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Testing of Steel Single L-Headers

RESEARCH REPORT RP03-3

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American Iron and Steel Institute



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PREFACE

This report was developed by the NAHB Research Center for the Steel Framing Alliance and the U.S. Department of Housing and Urban Development. The objectives of this project were to test single L-headers for short window and door openings (4'-0") for residential construction and to develop span-load tables for the AISI *Standard for Cold-Formed Steel Framing* - *Prescriptive Method for One and Two Family Dwellings*.

This project involved a series of 18 full-scale tests to evaluate the capacity of 6 different single L-header sizes. The findings provided a basis for the AISI Committee on Framing Standards to expand the design options in the AISI *Standard for Cold-Formed Steel Framing - Header Design* to include single L-headers.

Research Team Steel Framing Alliance

Testing Of Steel Single L-Headers

Prepared for

Steel Framing Alliance Washington, DC

And

The U.S. Department of Housing and Urban Development Office of Policy Development and Research Washington, DC

by

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February 2003

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Special recognition is given to Dale/Incor of Baltimore, Maryland for providing the steel L-header materials for testing.





The NAHB Research Center, U.S. Department of Housing and Urban Development (HUD) and the Steel Framing Alliance have worked cooperatively to introduce cold-formed steel framing into the residential construction market and to provide objective builders and homeowners with a cost-effective alternative construction material. This report investigates the structural adequacy of single steel L-headers for short openings in steel-framed residential buildings. Double L-headers have been previously investigated and proved to be a cost-effective alternative to conventional back-to-back and box-beam headers.

HUD and the Steel Framing Alliance commissioned the NAHB Research Center to test single L-headers for short door and window openings used in conventional steel-framed residential buildings (openings up to 4-feet wide). This report provides test results as well as span tables that can be used by builders.





Executive Summary

The purpose of this test report is to substantiate a more cost-competitive alternative to "box beam" and "back-to-back" headers by expanding upon the L-header testing program that was previously conducted at the NAHB Research Center, Inc. Previous testing of double L-shaped headers indicated that these headers could be a viable alternative to the existing box-beam or back-to-back headers in lightly loaded applications. The higher capacity of the previously tested L-headers was mainly due to the fact that there is an added benefit from the top track that may be neglected in the design process. However, the application of these previous tests were limited to double L-headers.

Single L-headers for short openings were tested in this report and span tables were developed for residential building that comply with the applicability limits of the *Prescriptive Method for One and Two Family Dwellings*.

This report starts by providing a literature review of existing tests and/ or design procedures for single L-headers. A list of single L-headers that were tested in this report is then tabulated. Test configuration and test procedures are described. Test results are tabulated and a prescriptive table is developed.





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INTRODUCTION

Although, the use of steel L-headers in residential cold-formed steel framing is relatively new to the U.S. homebuilding industry, L-headers have gained popularity among steel framers because of their simplicity and ease of construction (see Figure 1). A steel L-header consists of light-gauge steel angle(s) placed over the top track of a steel wall and fastened to the king, jack and cripple studs. The Steel Framing Alliance recently published an L-Header Guide [1] for builders containing span tables and construction details. The Guide, however, is limited to double angles (double L-headers). No span tables or details were published for single angles (single L-headers).

The purpose of this report is to investigate the feasibility of using single steel L-headers placed over steelframed walls. The investigation is limited to testing of single L-headers used for 4-foot (1.2 m) openings (i.e., window and door openings), which are common in residential buildings.



Figure 1 – Typical Steel L-Header



Review of Existing Test Data

A literature review of similar work was performed prior to testing. Little information pertaining to the benefits of single L-shaped headers was found in the literature. Tests of single L-shaped headers conducted at the NAHB Research Center, Inc. [2] using 6-inch and 8-inch angles with 33 mil and 43 mil thicknesses showed a noticeable increase in tested ultimate capacity over calculated ultimate capacity as shown in Table 1. The results show that a 6-inch, 33 mil L-header, with single angle does not provide adequate strength for wide openings in residential construction, but may be acceptable for smaller openings, especially for lightly loaded buildings. Tests also showed a significant increase in the single L-header's capacity as the thickness of the angle increases or the depth of the angle increases.

