lane	
1989-1990	104+33	Southbound	Both Lanes	
1989-1990	106+02	North and Southbound	Both Lanes	New Crack
1989-1990	106+19	Southbound	Outside Lane	

		VIII OR OTHER STATE		
TABLE 3, RECOMME	NDED SLABS TO BE REPLACED (CC	NTINUED)		
1989-1990	108+19 - 108+62	Southbound	Outside Lane	
1989-1990	108+80 - 108+94	Southbound	Inside Lane	
1989-1990	108+62	Southbound	Inside Lane	
1989-1990	108+45 - 108+80	Northbound	Both Lanes	Additional Cracking
1989-1990	109+25-109+43	Northbound	Both Lanes	Additional Cracking
1993	109+56	Northbound	Outside Lane	New Crack
1989-1990	109+68	Northbound	Outside Lane	Additional Cracking
1989-1990	109+25	Southbound	Both Lanes, Plus Turning Lane	Additional Cracking
1993	109+86	Northbound	Outside Lane	New Crack
1989-1990	110+03	Northbound	Outside Lane	Additional Cracking
1989-1990	110+16	Northbound	Inside Lane	
1989-1990	111+06	Northbound	Both Lanes	
1989-1990	112+09	Northbound	Outside Lane	
1989-1990	112+70 - 112+89	Northbound	Both Lanes	Additional Cracking
1989-1990	113+06	Northbound	Both Lanes	Additional Cracking
1993	116+69	Northbound	Both Lanes	New Crack
1989-1990	116+83	Northbound	inside Lane	
1989-1990	116+95 - 117+30	Northbound	Inside Lane	Additional Cracking
1993	117+56	Northbound	Both Lanes	New Crack
1993	117+74	Northbound	Inside Lane	New Crack

MILEPOST (2.669 - 3.011)			MILEPOST (3.011 - 3.661)		
DATE	NORTHBOUND	SOUTHBOUND	NORTHBOUND	SOUTHBOUND	
11/6/89	1.74	1.74	2.23	2.13	
5/7/90	1.34	1,66	1.54	1.76	
6/5/91	1.38	1.53	1.84	1.66	
6/17/92	1.46		1.63		
AVERAGE RI>	1.48	1.64	1.81	1.14	

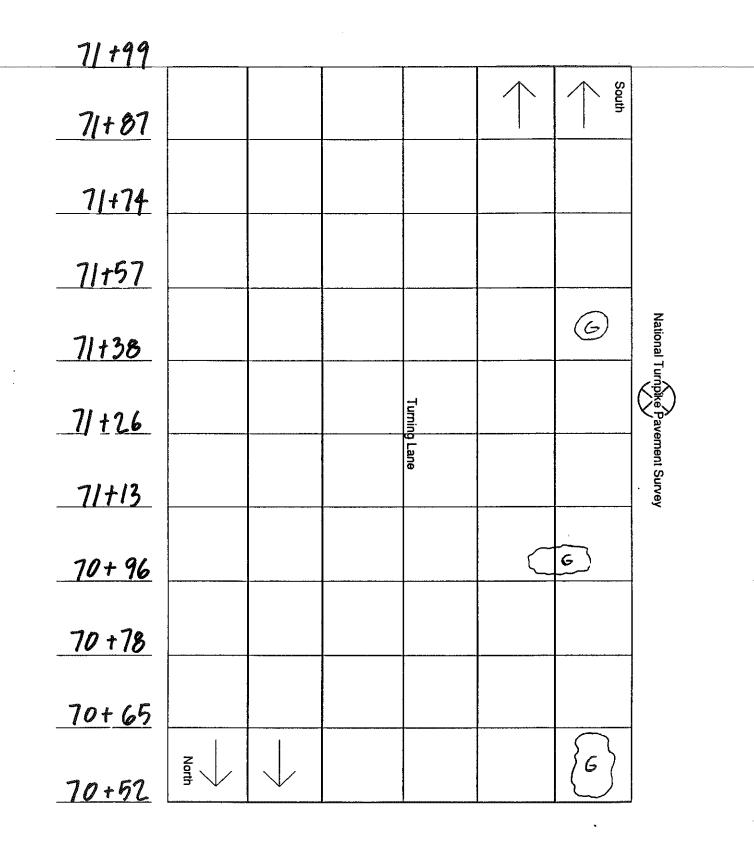
Appendix A Concrete Slab Distresses 1993

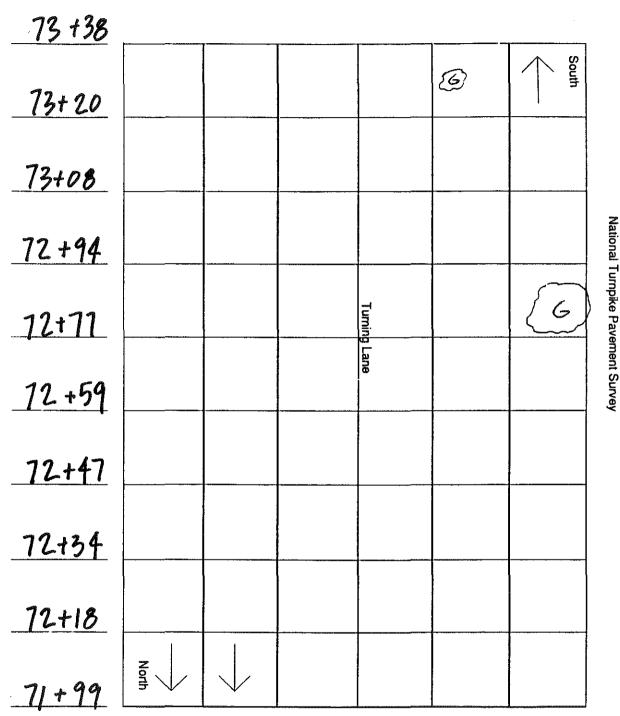


All and the second secon

1.142

.

