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Wave power extraction from multiple oscillating water columns along a straight coast

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The integration of oscillating water column (OWC) wave energy converters into a coastal structure (breakwater, jetty, pier, etc.) or, more generally, their installation along the coast is an effective way to increase the accessibility of wave power exploitation. In this paper, a theoretical model is developed based on the linear potential flow theory and eigenfunction matching method to evaluate the hydrodynamic performance of an array of OWCs installed along a vertical straight coast. The chamber of each OWC consists of a hollow vertical circular cylinder, which is half embedded in the wall. The OWC chambers in the theoretical model may have different sizes, i.e., different values of the radius, wall thickness and submergence. At the top of each chamber, a Wells turbine is installed to extract power. The effects of the Wells turbine together with the air compressibility are taken into account as a linear power take-off system. The hydrodynamic and wave power extraction performance of the multiple coast-integrated OWCs is compared with that of a single offshore/coast-integrated OWC and of multiple offshore OWCs. More specifically, we analyse the role of the incident wave direction, chamber size (i.e., radius, wall thickness and submergence), spacing between OWCs and number of OWCs by means of the present theoretical model. It is shown that wave power extraction from the coast-integrated OWCs for a certain range of wave conditions can be significantly enhanced due to both the constructive array effect and the constructive coast effect. (doi:10.1017/jfm.2019.656)

Key words: wave-structure interactions, surface gravity waves, wave scattering

1. Introduction

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Many different concepts for wave energy conversion have been proposed (Clément et al. 2002; Drew et al. 2009; Rusu & Onea 2018). However, compared with other renewable energy technologies, such as solar, wind or tidal, wave power is rather immature, and relatively few wave energy converters (WECs) have achieved fully commercial operation (Astariz & Iglesias 2015; Drew et al. 2009; Mustapa et al. 2017).

Among the various wave energy conversion concepts, the oscillating water column (OWC) is probably the most extensively investigated and best developed (Falcão & Henriques 2016; Heath 2012). An OWC is generally composed of a hollow chamber with its bottom open to the sea below the waterline. Subjected to ocean waves, the water column enclosed by the chamber moves up and down, applying pressure on the air within the chamber. The air is forced in and out of the chamber through a turbine installed at the top of the OWC, allowing for power extraction.

The cost of power is the major limitation to the uptake of WECs in commercial operation (Heath 2012; Di Lauro et al. 2019). This is a general issue with wave energy, not specific to OWC technology. A number of efforts have been made to achieve commercialization of OWCs (Pawitan et al. 2019; Viviano et al. 2016). The integration of OWCs into coastal structures, such as breakwaters, jetties and piers or along sections of the coast, presents an effective way to increase significantly the attractiveness of wave power exploitation. The fact that the capture factor of WECs may be enhanced by their deployment along the coast (which may be referred to, for simplicity, as the coast effect) was reported for flap-type WECs by Sarkar et al. (2015); Michele et al. (2016), and also for oscillating buoys by Evans (1988); Zhao et al. (2018); Zhang & Ning (2019). In this way, the economics of the OWC can be enhanced thanks to cost-sharing benefits, including construction, installation and maintenance (Arena et al. 2017; Boccotti 2007; Heras-Saizarbitoria et al. 2013; Mustapa et al. 2017). Reliability and survivability of the OWC can be improved as well, allowing power extraction during large wave conditions.

Many theoretical investigations have been devoted to wave power extraction by coast/breakwater-integrated OWCs. Evans & Porter (1995) proposed a two-dimensional (2-D) theoretical model to study the performance of an onshore OWC device that consists of a thin vertical surface-piercing lip in front of a vertical wall. It was illustrated that, by choosing proper submergence of the lip and the spacing distance between the lip and the wall, the incident wave power can be captured efficiently. The performance of a thin-walled OWC installed either at the tip of a thin fixed breakwater or along a straight coast was considered by Martins-Rivas & Mei (2009a,b), who developed theoretical models based on the linear potential flow theory to solve the three-dimensional (3-D) wave radiation/diffraction problems. To deal with the singular behaviours in the velocity field across the gap under the thin wall of OWC chamber, an integral equation for the horizontal velocity under the wall was employed in their models. The extracted power of the OWC at the tip of a thin breakwater was found to be reasonably insensitive to the incident wave direction, whereas the response of the OWC installed on a straight coast was strongly dependent on wave direction. The best performance occurred under normal incidence for most frequencies. Wave reflection at the coast means that the power captured by the OWC can be doubled. Lovas et al. (2010) extended the theoretical model by Martins-Rivas & Mei (2009a,b) into a more general model that can be applied to more complex situations, i.e., a thin-walled OWC installed at a coastal corner. The captured power by the OWC at a concave corner was found to be significantly greater than that when the OWC was installed at the tip of a convex corner of right angle.

More recently, Zheng et al. (2019) developed a theoretical model of a coast/breakwater-integrated OWC, in which the effect of the thickness of the OWC chamber wall was considered. Subjected to a fixed outer radius, the thinner the chamber wall, the larger and broader the main peaks of the frequency response of wave power capture width. Numerical and physical studies on a coast/breakwater-integrated OWC can be found in (Elhanafi et al. 2016; Falcão et al. 2016; He et al. 2012, 2017; Howe & Nader 2017; López & Iglesias 2014; López et al. 2016; Morris-Thomas et al. 2007; Zhang et al. 2012). However, most of these studies are focused on 2-D problems, and therefore miss fundamental

dynamics related to direction changes in wave diffraction and radiation from complexshaped structures.

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In order to fully harness the available wave power in a region and to produce large quantities of energy for electrical grids, wave farms, i.e., arrays of OWCs, are likely to be deployed. For these OWCs deployed not far away from each other, cost-sharing benefits of installation and electrical power transmission can be made as well. On the basis of an analytical solution of hydrodynamic problems from an oscillating circular patch on the water surface, Nihous (2012) presented a model to predict wave power absorption from an array of OWCs. The OWC chamber was assumed to have a sufficiently shallow draught; thus diffraction effects were neglected in the model. A finite array of fixed OWCs without the restriction of shallow draught was considered by Nader et al. (2012) by applying a 3-D finite element method model. The complexity of hydrodynamic interactions between the OWCs within the array was highlighted. Later, a more realistic model with the air compressibility inside the OWC chamber taken into account was proposed (Nader et al. 2014; Sarmento & Falcão 1985; López et al. 2019). The results showed that for some certain wave frequencies, more power can be harnessed by the array of fixed OWCs compared with the total power that the same number of OWCs working in isolation could extract. Recently, Konispoliatis & Mavrakos (2016) developed an efficient theoretical model to investigate the performance of an array of free-floating OWCs. Major improvements in terms of extracted power were demonstrated for arrays with certain spacings between the OWCs. More recently, the hydrodynamic characteristics of a hybrid wave farm consisting of both OWCs and point-absorber WECs were investigated by Zheng et al. (2018).

Apart from the integration of OWCs into coastal structures and the deployment of OWCs in an array, various studies have also been carried out on the development of individual OWCs (Elhanafi *et al.* 2017; Henriques *et al.* 2016; Kurniawan *et al.* 2017; López *et al.* 2014; Ning *et al.* 2018; Pereiras *et al.* 2015; Sheng & Lewis 2018; He *et al.* 2019).

To the authors' knowledge, most of the previous research work on OWCs has been focused on the investigation of either a single coast-integrated/offshore OWC or an array of offshore OWCs. In this article, the concept of integrating multiple OWCs into a straight coast is proposed. The chamber of each OWC mainly consists of a hollow vertical circular cylinder, which is cut away such that it is half open to the sea from a finite submergence to the seabed. To evaluate the hydrodynamic performance of these coastintegrated OWCs, a 3-D theoretical model is developed based on the linear potential flow theory and eigenfunction matching method. The water depth is assumed to be constant, in order to simplify the wave conditions. The effect of a Wells turbine installed at the top of each OWC together with the air compressibility are taken into account by means of a linear power take-off (PTO) system. Different from most of the previous reviewed 3-D theoretical models for a single coast-integrated thin-walled OWC, the present model can be used to study wave power extraction from multiple coast-integrated OWCs without the thin-wall restriction, i.e., the effect of the wall thickness of the OWC chamber is taken into consideration. The performance of the multiple coast-integrated OWC system is compared with that of a single individual coast-integrated OWC, and also with that of single and multiple offshore OWCs, which consists of a stationary hollow vertical cylinder located in the open sea with the whole cylinder cut off at a finite distance from the seabed. The theoretical model is applied to explore the influences of incident wave direction, chamber size (i.e., radius, wall thickness and submergence), spacing distance between the OWCs and the number of OWCs on power extraction systematically.

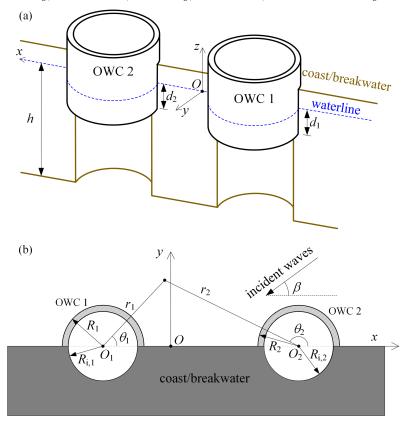


FIGURE 1. Definition sketch: (a) general layout of a pair of OWCs; (b) plan section with key dimensions.

2. Mathematical model

In the model, a number (N) of OWCs are conceptually installed along a straight coast in water of finite depth h (see figure 1, where N=2 is taken as an example). A global Cartesian coordinate system Oxyz is adopted with the Oxy plane at the mean water level and the Oxz plane at the sidewall of the coast. For the N vertical circular OWC chambers, the OWCs are numbered along the Ox axis in ascending order, and N local cylindrical coordinate systems, $O_n r_n \theta_n z_n$, are defined with their origins O_n at the central vertical axis of the n-th OWC (n=1,2,...,N). The O_n can be defined in Cartesian coordinate system Oxyz as $(x_n,0,0)$. In addition, R_n , $R_{i,n}$ and d_n denote the outer radius, inner radius and submergence of the n-th OWC chamber, respectively; and O_n represents the distance between O_n and O_{n+1} .

For the coast-integrated OWCs subjected to regular incident waves with small wave steepness propagating in the direction of β relative to the coast (see figure 1), in the framework of linear potential flow theory, the fluid flow in the water domain is described by the velocity potential

$$\phi(x, y, z, t) = \operatorname{Re}[\Phi(x, y, z)e^{-i\omega t}]. \tag{2.1}$$

Here Φ is a complex spatial velocity potential independent of time, which needs to satisfy Laplace's equation in the fluid, in addition to certain linear boundary conditions, which will be given shortly; i is the imaginary unit; ω denotes the circular frequency of incident waves; and t is the time. The linear potential flow theory is not suitable for extreme

waves, given that it does not account for either the viscous effect or the nonlinear wave dynamics.

Under linear theory, the spatial velocity potential Φ may be decomposed as the sum of scattering and radiation potentials, i.e.,

$$\Phi = \Phi_0 + \sum_{n=1}^{N} p_n \Phi_n, \tag{2.2}$$

where Φ_0 is the scattering spatial velocity potential representing the wave field when the coast-integrated OWCs with the top of each chamber entirely open to the air (i.e., no dynamic air pressure) are subjected to the incident waves; p_n is the complex air pressure amplitude inside the n-th OWC chamber; and Φ_n represents the spatial velocity potential due to a unit air pressure oscillation inside the n-th OWC chamber while all the others are at rest.

Outside the OWCs Φ_0 can be taken as the sum of two parts, $\Phi_0 = \Phi_I + \Phi_D$, where Φ_I is the spatial velocity potential representing the wave field due to the incident waves in the absence of OWCs, which includes both the incident plane wave and a plane wave reflected by the wall, and Φ_D is the diffracted spatial velocity potential due to the presence of the OWCs. In the *n*-th local cylindrical coordinate system $O_n r_n \theta_n z$, Φ_I can be written as (Zheng & Zhang 2015)

$$\Phi_{\rm I}(r_n, \theta_n, z) = -\frac{2igA}{\omega} \frac{Z_0(z)}{Z_0(0)} e^{-ik_0 x_n \cos\beta} \sum_{m=0}^{\infty} \varepsilon_m (-i)^m J_m(k_0 r_n) \cos(m\beta) \cos(m\theta_n). \quad (2.3)$$

Here A is the amplitude of incident waves; g denotes the gravitational acceleration; $\varepsilon_m = 1$ for m=0, whereas $\varepsilon_m = 2$ for m > 0; k_0 is the wavenumber, which satisfies the dispersion relation $\omega^2 = gk_0 \tanh(k_0h)$; J_m denotes the Bessel function of order m; and $Z_0(z)$ is an eigenfunction given by

$$Z_0(z) = N_0^{-1/2} \cosh[k_0(z+h)], \quad N_0 = \frac{1}{2} \left[1 + \frac{\sinh(2k_0h)}{2k_0h} \right].$$
 (2.4)

The governing equation in the water domain, the free-surface boundary conditions, and the body boundary conditions that Φ_{χ} ($\chi = 0, 1, 2, ..., N$) should satisfy are given as follows:

$$\nabla^2 \Phi_{\chi} = 0, \qquad \text{in water,} \tag{2.5}$$

$$\frac{\partial \Phi_{\chi}}{\partial n} = 0$$
, on all solid boundaries, (2.6)

$$\left. \left(\frac{\partial \Phi_{\chi}}{\partial z} - \frac{\omega^2}{g} \Phi_{\chi} \right) \right|_{z=0} = \delta_{\chi,n} \frac{\mathrm{i}\omega}{\rho g}, \quad \text{on the water surface inside the n-th OWC chamber,}$$
(2.7)

$$\left. \left(\frac{\partial \Phi_{\chi}}{\partial z} - \frac{\omega^2}{g} \Phi_{\chi} \right) \right|_{z=0} = 0, \quad \text{on the water surface outside the n-th OWC chamber, (2.8)}$$

in which $\delta_{\chi,n}$ is the Kronecker delta function, which is equal to 1 when $\chi=n,$ and is equal to 0 otherwise; and ρ represents the water density.

Additionally, it is required that $\Phi_{\rm D}$ and Φ_{χ} ($\chi = 1, 2, ..., N$) are outgoing for $r_n \to \infty$.

