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Characterization of linear viscoelastic, nonlinear viscoelastic and damage stages of asphalt mixtures¹

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¹ This is an Accepted Manuscript of an article published by Elsevier in *Construction and Building Materials*.
The final publication is available online via <http://doi.org/10.1016/j.conbuildmat.2016.08.039>

Abstract

It has been demonstrated that asphalt mixtures experienced linear viscoelastic stage, nonlinear viscoelastic stage and damage stage when subjected to controlled-strain repeated direct-tension (RDT) tests with increasing strain levels. However, the linear viscoelastic properties of asphalt mixtures are usually muddled up with their nonlinear viscoelastic properties. These confusions directly lead to the incorrect determination of the pseudostrains and dissipated pseudostrain energies (DPSEs) in the nonlinear viscoelastic stage and damage stage. This study investigated the material properties of fine aggregate mixture (FAM) specimens in all three stages. These three stages were differentiated and characterized in terms of the viscoelastic stress, pseudostrain and DPSE. The definitions of viscoelastic stress, reference modulus and pseudostrain were rigorously established to assure that the material properties in the linear viscoelastic stage were the reference properties and that the sole linear viscoelastic effect was eliminated when determining the pseudostrain and DPSE in the three stages. The characteristics of the DPSE in the three stages were found to be: 1) the DPSE of any loading cycle was zero in the linear viscoelastic stage; 2) in the nonlinear viscoelastic stage, the DPSE of each loading cycle remained approximately the same with the growth of the number of loading cycles, and the DPSE increased to a larger value when the strain level of the RDT test increased to a higher level; 3) in the damage stage, the DPSE of the loading cycle increased as the number of loading cycles increased. This study strictly distinguished the linear viscoelasticity from the nonlinear viscoelasticity of the asphalt mixtures, which is critical for the accurate determination of the DPSE spent in overcoming the nonlinear viscoelasticity and in developing damages, such as cracking and permanent deformation, in the asphalt mixtures.

Keywords:

Asphalt mixture; linear viscoelasticity; nonlinear viscoelasticity; viscoelastic stress; pseudostrain; dissipated pseudostrain energy.

66 1. Introduction

67 Paving asphalt mixtures are complex composite materials that may exhibit different
68 properties at different strain levels. It has been demonstrated that, when subjected to typical
69 controlled-strain repeated direct-tension (RDT) tests, an asphalt mixture experiences multiple
70 stages as the strain level increases, which include: 1) undamaged stage, consisting of the
71 linear viscoelastic stage and the nonlinear viscoelastic stage; and 2) damage stage [1][2].

72 These stages have the following characteristics:

73 (1) Undamaged stage:

- 74 a. At any specific strain level, the material properties stay constant despite of
75 the increase of the number of loading cycles;
- 76 b. The deformation of the asphalt mixture is completely recovered after
77 unloading;
- 78 c. As the strain level varies, the asphalt mixture has different properties in the
79 linear viscoelastic stage from those in the nonlinear viscoelastic stage:
 - 80 i. Linear viscoelastic stage: the material properties remain unchanged
81 if the strain level varies within this stage;
 - 82 ii. Nonlinear viscoelastic stage: the material properties change as the
83 strain level varies;

84 (2) Damage stage:

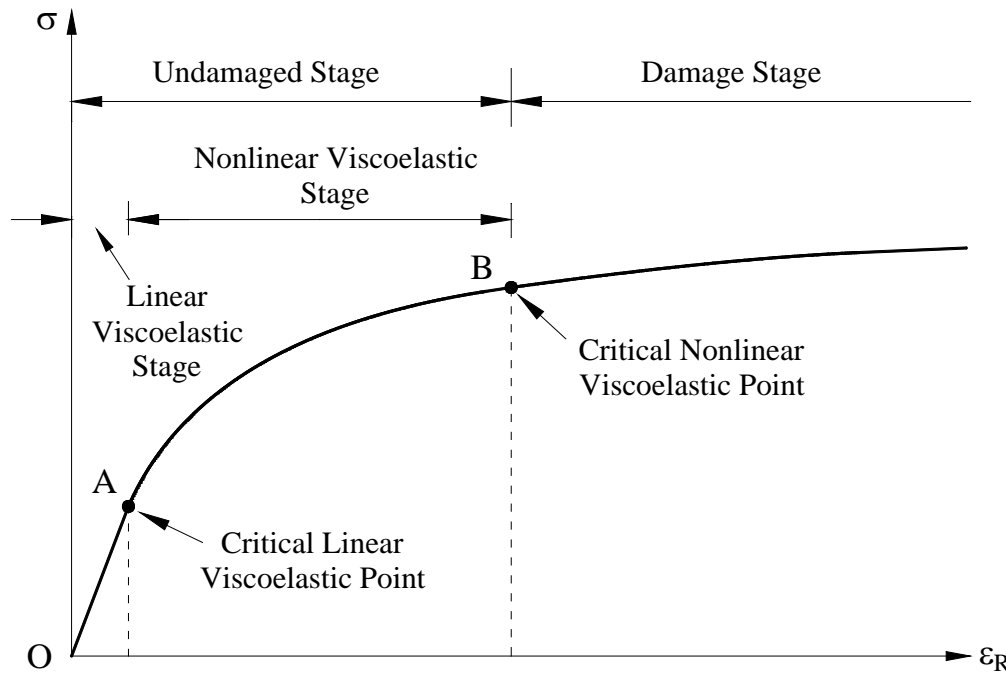
- 85 a. At any specific strain level, the material properties vary with the increase
86 of the number of loading cycles;
- 87 b. The deformation of the asphalt mixture cannot be completely recovered
88 after unloading; and
- 89 c. The material properties change as the strain level varies.

90 If using the pseudostrain defined in Equation 1 to eliminate the linear viscoelastic effect [3],
91 these stages can be illustrated via the stress-pseudostrain curve, as shown in Figure 1.

$$92 \quad \varepsilon_R = \frac{\sigma_{VE}(t)}{E_R} = \frac{\int_0^t E(t-\tau) \frac{\partial \varepsilon(\tau)}{\partial \tau} d\tau}{E_R} \quad (1)$$

93 where ε_R = pseudostrain, $\mu\varepsilon$; $\sigma_{VE}(t)$ = viscoelastic stress corresponding to the measured
94 strain history, Pa; E_R = reference modulus, MPa; t = loading time, s; τ = a dummy variable,
95 indicating any arbitrary time between 0 and t , s; $E(t)$ = relaxation modulus in the linear
96 viscoelastic stage, MPa; and $\varepsilon(t)$ = measured strain history, $\mu\varepsilon$.