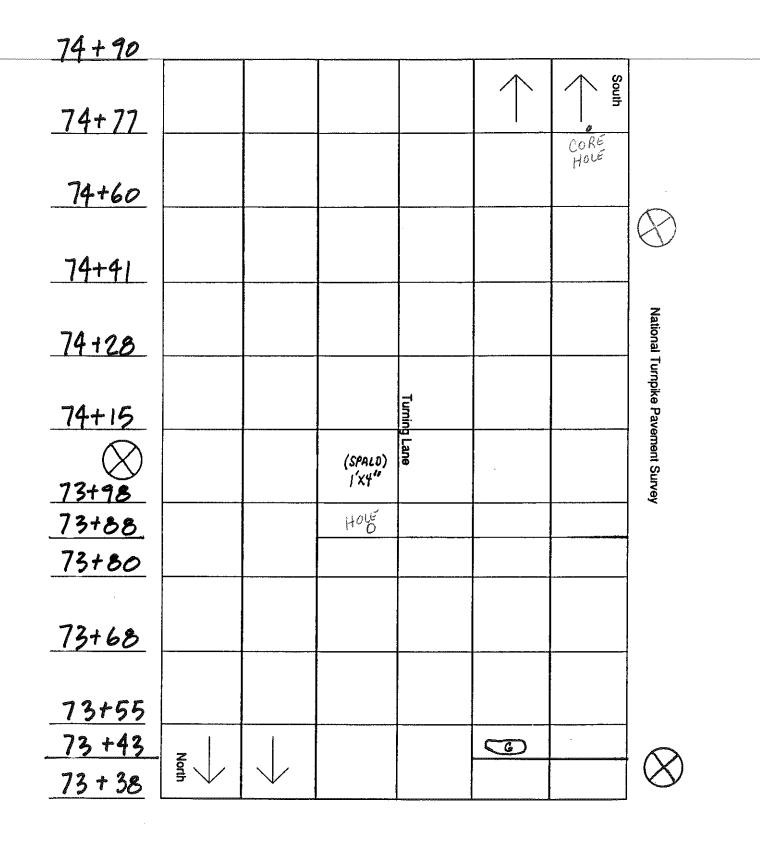


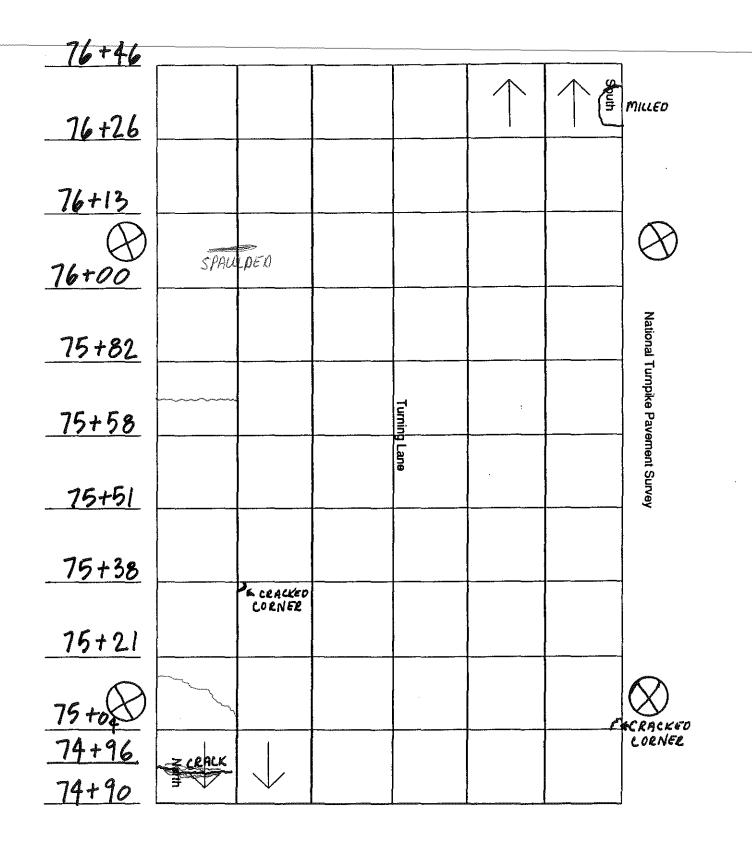


1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -

9

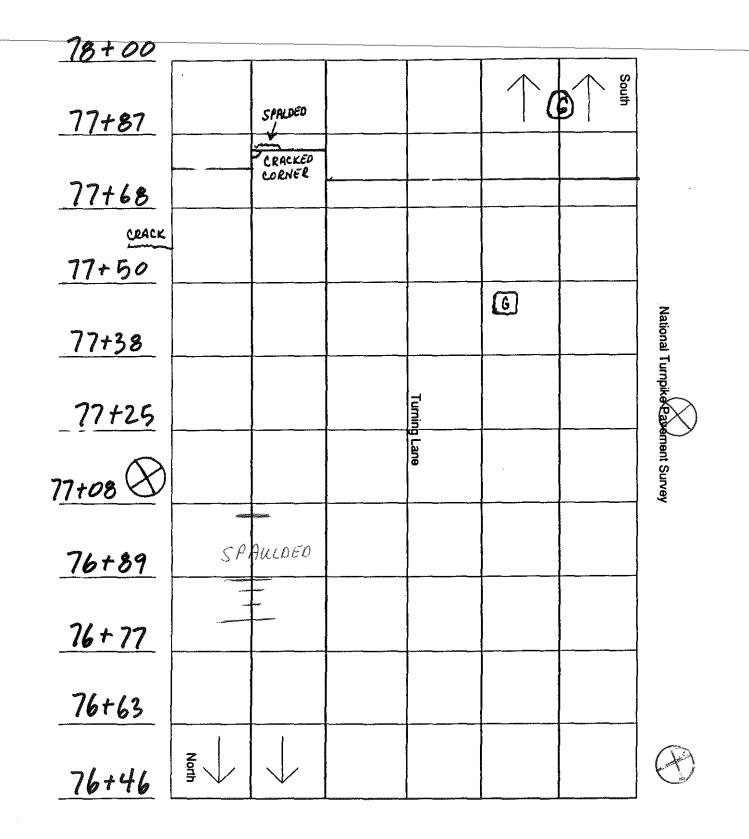
.





en en engenere Antiere is see⊻ere sin interest

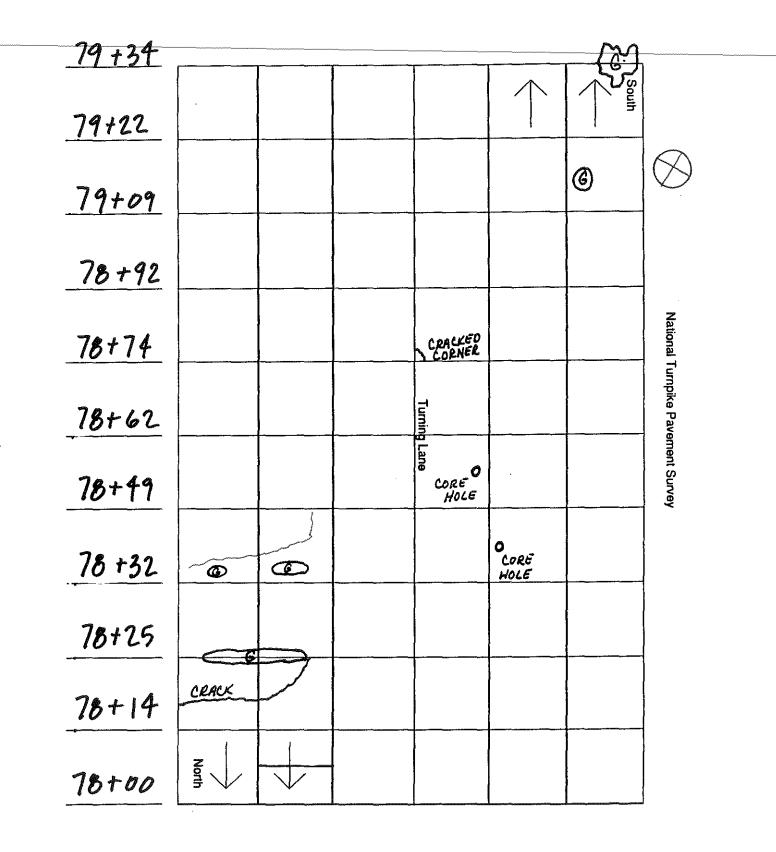
a second and a second as a



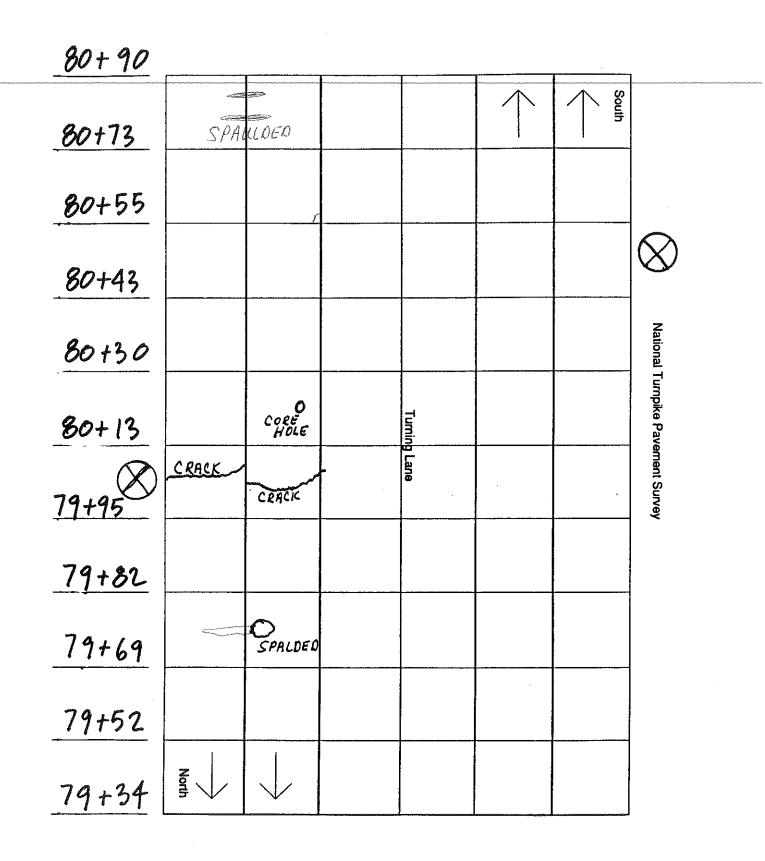
 Monormal Street Stre Street Str

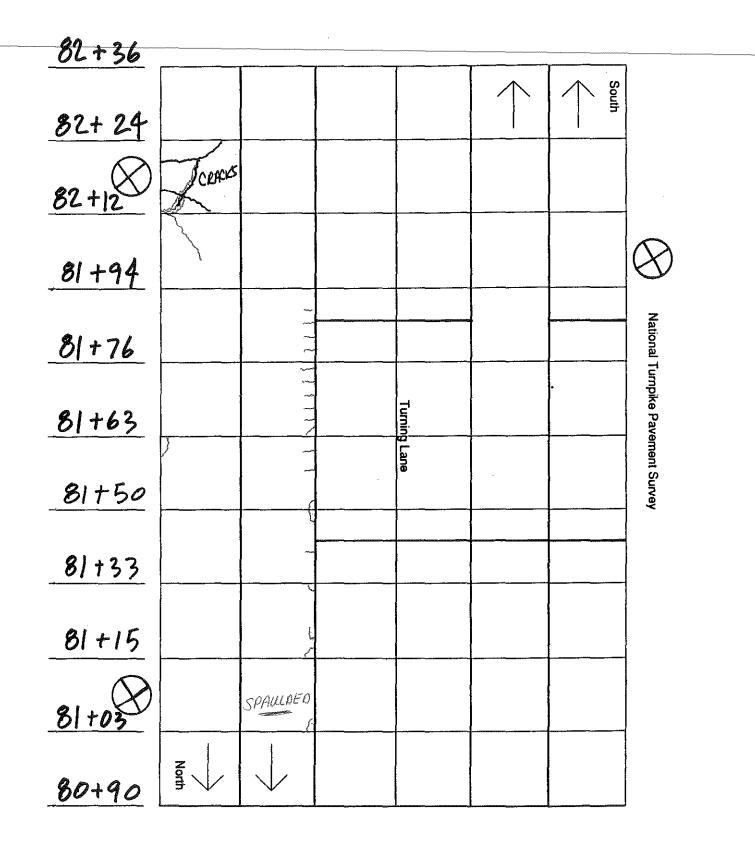
21.5

• • • .