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3. Solution of scattering and radiated potentials

3.1. Scattering and radiated spatial potentials in different regions

The general solution of the potential Φ_{χ} in the region enclosed by the *n*-th OWC, i.e., $r_n \in [0, R_{i,n}], \theta_n \in [0, 2\pi], z \in [-h, 0]$, is formally expressed as

$$\Phi_{\chi,n}^{\text{in}}(r_n,\theta_n,z) = \sum_{m=-\infty}^{\infty} \sum_{l=0}^{\infty} \frac{\tilde{I}_m(k_l r_n)}{k_l \tilde{I}'_m(k_l R_{i,n})} A_{m,l}^{\chi,n} Z_l(z) e^{im\theta_n} - i \frac{\delta_{\chi,n}}{\rho \omega}.$$
 (3.1)

Here $A_{m,l}^{\chi,n}$ are the unknown coefficients to be solved;

$$\tilde{I}_m(k_l r_n) = \begin{cases} J_m(k_l r_n), & l = 0\\ I_m(k_l r_n), & l = 1, 2, 3, \dots \end{cases}$$
(3.2)

in which I_m denotes the modified Bessel function of the first kind of order m; k_l is the eigenvalue, which is given by (e.g., Falnes (2002))

$$\omega^2 = -gk_l \tan(k_l h), \quad l = 1, 2, 3, ..., \tag{3.3}$$

and the corresponding eigenfunction $Z_l(z)$ is defined by

$$Z_l(z) = N_l^{-1/2} \cos[k_l(z+h)], \quad N_l = \frac{1}{2} \left[1 + \frac{\sin(2k_l h)}{2k_l h} \right], \quad l = 1, 2, 3, \dots$$
 (3.4)

The eigenfunctions $Z_0(z)$ and $Z_l(z)$, as given in equations (2.4) and (3.4) form a complete orthogonal set in $z \in [-h, 0]$:

$$\int_{-h}^{0} Z_m(z) Z_l(z) \, dz = h \delta_{m,l}, \quad m, \ l = 0, 1, 2, 3, \dots$$
 (3.5)

In the region beneath the *n*-th OWC chamber wall, i.e., $r_n \in [R_{i,n}, R_n], \theta_n \in [0, \pi],$ $z \in [-h, -d_n],$ the potential Φ_{χ} can be expressed as

$$\Phi_{\chi,n}^{\text{ring}}(r_n, \theta_n, z) = \sum_{m=0}^{\infty} \left[F_{m,0}^{\chi,n}(r_n) + \sum_{l=1}^{\infty} \left(C_{m,l}^{\chi,n} \frac{I_m(\beta_{n,l} r_n)}{I_m(\beta_{n,l} R_n)} + D_{m,l}^{\chi,n} \frac{K_m(\beta_{n,l} r_n)}{K_m(\beta_{n,l} R_n)} \right) \cos[\beta_{n,l}(z+h)] \right] \cos(m\theta_n)$$
(3.6)

which satisfies the no-flux boundary condition on the coast ($\theta_n = 0$ and π). Therein,

$$F_{m,0}^{\chi,n}(r_n) = \begin{cases} C_{m,0}^{\chi,n} + D_{m,0}^{\chi,n} \left[1 + \ln \left(\frac{r_n}{R_n} \right) \right], & m = 0 \\ C_{m,0}^{\chi,n} \left(\frac{r_n}{R_n} \right)^{|m|} + D_{m,0}^{\chi,n} \left(\frac{r_n}{R_n} \right)^{-|m|}, & m \neq 0 \end{cases}$$
(3.7)

in which $C_{m,l}^{\chi,n}$ and $D_{m,l}^{\chi,n}$ are the unknown coefficients to be determined; K_m is the modified Bessel function of the second kind of order m; and $\beta_{n,l}$ is the l-th eigenvalue given by

$$\beta_{n,l} = \frac{l\pi}{h - d_m}, \quad l = 0, 1, 2, 3, \dots$$
 (3.8)

In the region outside the OWC chambers and in front of the coast extending towards infinite distance horizontally, i.e., $r_n \in [R_n, \infty], \ \theta_n \in [0, \pi], \ z \in [-h, 0]$, the potential Φ_{χ}

can be expressed as

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$$\Phi_{\chi}^{\text{out}}(r_n, \theta_n, z) = \delta_{\chi,0} \Phi_{\text{I}} + \sum_{j=1}^{N} \Phi_{\chi,j}^{\text{out}},$$
(3.9)

where $\Phi_{\chi,j}^{\text{out}}$ represents the velocity potential component diffracted/radiated from the j-th OWC and it can be written in the j-th local cylindrical coordinate $O_j r_j \theta_j z$ as

$$\Phi_{\chi,j}^{\text{out}}(r_j, \theta_j, z) = \sum_{m=0}^{\infty} \sum_{l=0}^{\infty} E_{m,l}^{\chi,j} \frac{\tilde{K}_m(k_l r_j)}{\tilde{K}_m(k_l R_j)} \cos(m\theta_j) Z_l(z),$$
(3.10)

which satisfies the no-flux boundary condition on the coast ($\theta_j = 0$ and π). Here $E_{m,l}^{\chi,j}$ are the unknown coefficients to be determined; and

$$\tilde{K}_m(k_l r_j) = \begin{cases} H_m(k_l r_j), & l = 0\\ K_m(k_l r_j), & l = 1, 2, 3, \dots \end{cases}$$
(3.11)

where H_m denotes the Hankel function of the first kind of order m.

Following Graf's addition theorem for Bessel functions (Abramowitz & Stegun 1964),

$$\tilde{K}_{m}(k_{l}r_{j})\cos(m\theta_{j}) = \sum_{m'=-\infty}^{\infty} \tilde{K}_{m+m'}(k_{l}R_{jn})\tilde{I}_{m'}(k_{l}r_{n})e^{i(m\alpha_{jn}+m'\alpha_{nj})}\cos(m'\theta_{n}), \quad r_{n} \leqslant R_{jn},$$
(3.12)

where R_{jn} and α_{jn} denote the norm and the angle of vector $\overrightarrow{O_jO_n}$, respectively. Therefore, the expression of $\Phi_{\chi,j}^{\text{out}}$ can be transformed from the j-th local cylindrical coordinate into the n-th one, and equation (3.9) is ultimately expressed in the local cylindrical coordinate $O_n r_n \theta_n z$ by

$$\Phi_{\chi}^{\text{out}}(r_{n}, \theta_{n}, z) = \delta_{\chi,0} \Phi_{\text{I}} + \sum_{m=0}^{\infty} \sum_{l=0}^{\infty} E_{m,l}^{\chi,n} \frac{\tilde{K}_{m}(k_{l}r_{n})}{\tilde{K}_{m}(k_{l}R_{n})} \cos(m\theta_{n}) Z_{l}(z)
+ \sum_{\substack{j=1, \ j\neq n}}^{N} \sum_{m=0}^{\infty} \sum_{l=0}^{\infty} \frac{E_{m,l}^{\chi,j} Z_{l}(z)}{\tilde{K}_{m}(k_{l}R_{j})} \sum_{m'=-\infty}^{\infty} \tilde{K}_{m+m'}(k_{l}R_{jn}) \tilde{I}_{m'}(k_{l}r_{n}) e^{i(m\alpha_{jn}+m'\alpha_{nj})} \cos(m'\theta_{n}),
\text{for } r_{n} \leqslant R_{jn}.$$
(3.13)

3.2. Method of computation for unknown coefficients

It is easy to check that the governing equation and all the boundary conditions given in equations (2.5)-(2.8), except the no-flux condition on the inner and outer cylindrical surfaces of each OWC chamber $r_n = R_n$ and $r_n = R_{i,n}$, have been satisfied by the scattering and radiated spatial potentials in different regions, as expressed in Section 3.1, regardless of the values of the unknown coefficients. Note that the no-flux condition at $r_n = R_n$ and $r_n = R_{i,n}$, together with the pressure and velocity continuity conditions on the interfaces of each two adjacent regions should be satisfied as well, which can be applied to solve the unknown coefficients.

The continuity conditions for the scattering and radiated spatial potentials are given as follows:

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(1) Continuity of normal velocity at the boundary $r_n = R_{i,n}$:

$$\frac{\partial \Phi_{\chi,n}^{\text{in}}}{\partial r_n}\Big|_{r_n = R_{\text{i},n}} = \begin{cases}
0, & z \in [-d_n, 0], \ \theta_n \in [0, \pi]; \\
& \text{and } z \in [-h, 0], \ \theta_n \in [\pi, 2\pi], \\
\frac{\partial \Phi_{\chi,n}^{\text{ring}}}{\partial r_n}\Big|_{r_n = R_{\text{i},n}}, & z \in [-h, -d_n], \ \theta_n \in [0, \pi],
\end{cases} (3.14)$$

(2) Continuity of normal velocity at the boundary $r_n = R_n$:

$$\frac{\partial \Phi_{\chi,n}^{\text{out}}}{\partial r_n}\Big|_{r_n=R_n} = \begin{cases}
0, & z \in [-d_n, 0], \ \theta_n \in [0, \pi], \\
\frac{\partial \Phi_{\chi,n}^{\text{ring}}}{\partial r_n}\Big|_{r_n=R_n}, & z \in [-h, -d_n], \ \theta_n \in [0, \pi],
\end{cases} (3.15)$$

(3) Continuity of pressure at the boundary $r_n = R_{i,n}$

$$\Phi_{\chi,n}^{\text{ring}}\Big|_{r_n=R_{\mathbf{i},n}} = \Phi_{\chi,n}^{\text{in}}\Big|_{r_n=R_{\mathbf{i},n}}, \quad z \in [-h, -d_n], \ \theta_n \in [0, \pi],$$
 (3.16)

(4) Continuity of pressure at the boundary $r_n = R_n$:

$$\Phi_{\chi,n}^{\text{out}}\Big|_{r_n=R_n} = \Phi_{\chi,n}^{\text{ring}}\Big|_{r_n=R_n}, \quad z \in [-h, -d_n], \ \theta_n \in [0, \pi].$$
 (3.17)

Inserting the expressions of $\Phi_{\chi,n}^{\text{in}}$, $\Phi_{\chi,n}^{\text{ring}}$ and Φ_{χ}^{out} as given in Section 3.1 into the above continuity conditions, i.e., equations (3.14)-(3.17), and making use of the orthogonality of both trigonometric functions and eigenfunctions, the unknown coefficients $A_{m,l}^{\chi,n}, C_{m,l}^{\chi,n}$ and $D_{m,l}^{\chi,n}$ can be determined by solving a linear algebraic system after truncation (Yu et al. 2019; Zheng & Zhang 2015, 2016). For convenience, the details of the derivations can be found in Appendix A.

3.3. Wave excitation volume flux and hydrodynamic coefficients

The upward displacement of the water surface inside the n-th OWC chamber, i.e., the wave excitation volume flux of the n-th OWC, induced by scattering waves can be written as

$$Q_{e}^{(n)} = \int_{0}^{2\pi} \int_{0}^{R_{i,n}} \frac{\Phi_{0,n}^{\text{in}}(r_{n}, \theta_{n}, z)}{\partial z} \Big|_{z=0} r_{n} dr_{n} d\theta_{n}$$

$$= \frac{2\pi\omega^{2} R_{i,n}}{g} \left(-\frac{A_{0,0}^{0,n}}{k_{0}^{2}} Z_{0}(0) + \sum_{l=1}^{\infty} \frac{A_{0,l}^{0,n}}{k_{l}^{2}} Z_{l}(0) \right).$$
(3.18)

In a similar way, the volume flux of the n-th OWC due to the radiated velocity 240 potential induced by the unit air pressure oscillation inside the χ -th OWC chamber 241 can be evaluated by 242

$$Q_{\chi}^{(n)} = \frac{2\pi\omega^2 R_{i,n}}{g} \left(-\frac{A_{0,0}^{\chi,n}}{k_0^2} Z_0(0) + \sum_{l=1}^{\infty} \frac{A_{0,l}^{\chi,n}}{k_l^2} Z_l(0) \right) = -(c_{\chi}^{(n)} - ia_{\chi}^{(n)}), \tag{3.19}$$

in which $c_{\chi}^{(n)}$ and $a_{\chi}^{(n)}$ on the right hand of the second equals sign are the hydrody-243 namic coefficients, more specifically, the so-called radiation damping and added mass, respectively.

The method as shown in equation (3.19) is a straightforward way to calculate the hydrodynamic coefficient, and is referred to here as the direct method. It is worth noting that there is an alternative approach based on the Haskind relation which can be employed to evaluate $c_{\chi}^{(n)}$ indirectly (e.g., Falnes (2002); Martins-Rivas & Mei (2009 a)),

$$c_{\chi}^{(n)} = \frac{k}{8\pi\rho g c_q A^2} \int_0^{\pi} Q_{\rm e}^{(n)}(\beta) Q_{\rm e}^{(\chi)*}(\beta) \,\mathrm{d}\beta, \tag{3.20}$$

where the superscript * denotes complex conjugate, c_g denotes the wave group velocity and k is used to represent k_0 for the sake of simplicity. The Haskind-type identity as given in equation (3.20) links the radiation and scattering problems and presents a way to check the accuracy of the proposed theoretical model.

4. Relation between power take-off system and hydrodynamic problems

4.1. Response of the OWCs

The wave scattering and radiation problems are coupled by the PTO system. Assuming the mass flux through the Wells turbines is proportional to the chamber air pressure and the effect of air compressibility in the chamber is linear, following Sarmento & Falcão (1985); Martins-Rivas & Mei (2009a,b), the complex air pressure amplitude in each OWC chamber is related to the scattering and radiated velocity potentials, resulting in the following matrix equation:

$$[-i(\mathbf{M}_{PTO} + \mathbf{M}) + (\mathbf{C}_{PTO} + \mathbf{C})]\mathbf{p} = \mathbf{Q}_{e}. \tag{4.1}$$

Here $\mathbf{M}_{\mathrm{PTO}}$ is a diagonal matrix of size $N \times N$, adapted to consider the effect of air compressibility, and the n-th element in the diagonal of $\mathbf{M}_{\mathrm{PTO}}$ can be expressed as $\omega V_n/(v^2\rho_0)$, in which V_n is the air chamber volume of the n-th OWC, v denotes the sound velocity in air and ρ_0 represents the static air density; $\mathbf{C}_{\mathrm{PTO}}$ is a diagonal matrix of size $N \times N$ as well, and it is used to represent the damping of the PTO system of each OWC, which depends on the rotational speed of the turbines, their specification and design, and also the static air density; \mathbf{M} and \mathbf{C} are two matrices of size $N \times N$ that represent the hydrodynamic coefficients that correspond to $a_\chi^{(n)}$ and $c_\chi^{(n)}$, respectively; and p is a column vector of length N that includes all the air pressure responses of the multiple OWCs p_n , n=1,2,...,N. The forcing term $\mathbf{Q}_{\mathbf{e}}$ is a column vector of length N including the complex wave excitation volume flux acting on each OWC $Q_{\mathbf{e}}^{(n)}$, n=1,2,...,N.