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Figure 1 Stress-pseudostrain curve of an asphalt mixture in controlled-strain RDT tests
(after [1][2])

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Based on the identification of these distinct stages, testing to determine the mechanical properties of asphalt mixtures is made simpler and more precise by using pseudostrain concepts in analyzing the test data. However, these advantages are diminished or even lost if the analysis does not make clear distinctions and boundaries between these stages. In fact, the nonlinear viscoelastic properties are usually muddled up with the linear viscoelastic properties [1][2][4][5]. For example, $\sigma_{VE}(t)$ has been considered to be the same as the measured stress in the nonlinear viscoelastic stage, and E_R has been chosen to be the magnitude of the complex modulus at the critical nonlinear viscoelastic point (Point B) shown in Figure 1 when calculating the pseudostrain in the nonlinear viscoelastic stage [1][2]. These confusions directly lead to the incorrect determination of the pseudostrains and pseudostrain energies in the nonlinear viscoelastic stage and in the damage stage. Using these incorrectly determined results could hardly make accurate prediction of the development of the damages in the asphalt mixture, such as the fatigue cracking and permanent deformation, which are driven by the corresponding dissipated pseudostrain energies (DPSEs). As a result, there is an urgent need to rigorously determine the nonlinear viscoelastic properties of the asphalt mixture in typical controlled-strain RDT tests and to characterize the associated DPSEs in the nonlinear viscoelastic stage and damage stage.

119 To address this research need, this study employed a Dynamic Mechanical Analyzer
 120 (DMA) to perform controlled-strain RDT tests on fine aggregate mixture (FAM) specimens
 121 in order to investigate their material properties in the linear viscoelastic stage, nonlinear
 122 viscoelastic stage and damage stage. The DPSEs in these stages were also characterized for
 123 future applications to the prediction of the damage development in asphalt mixtures. The next
 124 section describes the configuration and procedure of the controlled-strain RDT tests. The
 125 subsequent section presents the determination of the asphalt mixture properties in different
 126 stages based on the test data. The following section details the differentiation and
 127 characterization of the linear viscoelastic stage, nonlinear viscoelastic stage and damage stage
 128 in terms of the viscoelastic stress, pseudostrain and DPSE. The final section summarizes the
 129 major findings of this study and briefs the authors' ongoing research on this subject.

130

131 **2. Configuration and procedure of the controlled-strain RDT tests**

132 *2.1. Specimen fabrication*

133 FAM specimens for the controlled-strain RDT tests were fabricated in the laboratory
 134 using an unmodified #70 petroleum asphalt binder (graded based on the penetration) and fine
 135 limestone aggregates passing No. 16 sieve with the opening of 1.18 mm. The gradation of the
 136 fine aggregates is listed in Table 1. The asphalt binder content was calculated to be 8.97% by
 137 weight of aggregates using the aggregate surface area method with the optimum asphalt
 138 content of the corresponding full asphalt mixture [6]–[9].

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Table 1 Gradation of the fine aggregates in FAM specimens

Sieve No.	No. 16	No. 30	No. 50	No. 200	PAN (-No. 200)
Sieve Size (mm)	1.18	0.60	0.30	0.75	<0.075
Individual Retaining (%)	0	44.23	23.46	18.85	13.46

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142 The procedure of fabricating and preparing the FAM specimens for testing was
 143 composed of five major steps as follows:

- 144 (1) Mixing and compaction: the aggregate batch was mixed with the asphalt binder at
 145 the temperature of 135°C; after being cured at 121°C for 2 hours, the asphalt
 146 mixture was compacted using the Superpave Gyrotory Compactor (SGC) into a
 147 cylindrical raw specimen 150 mm in diameter and 70 mm in height, as shown in
 148 Figure 2(a);

- 149 (2) Cutting: the upper and lower part of the raw specimen were cut off using an
150 automatic saw into a shorter specimen 40 mm in height, as presented in Figure
151 2(b);
- 152 (3) Coring: the shorter specimen were cored following the pattern illustrated using red
153 circles in Figure 2(c) to obtain cylindrical specimens to be tested, which were 12
154 mm in diameter and 40 mm in height; Figure 2(d) shows an example of the FAM
155 specimen;
- 156 (4) Gluing: each end of a FAM specimen was glued to an end platen using a 2 ton
157 epoxy with the aid of a specially designed gluing jig, as presented in Figure 2(e),
158 to assure the vertical pedestals of the two end platens were aligned; Figure 2(f)
159 presents an example of the test specimen glued to end platens; and
- 160 (5) Curing: all specimens with end platens were cured in an environmental chamber at
161 the temperature of 20°C for at least 1 hour to achieve the temperature equilibrium;
162 the specimens were therefore ready for testing.
- 163



(a) Raw specimen



(b) Shorter specimen after cutting



(c) Making cores



(d) FAM specimen



(e) Gluing jig



(f) Glued specimen for testing

Figure 2 Procedure of fabricating and preparing FAM specimens for testing

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166 2.2. Test configuration

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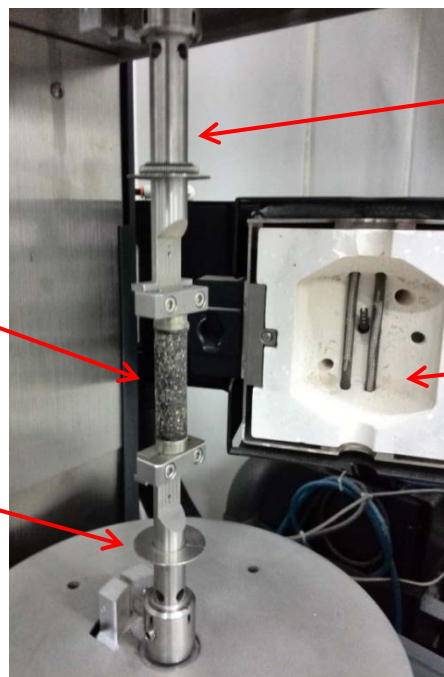
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The controlled-strain RDT tests were performed on the FAM specimens using the DMA, as shown in Figure 3. Before the test, the test specimen with end platens was mounted on the upper and lower tension fixtures that were attached to the DMA (see Figure 4). As the specimen was in place, the environmental chamber was closed, which was able to control the test temperature in a range from -60°C to 600°C . The test temperature in this study was 20°C . The test protocol was programmed in the software TRIOS designed specifically for the DMA.



Figure 3 Overview of Dynamic Mechanical Analyzer (DMA)



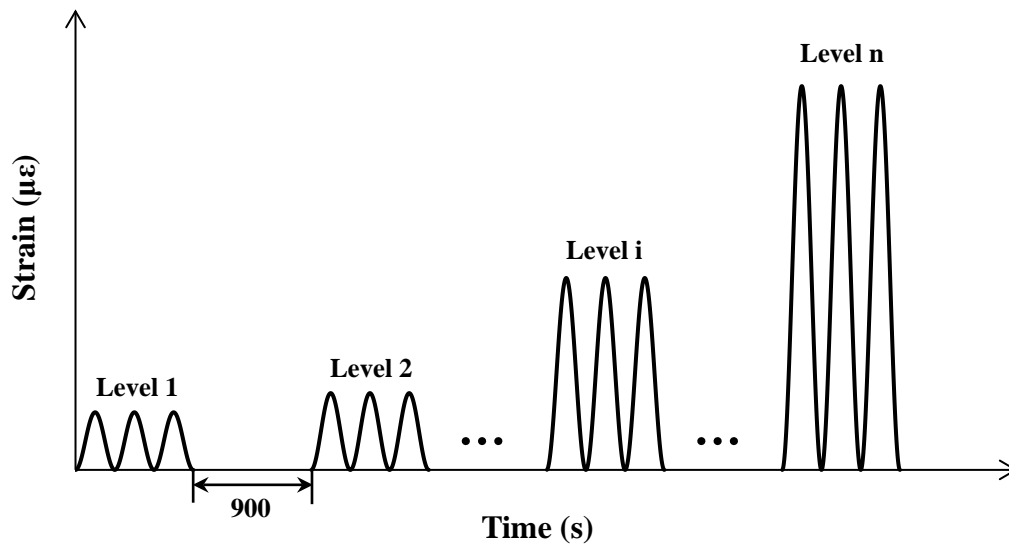
Upper tension fixture
 Environmental chamber
 FAM specimen
 Lower tension fixture

Figure 4 Configuration of DMA tests

2.3. Test procedure

The entire test procedure consisted of a sequence of consecutive controlled-strain RDT tests at different strain levels, as illustrated in Figure 5. A haversine strain curve was imposed on the specimen in each RDT test, which had 600 loading cycles with a loading frequency of 2π rad/s (1 Hz). There was a 900 s (15 min) rest period between two adjacent RDT tests in order to recover possible deformation in the previous RDT test [1][2].