L-Shape Header Specimen	Angle Size (inches)	Header Clear Span (feet)	Angle Thickness (mils)	Tested Ultimate Load (lb.)	Calculated Ultimate Load (lb.)	Load Ratio (Tested / Calculated)
L-Header #1	6"	4.0	33	1,446	949	1.52
L-Header #2	6"	4.0	33	1,306	971	1.34
L-Header #6	8"	4.0	43	2,662	1,806	1.47
L-Header #7	8"	4.0	43	2,693	1,788	1.51
L-Header #12	6"	4.0	33	1,354	952	1.42
L-Header #13	6"	4.0	33	1,384	953	1.45

TABLE 1SUMMARY DATA OF TESTED SINGLE L-HEADERS1

For SI: 1 inch = 25.4 mm, 1 lb. = 4.448 N

¹Reproduced from reference 2. For angle size, thickness, loading configuration, span length, material properties, number of tests, and other pertinent information, refer to reference 2.

Review of Design Procedures

The design of an L-header may be performed in accordance with the *North American Specification for the Design of Cold-Formed Steel Structural Members* [3]. The *AISI Standard for Cold-Formed Steel Framing - Header Design* [4] contains design equations that are limited to double L-headers.



EXPERIMENTAL APPROACH

Test Specimens

The L-header test specimens were assembled using construction materials and methods appropriate for residential construction of cold-formed steel framing. All steel materials used in the tests conform to the dimensional and material requirements of the *Standard for Cold-Formed Steel Framing - General Provisions* [5] and Table 2. Tensile and yield strength were verified by tensile tests in accordance with ASTM A370 [6]. Base steel thicknesses were also established and measured in accordance with ASTM A90 [7]. Mechanical properties were based on coupons cut from the center of the web of a sample of the test specimens.

Each single L-header assembly consisted of one cold-formed steel angle with one short leg lapping over the top track (350T125-33) and one long leg extending down the side of the wall above openings. Each angle was fastened to the top plate above an opening with No. 8 screws spaced 12-inches (305 mm) on center. All screws used were No. 8 self-drilling tapping screws with low profile heads. All screws protruded through steel a minimum of three exposed threads. Details of the built-up single L-header assembly are shown in Figures 2 and 3.

A total of 18 single L-header assemblies were constructed and tested, three for each L-header designation identified in Table 2.

WIEWIDEK DIWIENSIONS							
Member Designation	Measured Long Angle Size	Measured Short Angle Size	Nominal Thickness				
	(or Web Depth)	(or Flange Width)	(in.)				
	(in.)	(in.)					
L-Headers							
600L150-33	6.0	1.50	0.033				
600L150-43	6.0	1.50	0.043				
600L150-54	6.0	1.50	0.054				
800L150-33	8.0	1.50	0.033				
800L150-43	8.0	1.50	0.043				
800L150-54	8.0	1.50	0.054				
Tracks							
350T125-33	3.50	1.25	0.033				
Cripple Studs							
350\$162-33	3.50	1.625	0.033				
End Studs							
350S162-43	3.50	1.625	0.043				

TABLE 2 IEMBER DIMENSIONS

For SI: 1 inch = 25.4 mm



Figure 2 - Detail of a Built-up Single L-Header Assembly



Figure 3 – Section A-A (Single L-Header)



Test Apparatus

The header assemblies were tested using a 200,000 lb (900 kN) universal testing machine (UTM, Southwark-Emery Model 78075), a Satek Epsilon Series 2 inch deflectometer, and a Newvision II Data Acquisition System. The two 350S162-43 studs were fastened to each end of the L-header and to the bottom beam of the UTM. The test set-up is illustrated in Figure 4. The load is applied at a load rate of 1/20 inch per minute until each header failed. Failure constitutes failure of the header material (buckling, bearing or crippling), failure of the screws (shear or pull out), or failure of the head track. Deflections at the midpoint of the opening were recorded during the full range of loads using the 2-inch (51 mm) deflectometer. The assembly was laterally restrained against weak axis rotation and lateral movement. Rotation of the header and wall framing assembly was allowed in the plane of bending. Rollers were not used at the stud bearing points because rotation was provided by the configuration of the studs.