As M, C, and Q_e have already been theoretically evaluated in the previous sections, and $M_{\rm PTO}$ and $C_{\rm PTO}$ are known for a specified PTO system, the response of the OWCs can be easily determined by solving the Nth-order complex matrix equation (4.1).

4.2. Wave power extraction

Once the air pressure response in each OWC is obtained, the time-averaged power output by these coast-integrated OWCs can be directly calculated by (e.g., Falnes (2002))

$$P = \frac{1}{2} \boldsymbol{p}^{\dagger} \mathbf{C}_{\text{PTO}} \boldsymbol{p} = \frac{1}{2} \left\| \mathbf{C}_{\text{PTO}}^{1/2} \boldsymbol{p} \right\|^{2}, \tag{4.2}$$

where the superscript \dagger denotes complex-conjugate transpose. Since \mathbf{C}_{PTO} is a real diagonal matrix, the second equality holds, in which $\|\cdot\|$ represents the two-norm of a vector.

Following Lovas *et al.* (2010), the dimensionless coefficients of $Q_{\rm e}^{(n)}$, the hydrodynamic coefficients $c_{\chi}^{(n)}$ and $a_{\chi}^{(n)}$, and the corresponding PTO parameters can be defined as

follows:

$$\bar{Q}_{\rm e}^{(n)} = \frac{\sqrt{g/h}}{Aha} Q_{\rm e}^{(n)}; \quad (\bar{c}_{\chi}^{(n)}, \bar{a}_{\chi}^{(n)}, \bar{c}_{\rm PTO}^{(n)}, \bar{a}_{\rm PTO}^{(n)}) = \frac{\rho \sqrt{g/h}}{h} (c_{\chi}^{(n)}, a_{\chi}^{(n)}, c_{\rm PTO}^{(n)}, a_{\rm PTO}^{(n)}), \quad (4.3)$$

with which the time-averaged power absorption as given in equation (4.2) can be rewritten in terms of wave capture factor:

$$\eta = \frac{2kP}{\rho g A^2 c_g} = \frac{khg}{c_g \sqrt{g/h}} \left\| \bar{\mathbf{C}}_{\mathrm{PTO}}^{1/2} \left[-i(\bar{\mathbf{M}}_{\mathrm{PTO}} + \bar{\mathbf{M}}) + (\bar{\mathbf{C}}_{\mathrm{PTO}} + \bar{\mathbf{C}}) \right]^{-1} \bar{\boldsymbol{Q}}_{\mathrm{e}} \right\|^2, \tag{4.4}$$

where the overbar indicates that the corresponding matrix is written in non-dimensional format.

The rest of this paper focuses on the particular case in which all the OWCs have the same size and the spacing between adjacent OWCs is constant, unless otherwise specified. Moreover, all the OWCs are assumed to employ the same PTO system. Hence, for the sake of convenience, $R_n = R$, $R_{i,n} = R_i$, $d_n = d$, $D_n = D$, $c_{\text{PTO},n} = c_{\text{PTO}}$ and $a_{\text{PTO},n} = a_{\text{PTO}}$ are adopted, with which equation (4.4) simplifies to

$$\eta = \frac{khg\bar{c}_{\text{PTO}}}{c_{q}\sqrt{g/h}} \left\| \left[-i(\bar{a}_{\text{PTO}}\mathbf{I} + \bar{\mathbf{M}}) + (\bar{c}_{\text{PTO}}\mathbf{I} + \bar{\mathbf{C}}) \right]^{-1} \bar{\boldsymbol{Q}}_{\text{e}} \right\|^{2}, \tag{4.5}$$

where **I** represents the square identity matrix of size $N \times N$. The chamber size and chamber geometry of each OWC are fixed and cannot be easily adjusted. Following Lovas et~al.~(2010); Martins-Rivas & Mei $(2009\,a,b)$, here the value of $a_{\rm PTO}$ is calculated based on $\rho/\rho_0=1000,~v=340~{\rm m/s},~h=10~{\rm m}$ and $V_0=\pi R^2h$, as $a_{\rm PTO}=\omega V_0/(v^2\rho_0)$. As a comparison, it might be more feasible to vary the value of $c_{\rm PTO}$, e.g., to use several turbines and control the blade angle and rotation speed, to strive for high efficiency for a wide range of wave frequencies. In this paper, the corresponding optimal PTO damping is considered equal to the optimum coefficient of the same coast-integrated OWC when working in isolation (Lovas et~al.~2010; Martins-Rivas & Mei $2009\,a,b$). The wave power capture factor contributed by the n-th OWC is denoted by η_n .

Apart from the wave power capture factor η , a q-factor is adopted as well to evaluate the effect of the hydrodynamic interaction between the OWCs on power extraction:

$$q = \frac{\eta}{N\eta_0},\tag{4.6}$$

where η_0 represents the maximum wave capture factor of an isolated coast-integrated OWC. If q > 1, using an array of OWCs along the coast plays a constructive role in power absorption. Whereas if q < 1, a destructive effect is induced by the hydrodynamic interaction between the multiple coast-integrated OWCs.

In a similar way, the influence of the coast, i.e., the reflection effect, may be evaluated by

$$q_{\rm c} = \frac{\eta}{\eta'},\tag{4.7}$$

in which η' denotes the wave capture factor of the corresponding offshore OWCs.

As given in equations (4.6) and (4.7), the subscript 0 and the superscript prime represent the individual single isolated situation and the offshore situation, respectively. Hence, $q' = \eta'/(N\eta'_0)$ can be used as the array factor to denote the constructive or destructive hydrodynamic interaction between an array of offshore OWCs on power extraction, in which η'_0 is the power capture factor of a single offshore OWC. Similarly, $q_{c,0} = \eta_0/\eta'_0$ can be used to calculate the reflection effect of the coast on a single coast-integrated OWC.

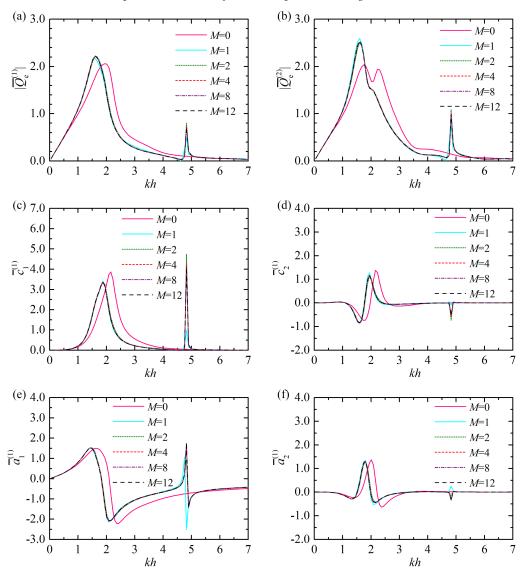


FIGURE 2. Impact of the angular cut-offs (i.e., in terms of M) on wave excitation volume flux and hydrodynamic coefficients, N=2, R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2, D/h=2.0, $\beta=\pi/6$, L=20: (a) $|\bar{Q}_{\rm e}^{(1)}|$; (b) $|\bar{Q}_{\rm e}^{(2)}|$; (c) $\bar{c}_{1}^{(1)}$; (d) $\bar{c}_{2}^{(1)}$; (e) $\bar{a}_{1}^{(1)}$; (f) $\bar{a}_{2}^{(1)}$.

5. Results and discussion

Figures 2 and 3 illustrate the impact of the angular and vertical truncated cutoffs (i.e., in terms of M and L), respectively, on the wave excitation volume flux and hydrodynamic coefficients for two coast-integrated OWCs with R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2, D/h=2.0 and $\beta=\pi/6$. Similarly, the convergence analysis was carried out for cases with a different number of OWCs and with different OWC geometry. In order to obtain the converged results, $M\geqslant 8$ and $L\geqslant 15$ are suggested. Hereinafter, M=12 and L=20 are adopted.

The present theoretical model is focused on an array of coast-integrated OWCs (i.e.,

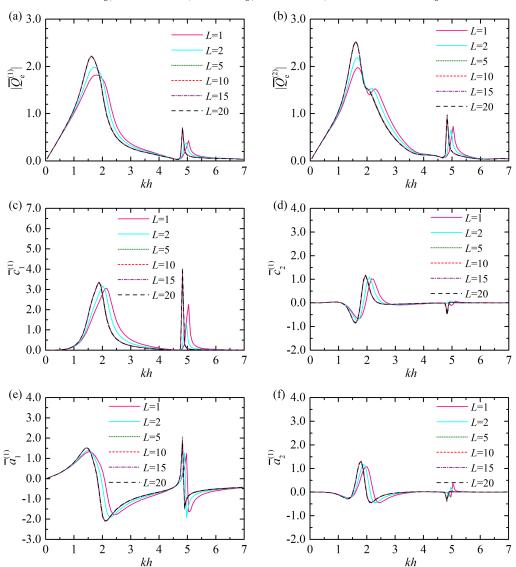


FIGURE 3. Impact of the vertical cut-offs (i.e., in terms of L) on wave excitation volume flux and hydrodynamic coefficients, N=2, R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2, D/h=2.0, $\beta=\pi/6$, M=12: (a) $|\bar{Q}_{\rm e}^{(1)}|$; (b) $|\bar{Q}_{\rm e}^{(2)}|$; (c) $\bar{c}_1^{(1)}$; (d) $\bar{c}_2^{(1)}$; (e) $\bar{a}_1^{(1)}$; (f) $\bar{a}_2^{(1)}$.

 $N \ge 2$) without the thin-walled assumption (i.e., $R_i < R$). By contrast, if the OWCs are 330 deployed far away from each other and the thickness of the OWC wall tends to zero (i.e., 331 $R_i \approx R$), the present model could be used to solve the hydrodynamic problems from a 332 thin-walled coast-integrated OWC (i.e., $R_i = R$) approximately, which was investigated 333 by Martins-Rivas & Mei (2009a). Figure 4 plots the frequency response of $c_1^{(1)}$ and $a_1^{(1)}$ 334 of the coast-integrated OWC(s) with R/h = 0.5 and d/h = 0.2. The present results with 335 N = 2, $R_{\rm i}/h = 0.49$, i.e., $(R - R_{\rm i})/R = 0.02$, and D/h = 200 are in good agreement with 336 those of a single coast-integrated OWC under the thin-wall restriction (Martins-Rivas & 337 Mei 2009a). 338

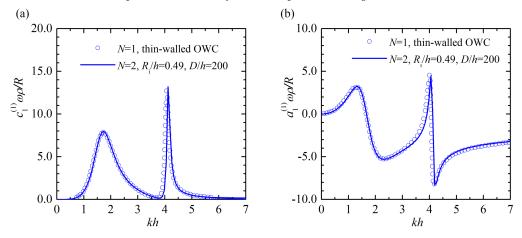


FIGURE 4. Frequency response of $c_1^{(1)}$ and $a_1^{(1)}$ of the coast-integrated OWC(s) with R/h=0.5, d/h=0.2: (a) $c_1^{(1)}$; (b) $a_1^{(1)}$. Circles: results from Martins-Rivas & Mei (2009 a) for a thin-walled OWC, i.e., $R_{\rm i}=R$; lines: present results for two OWCs far away from each other.

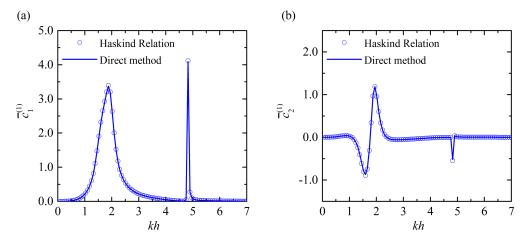


FIGURE 5. Results of wave damping coefficients by using the direct method and the indirect method based on the Haskind Relation, N=2, R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2, D/h=2.0: (a) $\bar{c}_1^{(1)}$; (b) $\bar{c}_2^{(1)}$.

Additionally, figure 5 illustrates the behaviour of $\bar{c}_1^{(1)}$ and $\bar{c}_2^{(1)}$ versus the non-dimensional wavenumber kh for two coast-integrated OWCs with R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2 and D/h=2.0. The plotted results of $\bar{c}_1^{(1)}$ and $\bar{c}_2^{(1)}$ obtained using the direct method and the Haskind relation cannot be distinguished. This excellent agreement between them, together with the results in figure 4, indicate the accuracy of separate computations of scattering and radiation potentials.

As displayed in figure 5a, there are two peaks of $\bar{c}_1^{(1)}$ (kh = 1.88 and 4.82) in the computed range of kh. Figure 6 presents the free-surface patterns ($\text{Re}(\xi_1 e^{-i\omega t})/A = \text{Re}(i\omega p_1 \Phi_1 e^{-i\omega t}/g)/A$) in- and outside the two OWC chambers corresponding to these two peaks of $\bar{c}_1^{(1)}$. Owing to the existence of the coast, the opening of each coast-integrated OWC is asymmetrical; as a result, in addition to the Helmholtz mode (the so-called pumping mode), another mode (i.e., the so-called sloshing mode) of the OWC is excited.