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Figure 5 Controlled-strain RDT test protocol

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The test protocol was designed as follows:

191 (1) First RDT test: the initial strain level (Level 1) of the first RDT test was selected

192 to be $20 \mu\epsilon$ to assure that the specimen was within the linear viscoelastic stage;

193 (2) Subsequent RDT tests from Strain Level 2 to $(n-1)$: the axial strain level was

194 increased with an increment of $10 \mu\epsilon$ until the specimen developed into the

195 damage stage; Strain Level $(n-2)$ corresponded to the critical nonlinear

196 viscoelastic point; and

197 (3) Final RDT test: the final strain level (Level n) was chosen to be $200 \mu\epsilon$ for the

198 purpose of introducing sufficient damages to the specimen.

199

200 3. Properties of test specimens in three stages

201 The strain and stress data measured from each RDT test were firstly processed in the

202 software MATLAB using the Fourier series to filter possible noise [10]. The strain amplitude

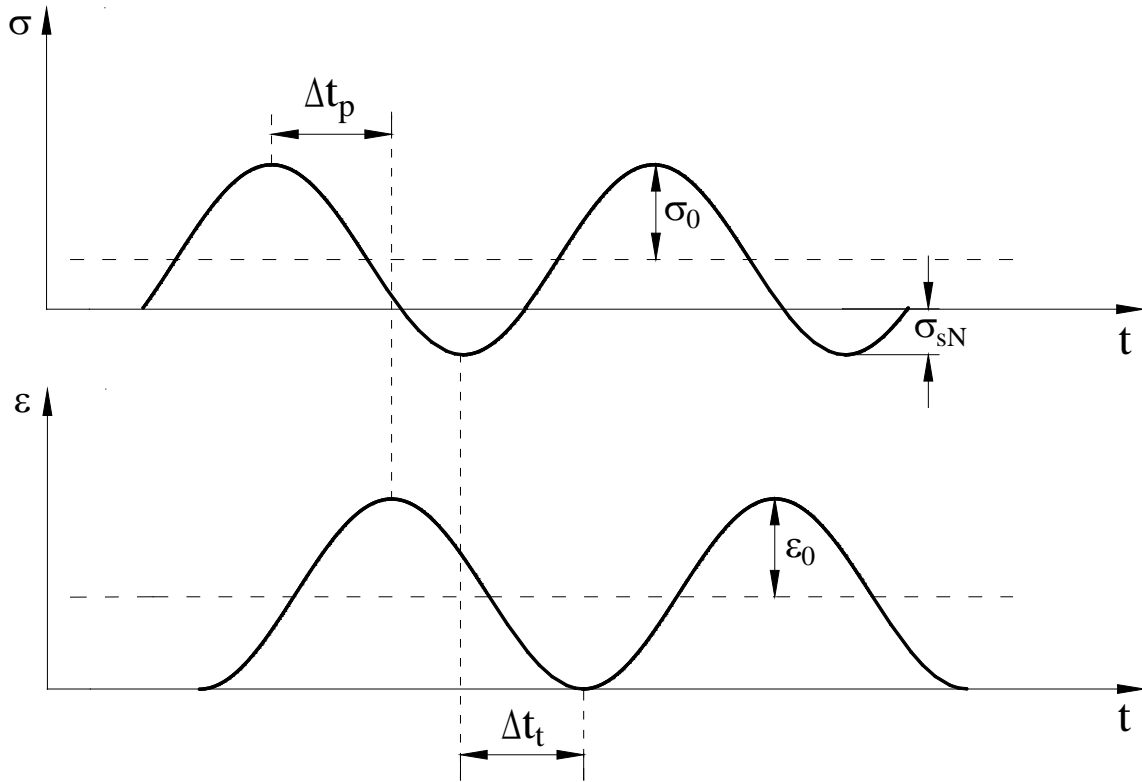
203 (ϵ_0) and stress amplitude (σ_0) of every loading cycle were therefore determined based on the

204 peaks and troughs of the strain wave and stress wave, respectively, as illustrated in Figure 6.