Figure 4 - L-Header Test Apparatus



RESULTS

The average steel physical properties are summarized in Table 3 (Table A1 of Appendix A contains all tested physical properties). The results of the single L-header tests are also shown in Appendix A (Test Plots). Summary of the single L-header test results is shown in Table 4. The ultimate loads shown in Table 4 are the loads measured by the load cell.

FAILURE MODE

All tested samples ultimately failed by buckling under the point load. The web of the angle bulged outwards under the point load that ultimately caused the screws, for some of the assemblies, at the point load location to fail (combination of pullout and shear failure). The steel angles in all tested specimens were severely buckled and deformed at ultimate loads. Each steel angle started to show signs of local buckling at load bearing points at approximately 60 percent of the ultimate load. The angles continued to deform and bulge as the load was increased. The deformation spread to the web of the angle between the load point and the end studs at loads close to the ultimate load. All screws that sheared off did so at ultimate loads. Refer to Figure 5 for illustrations of failure modes.

AVERAGE PHYS	AVERAGE PHYSICAL AND MECHANICAL PROPERTIES OF STEEL ANGLES							
L-Header Designation	Average Yield	Average Tensile	Average	Average				
	Point ¹	Strength ¹	Uncoated	Elongation ³				
	(psi)	(psi)	Thickness ²	(percent)				
			(in.)					
600L150-33	37,500	46,633	0.0339	18.9				
600L150-43	40,500	54,433	0.0448	20.3				
600L150-54	53,533	65,867	0.0551	20.2				
800L150-33	37,067	46,900	0.0337	22.1				
800L150-43	39,533	56,967	0.0440	21.4				
800L150-54	55,067	68,333	0.0541	21.2				

TABLE 3

For SI: 1 inch = 25.4 mm, 1 psi = 0.0703 kg/cm^2

¹ Yield point and tensile strength are actual yield point and tensile strength from coupons cut from the web of the angle specimen and tested per ASTM A370 [6].

² Uncoated thickness is the bare steel thickness of the steel angle as tested per ASTM A90 [7].

³ Tested in accordance with ASTM A370 [6] for a two-inch gauge length.

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TABLE 4SINGLE L-HEADER TEST RESULTS

					5	
Single L-Header	Web Depth	Flange Width	Top Track	Header Clear Span	Ultimate Load	Deflection at Ultimate Load
Designation	(in.)	(in.)		(ft)	(lb)	(in.)
600L150-33	6.0	1.5	350S125-33	4.0	1,870	0.165
600L150-33	6.0	1.5	350S125-33	4.0	1,889	0.230
600L150-33	6.0	1.5	350S125-33	4.0	1,701	0.294
Average		•			1,820	0.230
Standard D	eviation				103	0.064
COV					0.057	0.281
600L150-43	6.0	1.5	350S125-33	4.0	2,018	0.181
600L150-43	6.0	1.5	350S125-33	4.0	2,180	0.142
600L150-43	6.0	1.5	350S125-33	4.0	2,343	0.157
Average				•	2,180	0.160
Standard D	eviation				163	0.0200
COV					0.074	0.123
600L150-54	6.0	1.5	350S125-33	4.0	2,812	0.237
600L150-54	6.0	1.5	350S125-33	4.0	2,907	0.248
600L150-54	6.0	1.5	350S125-33	4.0	2,809	0.226
Average		•	•	•	2,843	0.237
Standard D	eviation				56	0.011
COV					0.0200	0.046
800L150-33	8.0	1.5	350S125-33	4.0	2,190	0.172
800L150-33	8.0	1.5	350S125-33	4.0	2,202	0.168
800L150-33	8.0	1.5	3508125-33	4.0	2,264	0.221
Average					2,219	0.187
Standard D	eviation				40	0.029
COV					0.018	0.158
800L150-43	8.0	1.5	350S125-33	4.0	2,810	0.142
800L150-43	8.0	1.5	350S125-33	4.0	2,768	0.166
800L150-43	8.0	1.5	350S125-33	4.0	2,933	0.133
Average					2,837	0.147
Standard D	eviation				86	0.017
COV					0.030	0.116
800L150-54	8.0	1.5	350S125-33	4.0	3,188	0.155
800L150-54	8.0	1.5	3508125-33	4.0	3,133	0.157
800L150-54	8.0	1.5	3508125-33	4.0	3,336	0.172
Average					3,219	0.161
Standard D	eviation				105	0.009
COV					0.033	0.058