. . .

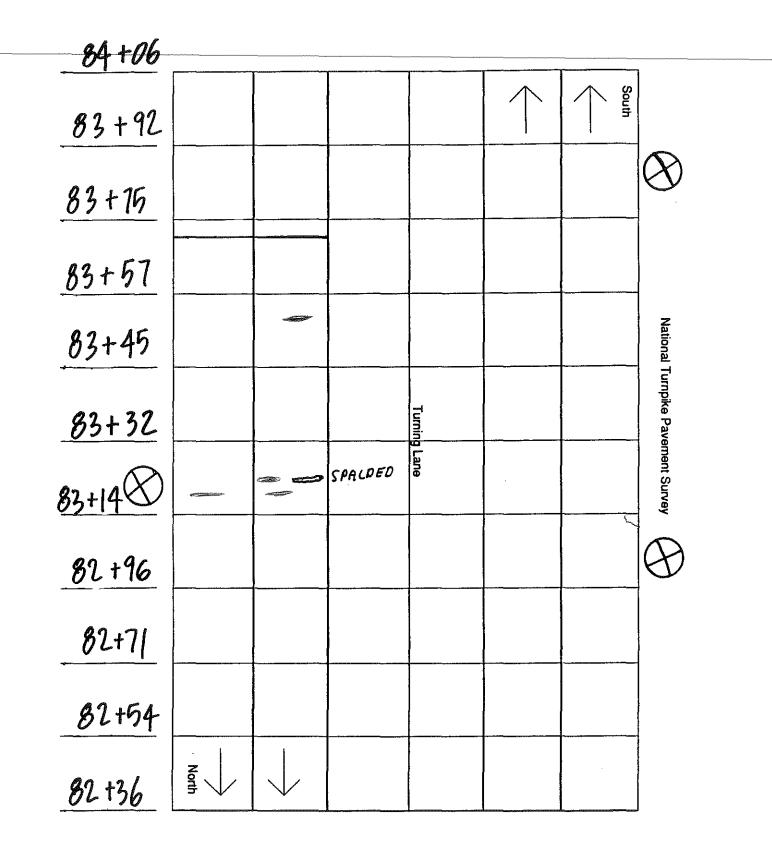




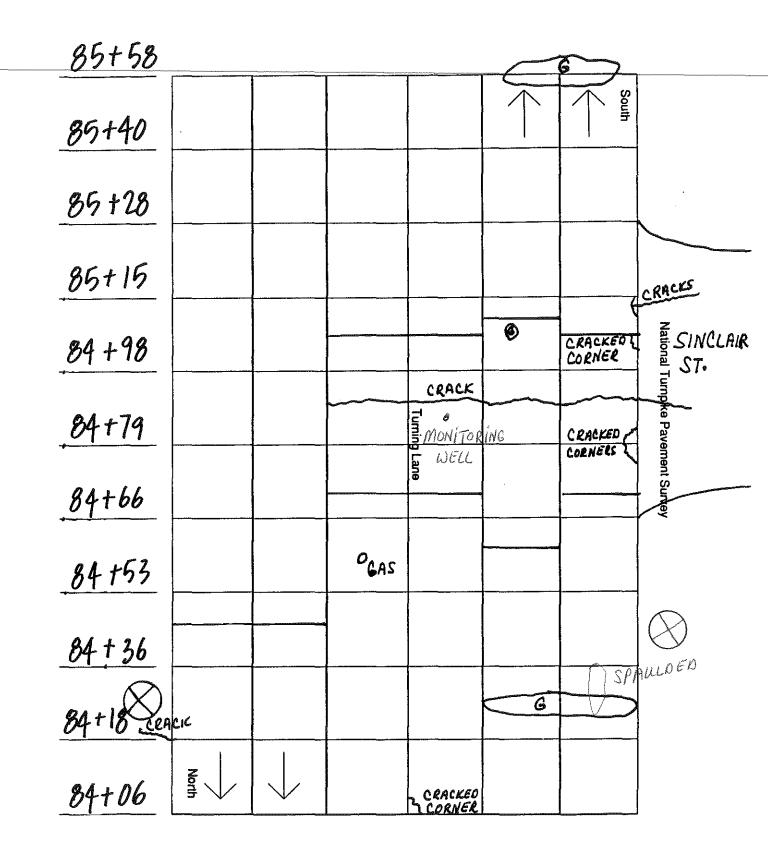
and the second second

n in state in a state of the st

1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -



n nagazan Pestika an an taona an taon an

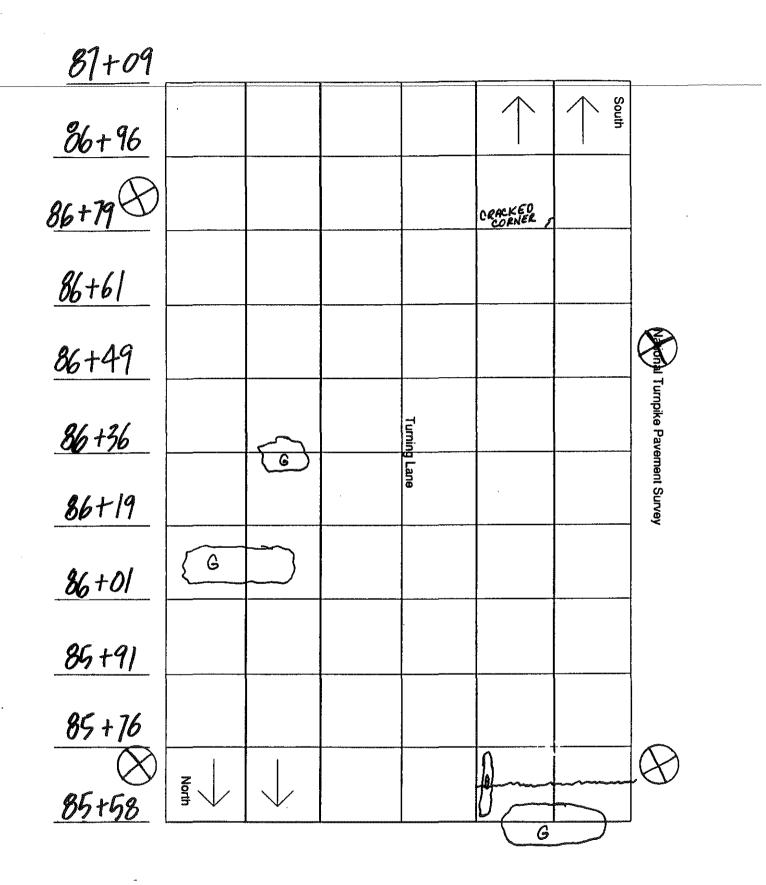