205 The magnitude of the complex modulus, $|E^*|$, of every cycle was then calculated to be:

$$206 \quad |E^*| = \frac{\sigma_0}{\epsilon_0} \quad (2)$$

207

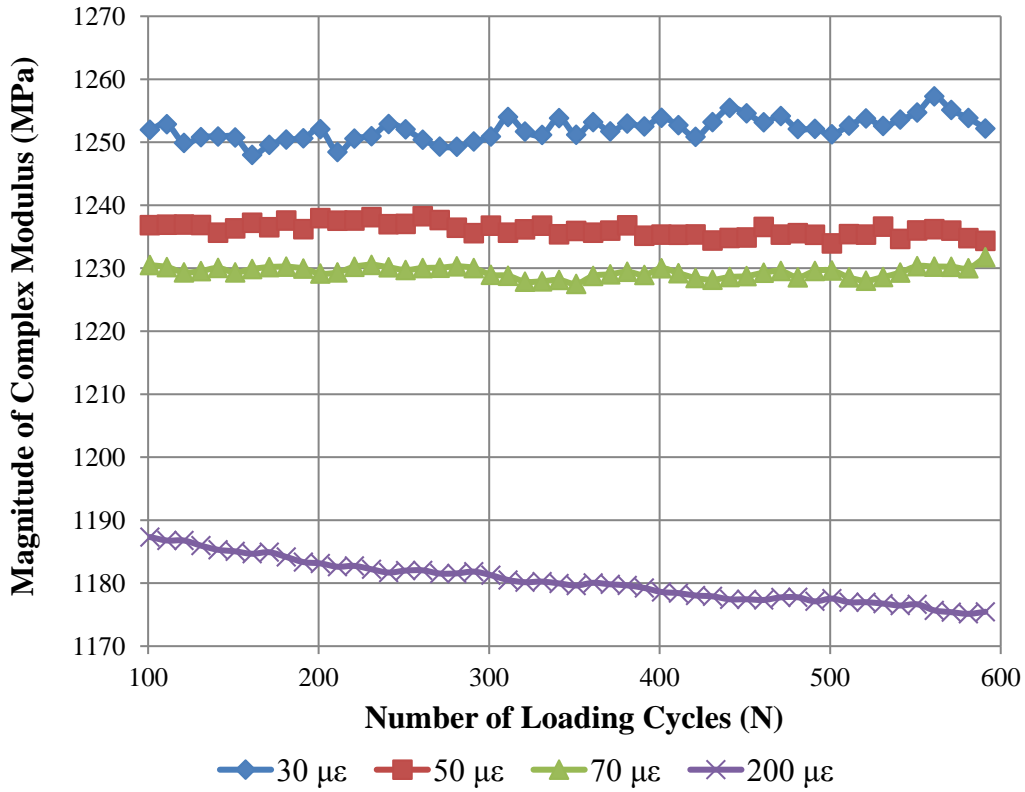


208
209 **Figure 6 Stress and strain waves**
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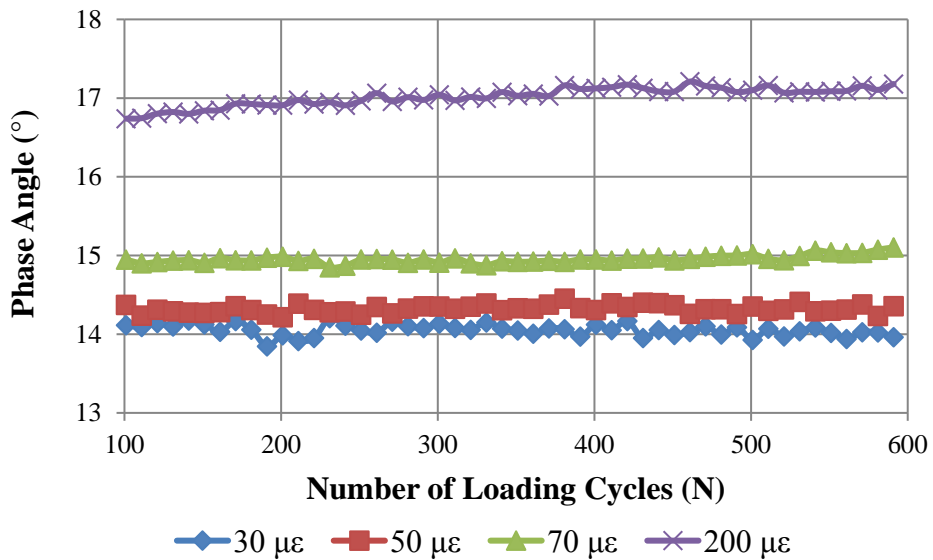
211 To determine the phase angle of the complex modulus, φ , the time lag between the
212 peaks and the time lag between the troughs of the strain and stress waves in the same loading
213 cycle were identified to be Δt_p and Δt_t , respectively, as shown in Figure 6. It was found that
214 Δt_p and Δt_t were not exactly the same in most loading cycles. Therefore, the average value
215 of Δt_p and Δt_t was used to compute the phase angle of the complex modulus in the
216 corresponding loading cycle as shown in Equation 3:

$$217 \quad \varphi = \frac{\Delta t_p + \Delta t_t}{2} \cdot \omega \quad (3)$$

218 where ω = loading frequency, 2π rad/s. Examples of the determined $|E^*|$ and φ at strain
219 levels of $30 \mu\epsilon$, $50 \mu\epsilon$, $70 \mu\epsilon$ and $200 \mu\epsilon$ are presented in Figure 7, in which φ is converted
220 into degrees for the convenience of visual comparison.
221



(a) Magnitude of complex modulus



(b) Phase angle of complex modulus

Figure 7 Examples of determined magnitudes and phase angles of complex moduli

The above data analysis was applied to each RDT test starting from Strain Level 1 that was $20 \mu\epsilon$. The $|E^*|$ and φ of every loading cycle in each RDT test were therefore

231 determined. According to their characteristics, as detailed in Introduction, the linear
 232 viscoelastic stage, nonlinear viscoelastic stage and damage stage were identified for the
 233 asphalt mixture specimens tested in this study. It was found that $30 \mu\epsilon$ corresponded to the
 234 critical linear viscoelastic point (Point A in Figure 1) and $80 \mu\epsilon$ corresponded to the critical
 235 nonlinear viscoelastic point (Point B in Figure 1). Based on the identification of the three
 236 stages, the viscoelastic stress, pseudostrain and DPSE will be determined in the next section.

237

238 4. Characterization of three stages

239 Based on the measured specimen properties and the identification of the linear
 240 viscoelastic, nonlinear viscoelastic and damage stages, these three stages were further
 241 characterized and differentiated in terms of the viscoelastic stress, pseudostrain and DPSE.
 242 First of all, the pseudostrain was rigorously defined in this study to eliminate the **linear**
 243 **viscoelastic** effect only. As a result, regarding the pseudo strain definition shown in Equation
 244 1, this study considered $\sigma_{VE}(t)$ to be the linear viscoelastic stress corresponding to the
 245 measured strain history, E_R to be the magnitude of the complex modulus in the linear
 246 viscoelastic stage and $E(t)$ to be the relaxation modulus in the linear viscoelastic stage. In
 247 other words, the material properties in the **linear viscoelastic** stage were the reference
 248 properties, based on which the viscoelastic stress, pseudostrain and DPSE were calculated in
 249 all three stages as follows.

250

251 4.1. Viscoelastic stress

252 To determine the viscoelastic stress, the strain and stress waves measured in each
 253 RDT test were firstly simulated using Equations 4 and 5, respectively:

$$254 \quad \varepsilon_m(t) = \varepsilon_0 [1 - \cos(\omega t - \varphi)] = \varepsilon_0 - \varepsilon_0 \cos(\omega t - \varphi) \quad (4)$$

$$255 \quad \sigma_m(t) = \sigma_{0N} [1 - \cos(\omega t)] - \sigma_{sN} \quad (5)$$

256 where $\varepsilon_m(t)$ = measured strain, $\mu\epsilon$; t = time, s; $\sigma_m(t)$ = measured stress, Pa; σ_{0N} = stress
 257 amplitude of the N^{th} loading cycle, Pa; σ_{sN} = absolute value of the downward shift of the
 258 stress curve in the N^{th} loading cycle, Pa. Equation 4 was then re-arranged as [2]:

$$259 \quad \varepsilon_m(t) = \varepsilon_1(t) - \varepsilon_2(t) \quad (6)$$

260 where $\varepsilon_1(t) = \varepsilon_0$, which was a constant strain history, $\mu\epsilon$; and $\varepsilon_2(t) = \varepsilon_0 \cos(\omega t - \varphi)$, which
 261 was a sinusoidal strain history, $\mu\epsilon$.