For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 lb. = 4.448 N.

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Testing of Steel L-Headers on Wood Walls















Figure 5 – Failure of Single L-Headers Tested Assemblies



The nominal L-header design value (R) can be determined by applying an appropriate factor of safety to average ultimate test values. The factor of safety is calculated in accordance with the AISI Specification [3] as follows:

 $\sum \gamma_i Q_i \leq \phi R_n$ $R_n = Average value of the test results.$ Where: ϕ = Resistance factor $\gamma_i Q_i$ Required strength based on the most critical load combination. $\phi = \text{Resistance factor} = C_{\phi} (M_m F_m P_m) e^{-\beta_0 \sqrt{V_M^2 + V_F^2 + C_p V_P^2 + V_Q^2}}$ Cø = Calibration coefficient = 1.52 M_m = Mean value of the material factor = 1.10 (bending) = Mean value of the fabrication factor = 1.00 F_{m} P_m = Mean value of the professional factor for the tested component = 1.0= Target reliability index = 2.5 β_0 = Coefficient of variation of the material factor = 0.10 (bending or compression) Vм = Coefficient of variation of the fabrication factor = 0.05 V_{F} = Correction factor = 5.7 (for n = 3) CP = Number of tests n = Coefficient of variation of the test results, $\geq 6.5\%$ (for V_p < 6.5%), use 6.5%) Vp = 6.5% for all single L-headers except 600L150-43 V_P = 7.4% for single L-header 600L150-43 Vo = Coefficient of variation of the load effect = 0.21 $\phi = 1.52(1.10x1.00x1.00)e^{-2.5\sqrt{0.10^2 + 0.05^2 + 5.7x0.065^2 + 0.21^2}}$

 $\phi = 0.747$ for all single L-headers except 600L150-43 $\phi = 0.723$ for single L-header 600L150-43

The appropriate ϕ factor computed above can be applied to the average ultimate capacity for each single header assembly shown in Table 4 to estimate the factored (design) capacity.

Therefore, the factored capacity (as measured by the load cell) for each single header is shown in Table 5 for use with LRFD design provisions and factored LRFD load combinations.

			SING	LE L-	HEADH	TABLE ER FAC	25 TOR	ED CAP	ACITY	Y		
	Sir]	ngle L- Design	Header ation		Av Ultim	erage ate Loa	d	Factor Capac	ed ity	Fac Mo	ctored ment ¹	
						(lb)		(lb)		(iı	n-lb)	
		600L15	50-33		1	,820		1,360)	16	5,320	
		600L15	50-43		2	,180		1,576	5	18	3,912	
		600L15	50-54		2	,843		2,124	ŀ	25	5,488	

2,219

2,837

3,219

19,896

25,428

28,860

1,658

2,119

2.405

For SI: 1 inch = 25.4 mm, 1 lb. = 4.448 N.

800L150-33

800L150-43

800L150-54

¹ Moment is calculated assuming simply supported beam.

DESIGN METHOD

The design equations contained in the AISI Header Design Standard [4] for the double L-headers will be used to investigate their applicability to the single L-headers tested in this report.

