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1120

RADIATION DAMPING OF SOIL-FOUNDATIONS

INTERACTION SYSTEMS

Paper № 4.06

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ABSTRACT

Based on the theory of wave scatter and the velocity, the wave radiation which is caused in soil of dynamic foundation and the range of the affected depth are presented in this paper. It is pointed out that the mass ratio of foundation and the location of the foundation in the layered soil, and its propagation velocity in the affected depth, are the major factors affecting radial damping of soil-foundation system on the half space.

KEYWORDS

Radiation damping Half space Layered strata Envelope velocity

INTRODUDTION

An important step in current methods of dynamic analysis of soil-foundation interaction problems is the determination of the rddiation damping of the rigid shallow foundation. The energy corresponding to radiation damping is generally carried away by body waves and surface waves. (prakash, S., puri, V.K. 1988; and Yan R.J. Wang Y.S. etc. 1981.) It is known, however, that energy of wave are propagnated by envelope velocity. When the different layered strata exists at a certain depth below the base of foundtion, it prevents the propagation of the body by scattering (with a small size layered strata in wide direction below the base) or refleting (with a large size one) them back to foundation and to the ground surface; as a result, their energy are lost within the layered media due to radiative damping and its material damping. When a rigid rock base exists at a certain depth below the soil base of foundation, their energy is either lost within the soil due to the material damping, or at some distance from the fondation, igets converted sintor surface wave energy (Gazetas, G. 1983.1991)^{edu}

RADIATION DAMPING OF SURFACE FOUNDATION

Damping of Wave Stress

As pointed out in references, the foundation block is assumed to be rigid mass, and the medium below the foundation may there fore be considered to be an elastic half rod as shown in fig.1.

The amplitude at operating frequency can be obtained by using theory of wave stress (Yang, X.J.1987):

$$A_{z} = \frac{Q}{\sqrt{(K_{z} - \omega^{2}M)^{2} + (A\rho V_{p}^{2}h)^{2}}} e^{i(\omega t - \alpha)}$$
(1)

$$tg\alpha = \frac{A\rho V_p^2 h}{K_z - \omega^2 M}$$
(2)

in waich

$$\begin{array}{l} \mathbf{h} = \frac{\omega}{V_{s}} \varphi \\ \varphi = \sqrt{\left(1 + \zeta^{2}\right) / \eta^{2}} \end{array} \right\}$$
(3)

when
$$\zeta = 0$$
, and $\varphi = \frac{1}{\eta} = \frac{V_s}{V_p}$

the damping item of the dynamic system in equation (1) is $(C_z \omega)^2 = (A \rho V_p \omega)^2$



Fig.1 Model of damping of wave stress

 $C_{z} = A\rho V_{p} \tag{4}$

in which

A – - the area of the foundation ρ – - unit mass density of soil Vs – - shear wave velocity of soil Vp – - longitudinal wave velocity of soil

Equation(4) is the radial damping coefficient of the soil below the foundation (Gazetas, G., 1983; Yama hara 1964)

Radiation Damping of Surface Foundation on Half Space

It is known from Lysmer's Analog that the damping coefficient of surface foundation on half space can be represented by.

$$C_z = \frac{3.4 r_0^2}{1-v} \sqrt{G\rho}$$
 (5)

Fourth International Conference on Case Histories in Geotechnical Engineering Missouri University of Science and Technology Where

G--the dynamic shear modulus of the medium;

1121

 $r_o - - radius$ of the footing.

By swbstituting the values:

$$V_s = \sqrt{\frac{G}{\rho}}$$
 and $A = \pi r_0^2$

v = - the poisson's ratio of the medium;

into Eq.(5), We get

$$C_z = A\rho V_{La} \tag{6}$$

Where

$$V_{LZ} = \frac{3.4}{\pi (1-v)} V_s$$

equation(6), which is known as Lysmer's Analog, illustrates that radial damping of a footing on elastic half-space can be represented in terms of an elastic half rod as shown in Eq.(4) except abowt the V_{La} and V_p .

Damping Ratio

The radiation damping coefficient C_z Eq. (6) do not include the soil hysteretic damping β_0 (Gazetas, G 1983), to incorporate such damping, the total damping coefficient of the vertical vibrations, C is given by

$$C = C_z + \frac{2K_{zd}\beta_0}{\omega}$$
(7)

in which

 $K_{zd} = \frac{4Gr_0}{1-v} f(v, a_0) - the dynamic equivalent$ spring constant of the soil for vertical vibrations $<math display="block">G = \rho V_s^2 - Shear \mod 0$ soil $V_s - the phase velocity of shear wave of soil$ $<math display="block">\beta_0 - the material damping coefficient of soil$

The damping ratio D_z is given by

$$D_z = \frac{C}{2\sqrt{k_{zd}M}}$$
(8)

RADIATION EFFECT OF RADIATION DAMPING

It is known from theory of wave motion in the elastic half-space that energy of wave are propagated by envelope velocity rather than phase velocity, and the former are usually smaller than the latter. Recall, in an elastic half rod, that the envelope velocity is minimum at the $r_0/\lambda = 0.45$. For this reason, the wave energy of radiation damping are generally carried away, in point of fact, by the smaller value from Eq.(6). For direct use in practical application the corrected radiation damping of surface foundation on half space can be written as

$$C_z = A\rho V_{La} e^{-\pi (1-v)\delta}$$
⁽⁹⁾

and Lysmer's Analog (Prakash, S.1988 and Richart. Jr. etc. 1970) damping ratio is obtained as

$$D_{z} = \frac{0.425}{\sqrt{B_{z}}} e^{-\pi (1-v)\delta}$$
(10)

in which, $\delta - -$ Constants in Table 1.