262 The viscoelastic stresses corresponding to $\varepsilon_1(t)$ and $\varepsilon_2(t)$ were determined to be:

$$263 \quad \sigma_{VE1}(t) = \varepsilon_1(t) E(t) = \varepsilon_0 E(t) \quad (7)$$

$$264 \quad \sigma_{VE2}(t) = \varepsilon_0 \left| E^* \right|_{LVE} \cos(\omega t - \varphi + \varphi_{LVE}) \quad (\text{Derivation detailed in Appendix}) \quad (8)$$

265 where $\sigma_{VE1}(t)$ = viscoelastic stress corresponding to $\varepsilon_1(t)$, Pa; $E(t)$ = relaxation modulus
266 in the linear viscoelastic stage, MPa; $\sigma_{VE2}(t)$ = viscoelastic stress corresponding to $\varepsilon_2(t)$, Pa;

267 $\left| E^* \right|_{LVE}$ = magnitude of the complex modulus in the linear viscoelastic stage, MPa; φ_{LVE} =

268 phase angle of the complex modulus in the linear viscoelastic stage, rad. Therefore, the

269 viscoelastic stress corresponding to $\varepsilon_m(t)$ was the difference between $\sigma_{VE1}(t)$ and $\sigma_{VE2}(t)$:

$$270 \quad \sigma_{VE}(t) = \varepsilon_0 E(t) - \varepsilon_0 \left| E^* \right|_{LVE} \cos(\omega t - \varphi + \varphi_{LVE}) \quad (9)$$

271 According to the general formulation of $\sigma_{VE}(t)$ presented in Equation 9, specific

272 formulations of $\sigma_{VE}(t)$ were established for the linear viscoelastic, nonlinear viscoelastic and

273 damage stages, respectively:

274 (1) Linear viscoelastic stage: since φ was equal to φ_{LVE} , $\sigma_{VE}(t)$ was simplified as:

$$275 \quad \sigma_{VE}(t) = \varepsilon_0 E(t) - \varepsilon_0 \left| E^* \right|_{LVE} \cos(\omega t) \quad (10)$$

276 (2) Nonlinear viscoelastic stage: at a specific strain level where the phase angle was

277 φ_{NL} , $\sigma_{VE}(t)$ was formulated as:

$$278 \quad \sigma_{VE}(t) = \varepsilon_0 E(t) - \varepsilon_0 \left| E^* \right|_{LVE} \cos(\omega t - \varphi_{NL} + \varphi_{LVE}) \quad (11)$$

279 (3) Damage stage: the formulation of $\sigma_{VE}(t)$ in a specific loading cycle with a phase

280 angle of φ_D was developed as:

$$281 \quad \sigma_{VE}(t) = \varepsilon_0 E(t) - \varepsilon_0 \left| E^* \right|_{LVE} \cos(\omega t - \varphi_D + \varphi_{LVE}) \quad (12)$$

282 The above formulations of $\sigma_{VE}(t)$ will be used to determine the pseudostrain and DPSE in
283 the following subsections.

284

285 4.2. Pseudostrain

286 To calculate the pseudostrains in all three stages, $E(t)$ was further derived based on

287 the formulation of $\sigma_{VE}(t)$ in the viscoelastic stage. Since $\sigma_{VE}(t)$ and $\sigma_m(t)$ were exactly the

288 same in the linear viscoelastic stage, Equation 13 was firstly established for the linear
289 viscoelastic stage:

$$290 \quad \sigma_{VE}(t) = \sigma_m(t) \quad (13)$$

291 According to Equation 13, $E(t)$ was then derived based on Equations 5 and 10:

$$292 \quad E(t) = \left| E^* \right|_{LVE} - \frac{\sigma_{s,LVE}(t)}{\varepsilon_{0,LVE}} \quad (14)$$

293 where $\sigma_{s,LVE}(t)$ = absolute value of the downward shift of the stress curve in an RDT test in
294 the linear viscoelastic stage, Pa; $\varepsilon_{0,LVE}$ = strain amplitude of the same RDT test, $\mu\epsilon$. For a
295 specific loading cycle in any RDT test, both $E(t)$ and $\sigma_{s,LVE}(t)$ were considered to be
296 constants within the loading cycle. As a result, the value of $E(t)$ in the N^{th} loading cycle of a
297 specific RDT test in the linear viscoelastic stage was determined to be:

$$298 \quad E_N = \left| E^* \right|_{LVE} - \frac{\sigma_{sN,LVE}}{\varepsilon_{0,LVE}} \quad (15)$$

299 where E_N = the value of the relaxation modulus in the N^{th} loading cycle of an RDT test in the
300 linear viscoelastic stage, MPa; and $\sigma_{sN,LVE}$ = absolute value of the downward shift of the
301 stress curve in the N^{th} loading cycle of the same RDT test, Pa.

302 Based on the determination of the relaxation modulus, the pseudostrains in the three
303 stages were determined using Equation 1 with the terms defined at the beginning of this
304 section, which are detailed as follows.

305 (1) Linear viscoelastic stage

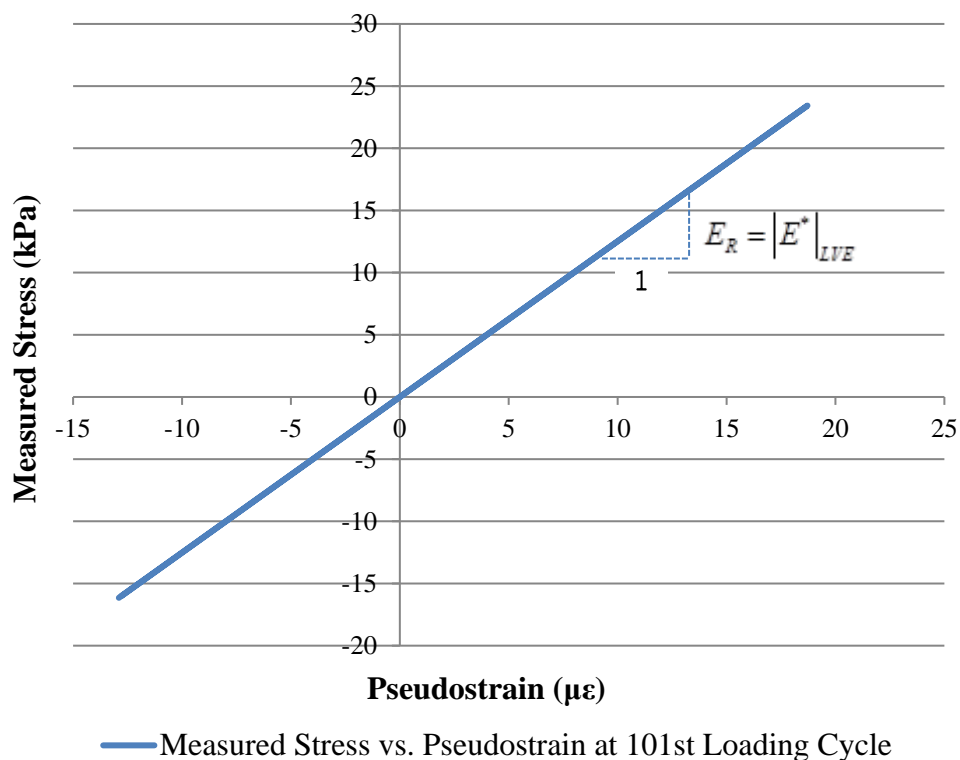
306 The pseudostrain was calculated based on Equations 1, 10 and 14:

$$307 \quad \begin{aligned} \varepsilon_R(t) &= \frac{\sigma_{VE}(t)}{E_R} \\ &= \frac{\varepsilon_0 E(t) - \varepsilon_0 \left| E^* \right|_{LVE} \cos(\omega t)}{\left| E^* \right|_{LVE}} \\ &= \frac{\varepsilon_0 \left(\left| E^* \right|_{LVE} - \frac{\sigma_{s,LVE}(t)}{\varepsilon_{0,LVE}} \right) - \varepsilon_0 \left| E^* \right|_{LVE} \cos(\omega t)}{\left| E^* \right|_{LVE}} \end{aligned} \quad (16)$$