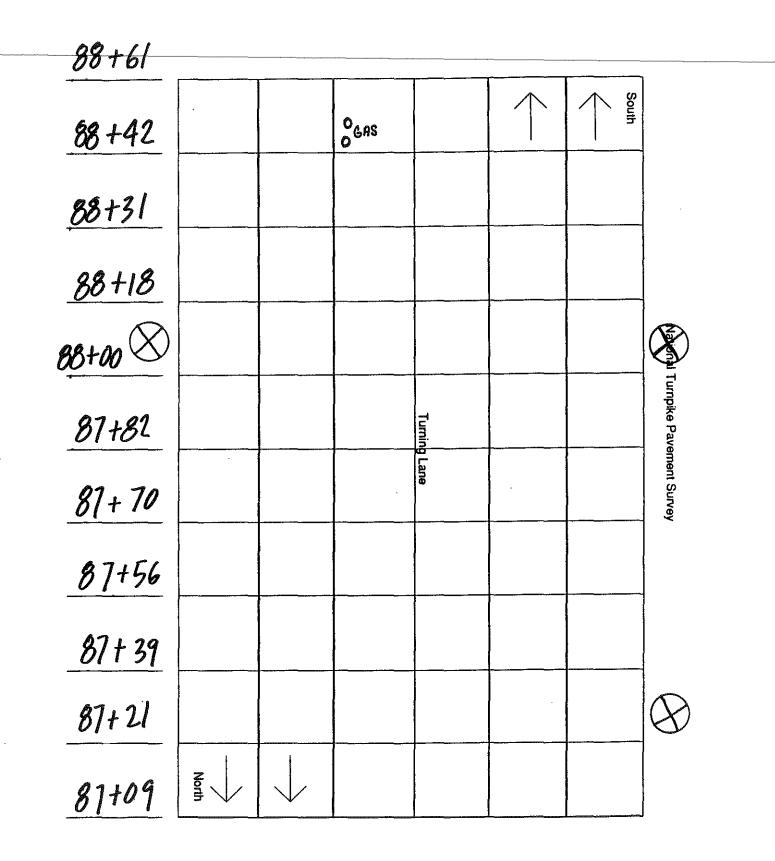
and the second second

-312 C

17

(





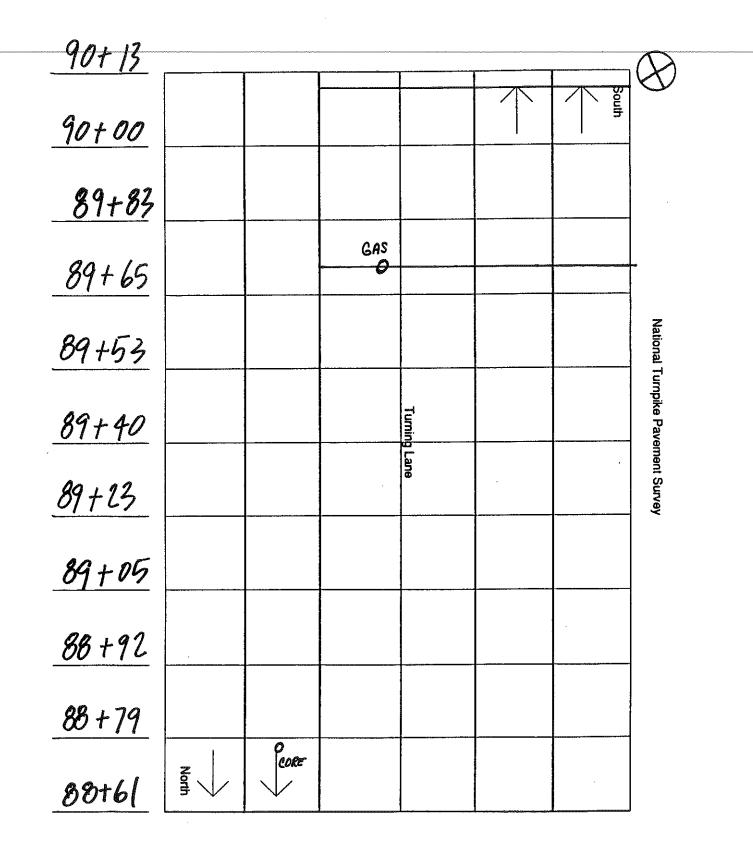
· · · · ·

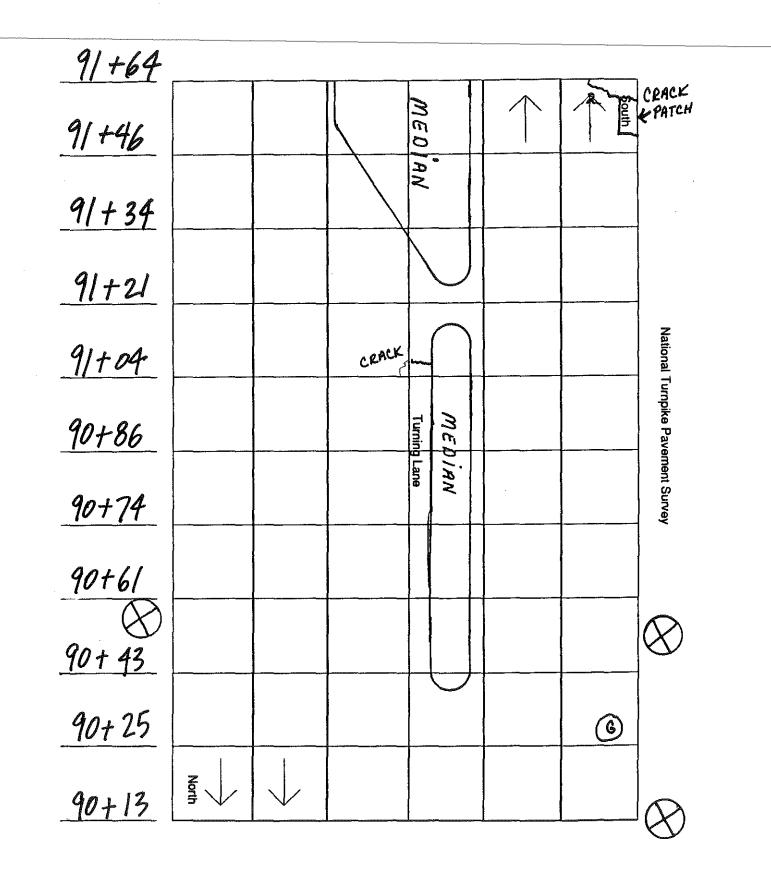
A second state of the second se

19

• •

· · · ·





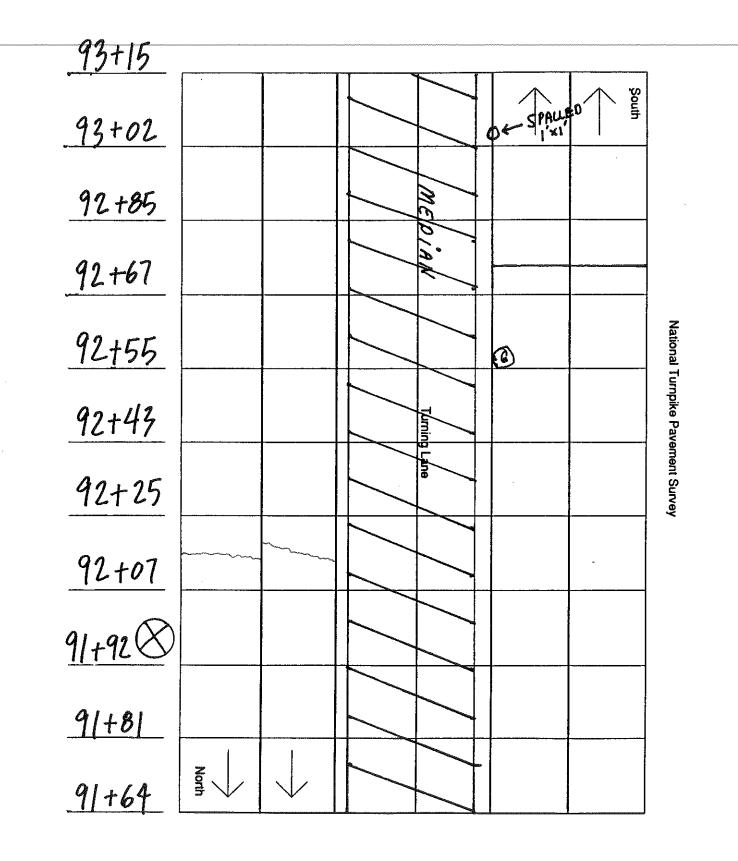
and a second second

1 - 12 general 1 - 12 general

