

Missouri University of Science and Technology Scholars' Mine

International Conference on Case Histories in Geotechnical Engineering

(1984) - First International Conference on Case Histories in Geotechnical Engineering

09 May 1984, 1:00 pm - 2:00 pm

Dynamic Response and Static Analysis of RCC Space Frames Supporting High Speed Centrifugal Machines with Coupled Soil-Structure Interaction

Dilip K. Chakravorty Development Consultants International, Calcutta, India

Dhiman K. Ghosh Development Consultants International, Calcutta, India

H. N. Batavyal Development Consultants International, Calcutta, India

Follow this and additional works at: https://scholarsmine.mst.edu/icchge

Part of the Geotechnical Engineering Commons

Recommended Citation

Chakravorty, Dilip K.; Ghosh, Dhiman K.; and Batavyal, H. N., "Dynamic Response and Static Analysis of RCC Space Frames Supporting High Speed Centrifugal Machines with Coupled Soil-Structure Interaction" (1984). *International Conference on Case Histories in Geotechnical Engineering*. 3. https://scholarsmine.mst.edu/icchge/1icchge/1icchge-theme6/3

This Article - Conference proceedings is brought to you for free and open access by Scholars' Mine. It has been accepted for inclusion in International Conference on Case Histories in Geotechnical Engineering by an authorized administrator of Scholars' Mine. This work is protected by U. S. Copyright Law. Unauthorized use including reproduction for redistribution requires the permission of the copyright holder. For more information, please contact scholarsmine@mst.edu.

Dynamic Response and Static Analysis of RCC Space Frames Supporting High Speed Centrifugal Machines with Coupled Soil-structure Interaction

Dilip K. Chakravorty, Dhiman K. Ghosh and H. N. Batavyal Development Consultants International, Calcutta, India

SYNOPSIS The paper reviews the current state of the art on the dynamic and static analyses of RCC space frames supporting high speed centrifugal machines e.g. large turbogenerators and compressors. The need to include the effects of soil-structure interaction formulations on overall behaviour of various analytical models are highlighted. At the same time, the uncertainties involved in evaluating essential geotechnical parameters and paucity of reliable and elaborate information from the machine manufacturers are discussed. The analysis and design aspects of this inter-disciplinary problem are illustrated with two typical design case studies selected from authors' own experience in this specialised field. The paper also discusses the usefulness, if any, of such rigorous analysis and identifies various shortcomings which still persist in finalising realistic design data and adopting suitable models to represent machine foundation-soil system.

INTRODUCTION

The rapid growth in industrialisation in recent years has necessitated the installation of heavy and complex machinery at various locations. The capacities of the machinery e.g. turbomachines, compressors and pumps have increased a great deal as also the sizes of their constituent units involved, calling for a modern analytical approach for their supporting structure.

All physical systems, built of material possessing mass and elasticity, are capable of vibra-ting at their own natural frequencies. Engineering structures, subjected to vibratory forces, experience vibration in different degrees and their design requires determination of their oscillatory behaviour. The current design office state of the art considers only their linear behaviour because of the convenience afforded by applying the principle of superposition, and also because the mathematical technique available for their treatment are well developed. In contrast, non-linear beha-viour of systems is less well known in spite of the fact that all structures tend to behave nonlinearly at exceedingly high amplitudes of vi bration, However, for reinforced concrete framed structures supporting high speed centrifugal machines, question of allowing high amplitude of vibration does not arise due to very stringent allowable design amplitude requirements put forward by various machine manufacturers.

Modeling of the real structure, machine and the supporting soil is of critical importance in obtaining results that will approach the actual performance of the combined soil-structure interactive system. Selection of non-realistic design parameters will also render these rigorous analysis appear meaningless. The link between the real structural system and the mathematically feasible solutions is provided by the mathematical model which is the symbolic designation for the substitute idealised system. Some of the modern techniques are lumped parameter model analysis, soil-structure interaction, elastic half-space theory and the latest computer programs available to the designers. This paper highlights synthesis of present state of the are in the analysis and design of two framed foundations supporting a compressor and a turbogenerator respectively. A comprehensive analytical approach using four computer models of the two types of fcundations under study are presented and discussed in this paper.