308 Since $\varepsilon_0 = \varepsilon_{0,LVE}$ for a specific RDT test in the linear viscoelastic stage, Equation 17 was then
309 established for the N^{th} loading cycle of the RDT test:

$$\varepsilon_{RN}(t) = \varepsilon_0 [1 - \cos(\omega t)] - \frac{\sigma_{SN,LVE}}{|E^*|_{LVE}} \quad (17)$$

311 Comparing Equations 5 to 17 showed that $\varepsilon_R(t)$ was in phase with $\sigma_m(t)$. As a
 312 result, if plotting the measured stress versus pseudostrain, it became a straight line instead of
 313 a hysteresis loop, and this straight line passed through the origin. Figure 8 presents an
 314 example of the $\sigma_m(t)$ vs. $\varepsilon_R(t)$ graph of the 101st loading cycle of the RDT test with a strain
 315 level of 30 $\mu\varepsilon$. This graph demonstrated that the entire linear viscoelastic effect was
 316 successfully removed in the linear viscoelastic stage, which validated the pseudostrain
 317 formulation established in this study.
 318



319 **Figure 8 Measured stress vs. pseudostrain in the linear viscoelastic stage**
 320 **(Strain level = 30 $\mu\varepsilon$)**
 321
 322

323 (2) Nonlinear viscoelastic stage

324 The pseudostrain in the nonlinear viscoelastic stage was determined based on
 325 Equations 1, 11 and 14:

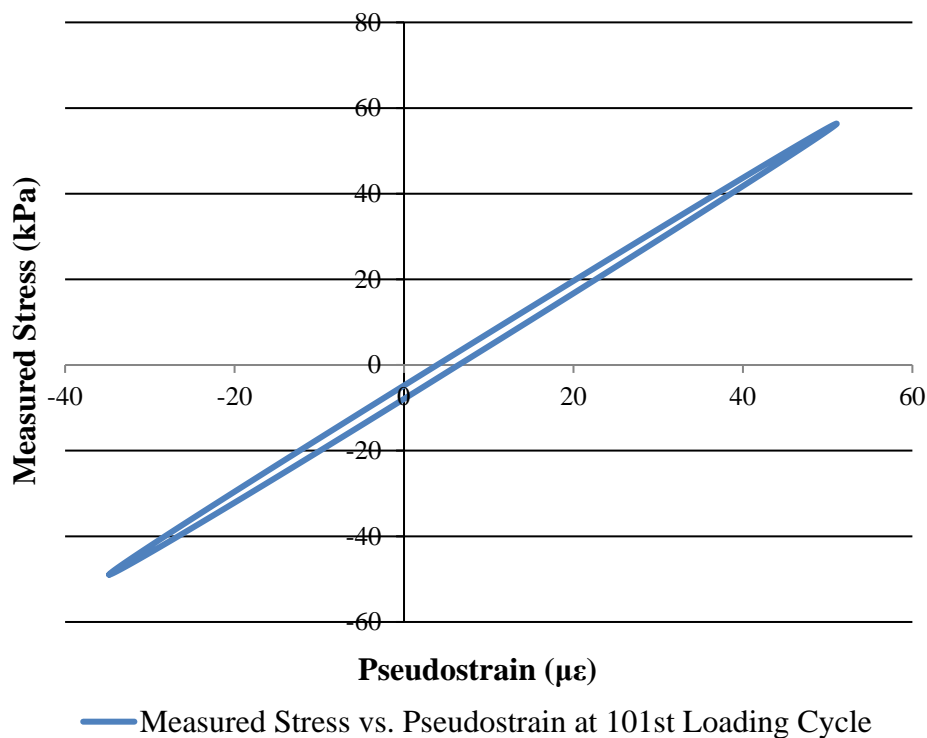
$$\begin{aligned}
\varepsilon_R(t) &= \frac{\sigma_{VE}(t)}{E_R} \\
&= \frac{\varepsilon_0 E(t) - \varepsilon_0 |E^*|_{LVE} \cos(\omega t - \varphi_{NL} + \varphi_{LVE})}{|E^*|_{LVE}} \\
326 \quad &= \frac{\varepsilon_0 \left(|E^*|_{LVE} - \frac{\sigma_{s,LVE}(t)}{\varepsilon_{0,LVE}} \right) - \varepsilon_0 |E^*|_{LVE} \cos(\omega t - \varphi_{NL} + \varphi_{LVE})}{|E^*|_{LVE}} \quad (18) \\
&= \varepsilon_0 \left[1 - \cos(\omega t - \varphi_{NL} + \varphi_{LVE}) \right] - \frac{\varepsilon_0}{\varepsilon_{0,LVE}} \cdot \frac{\sigma_{s,LVE}(t)}{|E^*|_{LVE}}
\end{aligned}$$

327 The pseudostrain formulation for the N^{th} loading cycle of an RDT test in the nonlinear
328 viscoelastic stage was then derived as:

$$329 \quad \varepsilon_{RN}(t) = \varepsilon_0 \left[1 - \cos(\omega t - \varphi_{NL} + \varphi_{LVE}) \right] - \frac{\varepsilon_0}{\varepsilon_{0,LVE}} \cdot \frac{\sigma_{sN,LVE}}{|E^*|_{LVE}} \quad (19)$$

330 Comparing Equations 5 to 19 indicated that $\varepsilon_R(t)$ was no longer in phase with $\sigma_m(t)$.
331 The phase angle between $\varepsilon_R(t)$ and $\sigma_m(t)$ was $(\varphi_{NL} - \varphi_{LVE})$, which was larger than zero
332 since $\varphi_{NL} > \varphi_{LVE}$. In a specific RDT test in the nonlinear viscoelastic stage, $(\varphi_{NL} - \varphi_{LVE})$
333 stayed unchanged as the number of loading cycles increased because of the characteristics of
334 this stage as stated in previous sections. The $\sigma_m(t)$ vs. $\varepsilon_R(t)$ graph of any loading cycle
335 exhibited an ellipse-shaped hysteresis loop, whose center was not located at the origin. The
336 area of this ellipse was the DPSE spent overcoming the sole effect of the nonlinear
337 viscoelasticity since the entire linear viscoelastic effect was eliminated already with the aid of
338 the pseudostrain formulation. Figure 9 presents an example of the $\sigma_m(t)$ vs. $\varepsilon_R(t)$ graph at
339 the 101st loading cycle of the RDT test with a strain level of $80 \mu\varepsilon$ in the nonlinear
340 viscoelastic stage.