The AISI Header Design Standard provides the following design equation for double L-header beam having vertical leg dimension of 8 inches (203 mm) or less:

 $M_{ng} = S_{ec}F_v$

Where: F_y = yield strength used for design S_{ec} = elastic section modulus of the effective section calculated at f=F_y in the extreme compression fibers.

$$\begin{split} M_a &= M_{ng} / \Omega \qquad \text{For ASD } (\Omega = 1.67) \\ M_a &= \phi \; M_{ng} \qquad \text{For LRFD } (\phi = 0.90) \end{split}$$
Design Moment Capacity,

The nominal flexural strength (Mng) for each of the tested assemblies is calculated using the above equations and tabulated in Table 6. The ratio of the tested-to-computed moments (M_{Test}/M_{ng}) is shown in Table 6 (un-factored moments). A ratio equals to 1.0 means that the double L-header design equations are applicable to single L-headers. A ratio greater than 1.0 indicates that the design equations are conservative while a ratio of less than 1.0 indicated that the design equations are unconservative for single L-headers.



 TABLE 6

 SINGLE L-HEADER MOMENT COMPARISON¹

Single L-Header Designation	Test Moment (M _{Test}) (in-lb)	S _{ec} (in ³)	Design Moment (M _{ng}) (in-lb)	Ratio (M _{Test})/ (M _{ng})
600L150-33	21,840	0.28809	10,803	2.02
600L150-43	26,260	0.40585	16,437	1.60
600L150-54	34,116	0.50797	27,193	1.25
800L150-33	26,628	0.41002	15,198	1.75
800L150-43	34,044	0.64456	25,481	1.34
800L150-54	38,628	0.79878	42,888	0.90

For SI: 1 inch = 25.4 mm, 1 lb. = 4.448 N.

1 The section modulus is computed based on the measured properties as shown in Tables 1 and 3.

It is concluded from the above table that the design equations for double L-headers can be conservatively used for the single L-headers tested in this report with the exception of the 800L150-54.

A trend in the ratios, $(M_{Test})/(M_{ng})$, can also be concluded from Table 6. It appears that as the thickness of the L-header increases the ratio decreases. The decrease in the ratio indicates that the other elements of the assembly do not contribute as much to enhancing the strength of the L-header. That is, the thinner headers gain more from the help given by the top track and other elements than the thicker headers. Also, the decrease in the ratio for the longer leg angles is consistent with the trend observed for the double L-header.

DEFLECTION CHECK

Table 4 shows the deflection for each tested assembly at ultimate load. The average deflection varies from 0.147 to 0.237 inches (3.73 to 6.02 mm). The average (tested) deflections at design loads (factored loads) can be estimated from the charts in Appendix A. These deflections range from 0.05 to 0.1 inches (1.27 to 2.54 mm).

The deflection criterion that is typically used for headers is L/360. This deflection limit is also specified in the *Prescriptive Method* [8]. Using this L/360 limit, the allowable deflection for each assembly would be: 48/360 = 0.133 inches (3.38 mm). The tested deflection is well below the deflection limit of L/360, and therefore, deflection need not be checked for such assemblies.

PRESCRIPTIVE TABLES

To investigate the feasibility of using the single L-header assemblies (tested in this report) in residential buildings, a simple example is provided as follows:

Using the applicability limits of the *Standard for Cold-Formed Steel Framing - Prescriptive Method for One and Two Family Dwellings* (Prescriptive Method) [8], determine the loads acting on a 4-foot (1220 mm) wide header supporting one roof and ceiling only for a 28-foot (8534 mm) wide building (with 24 inch, 610 mm, overhang) with a 30-psf (1.4364 MPa) ground snow load.



All other loads are in accordance with the Prescriptive Method. Select an appropriate single L-header for the 4-foot (1220 mm) opening. Refer to Appendix B for SI units.