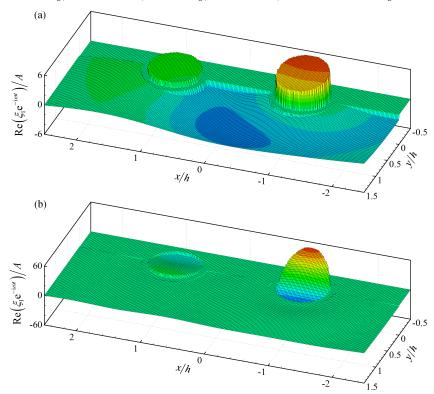


FIGURE 6. Radiation problem-free surface elevation inside and around the coast-integrated OWCs, N=2, R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2, D/h=2.0, $p_1=\rho gA$, $p_2=0$: (a) kh=1.88 at $t=3\pi/2\omega$; (b) kh=4.82, at $t=\pi/2\omega$.

As shown in figure 6a, the pumping mode dominates the wave motion inside each OWC chamber for kh = 1.88. For kh = 4.82, as can be seen from figure 6b, the wave motion inside the OWC chambers is dominated by the sloshing mode.

As illustrated in figure 5, compared to the lower peak of $\bar{c}_1^{(1)}$ (kh=1.88), the higher one (kh=4.82) is sharper and much narrower. Here, as given in figure 7, kh=1.88 is taken as an example to present the scattering results of the free-surface patterns ($\text{Re}(\xi_0 e^{-i\omega t})/A = \text{Re}(i\omega\Phi_0 e^{-i\omega t}/g)/A$) in- and outside the OWC chambers under incident waves with different angles of incidence: $\beta = \pi/6, \pi/4, \pi/3$ and $\pi/2$. Despite the fact that, generally speaking, the scattering wave motion around the integrated OWCs depends on the incident wave direction, the motion inside the OWC chambers is dominated by the pumping mode for kh=1.88, regardless of the incident wave direction (figure 7). Although the sloshing mode plays a rather weak role for such a wave condition, it can still be observed from figure 7 that the symmetrical axis of that mode tends to align itself with the incident wave direction. For $\beta = \pi/2$, as expected, the two water columns behave the same due to the symmetry of both OWC geometry and wave field.

5.1. Comparison between multiple and single coast-integrated/offshore OWCs

Figure 8 displays the frequency responses of the hydrodynamic coefficients for two coast-integrated OWCs, the same OWCs in the open sea and a single coast-integrated/offshore OWC (Zheng *et al.* 2018, 2019). For all four cases in the full range of kh, $\bar{c}_1^{(1)}$ is positive (figure 8a), which is reasonable from the perspective of energy

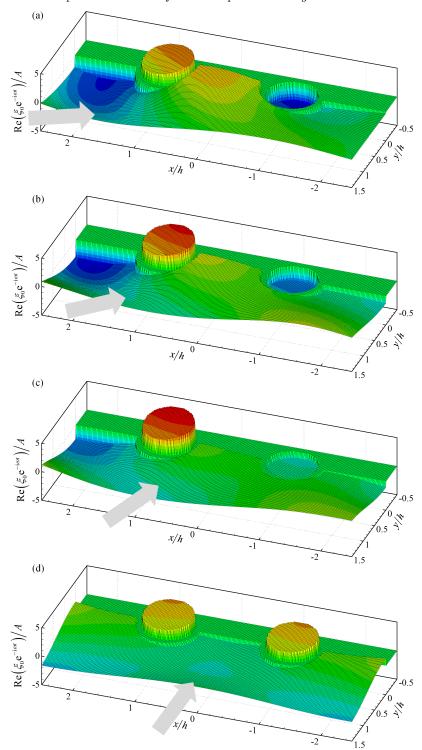


FIGURE 7. Scattering problem-free surface elevation inside and around the coast-integrated OWCs, N=2, R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2, D/h=2.0, t=0, kh=1.88: (a) $\beta=\pi/6$; (b) $\beta=\pi/4$; (c) $\beta=\pi/3$; (d) $\beta=\pi/2$.

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conservation and outgoing propagation of radiated waves (Zheng & Zhang 2018). For both the single and two offshore OWC(s) cases, there is only one peak of the $\bar{c}_1^{(1)}$ -kh curve at kh = 2.44 in the computed range of kh, which corresponds to a pumping mode. The peak value of $\bar{c}_1^{(1)}$ for the two offshore OWCs is somewhat larger than that of the single offshore OWC due to the hydrodynamic interaction between them. For the single/two coast-integrated OWC(s) cases, two modes are excited - the lower (around kh = 1.85) dominated by the pumping mode, the higher (at kh = 4.82) dominated by the sloshing mode. Owing to the constraint of the coastline, the wave motion inside 378 the OWC chamber is more restricted compared to that of the offshore cases, leading to smaller peaks of $\bar{c}_1^{(1)}$. As can be seen from figure 8c, the corresponding $\bar{a}_1^{(1)}$ parameter changes its sign rapidly around those kh values where the peaks of $\bar{c}_1^{(1)}$ occur. Since the effect of hydrostatic stiffness has already been included in $\bar{a}_1^{(1)}$, the kh values where $ar{a}_1^{(1)}$ vanishes correspond to natural resonance. The light grey line plotted in figure 8c represents $-\bar{a}_{\rm PTO}$, which is induced by the air compressibility. When taken into account, resonance happens at the kh values where the $\bar{a}_1^{(1)}$ and $-\bar{a}_{PTO}$ curves intersect each other. For the two offshore/coast-integrated OWCs (figures 8b and 8d), the ranges of $\bar{c}_2^{(1)}$ and $\bar{a}_2^{(1)}$ are comparable to those of $\bar{c}_1^{(1)}$ and $\bar{a}_1^{(1)}$, indicating the significant influence of the hydrodynamic interaction between multiple OWCs.

The frequency responses of the wave excitation volume flux in terms of the amplitude and phase for these four cases subjected to incident waves with $\beta = \pi/2$ are plotted in figure 9. The basic shapes of the $|\bar{Q}_{\rm e}^{(1)}|$ -kh curves (figure 9a) look similar to those of $\bar{c}_1^{(1)}$ -kh (figure 8a). However, due to the wave reflection from the vertical coastline, the peaks of $|\bar{Q}_{\rm e}^{(1)}|$ for the coast-integrated OWC(s) are larger than those of offshore situations, and clearly shift towards lower frequencies. The shift of the position of the peaks can be explained from the point of view of the natural modes: compared to the offshore OWC(s), for which the space under the chamber wall is entirely open to the water, in the case of the coast-integrated OWC(s), half of the space below the chamber on the coast side is closed, implying that a greater proportion of the water column is enclosed. This leads to smaller natural frequencies and, therefore, the OWC(s) are more likely to be significantly excited at lower frequencies. The peaks of $|\bar{Q}_{\rm e}^{(1)}|$ for the two OWCs, regardless of whether they are coast-integrated or offshore, can benefit from the hydrodynamic interaction between them, e.g., the peak value of $|\bar{Q}_{\rm e}^{(1)}|$ of 2.69 for the single coast-integrated OWC, which is reached for kh = 1.73, is enhanced to 3.64 for the two coast-integrated OWCs, and occurs at kh = 1.88. In long waves, e.g., kh < 1.5, the size of the OWCs is small compared to the wavelength, so that the primary effect on the wave field is reflection at the coast, leading to an overlapping of $\varphi_{\rm e}^{(1)}$ -kh for the single/two OWC(s) cases and a separation for offshore and coast-integrated situations (figure 9b).

The wave power extraction from these four cases of OWC(s) are displayed in figure 10 in terms of power capture factor, array factor, coast factor and PTO damping employed. The curve of $-\bar{a}_{PTO}$ intersects the curve of $\bar{a}_1^{(1)}$ at two values of kh, i.e., 2.47 and 4.19, in the computed range of kh for single/two offshore OWC(s) (see figure 8c), and the corresponding wave capture factors (η') and (η') as shown in figure 10a also attain their optimum at these two wave frequencies. The value of η'_0 is exactly 1.0 at the resonant frequencies displayed, which is reasonable and can be theoretically derived by using the Haskind relation (e.g., Falnes (2002)). For the two offshore OWCs case, the largest value of the wave capture factor (η' as displayed in figure 10a) can reach 2.27. There

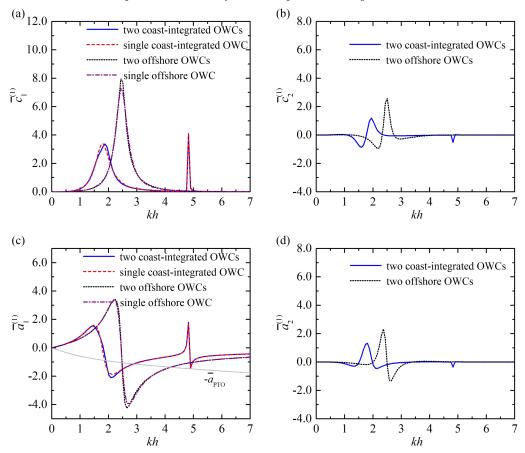


FIGURE 8. Radiation problem, R/h = 0.5, $(R - R_{\rm i})/h = 0.1$, d/h = 0.2, D/h = 2.0, h = 10 m: (a) $\bar{c}_1^{(1)}$; (b) $\bar{c}_2^{(1)}$; (c) $\bar{a}_1^{(1)}$ and $-\bar{a}_{\rm PTO}$; (d) $\bar{a}_2^{(1)}$.

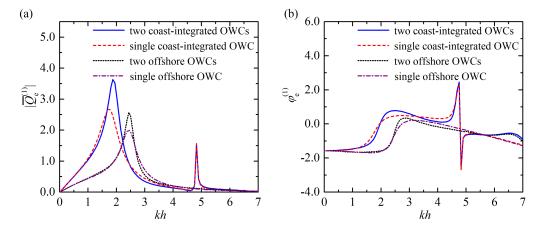


FIGURE 9. Scattering problem, R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2, D/h=2.0, $\beta=\pi/2$: (a) amplitude of wave excitation volume flux, $|\bar{Q}_{\rm e}^{(1)}|$; (b) phase of wave excitation volume flux, $\varphi_{\rm e}^{(1)}$.

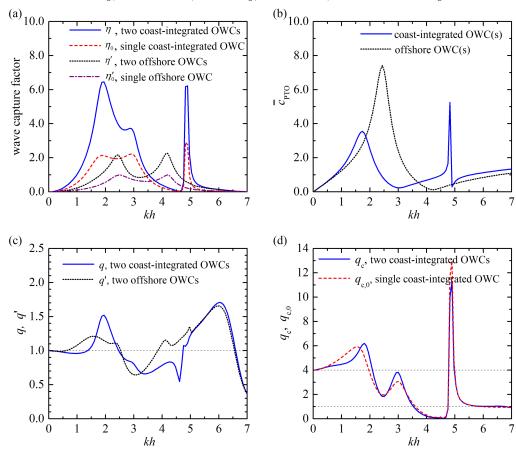


FIGURE 10. Power extraction, R/h = 0.5, $(R - R_i)/h = 0.1$, d/h = 0.2, D/h = 2.0, $\beta = \pi/2$, h = 10 m: (a) wave capture factor, η , η_0 , η' and η'_0 ; (b) \bar{c}_{PTO} ; (c) array factor, q and q'; (d) coast factor, q_c and $q_{c,0}$.

is an obvious drop in η' between these two resonant frequencies, for $kh \in (3.0, 3.5)$, implying that the two OWCs cannot continuously capture wave power effectively in a large range of kh. When the OWC(s) is(are) integrated into a coast, three intersections of $\bar{a}_1^{(1)}$ and $-\bar{a}_{PTO}$ occur, at kh=1.88, 2.92 and 4.82 (figure 8c), resulting in three peaks of η and η_0 (figure 10a). Compared to those of the offshore cases, although the first two resonant frequencies of the coast-integrated cases are closer to each other, η_0 remains mostly around 2.0 for kh between these two frequencies, leading to an even broader bandwidth of high efficiency. For the two coast-integrated OWCs, thanks to the constructive hydrodynamic interaction between them, a large peak value of η , 6.46, is achieved around kh=1.88. The corresponding \bar{c}_{PTO} (figure 10b) shows that, in order to reach optimum efficiency for all wave conditions, the turbine parameter for the coast-integrated cases does not need to be varied as much as that in the offshore cases, making it relatively easy to achieve in practice.

The variation of the array factor for the coast-integrated and offshore cases (i.e., q and q') with kh (figure 10c) indicates that both q and q' tend to unity as kh tends to 0. As a comparison, the coast reflection effect factor for single/two coast-integrated cases (i.e., q_c and $q_{c,0}$, figure 10d) tends to 4.0 when kh tends to 0. This is due to the fact that incident

waves are the dominant element in the excitation volume flux under long waves. In the 435 open sea, the amplitude of undisturbed incident waves is A, whereas the amplitude of 436 incident waves subjected to reflection from the vertical coast turns out to be 2A, leading 437 to a doubling of the excitation volume flux and, in turn, affecting wave power extraction. 438 For short waves, e.g., kh > 5.5, the curves of q and q' tend to overlap each other and 439 the values of q_c and $q_{c,0}$ both approach unity. This can be explained by the dominant 440 role of \bar{c}_{PTO} and \bar{a}_{PTO} in the wave power capture factor (refer to equation (4.5)) in short 441 wave conditions. For kh > 6.0, all hydrodynamic coefficients vanish alongside the wave 442 excitation volume flux (figures 8-10), whereas \bar{c}_{PTO} and \bar{a}_{PTO} remain non-vanishing and 443 become even larger with the increase of kh. As displayed in figure 10d, for most wave 444 conditions, except $kh \in (3.5, 4.8)$ and kh > 5.5, the coast factors remain far above unity, 445 indicating a constructive effect of coast reflection on wave power absorption. However, 446 the array factor oscillates around 1.0 and shows alternating constructive and destructive 447 effects with the change of kh (figure 10c). In the following sections, only the wave capture 448 factor and array factor are preserved to indicate power extraction of the coast-integrated 449 OWCs. As can be seen from figures 10a, 10c and 10d, the dramatic peak of η occurring at 450 kh = 1.88 benefits from both the constructive array effect (q = 1.5) and the constructive 451 coast effect $(q_c = 6.0)$. 452