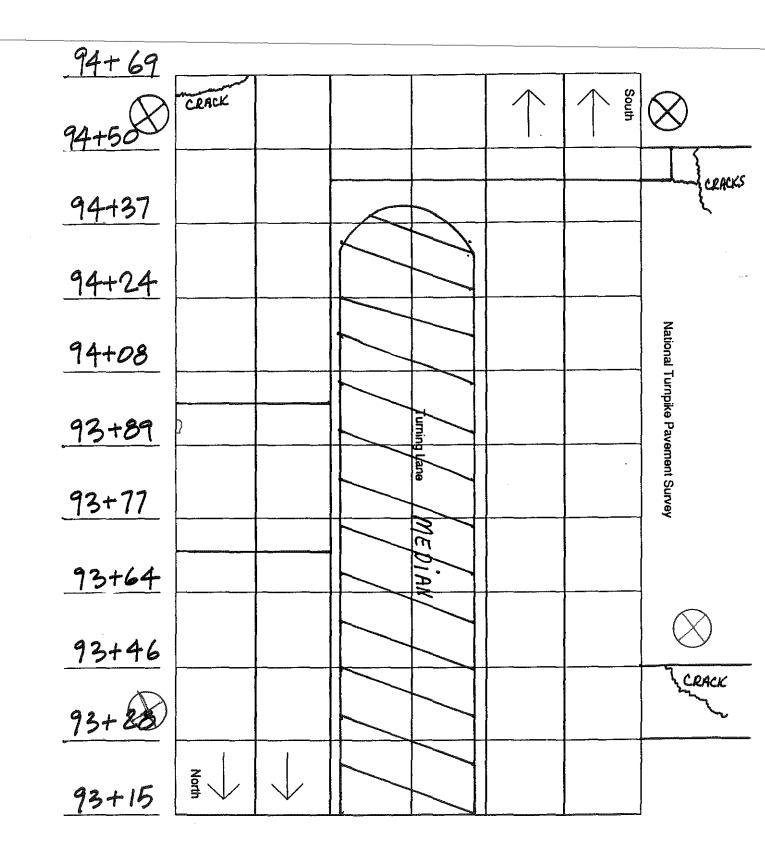
an an tao an t

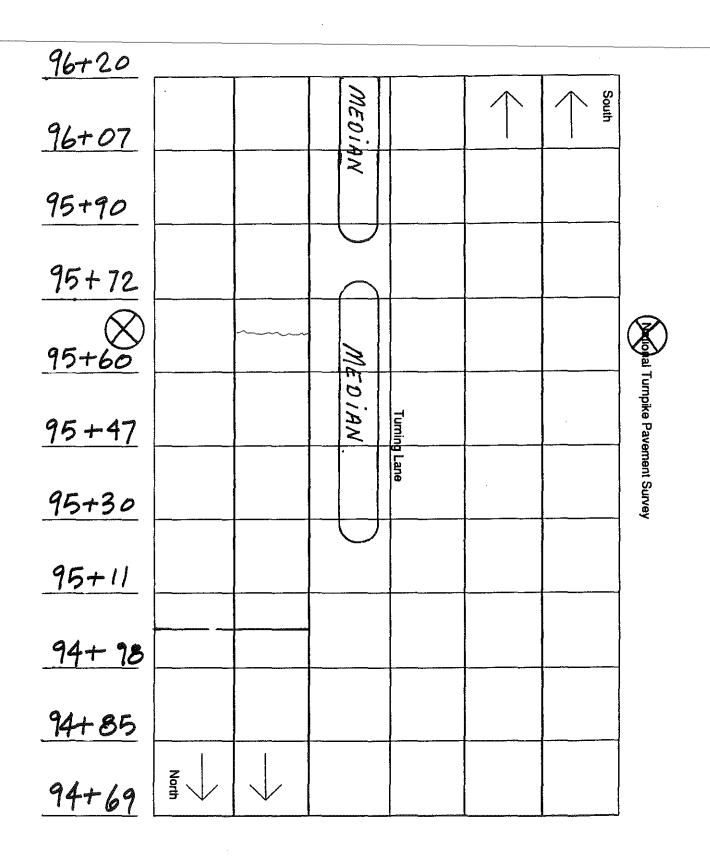
21

.



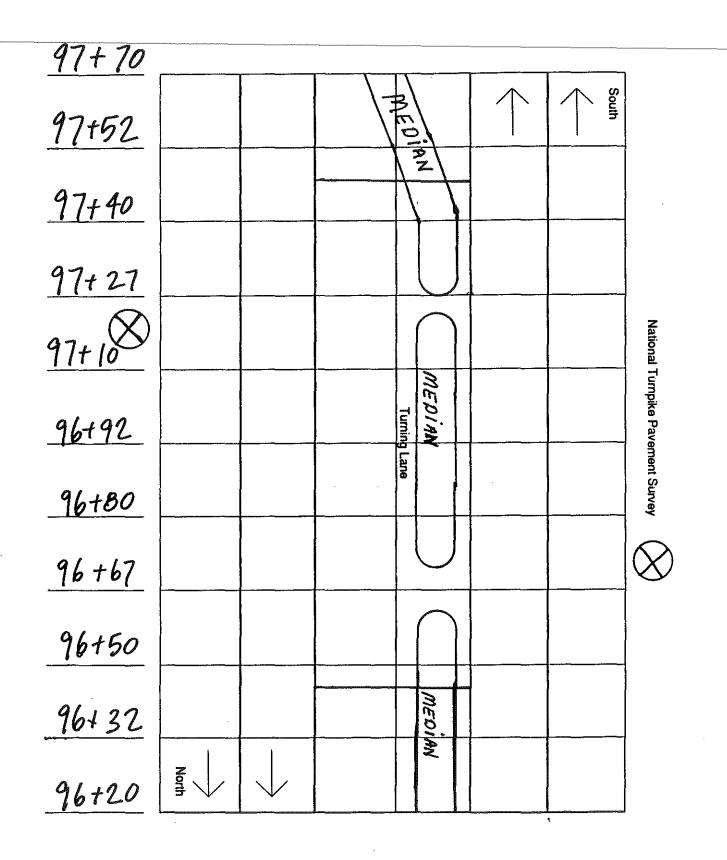
 f_{i}^{hp}





· - ...

 $\mathbf{24}$

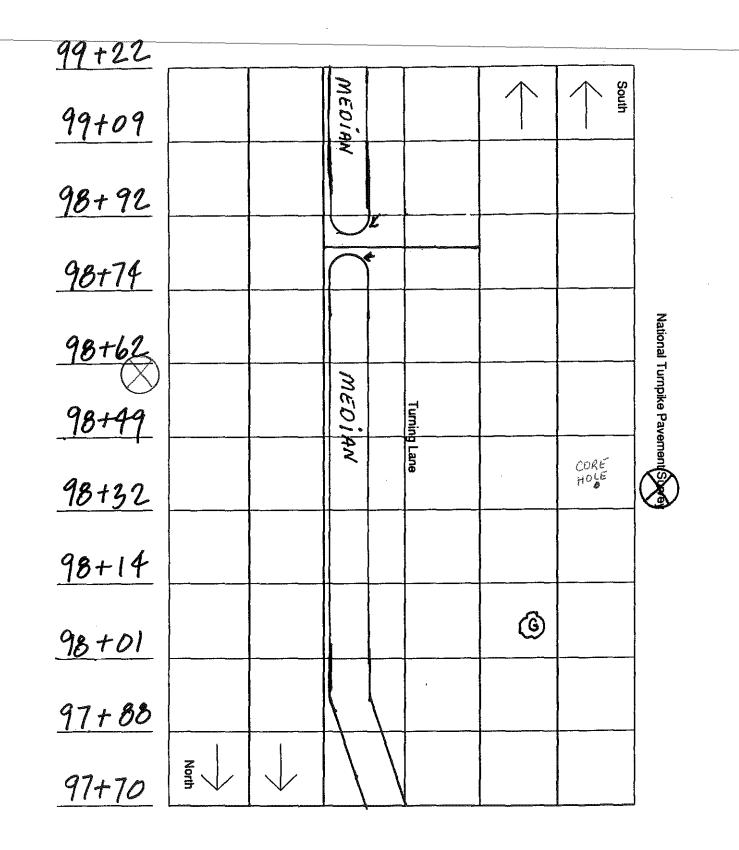


an in gayanna. Ta ta sa

2017 N

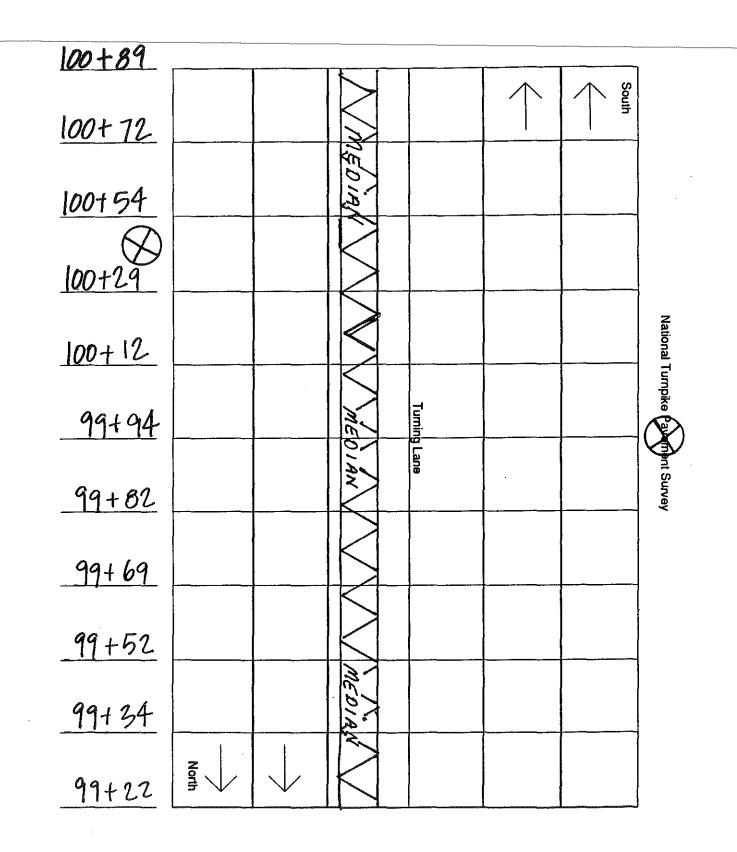
25

٠.