Load Combinations:	$\begin{array}{llllllllllllllllllllllllllllllllllll$	$\begin{array}{l} 1.4D \\ 1.2D + 1.6(L_r \mbox{ or } S) + 0.5L \\ 1.2D + 0.5(L_r \mbox{ or } S) + 1.6L \end{array}$					
Loads	Roof dead load Ceiling dead load Roof live load=	= 7 psf = 5 psf = 16 psf	[8] [8] [8]				
Dead Loads:		I					
	Ceiling Dead Load =	5 psf(28/2)	=	70 plf			
	Roof Dead Load =	7 psf(32/2)	=	<u>112 plf</u>			
	Total Dead Load		=	182 plf			
Live Loads:							
	Roof Live Load =	16 psf(28 + 4)/2	2=	256 plf			
	Roof Snow Load =	0.7(30 psf)(32/2	2)=	336 plf \leftarrow controls			

Design load acting on header = P

1.	1.4(182) =	255 plf
2.	1.2(182) + 1.6(336) =	756 plf ⇐ Controls
3.	1.2(182) + 0.5(336) =	386 plf

The moment due to a 756 plf distributed load is 18,144 in-lb. Using Table 5, the appropriate single L-header to use is 600L150-43 or 800L150-33 (with factored moment capacities of 18912 and 19896 respectively).

Table 7 was developed using the *Prescriptive Method* applicability limits and the design equations from the AISI *Header Design Standard*. The values in Table7 were derived in a similar manner to the example shown in the previous section. The moment capacity used for the 800L150-54 was normalized for the minimum thickness, 0.054" (1.37 mm) and the yield strength, 50 ksi (345 MPa). The factored moment capacity for the 800L150-54 is 26,156 in-lb (2956 N-m).



TABLE 7 MINIMUM THICKNESS (MILS) OF SINGLE L-HEADERS FOR OPENINGS NOT GREATER THAN 4-FEET (HEADERS SUPPORTING ROOF AND CEILING ONLY)

Building	L-Header	Ground Snow Load (psf) ²					
Width ¹ (Feet)	Designation	20	30	50	70		
24	600L150-	43	54	54	-		
24	800L150-	33	43	54	-		
28	600L150-	54	54	-	-		
28	800L150-	43	43	-	-		
22	600L150-	54	54	-	-		
32	800L150-	43	43	-	-		
26	600L150-	54	54	-	-		
50	800L150-	43	43	-	-		
40	600L150-	54	-	-	-		
40	800L150-	43	54	-	-		

For SI: 1 mil = 1/1000 inch = 25.4 mm, 1 foot = 305 mm, 1 psf = 0.0479 kN/m².

¹ Building width is measured in the direction of horizontal framing members supported by the header.

² Design Assumptions:

Roof and ceiling dead load = $12 \text{ psf}(0.58 \text{ kN/m}^2)$ 2 foot (0.61 m) roof overhang



Single L-shaped header is a viable alternative to conventional back-to-back and box-beam headers for common window and door openings in lightly loaded buildings. It is anticipated that possible applications will include headers supporting short or moderate roof spans and other light structures. Tables were developed for single L-headers with a maximum span of 4-feet (1220 mm).



REFERENCES

- [1] *L-Shaped Header, Field Design Guide*. North American Steel Framing Alliance, Washington DC. December 1999.
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- [3] AISI North American Specification for the Design of Cold-Formed Steel Structural Members, 2001 Edition. American Iron and Steel Institute (AISI), Washington, DC. 2001.
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- [5] AISI *Standard for Cold-Formed Steel Framing General Provisions*, 2001 Edition. American Iron and Steel Institute (AISI), Washington, DC. 2000.
- [6] ASTM A370-02 Standard Test Methods and Definitions for Mechanical Testing of Steel Products, American Society for Testing and Materials (ASTM), West Conshohocken, PA. 2002.
- [7] ASTM A90/A90M-01 Standard Test Method of Weight [Mass] of Coating on Iron and Steel Articles with Zinc or Zinc-Alloy Coatings. American Society for Testing and Materials (ASTM), West Conshohocken, PA. 2001.
- [8] AISI Standard for Cold-Formed Steel Framing-Prescriptive Method for One and Two Family Dwellings, 2001 Edition. American Iron and Steel Institute (AISI), Washington, DC. 2001.