5.2. Effect of incident wave direction

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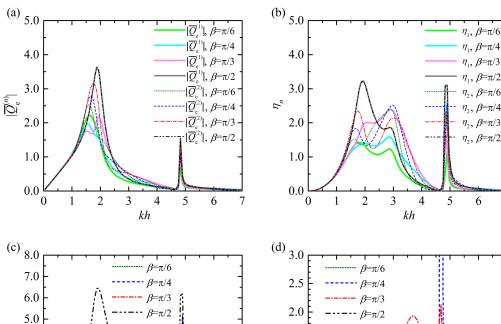
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The wave excitation volume flux of each OWC, the power capture factor of each OWC and both together with the q-factor for different incident wave directions β are displayed in figure 11. As β increases from $\pi/6$ to $\pi/2$, the main peak of the wave excitation volume flux of the up-wave OWC $(|\bar{Q}_{\rm e}^{(2)}|)$ becomes larger and shifts towards large kh. The first peak for the other OWC ($|\bar{Q}_{\rm e}^{(1)}|$), on the contrary, first falls and shifts towards small kh, and then rises and shifts in the opposite direction rapidly to the same position of $|\bar{Q}_{\rm e}^{(2)}|$ for $\beta=\pi/2$. Note that, at $kh\approx 2.0$, a slight rise of the $|\bar{Q}_{\rm e}^{(1)}|$ or $|\bar{Q}_{\rm e}^{(2)}|$ curves with specified values of β is observed, e.g., more particularly, an additional peak of the $|\bar{Q}_{\rm e}^{(1)}|$ -kh curve for $\beta = \pi/3$ can be excited. This appears to be induced by the resonance of water waves between the two OWCs, for $k(D-R) \approx \pi$ is satisfied for these cases at $kh \approx 2.0$. For $\beta = \pi/6$ and $\pi/4$, the wave power capture factor of the up-wave OWC (η_2) is generally larger than the down-wave one (η_1) for $kh \in (1.5, 3.5)$ (figure 11b), whereas for $\beta = \pi/3$, $\eta_1 > \eta_2$ is observed for $kh \in (2.0, 3.0)$. As displayed in figure 11c, for $kh \in (1.5, 2.0)$, the overall power capture factor (η) increases dramatically with the increase of β from $\pi/6$ to $\pi/2$. For $kh \in (2.6, 4.0)$, the η corresponding to $\beta = \pi/3$ is significantly greater than those for all three other incident wave directions, including $\beta = \pi/2$. This can be explained from the perspective of the array effect as illustrated in figure 11d, in which constructive (q > 1.0) and destructive (q < 1.0) effects are indicated for $\beta = \pi/3$ and $\pi/2$, respectively, for $kh \in (2.6, 4.0)$. The following sections focus on the cases with $\beta = \pi/2$.

Note that, at kh=1.88, the maximum η for $\beta=\pi/2$ is dramatically higher than 4.0, while the η values for $\beta=\pi/6$ and $\pi/4$ are obviously lower than 4.0. Instead, at kh=2.92, the η values for the four cases with different values of β are all concentrated around 4.0. In fact, for any certain wave frequency, there is a general identity of the optimum wave capture factor (η_{MAX}) over all incidence angles that multiple coast-integrated OWCs must hold regardless of the OWC dimension, i.e.,

$$\frac{1}{\pi} \int_0^{\pi} \eta_{\text{MAX}}(\beta) \, \mathrm{d}\beta = 2N,\tag{5.1}$$



≈ 4.0

3.0

2.0

1.0 0.0

kh

FIGURE 11. Comparison for different incident direction, β , with N=2, R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2, D/h=2.0, h=10 m: (a) $|\bar{Q}_{\rm e}^{(n)}|$; (b) η_n ; (c) η ; (d) q-factor.

₾ 1.5

1.0

0.5

0.0

kh

which can be theoretically confirmed by invoking the ideal optimization criteria and the Haskind relation (Wolgamot *et al.* 2012). A detailed derivation is given in Appendix B.

5.3. Effect of radius of the OWCs

The effect of radius (R/h) of the OWCs on wave excitation volume flux, hydrodynamic coefficients, wave capture factor and array factor were investigated (figure 12). As R/h increases from 0.3 to 0.7, the main peak of the $|\bar{Q}_{\rm e}^{(n)}|$ curve, as shown in figure 12a, shifts towards lower frequencies and tends to be flatter. The peak value first becomes larger and then smaller after reaching the largest value with R/h = 0.5. As illustrated in figure 12b, for the smallest column R/h = 0.3, the curve of $\bar{c}_1^{(1)}$ has only one peak in the computed range of kh. For larger R/h, i.e., $R/h = 0.4 \sim 0.6$ and R/h = 0.7, two and three peaks, are evident, respectively. This is associated with the natural resonance modes (without PTO system), which are strongly dependent on the relative size of OWC chamber to wavelength. Figure 12(c,e) indicates that, with the increase of R/h, the oscillation amplitudes of the $\bar{c}_2^{(1)}$ and $\bar{a}_2^{(1)}$ curves become larger, and the kh corresponding to these largest amplitudes get smaller. As previously defined in Section 4, $\bar{a}_{\rm PTO}$ depends on the chamber volume $V_0 = \pi R^2 h$, which in turn depends critically on R/h.

 In figure 12d, apart from the five curves of $\bar{a}_1^{(1)}$, five solid thin curves of $-\bar{a}_{\rm PTO}$ relating to five different values of R/h are plotted in the corresponding colour. For R/h=0.3, there are two points of interaction between $-\bar{a}_{\rm PTO}$ and $\bar{a}_1^{(1)}$ in the range of kh plotted; while for larger R/h, more points of interaction can be achieved, e.g., four interaction points for R/h=0.7. As R/h increases from 0.3 to 0.7, the curve of $-\bar{a}_{\rm PTO}$ is slanted downwards, resulting in the first two points of interaction moving towards lower frequencies and the horizontal distance between them getting smaller. The kh values corresponding to the points of interaction between $-\bar{a}_{\rm PTO}$ and $\bar{a}_1^{(1)}$ identified from figure 12d coincide well with the peak positions of the η curves (figure 12f). As R/h increases, the main peaks of the η curve shift towards lower kh and gain intensity. Figure 12g shows that the q-factor remains above unity for $kh \in (1.4, 2.5)$ regardless of the value of R/h. For R/h=0.7, constructive array effects can be obtained for a rather large range of kh, i.e., $kh \in (1.4, 3.3)$. Although some higher and broader peaks of q can be achieved for kh > 4.5, they are not attractive, because either η is too small, or η is only large in a narrow bandwidth.

5.4. Effect of wall thickness of the OWCs

The theoretical results for the OWCs with different chamber wall thickness are plotted in figure 13. As the wall thickness of the OWC chambers increases, i.e., the inner radius of the chamber decreases while the outer radius remains constant, the main peak of $|\bar{Q}_{\rm e}^{(n)}|$ shifts slightly towards higher wave frequencies with a narrower bandwidth (figure 13a), while its peak height remains approximately the same. A similar change occurs for $\bar{c}_1^{(1)}$ (figure 13b), with the main peak becoming higher and more abrupt with the increase of wall thickness of the OWCs. Correspondingly, the first sign changing point of $\bar{a}_1^{(1)}$ (figure 13d) occurs at a lower frequency, and its variation in amplitude gets larger, and happens in a narrower range of kh. With the increase of wall thickness, the position of the largest oscillation amplitude of $\bar{c}_2^{(1)}$ and $\bar{a}_2^{(1)}$ (figure 13c,e) moves towards large kh and the variation becomes more abrupt as well. As illustrated in figure 13f, the peaks of η at $kh \in (1.0, 3.5)$ are lower and the overall bandwidth is narrower for a thicker wall of each OWC chamber. This can also be reflected by the intersections between the $-\bar{a}_{\rm PTO}$ and $\bar{a}_{1}^{(1)}$ curves (figure 13d), which get closer to each other horizontally. In figure 13g, a smaller q-factor is shown to be obtained for the OWCs with a thicker chamber wall for most $kh \in (1.8, 3.5)$, suggesting a relatively more destructive array effect. Hence it may be concluded that to achieve higher wave power absorption efficiency in a broader bandwidth, the OWC chambers with a thinner wall are more appropriate. It should be noted that in practice the chamber wall should not be so thin as to lose structural robustness.

5.5. Effect of submergence of the OWCs

The submergence of the chamber, d/h, can also strongly affect the hydrodynamics and power extraction of the coast-integrated OWCs. As indicated in figures 14b and 14d, the peaks of $\bar{c}_1^{(1)}$ and the sign changing points of $\bar{a}_1^{(1)}$ are found to shift towards lower kh with the increase of d/h. This is reasonable, since a larger d/h means a higher, heavier water column enclosed within the chamber, leading to a smaller natural frequency. As d/h increases, the curve of $|\bar{Q}_e^{(n)}|$ becomes more abrupt (figure 14a), and the peaks become higher and move towards low kh. As indicated in figures 14c and 14e, the frequencies corresponding to the dramatic variations of $\bar{c}_2^{(1)}$ and $\bar{a}_2^{(1)}$ decrease with increasing d/h. It is worth noting that, with the decrease of d/h, although the peaks of the η curve remain

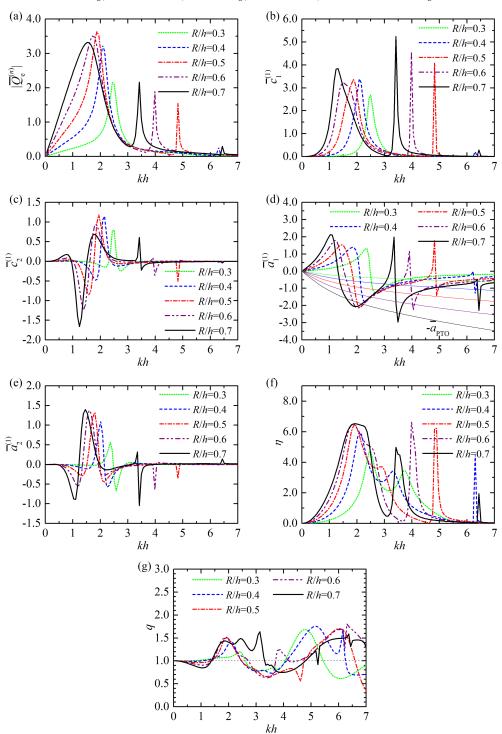


FIGURE 12. Comparison for different radius of the OWCs, R/h, with N=2, $(R-R_{\rm i})/h=0.1$, d/h=0.2, D/h=2.0, $\beta=\pi/2$, h=10 m: (a) $|\bar{Q}_{\rm e}^{(n)}|$; (b) $\bar{c}_{1}^{(1)}$; (c) $\bar{c}_{2}^{(1)}$; (d) $\bar{a}_{1}^{(1)}$ and $-\bar{a}_{\rm PTO}$ (solid thin curves in the same colour of $\bar{a}_{1}^{(1)}$ for the same value of R/h); (e) $\bar{a}_{2}^{(1)}$; (f) η ; (g) q-factor.

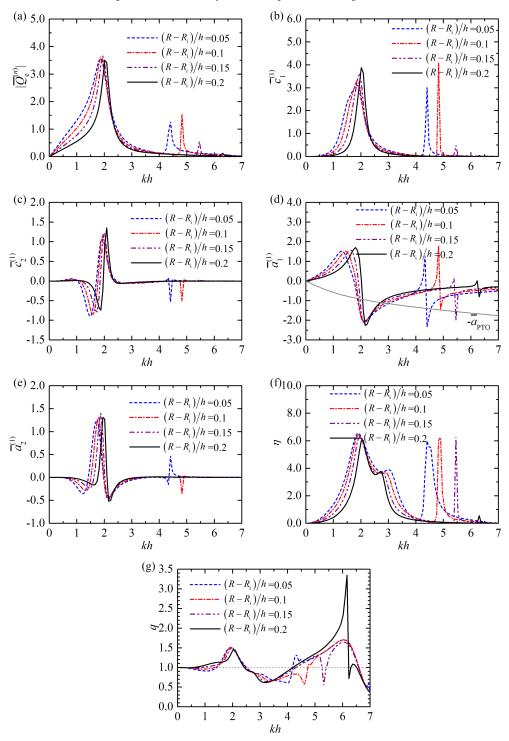


FIGURE 13. Comparison for different wall thickness of the OWCs, $(R-R_{\rm i})/h$, with N=2, R/h=0.5, d/h=0.2, D/h=2.0, $\beta=\pi/2$, h=10 m: (a) $|\bar{Q}_{\rm e}^{(n)}|$; (b) $\bar{c}_{1}^{(1)}$; (c) $\bar{c}_{2}^{(1)}$; (d) $\bar{a}_{1}^{(1)}$ and $-\bar{a}_{\rm PTO}$; (e) $\bar{a}_{2}^{(1)}$; (f) η ; (g) q-factor.

at approximately the same levels (figure 14f), there is a distinct movement of these peaks towards large kh, which can be indicated as well from the position changes of the intersections between the $-\bar{a}_{\text{PTO}}$ and $\bar{a}_{1}^{(1)}$ curves (figure 14d). Furthermore, broader and smoother peaks of η are achieved for a smaller d/h as shown in figure 14f. For short waves, e.g., $kh \in (5.5, 7.0)$, more power can be captured with the decrease of d/h, and this constructive effect becomes stronger and stronger. This is due to the fact that most wave power (approximately 95%) is concentrated at no more than one-quarter of a wavelength below the still-water level, where the kinetic energy at a shallower position is more intensive compared to that at a deeper position. For most wave conditions at $kh \in (2.2, 3.6)$, a larger q-factor is obtained for a smaller d/h (figure 14g). Therefore, to have a better array effect and ultimately to achieve high power absorption in a rather broader bandwidth, the submergence of the OWC chambers should be as small as possible. However, the realistic chamber submergence cannot be too small, otherwise the opening may not be continuously submerged in the water when the OWCs are subjected to either strong waves or a large tidal range.