DESIGN CASE STUDIES

Extracts of computer solutions of two typical design case studies using computer software package SAP IV are furnished through Tables I to IV. These examples are selected to highlight the influence of soil-structure interaction and related design parameters pertaining to dynamic soil properties and the unbalanced forces of the machinery on the static stress analysis and dynamic response of the supporting reinforced concrete space frames.

Foundation Sizing

The trial dimensions of these two foundations are selected to meet respective machine manufacturer's basic guidelines viz. machine assembly and piping requirements and also to satisfy preliminary criteria under trial sizing of elevated foundations which are to check whether(1) thickness of the mat is adequate to assure its rigid behaviour; (2) centre of column resistance coincides with the centre of gravity of the equipments plus the top half of the struc-ture (3) centre of resistance of the soil is found to coincide with the centroid of all superimposed loads i,e, structure plus machine (4) ratio of mass of structure to mass of machine is more than 2.5; (5) column and beam static deflections considering various combinations of static machine loads, creep, thermal

TABLE I Free Vibration Analysis of Compressor Foundation Models

Mode No.	E (C Model ^A l	Frequenc Cycles/s Model ^B 1	y ec) Model ^C l	Model ^A l	Period (sec) Model ^B l	Model C _l
lst 2nd 3rd 4th 5th 10th 15th 20th	7.19 8.55 9.92 21.23 28.74 55.9 73.9 129.3	1,46 2.26 2,89 4.36 4.70 25.3 49.13 67.8	1.36 1.91 2.09 3.93 3.00 21.2 49.15 69.39	0,14 0,117 0.101 0.047 0.035 0.018 0,013 0,008	0.686 0.443 0.346 0.229 0.212 0.039 0.020 0.015	0.743 0.523 0.477 0.340 0.333 0.047 0.023 0.014
Note:	Model Model	B _l is w C _l is w	vith bas vith bas	e raft e raft	1.0m th 3.0m th	ick.

TABLE II Static Stress Outputs of Typical Prismatic Beam Elements of Compressor Foundation Computer Models Unit of force-KN, moment-KN metre

Ele- ment		Stress Component				
No.	Axial	Shear	Shear	Torsion	Moment	Moment
1	277,8	3,41	-1.8	0,04	3,95	5.9
Al .	-277,8	-3.41	1.8	-0.04	5.13	11.8
Model	288,1	. 1,47	2.3	0.03	-7.94	-13,5
^B 1	-288,1	1,47	-2,3	-0,03	-3.88	5,9
9 Model	21,73	-170.4	*	*	0,097	-86,13
Al	-21.73	170.4	*	*	-0.097	-173,8
Model	14.23	-170,4	<u>+</u> *	*	0.079	-80,1
Bl	-14.23	170.4	! *	*	-0.079	-180.2
10 Model	21.73	170,4	<u>+</u> *	*	0,097	173.8
Al	-21.73	-170,4	<u></u> *	*	- 0.097	86,13
Mode 1	. 14,23	170.4	1 *	*	0,079	180,2
Bl	. 14.23	-170;4	1 *	*	-0.079	80,1
	* yalu	es are	found	to be r	egligib	le

TABLE III Free Vibration Analysis of Turbogenerator Foundation Models

Mode No.	Fre (cyc	quency le/sec)	Period (sec)		
	Model A ₂	Model B ₂	Model A ₂	Model B ₂	
lst 2nd `~d `	1,569 1,748 2,089 5,737 10,32 11,79	1.016 1,133 1.596 5,198 5.399 7,64	0.637 0.572 0.478 0,174 0.097 0.085	0.984 0.882 0.627 0.192 0.185 0 131	

TABLE III (contd.)

20th13.4618.730.0740.05325th15.0425.440.0670.03930th15.4830.530.0650.033	15th 20th 25th 30th	12.06 13.46 15.04 15.48	15.80 18.73 25,44 30,53	0.083 0.074 0.067 0.065	0.063 0.053 0.039 0.033	
--	------------------------------	----------------------------------	----------------------------------	----------------------------------	----------------------------------	--

TABLE IV Dynamic Stress Output of Two Typical Prismatic Beam Elements of Turbogenerator Foundation Computer Models

Unit of force-KN, moment-KN metre, time-sec.