in tragger State

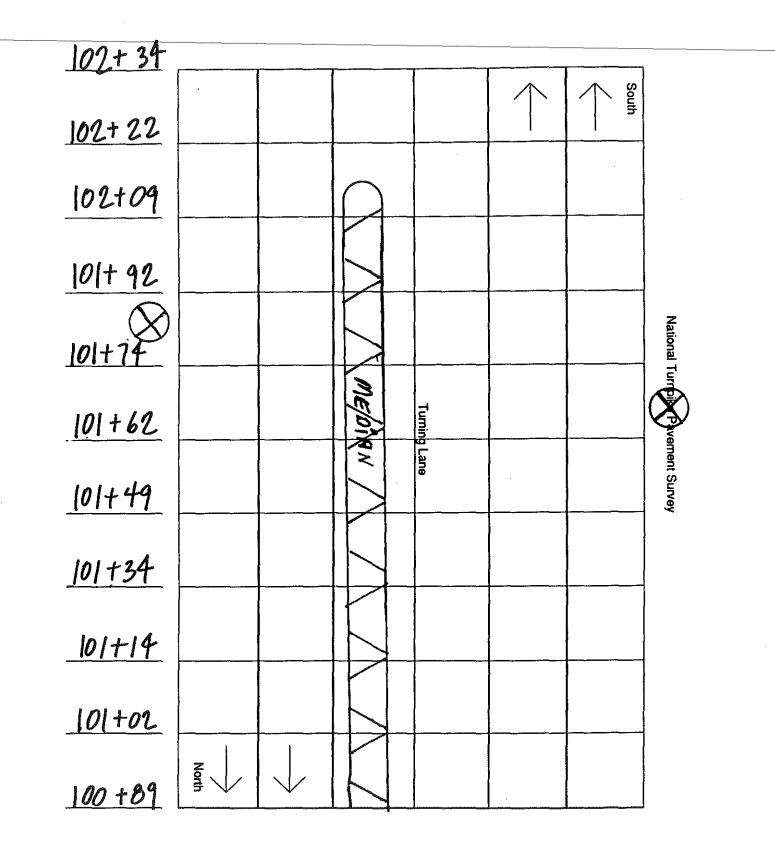
$\mathbf{26}$



and the second sec

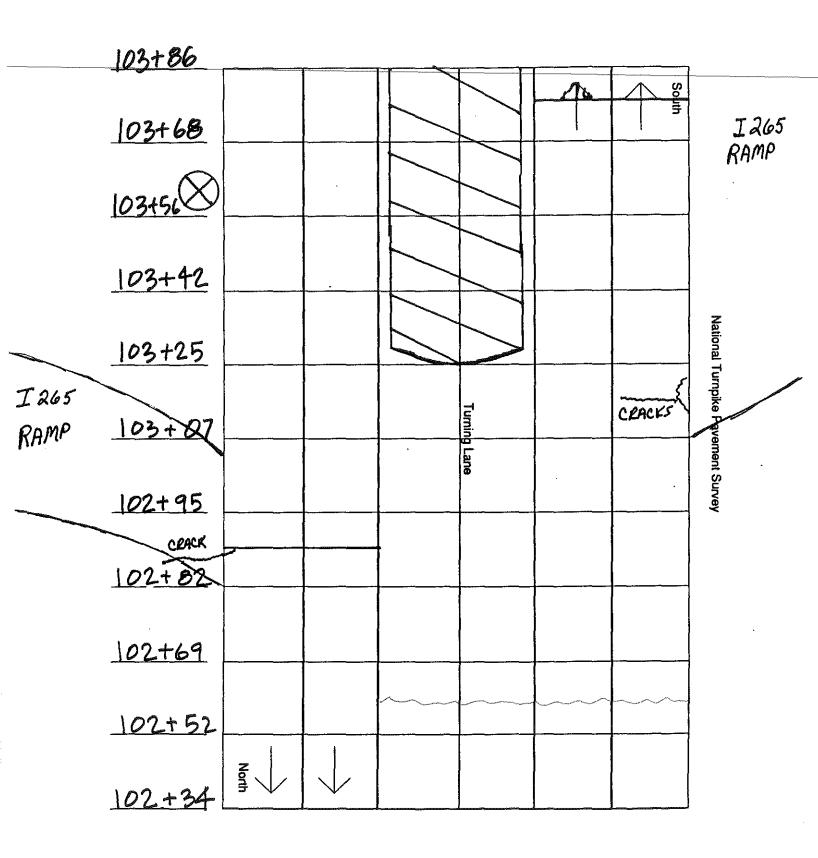
n ngarnin Sti≰tig and the second sec

14.20



1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 -1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 28

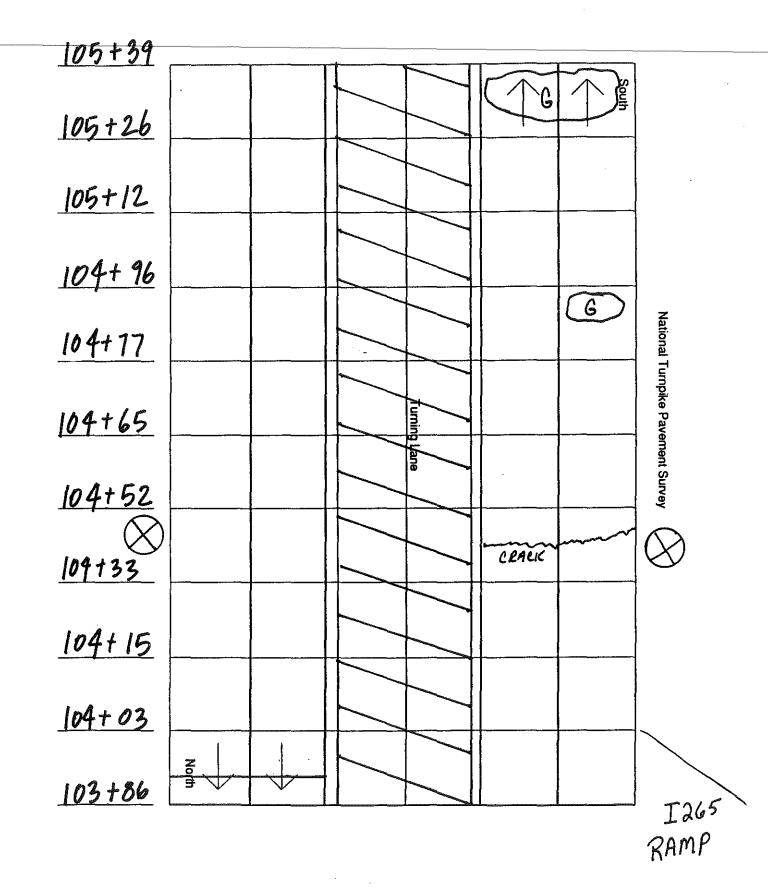
• -



and Kar

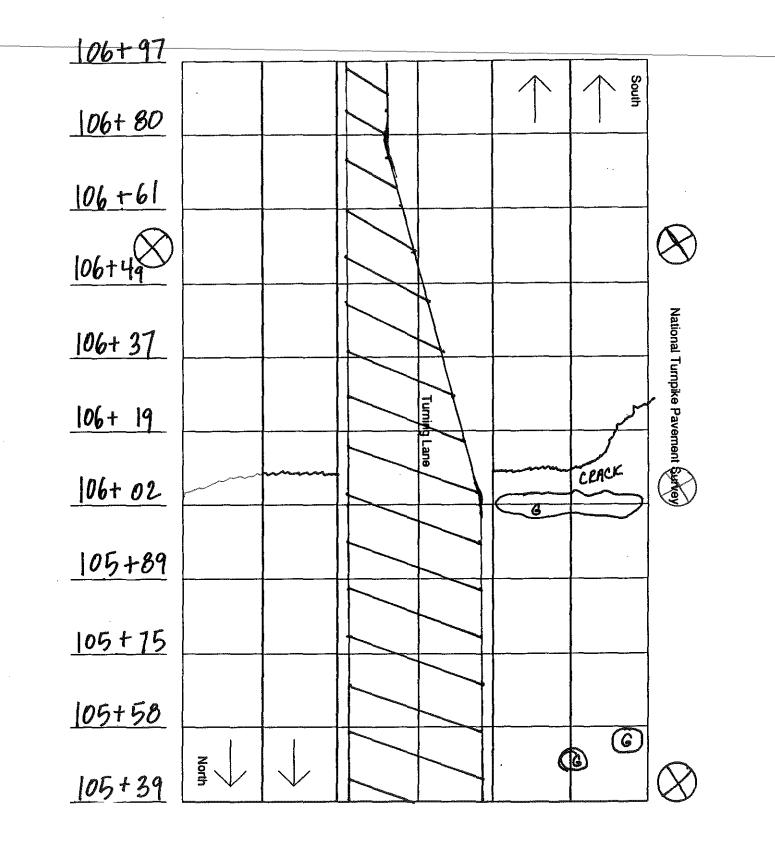
• .