5.6. Effect of distance between the OWCs

Figure 15 presents the effect of distance between the OWCs. Similar results for the individually isolated single coast/breakwater-integrated OWC (denoted as "isolated") are also displayed for comparison. Figure 15a shows that there are two peaks of the $|\bar{Q}_{\rm e}^{(n)}|$ -kh curve over the computed range of kh, with the main one around $kh \approx 1.8$ and the second sharp one at a higher frequency, i.e., $kh \approx 4.82$. As D/h increases from 1.5 to 3.0, the amplitude of the main peak first increases and then decreases. Although the amplitude of the main peak for D/h = 1.5 is merely 2.4, large values of $|\bar{Q}_{\rm e}^{(n)}|$ compared to the other cases are obtained at $kh \in (2.1, 3.0)$. The kh corresponding to the main peak shifts towards lower frequencies. The second sharp peak is nearly independent of D/h. As shown in figures 15b and 15d, a rather limited impact of D/h on $\bar{c}_1^{(1)}$ and $\bar{a}_1^{(1)}$ is observed at $kh \in (1.5, 2.5)$, where the main peak of the $\bar{c}_1^{(1)}$ -kh curve and the corresponding drop of $\bar{a}_1^{(1)}$ occur. As D/h varies, the $\bar{c}_1^{(1)}$ ($\bar{a}_1^{(1)}$)-kh curve of the two OWCs oscillates slightly around that of the "isolated" case. This is due to the fact that the waves radiated from each coast/breakwater-integrated OWC, and also those waves diffracted from the other OWC, act on the OWC in question simultaneously. The change in D/h leads to alteration of the phase difference between the two-OWCs mutual radiated and diffracted waves, resulting in the switch of reinforcing and diminishing influences. The amplitudes of the peak of $\bar{c}_1^{(1)}$ and the drop of $\bar{a}_1^{(1)}$ at kh=1.8 are both approximately 3.5.

As a comparison (figures 15c and 15e), the variations of $\bar{c}_2^{(1)}$ and $\bar{a}_2^{(1)}$, especially for $kh \in (1.0, 3.0)$, are significantly dependent upon D/h. The amplitudes of the drops of $\bar{c}_2^{(1)}$ and $\bar{a}_2^{(1)}$ around kh = 1.8 are both found to be no smaller than 1.6, revealing a strong hydrodynamic interaction between the OWCs for the four cases of D/h examined. As D/h increases from 1.5 to 3.0, these drops of $\bar{c}_2^{(1)}$ and $\bar{a}_2^{(1)}$ become progressively weaker, and it can be expected that for $D/h \to \infty$, $\bar{c}_2^{(1)} \approx 0$ and $\bar{a}_2^{(1)} \approx 0$ will be obtained. The wave power capture factor of the "isolated" case, i.e., η_0 , is no more than 2.0 (figure 15f). However, for the cases consisting of two OWCs, the value of $\eta > 6.0$ can be obtained for certain values of D/h due to hydrodynamic interactions. From the perspective of the peak value of η , the OWCs with D/h = 2.0 could be the best solution for power absorption. However, in practice, the OWCs with D/h = 1.5 might be a better choice for their good performance over a broader bandwidth, with a sufficiently large wave capture factor. It can be learned (figure 15g) that, indeed, the D/h ratio has a strong effect on the

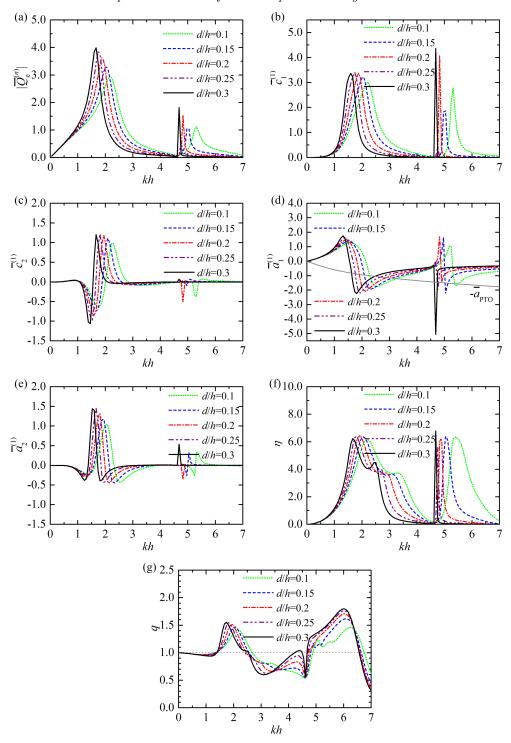


FIGURE 14. Comparison for different submergence of the OWCs, d/h, with N=2, R/h=0.5, $(R-R_{\rm i})/h=0.1$, D/h=2.0, $\beta=\pi/2$, h=10 m: (a) $|\bar{Q}_{\rm e}^{(n)}|$; (b) $\bar{c}_{1}^{(1)}$; (c) $\bar{c}_{2}^{(1)}$; (d) $\bar{a}_{1}^{(1)}$ and $-\bar{a}_{\rm PTO}$; (e) $\bar{a}_{2}^{(1)}$; (f) η ; (g) q-factor.

shape of the q-factor curve as well as on its amplitude. For D/h = 1.5, q > 1.0 is satisfied at $kh \in (1.8, 3.4)$, meaning that a constructive hydrodynamic interaction between the OWCs is achieved in a large range of wave conditions.

5.7. Effect of the number of OWCs

The frequency responses of the wave power capture factor of each OWC for N=2, 3, 4and 5, together with the overall q-factor, are plotted in figure 16. The wave power capture factor of the isolated single coast/breakwater-integrated OWC (i.e., η_0) are also displayed as a comparison. Since the OWCs with the same size are uniformly distributed along the straight coast and are subjected to incident waves with $\beta = \pi/2$, the performance of an individual OWC is the same as the one symmetrical about the centrosymmetric plane of the OWC array. For the sake of simplicity, only the results of the first half number of OWCs are displayed, including the middle one as well if N is odd. It is shown in figures 16a - 16d that, from the perspective of the peak value of the power capture factor, the performance of each OWC among the multiple OWCs is better than that of the single isolated coast-integrated OWC. The closer the OWC is to the middle position of the array, the higher its peak power capture factor. For other wave conditions rather than the peak frequencies, e.g., $kh \in (2.8, 3.5)$, less power can be extracted by an OWC in an array of OWCs, compared to the single isolated coast-integrated case. Moreover, much less can be captured by the OWC closer to the middle position of the array. This tendency of the performances of an array of OWCs compared to a single OWC can also be clearly detected from the q-factor as plotted in figure 16e. For $kh \in (2.0, 2.5)$, although q>1 is achieved for all the examples considered, the q value for two OWCs is obviously smaller than those with more OWCs. It might be concluded that for such a range of wave conditions, an array of coast-integrated OWCs consisting of three or more OWCs could be a better choice, to benefit more fully from array effects, and in turn to extract wave power more efficiently.

5.8. Effect of the distance difference

We consider the effect of varying the distances on wave power extraction from five coast-integrated OWCs subjected to incident waves with $\beta=\pi/2$. The overall length of the array is fixed as $D_1+D_2+D_3+D_4=8h$ and the OWCs are symmetrically deployed about the central OWC, i.e., $D_1=D_4$, $D_2=D_3$. Seven cases with $(D_2-D_1)/h=\Delta D/h=-1.5$, -1.0, -0.5, 0, 0.5, 1.0 and 1.5 are examined. Figure 17 presents the frequency responses of η_n , η and q-factor for these seven cases. Figure 17a demonstrates that for $kh \in (2.1, 2.8)$, when the second and the fourth OWCs are placed closer to the ends of the array (i.e., $\Delta D/h > 0$), more power can be captured by the two OWCs at the ends of the array compared to the uniform distribution (i.e., $\Delta D/h = 0$). Whereas when the second and the fourth OWCs are placed closer to the central OWC (i.e., $\Delta D/h < 0$), less power can be captured by the two end OWCs.

On the contrary, figure 17c indicates an opposite effect of $\Delta D/h$ on the power absorption of the central OWC in terms of the peak value of η_3 : the peak value of η_3 is no more than 3.0 for $\Delta D/h > 0$, while it can be larger than 4.3 for each case with $\Delta D/h \leqslant 0$. As shown in figure 17b, the shape of the η_2 curve is significantly influenced by $\Delta D/h$. As $|\Delta D/h|$ increases from 0 to 1.5, the η_2 curve at $kh \in (1.5, 3.5)$ turns from a single peak curve into a bimodal curve. The less uniform the array layout, i.e., the larger $|\Delta D/h|$, the greater the separation between the two peaks of the curve. This is reasonable, since the hydrodynamic interaction between each pair of adjacent OWCs is dependent on the distance between them (as demonstrated in figure 15), leading to two reinforcing peaks

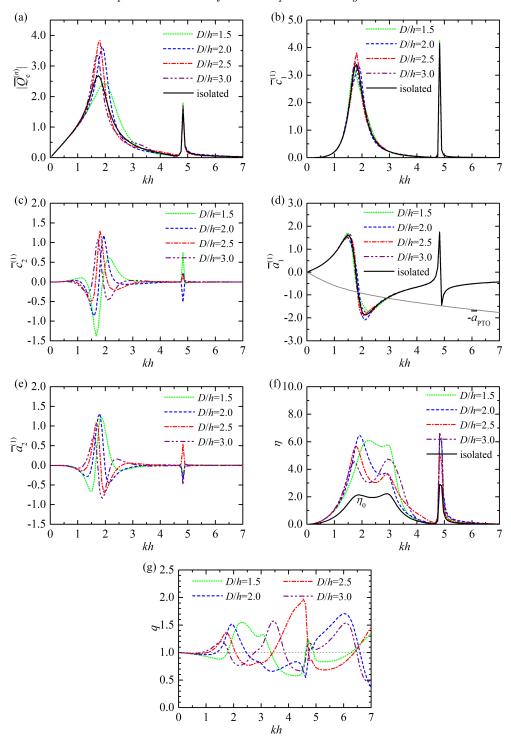


FIGURE 15. Comparison for different spacing distance between the OWCs, D/h, with N=2, R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2, $\beta=\pi/2$, h=10 m: (a) $|\bar{Q}_{\rm e}^{(n)}|$; (b) $\bar{c}_{1}^{(1)}$; (c) $\bar{c}_{2}^{(1)}$; (d) $\bar{a}_{1}^{(1)}$ and $-\bar{a}_{\rm PTO}$; (e) $\bar{a}_{2}^{(1)}$; (f) η ; (g) q-factor.

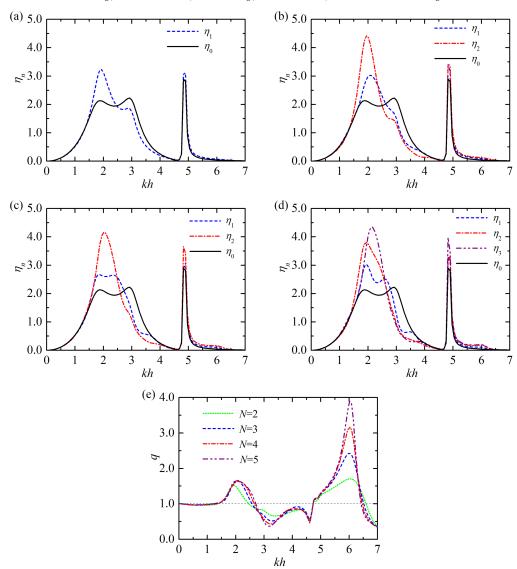


FIGURE 16. Comparison for different number of the OWCs, N, with R/h = 0.5, $(R-R_i)/h = 0.1$, d/h = 0.2, D/h = 2.0, $\beta = \pi/2$, h = 10 m: (a) η_n with N = 2; (b) η_n with N = 3; (c) η_n with N = 4; (d) η_n with N = 5; (e) q-factor.

of η_2 at two different frequencies when $|(D_2 - D_1)/h| = |\Delta D/h|$ is large enough. Figure 17d indicates that the main peak of the total wave power capture factor of the array, η , for $\Delta D/h = 0$ and 0.5 is larger than in other cases. Of these two options, the array with $\Delta D/h = 0.5$ might be of greater practical interest, for the power captured by each OWC is more balanced than in the case with $\Delta D/h = 0$.

Although the peak value of η is reduced with a non-uniform array layout, the peak is broadened. Therefore, the array with different distances may well be attractive in practice, especially for a broad-banded wave spectrum. A peak value of the q-factor larger than 2.9 is achieved for $\Delta D/h = \pm 1.5$ around kh = 4.0. Thanks to the constructive

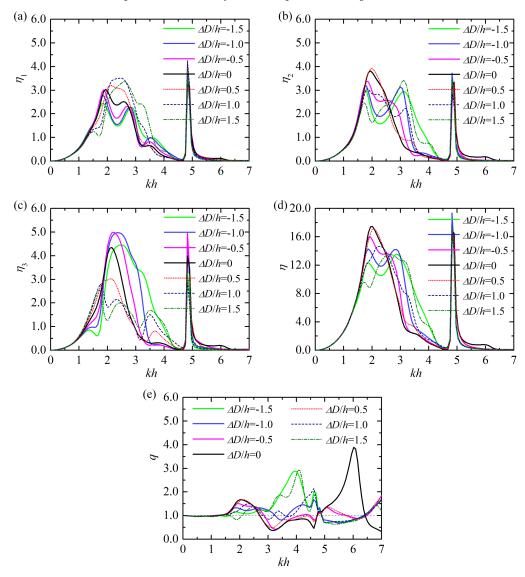


FIGURE 17. Comparison for different distance difference, $\Delta D/h$, with N=5, R/h=0.5, $(R-R_{\rm i})/h=0.1$, d/h=0.2, $D_1+D_2+D_3+D_4=8h$, $D_1=D_4$, $D_2=D_3$, $\beta=\pi/2$, h=10 m: (a) η_1 ; (b) η_2 ; (c) η_3 ; (d) η ; (e) q-factor.

hydrodynamic interaction, the array with $\Delta D/h = \pm 1.5$ absorbs more power than the other cases for $kh \in (3.1, 4.3)$.

6. Conclusions

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An array of coast-integrated OWCs is considered in this paper. The chamber of each OWC is mainly composed of a hollow vertical circular cylinder. Each OWC cylinder is half-embedded in the wall, with the other half on the seaward side open from a finite submergence to the seabed. Based on the linear potential flow theory and eigenfunction matching method, a theoretical model was developed to solve the wave scattering and

wave radiation problems of these OWCs. The effects induced by the Wells turbine installed at the top of each OWC and the compressibility of air inside each chamber were represented by a linear PTO system. The present theoretical model was developed without the thin-wall restriction; hence the influence of the wall thickness of the OWC chamber on power extraction can be examined. The performances of the multiple and single coast-integrated/offshore OWCs in wave power extraction were compared with each other. The theoretical model was ultimately applied to explore the influence of the wave conditions, chamber size, spacing between the OWCs and number of OWCs on power extraction. The following conclusions may be drawn.