Ele- Stress ment Component No.*		s onent	Maximum t Value		Time at Maximum	
43 Model ^A 2	Axial Shear Shear Torsion Moment Moment	Axial Shear Shear Torsion Moment Moment	95.43 12.66 2.602 0.546 16.22 91.03	95.43 12.66 2.602 0.546 8.718 43.19	2.12×10 ⁻² 2.62×10 ⁻² 6.37×10 ⁻² 9.75×10 ⁻² 1.50×10 ⁻² 1.38×10 ⁻²	
10 Model ^B 2	Axial Shear Shear Torsion Moment Moment	Axial Shear Shear Torsion Moment Moment	75,64 4,25 0.736 0,140 5.57 28,9	75.64 4.25 0.736 0.140 9.53 56.75	$2.13x10_{-2}$ $1.37x10_{-2}$ $1.50x10_{-2}$ $4.00x10_{-2}$ $4.50x10_{-2}$ 1.38x10	
41 Model ^A 2	Axial Shear Shear Torsion Moment Moment	Axial Shear Shear Torsion Moment Moment	30.94 6.22 152.6 0.836 251.1 21.7	30.94 6.22 152.6 0,836 483.1 11.78	9.40×10^{-2} 9.50×10^{-2} 1.25×10^{-2} 6.12×10^{-2} 1.25×10^{-2} 9.62×10^{-2}	
42 Model ^A 2	Axial Shear Shear Torsion Moment Moment	Axial Shear Shear Torsion Moment Moment	34.37 7.41 151.6 0.836 483.1 11.80	34.37 7.41 151.6 0.836 246.1 24.50	$\begin{array}{c} 8.90 \times 10^{-2} \\ 8.80 \times 10^{-2} \\ 1.25 \times 10^{-2} \\ 6.12 \times 10^{-2} \\ 1.38 \times 10^{-2} \\ 9.00 \times 10^{-2} \end{array}$	
23 Model ^B 2	Axial Shear Shear Torsion Moment Moment	Axial Shear Shear Torsion Moment Moment	4.90 3.43 132,7 0.223 191.1 9.56	4.90 3.43 132.7 0,223 445.7 7.10	7.25×10^{-2} 7.25×10^{-2} 1.38×10^{-2} 8.00×10^{-2} 1.38×10^{-2} 7.25×10^{-2}	
24 Model ^B 2	Axial Shear Shear Torsion Moment Moment	Axial Shear Shear Torsion Moment Moment	4.13 2,91 132.0 0,223 445.7 7,10	4.13 2.91 131.9 0.223 187.6 8.00	7.62×10^{-2} 9.12×10^{-2} 1.38×10^{-2} 8.00×10^{-2} 1.38×10^{-2} 7.38×10^{-2}	

* It may be noted that the dynamic stress outputs of element numbers 43/10, 41/42 and 23/24 respectively are to be compared. Element numbers 41, 42 and 43 belong to fixed base model and their corresponding numbers in coupled soil-structure interaction model are 10, 23 and 24 respectively.

Note: Stress outputs for an element are furnished for end I and end J.

First International Conference on Case Histories in Geotechnical Engineering Missouri University of Science and Technology



First International Conference on Case Histories in Geotechnical Engineering Missouri University of Science and Technology http://ICCHGE1984-2013.mst.edu



First International Conference on Case Histories in Geotechnical Engineering Missouri University of Science and Technology http://ICCHGE1984-2013.mst.edu 784



785

First International Conference on Case Histories in Geotechnical Engineering Missouri University of Science and Technology http://ICCHGE1984-2013.mst.edu

and other service loads must meet the machine manufacturers' stringent design stipulations to guard against shaft misalignment.

Once the trial design is judged satisfactory, the computer analysis for the proposed frame foundation may be performed. For the two case studies considered in this paper, the idealised analytical computer models viz. Al, Bl, A2 and B2 are shown in Figs. 2, 3, 5 and 6 respectively.