APPENDIX A

Coupon Test Results and L-Header Assembly Test Results

Testing of Steel L-Headers on Wood Walls

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L-Header (Angle)	Yield Point ¹ (psi)	Tensile Strength ¹ (psi)	Uncoated Thickness ²	Elongation ³ (percent)
Designation			(inch)	
600L150-33	36,800	46,100	0.0337	18.5
600L150-33	38,200	45,200	0.0341	18.7
600L150-33	37,500	48,600	0.0340	19.4
600L150-43	41,100	55,300	0.0444	19.6
600L150-43	39,900	54,600	0.0451	21.2
600L150-43	40,500	53,400	0.0448	20.0
600L150-54	53,700	64,600	0.0551	19.5
600L150-54	52,800	67,800	0.0555	19.9
600L150-54	54,100	65,200	0.0546	21.3
800L150-33	36900	48600	0.0339	23.2
800L150-33	37200	45900	0.0335	22.1
800L150-33	37100	46200	0.0336	20.9
800L150-43	39800	56900	0.0439	22.9
800L150-43	38700	58200	0.0441	21.6
800L150-43	40100	55800	0.0440	19.8
800L150-54	56500	69200	0.0540	21.6
800L150-54	54900	67500	0.0541	19.8
800L150-54	53800	68300	0.0541	22.3

 TABLE A1

 PHYSICAL AND MECHANICAL PROPERTIES OF STEEL ANGLES

For SI: 1 inch = 25.4 mm, 1 psi = 0.0703 kg/cm^2

¹ Yield point and tensile strength shown are based on coupons cut from each sample and tested per ASTM A370 [6].

² Uncoated thickness shown is based on uncoated thickness taken from each sample per ASTM A90 [7].

³ Tested in accordance with ASTM A370 [6] for a two-inch gauge length.

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Member DesignationYield Point1 (psi)		Tensile Strength ¹ (psi)	Uncoated Thickness ² (inch)	Elongation ³ (percent)	
350S162-33	37,850	48,620	0.0336	21.6	
350S162-33	36,690	47,740	0.0334	23.4	
350S162-33	36,230	47,020	0.0334	22.1	
350T125-33	33,420	42,130	0.0332	21.9	
350T125-33	34,050	41,010	0.0332	22.3	
350T125-33	34,190	40,670	0.0331	22.6	

 TABLE A2

 PHYSICAL AND MECHANICAL PROPERTIES OF STEEL MEMBERS

For SI: 1 inch = 25.4 mm, 1 psi = 0.0703 kg/cm^2

¹ Yield point and tensile strength shown are based on coupons cut from each sample and tested per ASTM A370 [6].

² Uncoated thickness shown is based on uncoated thickness taken from each sample per ASTM A90 [7].

³ Tested in accordance with ASTM A370 [6] for a two-inch gauge length.













Testing of Steel L-Headers on Wood Walls









APPENDIX B

Metric Conversion



The following list provides the conversion relationship between U.S. customary units and the International System (SI) units. A complete guide to the SI system and its use can be found in ASTM E 380, Metric Practice.