Wave reflection at the coast plays a constructive role in wave power absorption for most of the wave conditions examined. The hydrodynamic interaction between the coast-integrated OWCs, which is referred to in this work as the array effect, can enhance power extraction of the OWCs dramatically. A dramatic peak wave power capture factor, much higher than that of a single offshore/coast-integrated OWC and of multiple offshore OWCs, can be achieved due to both the constructive array effect and the constructive coast effect. For any certain wave frequency, there is a general identity, i.e., equation (5.1), of the optimum wave capture factor over all incidence angles that multiple coast-integrated OWCs must hold regardless of the OWC dimension. It means a higher peak in the curve of wave power capture factor at some incident wave directions must be associated with less power absorption at other wave incident angles.

As the radius of the coast-integrated OWC chambers increases, the main peaks of the frequency response curve of power capture factor shift towards lower wave frequencies and gain intensity. The wall thickness and submergence of the OWC chambers should be as small as possible to yield high wave power extraction across a broad bandwidth.

The spacing between two coast-integrated OWCs has a strong effect on the shape of the array factor frequency response curve as well as on its amplitude. For multiple coast-integrated OWCs with the same spacing, the one(s) closest to the central position has the highest peak power capture factor. The power absorption by individual OWCs in an array can be balanced and the frequency response of the overall wave capture factor can be improved by adopting a non-uniform array layout.

The linear approximation for small wave steepness was used throughout and no viscous effect was considered; hence the model is not suitable for extreme wave-structure interactions. In future work we will consider the optimization of the array from a general point of view, i.e., considering directional wave spectra, coast-integrated OWCs with different geometries (different diameters, wall thicknesses and submergences) and more elaborate PTO control strategies. "Near-trapping" effects as described by, for example, Maniar & Newman (1997); Thompson et al. (2008) were not registered in our results. However, this is an aspect of interest that we plan to investigate as a continuation of this line of research.

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Appendix A. Integral equations of the scattering/radiation problems

Inserting equations (3.1) and (3.6) into equation (3.14), after multiplying both sides by $Z_{\zeta}(z)e^{-i\tau\theta_n}$ and integrating for $z \in [-h, 0]$ and $\theta_n \in [0, 2\pi]$, for any pair of integers

 (τ,ζ) , it can be shown that

$$2\pi h A_{\tau,\zeta}^{\chi,n} = \sum_{l=0}^{\infty} \left[\frac{\pi}{\varepsilon_{|\tau|}} (X_{|\tau|,l}^{(1,n)} C_{|\tau|,l}^{\chi,n} + Y_{|\tau|,l}^{(1,n)} D_{|\tau|,l}^{\chi,n}) + i \sum_{\substack{m=0,\\m\neq |\tau|}}^{\infty} \frac{\tau[(-1)^{\tau-m} - 1]}{\tau^2 - m^2} (X_{m,l}^{(1,n)} C_{m,l}^{\chi,n} + Y_{m,l}^{(1,n)} D_{m,l}^{\chi,n}) \right] L_{l,\zeta}^{(n)},$$
(A 1)

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$$X_{m,l}^{(1,n)} = \begin{cases} \frac{m}{R_n} \left(\frac{R_{i,n}}{R_n}\right)^{m-1}, & l = 0\\ \frac{\beta_{n,l} I_m'(\beta_{n,l} R_{i,n})}{I_m(\beta_{n,l} R_n)}, & l \neq 0 \end{cases}, \quad Y_{m,l}^{(1,n)} = \begin{cases} \frac{1}{R_{i,n}}, & l = 0, \ m = 0\\ -\frac{m}{R_n} \left(\frac{R_n}{R_{i,n}}\right)^{m+1}, & l = 0, \ m \neq 0, \end{cases}$$

$$\frac{\beta_{n,l} K_m'(\beta_{n,l} R_{i,n})}{K_m(\beta_{n,l} R_n)}, \quad l \neq 0$$
(A 2)

$$L_{l,\zeta}^{(n)} = \int_{-h}^{-d_n} \cos[\beta_{n,l}(z+h)] Z_{\zeta}(z) dz$$

$$= \begin{cases} \frac{(-1)^l (h - d_n)^2 k_0 Z_0(0) \sinh[k_0 (h - d_n)]}{[(h - d_n)^2 k_0^2 + l^2 \pi^2] \cosh(k_0 h)}, & \zeta = 0\\ \frac{(-1)^l (h - d_n)^2 k_\zeta Z_{\zeta}(0) \sin[k_\zeta (h - d_n)]}{[(h - d_n)^2 k_\zeta^2 - l^2 \pi^2] \cos(k_\zeta h)}, & \zeta \neq 0 \end{cases}$$
(A 3)

Inserting equations (3.6) and (3.13) into equation (3.15), after multiplying both sides by $Z_{\zeta}(z)\cos(\tau\theta_n)$ and integrating for $z\in[-h,0]$ and $\theta_n\in[0,\pi]$, for any pair of integers (τ,ζ), it can be shown that

$$\sum_{l=0}^{\infty} (X_{\tau,l}^{(2,n)} C_{\tau,l}^{\chi,n} + Y_{\tau,l}^{(2,n)} D_{\tau,l}^{\chi,n}) L_{l,\zeta}^{(n)} - h Z_{\tau,\zeta}^{(2,n)} E_{\tau,\zeta}^{\chi,n} - \sum_{\substack{j=1, \ j \neq n}}^{N} \sum_{m=0}^{\infty} E_{m,\zeta}^{\chi,j} T_{m,\tau,\zeta}^{\prime n,j} \\
= -\frac{2\delta_{\chi,0} \delta_{\zeta,0} \varepsilon_{\tau} igAk_0 h}{\omega Z_0(0)} e^{-ik_0 x_n \cos \beta} (-i)^{\tau} J_{\tau}^{\prime}(k_0 R_n) \cos(\tau \beta), \tag{A 4}$$

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$$T_{m,\tau,\zeta}^{\prime n,j} = \frac{\varepsilon_{\tau} k_{\zeta} h \tilde{I}_{\tau}^{\prime}(k_{\zeta} R_n)}{2\tilde{K}_m(k_{\zeta} R_j)} [\tilde{K}_{m+\tau}(k_{\zeta} R_{jn}) + (-1)^{\tau \delta_{\zeta,0}} \tilde{K}_{m-\tau}(k_{\zeta} R_{jn})] e^{i(m\alpha_{jn} + \tau \alpha_{nj})}, \quad (A5)$$

$$X_{\tau,\zeta}^{(2,n)} = \begin{cases} \frac{\tau}{R_n}, & \zeta = 0\\ \frac{\beta_{n,\zeta} I_{\tau}'(\beta_{n,\zeta} R_n)}{I_{\tau}(\beta_{n,\zeta} R_n)}, & \zeta \neq 0 \end{cases}, \quad Y_{\tau,\zeta}^{(2,n)} = \begin{cases} \frac{1}{R_n}, & \zeta = 0, \ \tau = 0\\ -\frac{\tau}{R_n}, & \zeta = 0, \ \tau \neq 0\\ \frac{\beta_{n,\zeta} K_{\tau}'(\beta_{n,\zeta} R_n)}{K_{\tau}(\beta_{n,\zeta} R_n)}, & \zeta \neq 0 \end{cases}$$
(A 6)

$$Z_{\tau,\zeta}^{(2,n)} = \begin{cases} \frac{k_0 H_{\tau}'(k_0 R_n)}{H_{\tau}(k_0 R_n)}, & \zeta = 0\\ \frac{k_{\zeta} K_{\tau}'(k_{\zeta} R_n)}{K_{\tau}(k_{\zeta} R_n)}, & \zeta = 1, 2, 3, \dots \end{cases}$$
(A 7)

Inserting equations (3.1) and (3.6) into equation (3.16), after multiplying both sides by $\cos[\beta_{n,\zeta}(z+h)]\cos(\tau\theta_n)$ and integrating for $z\in[-h,-d_n]$ and $\theta_n\in[0,\pi]$, for any pair of integers (τ,ζ) , it can be shown that

$$\begin{split} &\sum_{l=0}^{\infty} \left[\frac{\pi}{2} \left(\frac{\tilde{I}_{\tau}(k_{l}R_{\mathbf{i},n})}{k_{l}\tilde{I}_{\tau}'(k_{l}R_{\mathbf{i},n})} A_{\tau,l}^{\chi,n} + \frac{\tilde{I}_{-\tau}(k_{l}R_{\mathbf{i},n})}{k_{l}\tilde{I}_{-\tau}'(k_{l}R_{\mathbf{i},n})} A_{-\tau,l}^{\chi,n} \right) \\ &- \mathrm{i} \sum_{\substack{m=-\infty,\\m\neq\pm\tau}}^{\infty} \frac{m[(-1)^{m-\tau}-1]}{m^{2}-\tau^{2}} \frac{\tilde{I}_{m}(k_{l}R_{\mathbf{i},n})}{k_{l}\tilde{I}_{m}'(k_{l}R_{\mathbf{i},n})} A_{m,l}^{\chi,n} \right] L_{\zeta,l}^{(n)} \\ &= \frac{\pi(h-d_{n})}{\varepsilon_{\tau}\varepsilon_{\zeta}} (X_{\tau,\zeta}^{(3,n)} C_{\tau,\zeta}^{\chi,n} + Y_{\tau,\zeta}^{(3,n)} D_{\tau,\zeta}^{\chi,n}) + \frac{\delta_{\chi,n}\delta_{\tau,0}\delta_{\zeta,0}\mathrm{i}\pi(h-d_{n})}{\rho\omega}, \end{split} \tag{A 8}$$

708 where

$$X_{\tau,\zeta}^{(3,n)} = \begin{cases} \left(\frac{R_{i,n}}{R_n}\right)^{\tau}, & \zeta = 0\\ \frac{I_{\tau}(\beta_{n,\zeta}R_{i,n})}{I_{\tau}(\beta_{n,\zeta}R_n)}, & \zeta \neq 0 \end{cases}, \quad Y_{\tau,\zeta}^{(3,n)} = \begin{cases} 1 + \ln\left(\frac{R_{i,n}}{R_n}\right), & \zeta = 0, \ \tau = 0\\ \left(\frac{R_n}{R_{i,n}}\right)^{\tau}, & \zeta = 0, \ \tau \neq 0 \end{cases}. \quad (A.9)$$

Inserting equations (3.6) and (3.13) into equation (3.17), after multiplying both sides by $\cos[\beta_n, \zeta(z+h)]\cos(\tau\zeta_n)$ and integrating for $z \in [-h, -d_n]$ and $\theta_n \in [0, \pi]$, for any pair of integers (τ, ζ) , the following expression is obtained

$$\frac{h - d_n}{\varepsilon_{\zeta}} (C_{\tau,\zeta}^{\chi,n} + D_{\tau,\zeta}^{\chi,n}) - \sum_{l=0}^{\infty} E_{\tau,l}^{\chi,n} L_{\zeta,l}^{(n)} - \sum_{\substack{j=1, \ j \neq n}}^{N} \sum_{m=0}^{\infty} \sum_{l=0}^{\infty} E_{m,l}^{\chi,j} T_{m,\tau,l}^{n,j} L_{\zeta,l}^{(n)}$$

$$= -\frac{2\delta_{\chi,0} \varepsilon_{\tau} i g A L_{\zeta,0}^{(n)}}{\omega Z_0(0)} e^{-ik_0 x_n \cos \beta} (-i)^{\tau} J_{\tau}(k_0 R_n) \cos(\tau \beta), \tag{A 10}$$

712 in which

$$T_{m,\tau,l}^{n,j} = \frac{\varepsilon_{\tau} \tilde{I}_{\tau}(k_l R_n)}{2\tilde{K}_m(k_l R_j)} [\tilde{K}_{m+\tau}(k_l R_{jn}) + (-1)^{\tau \delta_{l,0}} \tilde{K}_{m-\tau}(k_l R_{jn})] e^{\mathrm{i}(m\alpha_{jn} + \tau \alpha_{nj})}.$$
 (A 11)

A linear algebraic system can be formed by equations (A 1), (A 4), (A 8) and (A 10), and can be used to solve the unknown coefficients $A_{m,l}^{\chi,n}$, $C_{m,l}^{\chi,n}$, $D_{m,l}^{\chi,n}$ and $E_{m,l}^{\chi,n}$ numerically after truncation. In the present model, the infinite terms of $e^{-\mathrm{i}m\theta_n}/\cos(m\theta_n)$ and $Z_l(z)/\cos[\beta_{n,l}(z+h)]$ are truncated at m=M and l=L, respectively. Accurate results can be obtained by choosing M=12 and L=20.

Appendix B. Identity of optimum wave capture factor over all incidence angles

Following Evans (1980); Falnes (1980), the theoretical maximum power that may be extracted by multiple coast-integrated OWCs can be expressed as

$$P_{\text{MAX}} = \frac{1}{8} \mathbf{Q}_{\text{e}}^{\dagger}(\beta) \mathbf{C}^{-1} \mathbf{Q}_{\text{e}}(\beta), \tag{B1}$$

which is obtained when an ideal PTO system is applied, such that

$$p(\beta) = p_{\text{opt}}(\beta) = \frac{1}{2} \mathbf{C}^{-1} Q_{\text{e}}(\beta)$$
 (B2)

is satisfied (provided C is non-singular).