- -



an de la sur 🗧 😞

1997) 1997 - 1997 1997 - 1997



a subdemander and a subserver a

n in standigeren er Standigeren

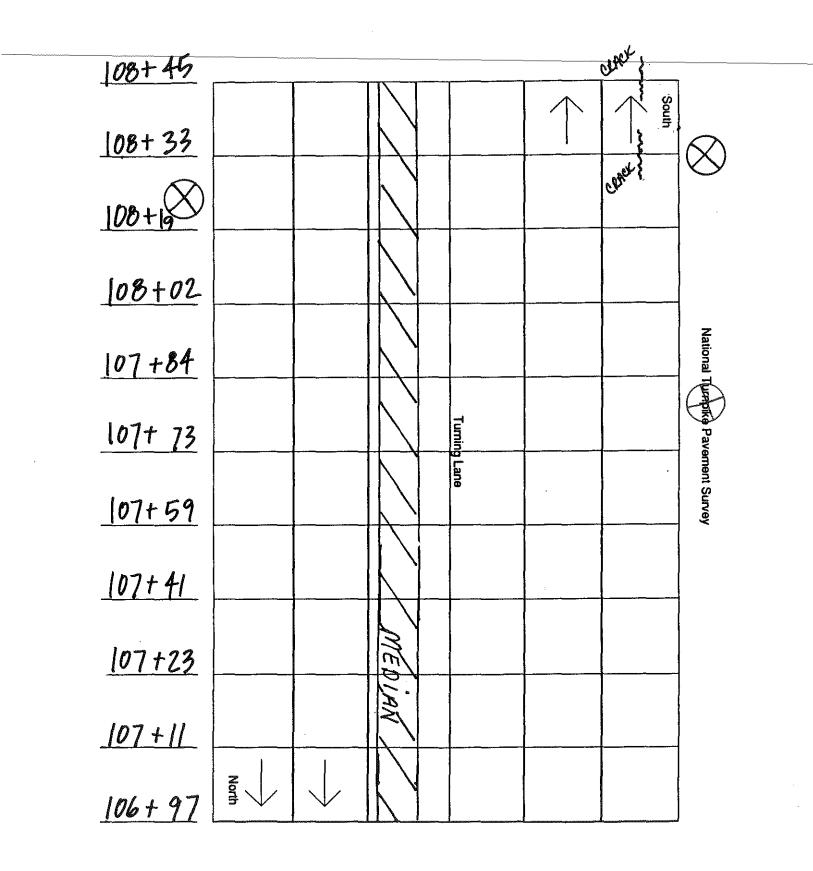
and the second secon

 $\mathbf{31}$

,

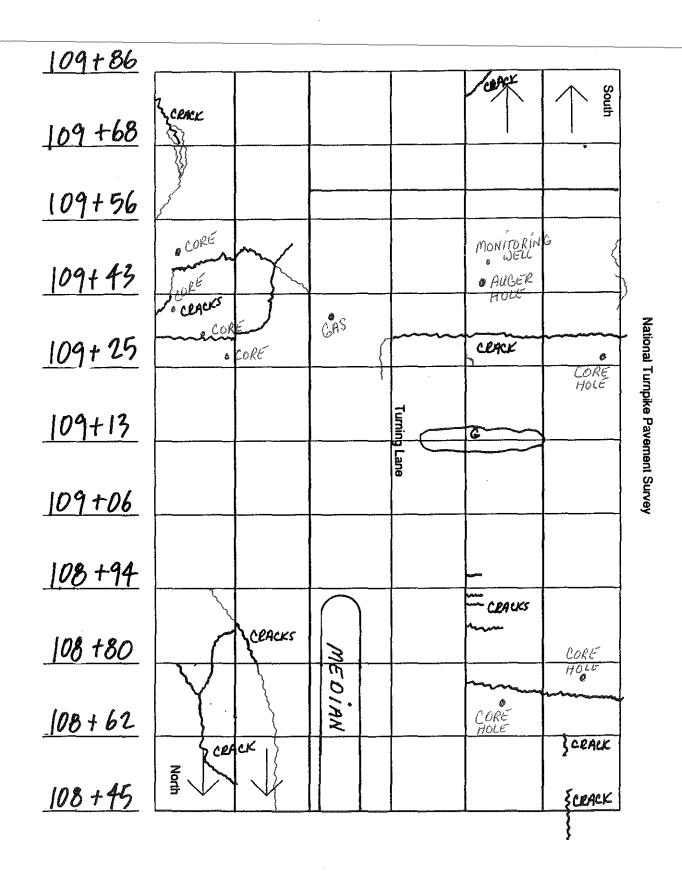
.

.