341



342

343

Figure 9 Measured stress vs. pseudostrain in the nonlinear viscoelastic stage

344

(Strain level = 80 με)

345

(3) Damage stage

346

The pseudostrain in the damage stage was formulated based on Equations 1, 12 and

348 14:

$$\begin{aligned}
 \varepsilon_R(t) &= \frac{\sigma_{VE}(t)}{E_R} \\
 &= \frac{\varepsilon_0 E(t) - \varepsilon_0 |E^*|_{LVE} \cos(\omega t - \varphi_D + \varphi_{LVE})}{|E^*|_{LVE}} \\
 &= \frac{\varepsilon_0 \left(|E^*|_{LVE} - \frac{\sigma_{s,LVE}(t)}{\varepsilon_{0,LVE}} \right) - \varepsilon_0 |E^*|_{LVE} \cos(\omega t - \varphi_D + \varphi_{LVE})}{|E^*|_{LVE}} \\
 &= \varepsilon_0 \left[1 - \cos(\omega t - \varphi_D + \varphi_{LVE}) \right] - \frac{\varepsilon_0}{\varepsilon_{0,LVE}} \cdot \frac{\sigma_{s,LVE}(t)}{|E^*|_{LVE}}
 \end{aligned} \tag{20}$$

349

350 For the N^{th} loading cycle of an RDT test in the damage stage, the pseudostrain was

351 formulated as:

$$\varepsilon_{RN}(t) = \varepsilon_0 \left[1 - \cos(\omega t - \varphi_D + \varphi_{LVE}) \right] - \frac{\varepsilon_0}{\varepsilon_{0,LVE}} \cdot \frac{\sigma_{sN,LVE}}{|E^*|_{LVE}} \tag{21}$$

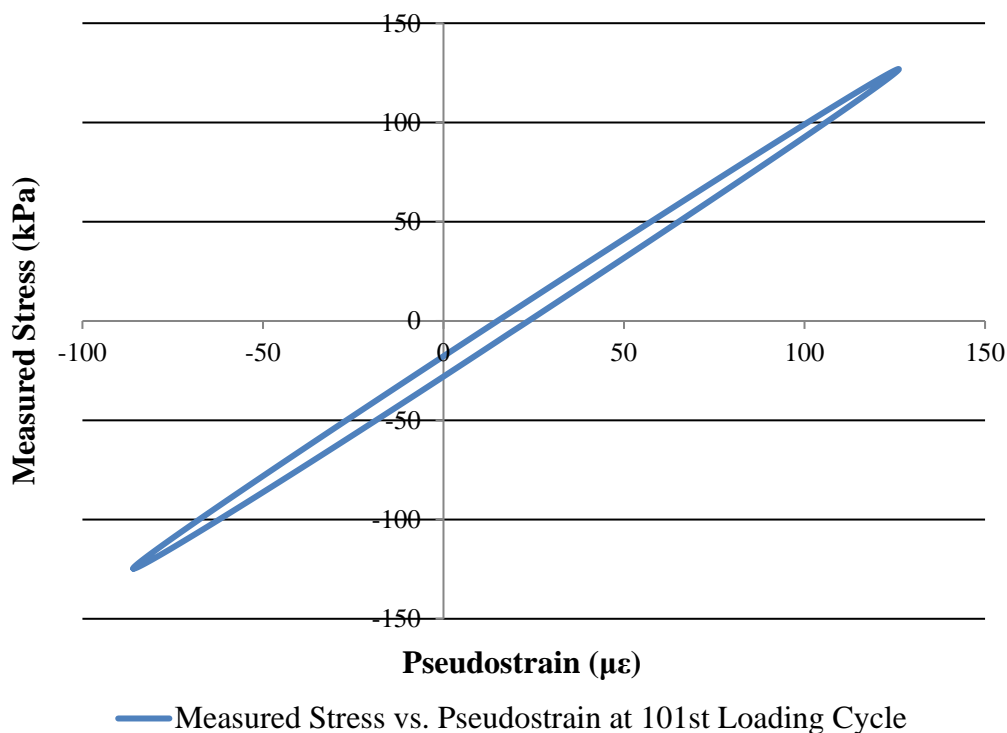
352

353 When comparing Equation 5 to Equation 21, it was obviously concluded that $\varepsilon_R(t)$
 354 was out of phase with $\sigma_m(t)$ in the damage stage. The phase angle was $(\varphi_D - \varphi_{LVE})$, which
 355 was increasing as the number of loading cycles increased in the destructive RDT test. The
 356 $\sigma_m(t)$ vs. $\varepsilon_R(t)$ hysteresis loop of any loading cycle also exhibited an ellipse, whose center
 357 was not located at the origin either. The area of this ellipse was the DPSE spent for the
 358 following purposes:

- 359 • Overcoming the nonlinear viscoelastic effect; and
- 360 • Developing damages such as cracking and permanent deformation.

361 Figure 10 shows the $\sigma_m(t)$ vs. $\varepsilon_R(t)$ graph of the 101st loading cycle of the RDT test with a
 362 strain level of 200 $\mu\varepsilon$ in the damage stage.

363



364

365 **Figure 10 Measured stress vs. pseudostrain in the damage stage**

366

(Strain level = 200 $\mu\varepsilon$)

367

368 4.3. Dissipated pseudostrain energy (DPSE)

369 As previously explained, the value of the DPSE was the area of the $\sigma_m(t)$ vs. $\varepsilon_R(t)$
 370 hysteresis loop. The mathematical formulation of the DPSE was shown in Equation 22 [1][2]
 371 [11][12]:

$$DPSE = \int_{t_1}^{t_2} \sigma_m(t) \frac{\partial \varepsilon_R(t)}{\partial t} dt \quad (22)$$

Based on Equation 22, the DPSEs of representative loading cycles in the linear viscoelastic, nonlinear viscoelastic and damage stages were determined, respectively.

(1) Linear viscoelastic stage

Since the $\sigma_m(t)$ vs. $\varepsilon_R(t)$ hysteresis loop was in fact a straight line, the DPSE in a loading cycle of any RDT test in the linear viscoelastic stage was equal to zero. This fact indicated that there was no DPSE spent overcoming the linear viscoelastic effect.

(2) Nonlinear viscoelastic stage

For any loading cycle in the nonlinear viscoelastic stage, the terms of the integrand in Equation 22, $\sigma_m(t)$ and $\varepsilon_R(t)$, were presented in Equations 5 and 19, respectively.