To convert from	То	Multiply by	To convert from	То	Multiply by
Length			Stress or pressure		
Inch (in.)	Micrometer (µm)	25,400	Pound/sq. foot (psf)	Kilogram/square meter (kg/sq. m)) 4.8824
Inch (in.)	Millimeter (mm)	25.4	Pound/sq. foot (psf)	Pascal (Pa)	47.88
Inch (in.)	Centimeter (cm)	2.54	Mass (weight)		
Inch (in.)	Meter (m)	0.0254	Pound (lb.)	Kilogram (kg)	0.4535924
Foot (ft)	Meter (m)	0.3048	Ton, 2000 lb	Kilogram (kg)	907.1848
Yard (yd)	Meter (m)	0.9144	Grain	Kilogram (kg)	0.0000648
Mile (mi)	Kilometer (km)	1.6	Mass (weight) per l	ength)	
Area			Kip per linear foot (klf)	Kilogram per meter (kg/m)	0.001488
Square foot (sq. ft)	Square meter (sq. m)	0.0929	Pound per linear foot (plf)	Kilogram per meter (kg/m)	1.488
Square inch (sq. in)	Square centimeter (sq. cm)	6.452	Moment		
Square inch (sq. in.)	Square meter (sq. m)	0.00064516	1 foot-pound (ft-lb.)	Newton-meter (N-m)	1.356
Square yard (sq. yd)	Square meter (sq. m)	0.8391	Mass par volume (d	longity)	
		2.6	Niass per volume (u	tensity)	16 01046
Square mile (sq. mi)	Square kilometer (sq. km)	2.6	foot (pcf)	(kg/cu m)	16.01846
Volume			Pound per cubic yard (lb/cu yd)	Kilogram per cubic meter (kg/cu m)	0.5933
Cubic inch (cu in.)	Cubic centimeter (cu cm)	16.387064	Velocity		
Cubic inch (cu in.)	Cubic meter (cu m)	0.00001639	Mile per hour (mph)	Kilometer per hour (km/hr)	1.60934
Cubic foot (cu ft)	Cubic meter (cu m)	0.02831685	Mile per hour (mph)	Kilometer per second (km/sec)	0.44704
Cubic vard (cu vd)	Cubic meter (cu m)	0.7645549	Temperature		
Gallon (gal) Can.	Liter	4.546	Degree Fahrenheit	Degree Celsius (°C)	$t_{\rm C} = (t_{\rm F} - 32)/1.8$
liquid Callar (aal) Car	Cubic motor (ou m)	0.004546	(°F) Decree Februarie		4 = (4 + 50.7)/1.9
Gallon (gal) Can.	Cubic meter (cu m)	0.004546	Degree Fanrenneit	Degree Kelvin (°K)	$t_{\rm K} = (t_{\rm F} + 39.7)/1.8$
Callar (cal) U.S.	Litan	2 7054110	$(^{*}F)$		4 = (4 - 272, 15)
Gallon (gal) U.S.	Liter	3./834118	Degree Kelvin (°K)	Degree Celsius (°C)	$t_{\rm C} = (t_{\rm K} - 2/3.13)$
Gallon (gal) U.S.	Cubic meter (cu m)	0.00378541	Degree Celsius (°C)	Degree Fahrenheit (°F)	$t_{\rm F} = 1.8 t_{\rm C} + 32$
Fluid ounce (fl.oz)	Milliliters (ml)	20 57353	* One U.S. gallon a	aguals 0.8327 Canadian gallon	
Fluid ounce (fl oz)	Cubic meter (cu m)	0.00002957	** A Pascal equals 1	000 Newton per square meter	
Former	Cubic meter (eu m)	0.00002937	The profixed and sur	whole below are commonly used to	form names and
Force Vin (1000 lb.)	Kilo men (lee)	152 6	symbols of the de	acimal multiples and submultiples	of the SI units
Kip(1000 lb.)	Nilogram (Kg)	433.0	Multinlightion Eq	nator Duefy	Symbol
$\operatorname{Rep}\left(1000 \text{ ID.}\right)$	Kilogram (kg)	4,448.222	$1 000 000 000 - 10^9$	Cigo	Symbol
Pound (lb.)	Nowton (N)	0.4353924	1,000,000,000 = 10 $1,000,000 = 10^{6}$	Maga	U M
Pouliu (ID.)	Newton (N)	4.440222	1,000,000 - 10 $1,000 = 10^3$	Kilo	lvi k
Stress or pressure		6 00 4858	1,000 - 10	Kilo	K
Kip/sq. inch (ksi)	Mega Pascal (Mpa)	6.894757	$0.01 = 10^{-3}$	Centi	с
Kıp/sq. inch (ksi)	(kg/sq. cm)	/0.31	$0.001 = 10^{-1}$	Milli	m
Pound/sq. inch (psi)	Kilogram/square centimeter (kg/sq. cm)	0.07031	$0.000001 = 10^{-6}$	Micro	μ
Pound/sq. inch (psi)	Pascal (Pa) **	6,894.757	$0.00000001 = 10^{-9}$	Nano	n
Pound/sq. inch (psi)	Mega Pascal (Mpa)	0.00689476			





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