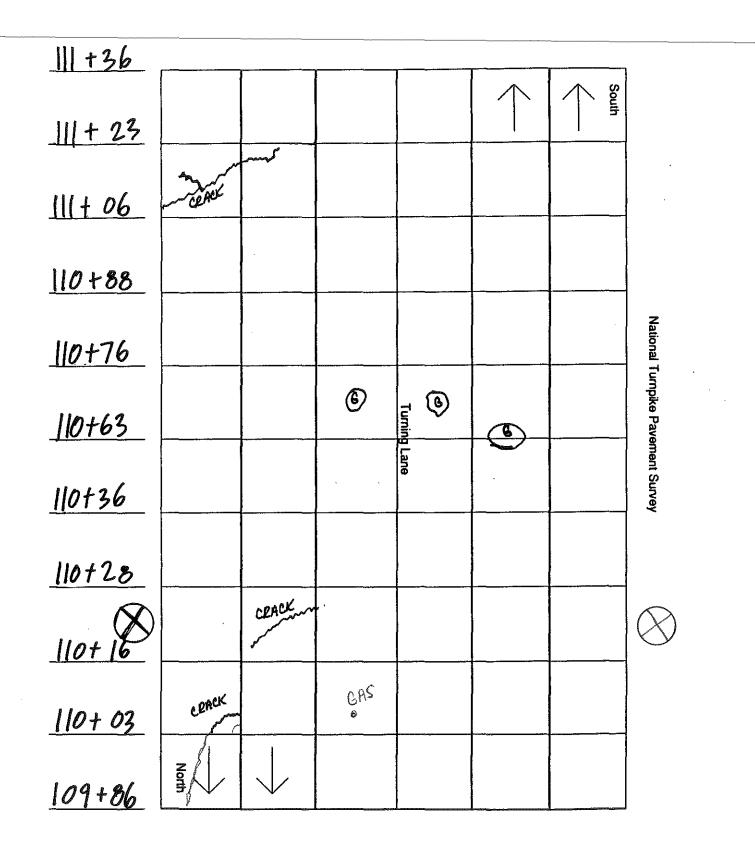
a décendre de la sec

41 K.S.

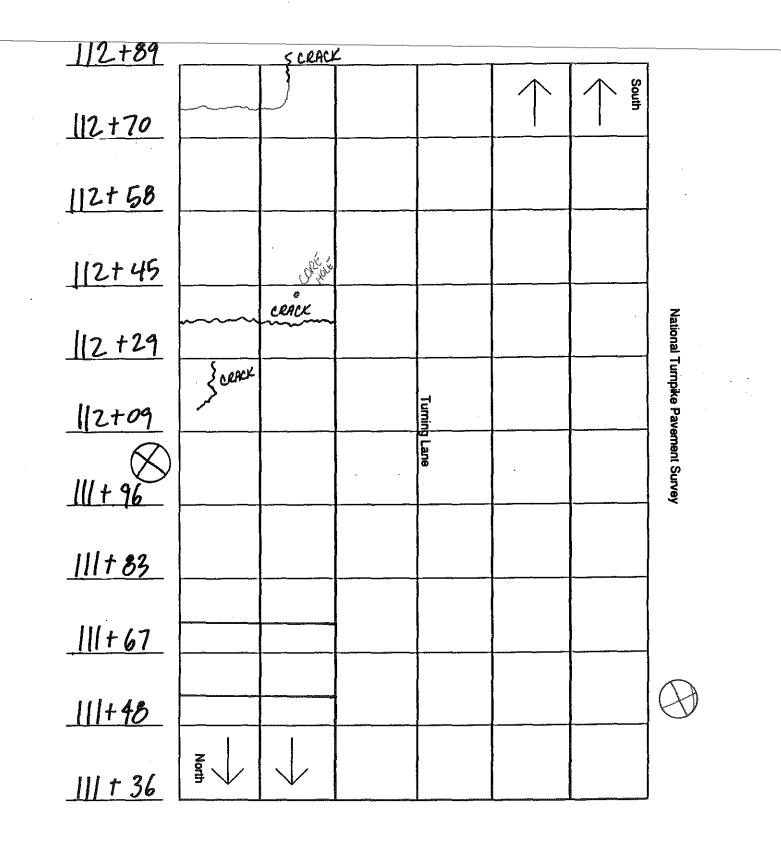


n na spinara n Spinara na in the second second

. ,



-12.5 C



··· ·· ·· ·

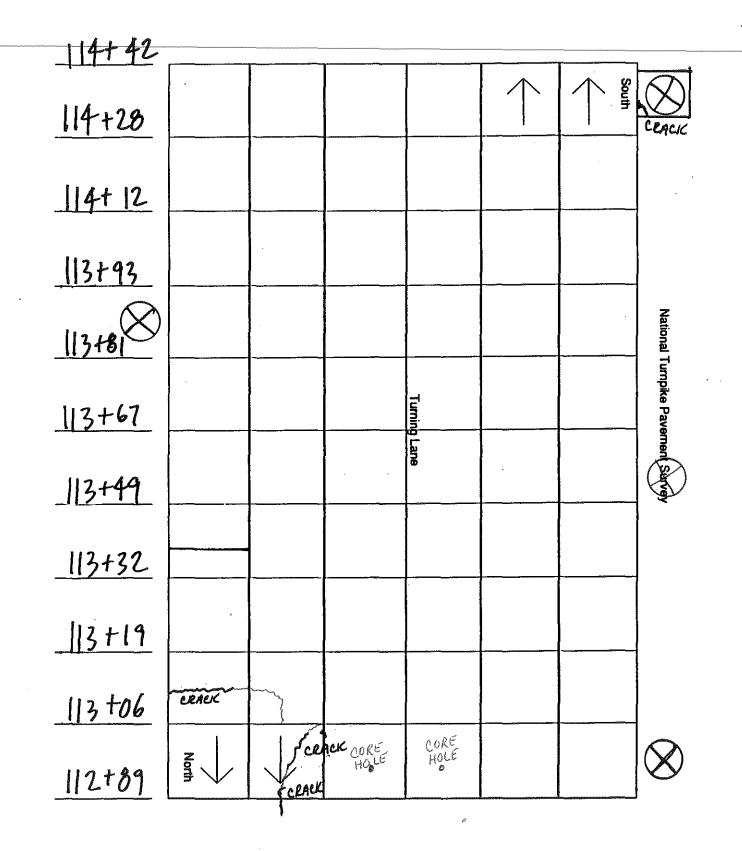
1

. . . .

1.1.1.1

35

•

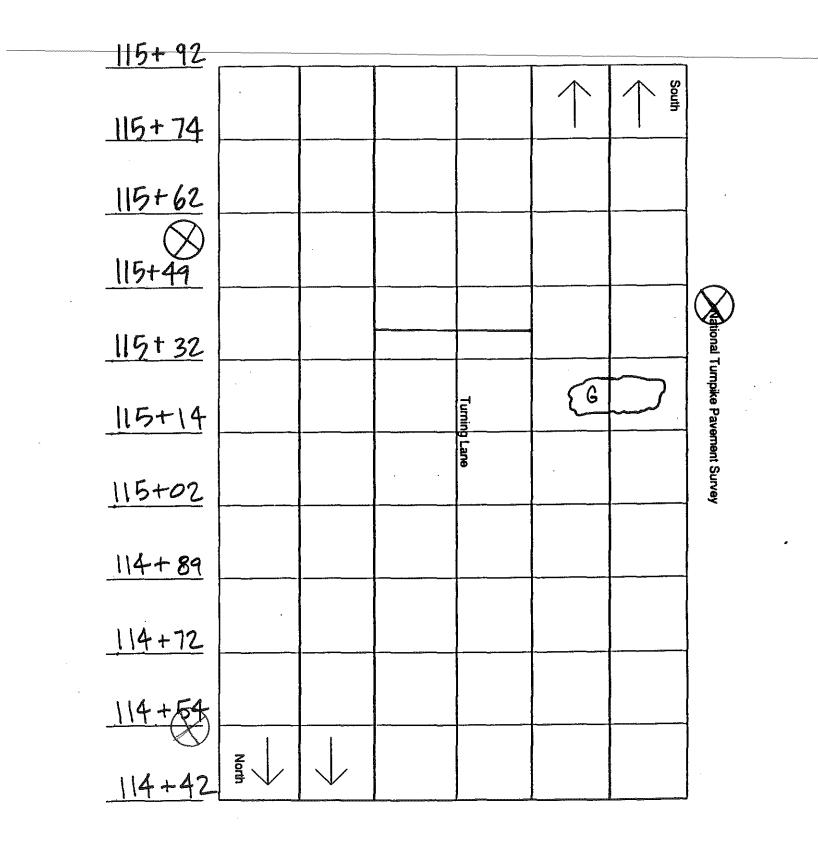


•

....

۰.

-



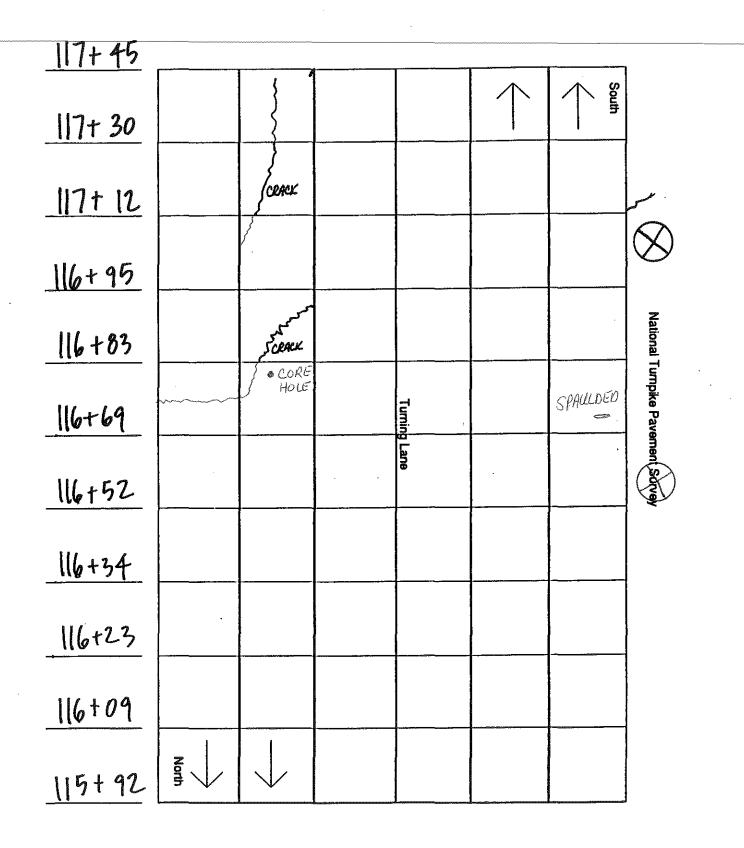
51 K S

• .

. .

•

....

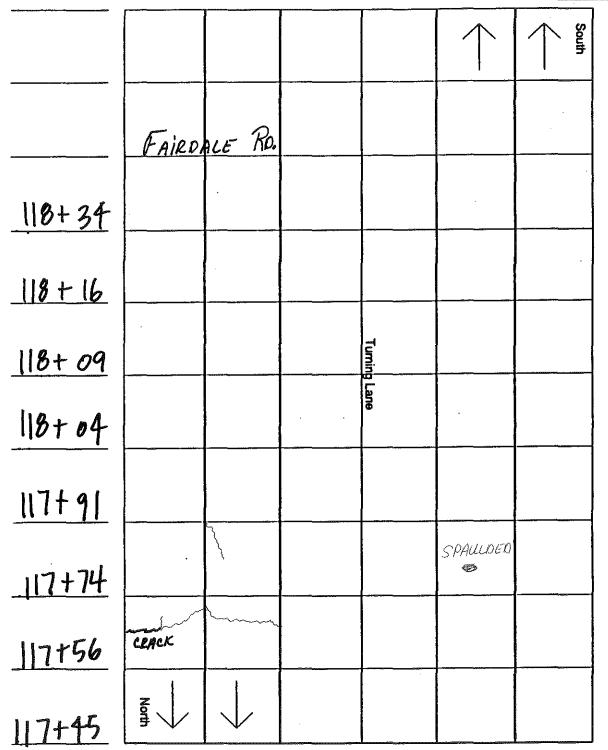


en an againm an Thailte a a construction de la construction

··· ·

1

• •



a a a a a

107

1474

,

National Turnpike Pavement Survey

e de l'activité de la sec