Note that **C** is composed of real elements, and, more specifically, it can be shown from equation (3.20) that **C** is symmetric. Assuming **C** is positive definite (Wolgamot *et al.* 2012), **C** can be written as the product of an upper real triangular matrix **H** and its transpose with the employment of the Cholesky decomposition,

$$\mathbf{C} = \mathbf{H}^{\mathrm{T}}\mathbf{H},\tag{B3}$$

 $_{728}$ $\,$ where T denotes the conjugate transpose. Hence,

$$\mathbf{C}^{-1} = \mathbf{H}^{-1}(\mathbf{H}^{\mathrm{T}})^{-1}.\tag{B4}$$

For the sake of convenience, a column vector of length N is defined as (Wolgamot et al, 2012)

$$S(\beta) = (\mathbf{H}^{\mathrm{T}})^{-1} \mathbf{Q}_{e}(\beta), \tag{B5}$$

from which equation (B1) can be rewritten as

$$P_{\text{MAX}}(\beta) = \frac{1}{8} \mathbf{S}^{\dagger}(\beta) \mathbf{S}(\beta). \tag{B6}$$

Rewriting (3.20) in the matrix format gives

$$\mathbf{C} = \frac{k}{8\pi\rho g c_a A^2} \int_0^{\pi} \mathbf{Q}_{e}(\beta) \mathbf{Q}_{e}^{\dagger}(\beta) \, \mathrm{d}\beta. \tag{B7}$$

Multiplying two **H** related inverse matrices results in

$$(\mathbf{H}^{\mathrm{T}})^{-1}\mathbf{C}\mathbf{H}^{-1} = \frac{k}{8\pi\rho q c_{a} A^{2}} \int_{0}^{\pi} \mathbf{S}(\beta) \mathbf{S}^{\dagger}(\beta) \,\mathrm{d}\beta = \mathbf{I}, \tag{B8}$$

734 leading to the integral

$$\int_0^{\pi} S_i(\beta) S_j^*(\beta) \, \mathrm{d}\beta = \delta_{i,j} \frac{8\pi \rho g c_g A^2}{k}.$$
 (B9)

Integrating equation (B 6) over $\beta \in [0, \pi]$ and adopting equation (B 9) gives

$$\int_0^{\pi} P_{\text{MAX}} \, \mathrm{d}\beta = \frac{N\pi \rho g c_g A^2}{k},\tag{B 10}$$

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$$\frac{1}{\pi} \int_0^{\pi} \eta_{\text{MAX}}(\beta) \, \mathrm{d}\beta = \frac{2k}{\pi \rho g c_g A^2} \int_0^{\pi} P_{\text{MAX}}(\beta) \, \mathrm{d}\beta = 2N.$$
 (B 11)

- ABRAMOWITZ, M. & STEGUN, I. A. 1964 Handbook of mathematical functions. Washington,
 D.C.: Government Printing Office.
- ARENA, F., ROMOLO, A., MALARA, G., FIAMMA, V. & LAFACE, V. 2017 The first full operative U-OWC plants in the port of civitavecchia. In ASME 2017 36th International Conference on Ocean, Offshore and Arctic Engineering, p. V010T09A022. American Society of Mechanical Engineers.
- ASTARIZ, S. & IGLESIAS, G. 2015 The economics of wave energy: A review. Renewable and
 Sustainable Energy Reviews 45, 397–408.
- POCCOTTI, P. 2007 Caisson breakwaters embodying an OWC with a small opening-part I:
 Theory. Ocean Engineering 34 (5-6), 806-819.
- CLÉMENT, A., MCCULLEN, P., FALCÃO, A. F. DE O., FIORENTINO, A., GARDNER, F.,
 HAMMARLUND, K., LEMONIS, G., LEWIS, T., NIELSEN, K., PETRONCINI, S., PONTES,
 M. T., SCHILD, P., SJÖSTRÖM, B. O., SØRENSEN, H. C. & THORPE, T. 2002 Wave
 energy in Europe: current status and perspectives. Renewable and Sustainable Energy
 Reviews 6 (5), 405 431.
- DI LAURO, E., LARA, J. L., MAZA, M., LOSADA, I. J., CONTESTABILE, P. & VICINANZA,
 D. 2019 Stability analysis of a non-conventional breakwater for wave energy conversion.
 Coastal Engineering 145, 36–52.
- DREW, B., PLUMMER, A. R. & SAHINKAYA, M. N. 2009 A review of wave energy converter technology. Proceedings of the Institution of Mechanical Engineers Part A-Journal of Power and Energy 223 (A8), 887–902.
- ELHANAFI, A., FLEMING, A., MACFARLANE, G. & LEONG, Z. 2016 Numerical energy balance
 analysis for an onshore oscillating water column—wave energy converter. Energy 116, 539–
 557.
- ELHANAFI, A., FLEMING, A., MACFARLANE, G. & LEONG, Z. 2017 Underwater geometrical impact on the hydrodynamic performance of an offshore oscillating water column—wave energy converter. Renewable Energy 105, 209–231.
- EVANS, D. V. 1980 Some analytic results for two-and three-dimensional wave-energy absorbers.
 London: Academic Press.
- EVANS, D. V. 1988 The maximum efficiency of wave-energy devices near coast lines. Applied
 Ocean Research 10 (3), 162 164.
- EVANS, D. V. & PORTER, R. 1995 Hydrodynamic characteristics of an oscillating water column
 device. Applied Ocean Research 17 (3), 155–164.
- Falcão, A. F. de O. & Henriques, J. C. C. 2016 Oscillating-water-column wave energy converters and air turbines: A review. *Renewable Energy* 85, 1391–1424.
- FALCÃO, A. F. DE O., HENRIQUES, J. C. C. & GATO, L. M. C. 2016 Air turbine optimization for a bottom-standing oscillating-water-column wave energy converter. *Journal of Ocean Engineering and Marine Energy* 2 (4), 459–472.
- Falnes, J. 1980 Radiation impedance matrix and optimum power absorption for interacting oscillators in surface waves. *Applied Ocean Research* 2 (2), 75–80.
- Falnes, J. 2002 Ocean waves and oscillating systems: linear interactions including wave-energy extraction. Cambridge university press.
- HE, F., Huang, Z. & Law, A. W. K. 2012 Hydrodynamic performance of a rectangular floating breakwater with and without pneumatic chambers: An experimental study. Ocean Engineering 51, 16–27.
- HE, F., LENG, J. & ZHAO, X. 2017 An experimental investigation into the wave power extraction
 of a floating box-type breakwater with dual pneumatic chambers. Applied Ocean Research
 67, 21–30.
- HE, F., Zhang, H., Zhao, J., Zheng, S. & Iglesias, G. 2019 Hydrodynamic performance of a pile-supported OWC breakwater: An analytical study. Applied Ocean Research 88, 326 340.
- HEATH, T. V. 2012 A review of oscillating water columns. Philosophical Transactions of the Royal Society A: Mathematical, Physical and Engineering Sciences 370 (1959), 235–245.
 - Henriques, J. C. C., Gato, L. M. C., Falcão, A. F. de O., Robles, E. & Faÿ, F. X. 2016 Latching control of a floating oscillating-water-column wave energy converter. *Renewable Energy* 90, 229–241.
- 793 HERAS-SAIZARBITORIA, I., ZAMANILLO, I. & LASKURAIN, I. 2013 Social acceptance of ocean

wave energy: A case study of an OWC shoreline plant. Renewable and Sustainable Energy 794 Reviews 27, 515–524. 795

796

797 798

804

806

- HOWE, D. & NADER, J. R. 2017 OWC WEC integrated within a breakwater versus isolated: Experimental and numerical theoretical study. International Journal of Marine Energy **20**, 165–182.
- Konispoliatis, D. N. & Mavrakos, S. A. 2016 Hydrodynamic analysis of an array of 799 interacting free-floating oscillating water column (OWC's) devices. Ocean Engineering 800 **111**, 179–197. 801
- Kurniawan, A., Chaplin, J. R., Greaves, D. M. & Hann, M. 2017 Wave energy absorption 802 by a floating air bag. Journal of Fluid Mechanics 812, 294–320. 803
- LÓPEZ, I., CARBALLO, R., TAVEIRA-PINTO, F. & IGLESIAS, G. 2019 Sensitivity of owc performance to air compressibility. Renewable Energy in press. 805
 - LÓPEZ, I. & IGLESIAS, G. 2014 Efficiency of OWC wave energy converters: A virtual laboratory. Applied Ocean Research 44, 63–70.
- LÓPEZ, I., PEREIRAS, B., CASTRO, F. & IGLESIAS, G. 2014 Optimisation of turbine-induced 808 damping for an OWC wave energy converter using a RANS-VOF numerical model. 809 810 Applied Energy 127, 105–114.
- LÓPEZ, I., PEREIRAS, B., CASTRO, F. & IGLESIAS, G. 2016 Holistic performance analysis 811 and turbine-induced damping for an OWC wave energy converter. Renewable Energy 85, 812 1155-1163.
- LOVAS, S., MEI, C. C. & LIU, Y. 2010 Oscillating water column at a coastal corner for wave 814 815 power extraction. Applied Ocean Research 32 (3), 267–283.
- Maniar, H. D. & Newman, J. N. 1997 Wave diffraction by a long array of cylinders. Journal 816 of Fluid Mechanics **339**, 309330. 817
- Martins-Rivas, H. & Mei, C. C. 2009a Wave power extraction from an oscillating water 818 column along a straight coast. Ocean Engineering 36 (6-7), 426–433. 819
- Martins-Rivas, H. & Mei, C. C. 2009b Wave power extraction from an oscillating water 820 column at the tip of a breakwater. Journal of Fluid Mechanics 626, 395–414. 821
- MICHELE, S., SAMMARCO, P. & D'ERRICO, M. 2016 The optimal design of a flap gate array in 822 front of a straight vertical wall: Resonance of the natural modes and enhancement of the 823 exciting torque. Ocean Engineering 118, 152 – 164. 824
- MORRIS-THOMAS, M. T., IRVIN, R. J. & THIAGARAJAN, K. P. 2007 An investigation into the 825 hydrodynamic efficiency of an oscillating water column. Journal of Offshore Mechanics 826 and Arctic Engineering **129** (4), 273–278. 827
- Mustapa, M. A., Yaakob, O. B., Ahmed, Y. M., Rheem, C. K., Koh, K. K. & Adnan, 828 F. A. 2017 Wave energy device and breakwater integration: A review. Renewable and 829 Sustainable Energy Reviews 77, 43–58. 830
- Nader, J. R., Zhu, S. P. & Cooper, P. 2014 Hydrodynamic and energetic properties of a 831 finite array of fixed oscillating water column wave energy converters. Ocean Engineering 832 **88**, 131–148. 833
- NADER, J. R., ZHU, S. P., COOPER, P. & STAPPENBELT, B. 2012 A finite-element study of the 834 efficiency of arrays of oscillating water column wave energy converters. Ocean Engineering 835 **43**, 72–81. 836
- Nihous, G. C 2012 Wave power extraction by arbitrary arrays of non-diffracting oscillating 837 water columns. Ocean Engineering 51, 94–105. 838
- NING, D., ZHOU, Y. & ZHANG, C. 2018 Hydrodynamic modeling of a novel dual-chamber OWC 839 wave energy converter. Applied Ocean Research 78, 180–191. 840
- PAWITAN, K. A., DIMAKOPOULOS, A. S., VICINANZA, D., ALLSOP, W. & BRUCE, T. 2019 841 A loading model for an OWC caisson based upon large-scale measurements. Coastal 842 Engineering **145**, 1–20. 843
- Pereiras, B., López, I., Castro, F. & Iglesias, G. 2015 Non-dimensional analysis for 844 matching an impulse turbine to an OWC (oscillating water column) with an optimum 845 energy transfer. Energy 87, 481–489.
- Rusu, E. & Onea, F. 2018 A review of the technologies for wave energy extraction. Clean 847 Energy 2 (1), 10–19.
- SARKAR, D., RENZI, E. & DIAS, F. 2015 Effect of a straight coast on the hydrodynamics and 849 performance of the Oscillating Wave Surge Converter. Ocean Engineering 105, 25 – 32. 850

860

861

862

863

864

- SARMENTO, A. J. N. A. & FALCÃO, A. F. DE O. 1985 Wave generation by an oscillating surfacepressure and its application in wave-energy extraction. *Journal of Fluid Mechanics* **150**, 467485.
- SHENG, W. & LEWIS, A. 2018 Power takeoff optimization to maximize wave energy conversions for oscillating water column devices. *IEEE Journal of Oceanic Engineering* **43** (1), 36–47.
- Thompson, I., Linton, C. M. & Porter, R. 2008 A new approximation method for scattering by long finite arrays. The Quarterly Journal of Mechanics & Applied Mathematics 61 (3), 333–352.
 - VIVIANO, A., NATY, S., FOTI, E., BRUCE, T., ALLSOP, W. & VICINANZA, D. 2016 Large-scale experiments on the behaviour of a generalised oscillating water column under random waves. *Renewable Energy* **99**, 875 887.
 - WOLGAMOT, H. A., TAYLOR, P. H. & TAYLOR, R. EATOCK 2012 The interaction factor and directionality in wave energy arrays. *Ocean Engineering* 47, 65 73.
 - Yu, H., Zheng, S., Zhang, Y. & Iglesias, G. 2019 Wave radiation from a truncated cylinder of arbitrary cross section. *Ocean Engineering* 173, 519 530.
- ZHANG, C. & NING, D. 2019 Hydrodynamic study of a novel breakwater with parabolic openings
 for wave energy harvest. Ocean Engineering 182, 540 551.
- ZHANG, Y., ZOU, Q. P. & GREAVES, D. 2012 Airwater two-phase flow modelling of hydrodynamic performance of an oscillating water column device. Renewable Energy 41, 159 170.
- ZHAO, X., NING, D., JOHANNING, L. & TENG, B. 2018 Numerical investigation on hydrodynamic performance of a wec array integrated into a pontoon. In *The 28th International Ocean and Polar Engineering Conference*. International Society of Offshore and Polar Engineers.
- ZHENG, S. & ZHANG, Y. 2015 Wave diffraction from a truncated cylinder in front of a vertical
 wall. Ocean Engineering 104, 329 343.
- ZHENG, S. & ZHANG, Y. 2016 Wave radiation from a truncated cylinder in front of a vertical wall. Ocean Engineering 111, 602-614.
- ZHENG, S. & ZHANG, Y. 2018 Theoretical modelling of a new hybrid wave energy converter in regular waves. *Renewable Energy* **128**, 125 141.
- ZHENG, S., ZHANG, Y. & IGLESIAS, G. 2018 Wave-structure interaction in hybrid wave farms.
 Journal of Fluids and Structures 83, 386 412.
- ZHENG, S., ZHANG, Y. & IGLESIAS, G. 2019 Coast/breakwater-integrated OWC: A theoretical model. Marine Structures 66, 121 – 135.