Machine Data & Relevant Design Parameters

The basic machine data and related soil and other relevant parameters considered for the analysis of two foundations are as mentioned below :

Compressor Foundation

Total machine weight= 680 KN, Rotor weight= 20 KN, Machine speed = 7000 cpm (both turbine and compressor) Dynamic modulus of elasticity of concrete

= 3.6×10^7 KN/sq.metre,

Static modulus of elasticity of concrete
= 2.85x10⁷ KN/sq.metre,

Poissons ratio of concrete = 0.15 Average viscous damping = 5% of critical, Density of concrete = 24 KN/cubic metre, Shear modulus of soil = 6700 KN/sq.metre, Poisson's ratio of soil = 0.45 Allowable bearing pressure of soil = 75 KN/sq.metre

Turbogenerator Foundation

Total machine weight = 23,638 KN Rotor weight = 1919 KN Machine speed = 3000 cpm Shear modulus of soil = 117000 KN/sq.metre Poisson's ratio of soil = 0.30Allowable bearing pressure of soil = 300 KN/sq.metre All data related to concrete material are similar as stated above. In the typical forced response analysis results shown in Table IV, the maximum allowable unbalanced forces as furnished by the machine manufacturer are considered to be acting at bearing no.3 which corresponds to node 67 for model A2 and node 38 for model B2 respectively. These unbalanced forces are (1) vertical force, Fy, = + 488 KN and (2) transverse force, Fz, = \mp 166 KN acting with 90 degree phase differ-

Modeling of Idealised Systems

ence to Fy,

Isometric view of the computer models, which are predominantly composed of prismatic beam elements, for the compressor and turbogenerator foundations are shown on Figs, 2, 3, 5 and 6. In soil-structure interaction models, the beam elements at base level are connected to soil springs. The foundation mat may be represented by plate elements also. These models are suitable for use in any finite element structural analysis program with static and dynamic capabilities. Because of the complexity and expense of vigorously computing the effects of ratiation damping in the foundation an equivalent viscous damping for the soil-structure system is considered and this concept may be reliable since the top half of both the foundations are having uniform mass and stiffness distributions. The soil is modelled in both the cases by using springs at base level of space frame models to represent horizontal and vertical stiffness of the soil which also includes effect of footing embedment. These stiffnesses are derived from elastic half-space theory, These spring stiffnesses are dependent on the shear modulus which in turn varies with the level of shear strain in the soil, Hence for linear elastic models, spring stiffnesses should be incorporated corresponding to a value of shear strain which is less than the maximum expected shear strain.

Discussion on Results of Analysis

A close scrutiny of the results furnished through Tables I to IV reveal that mode shapes, predominant mode of a given structure, static and dynamic stress outputs are all affected if soil-structure interaction are included in the analysis, Since realistic values of soil stiffnesses depend fully on accurate field determination of low strain shear modulus of the supporting soil, which in the opinion of the authors is not yet an established and definitive procedure, the validity of carrying out such rigorous analysis may be questioned. Moreover, while fixing up other important parameters like dynamic elastic modulus of concrete, damping of concrete frame and supporting soil no uniform and rational basis is followed. But variations of these parameters will significantly affect the outcome of entire analysis eventhough model may remain same. Similarly the dynamic stress outputs and effective vibration amplitudes will be influenced considerably in the forced response analysis if the unbalanced forces data change arbitrarily. In view of these factors, start-ing from basic data processing to actual analysis and design; a unified approach should be adopted on an international basis.

CONCLUSIONS

The paper emphasizes the need for analysis and design of framed type turbo-machine and centrifugal compressor foundations with coupled soilstructure interaction due to its significant influence on dynamic response and static stress analysis. At the same time, a meaningful and unified approach in establishing and adopting realistic machine load data and relevant design parameters in order to make such rigorous analysis usable, is also highlighted.

PFFERENCES

"SAP IV Engineering User's Manual, EERC 73-11", University of California, Berkeley.

Arya, S.C., O'Neill, M., and Pincus, G., "Design of Structures & Foundations for Vibrating Machines", Gulf Publishing Co., Houston, 1979.

Whitman, R.V., "Analysis of Soil-Structure Interaction - A State of the Art Review", S.P. No. 300, M.I.T., Massachusetts, 1972.

Barkan, D.D., "Dynamics of Bases&Foundations", McGraw-Hill Co., New York, 1962.

First International Conference on Case Histories in Geotechnical Engineering Missouri University of Science and Technology http://ICCHGE1984-2013.mst.edu