TABLE 1. Affected Parameters δ of Radiation Damping

$\frac{r_0}{\lambda}$	0.2	0.3	0.4	0.5	0.6	0.8	1.2	1.6	2.0
δ	0 .110	0.285	0.450	0.400	0.330	0.240	0.205	0.185	0.170

RADIATION DAMPING IN LAYERED STRATA AND COMPLICATED BASE

When the elastic wave, which is applied below the footing, propagating in a medium encounters another medium which has different wave velocity and mass density from the propagation medium, the progressing wave will generate reflection, scattering and diffraction, and then the energy has been partly shielded. It has been found that Vs in Eq.(6) and Eq.(9), leds to results

Fourth International Conference on Case Histories in Geotechnical Engineering Missouri University of Science and Technology http://ICCHGE1984-2013.mst.edu consistent with solutions for radial damping of surface foundations. In practical engineering, therefore, how to predict the V_s and the dependent poisson's ratio vin this kind medium in a simple and clear way is a very important subject in engineering decision making. It can be assumed that the heterobody to be a barrier in the medium as follows (Yang X. J1991).

$$V_{SL} = T_u V_s$$

in which

$$T_{u} = \frac{U_{t}}{U_{i}}$$
(11)

in which

- U_t the amplitude of vibration when the different layered strata exist below the footing.
- $$\label{eq:Ui} \begin{split} U_i - the \ \text{amplitude of vibration on homogeneous half} \\ space \quad \text{below the footing} \end{split}$$

The transmissivity T_u is the function of the ratio of wave impedance α , in whch

$$\alpha = \frac{\rho_2 V_{sz}}{\rho_1 V_{s_1}} \tag{12}$$

Where

 $\rho_1 V_{s1}$ – The unit mass density and shear wave velocity of media below the footing

 $\rho_2 V_{s2}$ - The unit mass density and shear wave velocity of the different layered strata

It can be seen from the Eq. (12), when $\alpha = 1$, no hetero-stratum exists in the medium, and $T_u=1$ (i.e. all incident wave passed through).

AFFECTED DEPTH FOR SURFACE FOUNDATION

By theory of wave analysis, this section presents the vertical "depths of influence" H_z below the footing as follows:

$$a_{o} = 0 \sim 0.5, \qquad H_{z} = (0.10 \sim 0.40)\lambda_{R}$$

 $a_{o} > 0.8, \qquad H_{z} = (0.80 \sim 1.55) B$ (13)

in which

a

CASE HISTORY

The results of tested and calculated case historys are tabulated in table 2. In this table, the F1 is a large-sized press foundation was founded on the layers existing composite siltstone and clystone base. The wave radiation, which was caused by layered strata,

$$a_o = \frac{\omega r_o}{V_s}$$
 - dimensionless frequency ratio

1123

 $\lambda_{R} - -R$ wave length B – width of the footing ω - - circular excitation frequency

was consisted in the design. The size the bulk of concrete of foundation was reduced 1160m³ than that of winkler-Viigt model. The beneficial result of the theory of envelope velocity and wave scatter in radiation damping with practical engineering value is gained.

TABLE 2

The Results of Tested and Calculoted Case Historys

	$\frac{b}{(\frac{m}{\rho r_{o}^{3}})}$	Stiffness of base		Dz		Calculated		Tested		-	
N⁰		Static K _z	Dynamic K _{zd}	Eq.(8) Eq.(9)	Eq.(10)	A _z (mm)	f _z (Hz)	A _z (mm)	f _z (Hz)	Dz	Remarks
F ₁	6.10	193.0 × 10 ⁶	174.0 × 10 ⁶	0.253	0.332	0.172	37.43	0.125	31.64	0.248	
F ₂	8.93	170×10^{6}	153.5×10^{6}	0.055	0.280	0.258	33.00	0.303	30.0	0.08	
F ₃	8.16	2.23 × 10 ⁶	1.12×10 ⁶	0.311	0.215	0.533	7.30	0.606	11.0		
F₄	28.80	89.3×10 ⁶	80.3 × 10 ⁶	0.314	0.151	0.065	70.27	0.070	62.7	0.16	
F,	2.53	6.7 × 10 ⁶	5.03×10 ⁶	0.610	0.469	0.016	6.82	0.015	6.90	0.33	
F ₆	3.70	1.3 × 10 ⁶	1.03×10^{6}	0.518	0.453	0.0242	33.10	0.0184	37.6	0.43	- /- /- /- /- /- /- /- /- /- /- /- /- /-

CONCLUSIONS

Lysmer's Analog illustrates that radial damping coefficient of a footing on elastic half-space can be represented in terms of an elastic half rod except about the V_{La} and V_{P} .

In the elastic half-space that energy of wave are propagated by envelope Velocity and the wave energy of radiation damping are generally carried away by the smaller Value from Lysmer's Analog. The corrected radiation damping ratio can be written as Eq. (10).

The elastic wave propagated in a medium will generate reflection, scattering when encountering the different medium. In practical engineering can be assumed that the heterbody to be a barrier in the mediun. It has been found that $V_{SL} = T_u V_s$ leds to results consistent with solutions for radial damping of surface foundations. By engineering prototype measurement and theory analysis, it has been proved that the V_{SL} can be applied in practical engineering for radiation damping in layered media.



Fig.2 The block and the base soil of press foundation



Fig.3 Tested Curves of press foundation

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