Therefore, the DPSE of a loading cycle in this stage was calculated to be:

$$DPSE = \int_{t_0}^{t_0 + \frac{2\pi}{\omega}} \left\{ \sigma_{0N} [1 - \cos(\omega t)] - \sigma_{sN} \right\} \frac{\partial \left\{ \varepsilon_0 [1 - \cos(\omega t - \varphi_{NL} + \varphi_{LVE})] - \frac{\varepsilon_0}{\varepsilon_{0,LVE}} \cdot \frac{\sigma_{sN,LVE}}{|E^*|_{LVE}} \right\}}{\partial t} dt \quad (23)$$

Since $\omega = 2\pi$ rad/s, Equation 23 was simplified to be:

$$DPSE = \pi \sigma_{0N} \varepsilon_0 \sin(\varphi_{NL} - \varphi_{LVE}) \quad (24)$$

(3) Damage stage

When calculating the DPSE in a complete loading cycle in the damage stage, the formulations of $\sigma_m(t)$ and $\varepsilon_R(t)$ in the integrand in Equation 22 were presented in Equations 5 and 21, respectively. Consequently, the DPSE in a loading cycle in the damage stage was formulated as:

$$DPSE = \int_{t_0}^{t_0 + \frac{2\pi}{\omega}} \left\{ \sigma_{0N} [1 - \cos(\omega t)] - \sigma_{sN} \right\} \frac{\partial \left\{ \varepsilon_0 [1 - \cos(\omega t - \varphi_D + \varphi_{LVE})] - \frac{\varepsilon_0}{\varepsilon_{0,LVE}} \cdot \frac{\sigma_{sN,LVE}}{|E^*|_{LVE}} \right\}}{\partial t} dt \quad (25)$$

The definite integral was calculated to be:

$$DPSE = \pi \sigma_{0N} \varepsilon_0 \sin(\varphi_D - \varphi_{LVE}) \quad (26)$$

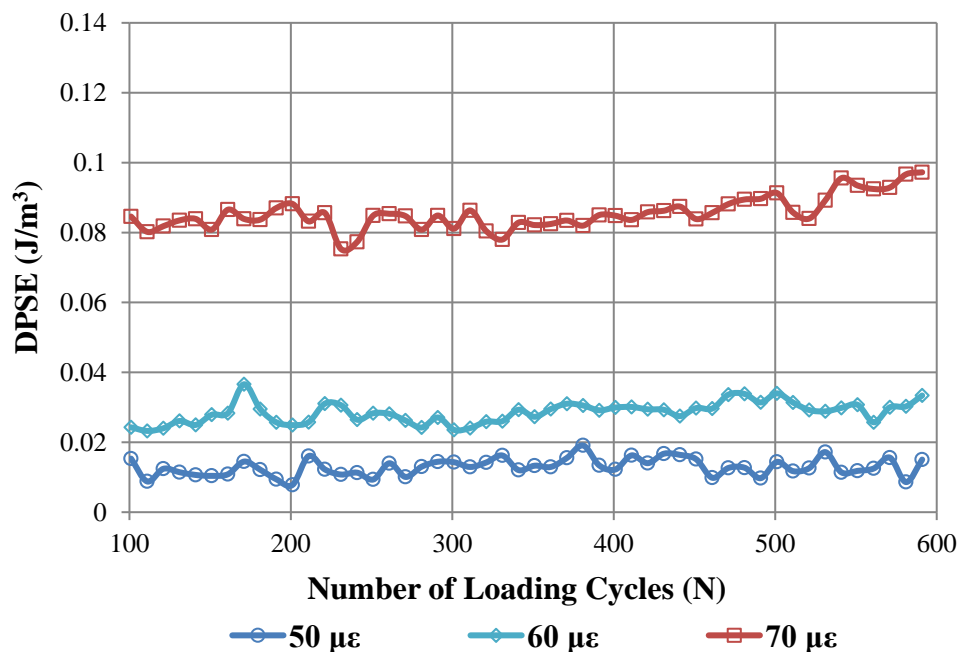
Using Equations 24 and 26, the DPSE of every loading cycle was determined for each RDT test in the nonlinear viscoelastic stage and the damage stage. The determined DPSEs of selected loading cycles in the RDT tests with different strain levels in the nonlinear

399 viscoelastic stage are presented in Figure 11, which demonstrates the following
 400 characteristics:

- 401 (1) The DPSEs of all loading cycles stayed approximately constant as the number of
 402 loading cycles increased; and
 403 (2) When the strain level increased to a higher level within this stage, the DPSE of the
 404 loading cycle increased to a larger value.

405 Figure 12 exhibits the determined DPSEs of loading cycles in the RDT test with a
 406 strain level of $200 \mu\epsilon$, which is in the damage stage. It is clearly illustrated that the DPSE had
 407 sustained growth while the number of loading cycles was increasing. This fact indicated that,
 408 with the increasing number of loading cycles, an increasing amount of DPSE accumulated to
 409 drive the development of damages such as cracking and permanent deformation in the asphalt
 410 mixture specimen.

411



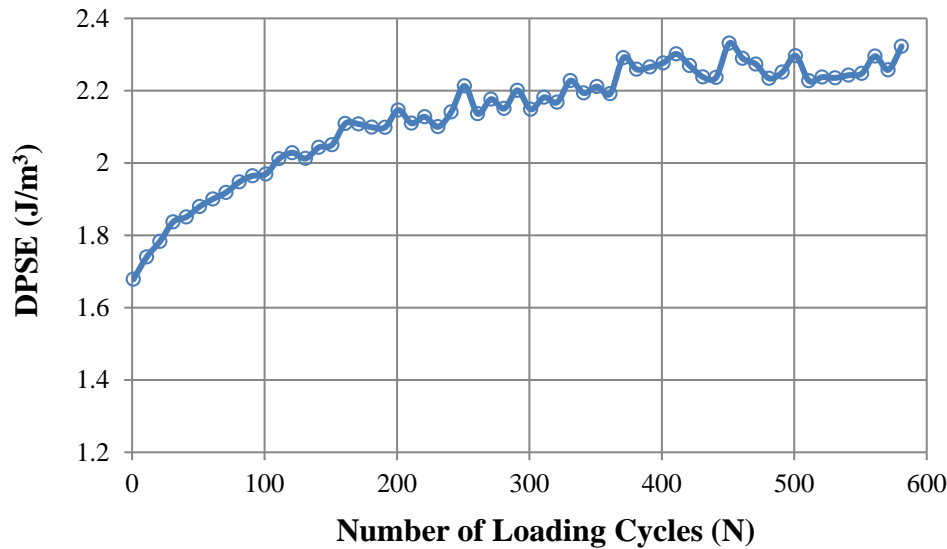
412

413 **Figure 11 DPSE of loading cycles in RDT tests in nonlinear viscoelastic stage**

414

(Strain level = 50, 60, 70 $\mu\epsilon$)

415



416
 417 **Figure 12 DPSE of loading cycles in an RDT test in damage stage**
 418 **(Strain level = 200 $\mu\epsilon$)**
 419

420 5. Conclusions

421 This study investigated the material properties of FAM specimens in the linear
 422 viscoelastic stage, nonlinear viscoelastic stage and damage stage. These three stages were
 423 differentiated and characterized in terms of the viscoelastic stress, pseudostrain and DPSE
 424 based on the measurements of controlled-strain RDT tests at a variety of strain levels, which
 425 were performed using the DMA. The definitions of viscoelastic stress, reference modulus and
 426 pseudostrain were rigorously established in the analysis in order to assure that the material
 427 properties in the linear viscoelastic stage were the reference properties. As a result, only the
 428 linear viscoelastic effect was eliminated when determining the pseudostrain and DPSE in the
 429 three stages. With the successful elimination of the sole linear viscoelastic effect, the
 430 measured stress versus pseudostrain in any loading cycle exhibited a straight line passing
 431 through the origin in the linear viscoelastic stage but an ellipse-shaped hysteresis loop, whose
 432 center was not located at the origin, in both nonlinear viscoelastic stage and damage stage.
 433 The area within the hysteresis loop was the DPSE, which was used for different purposes in
 434 the nonlinear viscoelastic stage and damage stage:

- 435 (1) Nonlinear viscoelastic stage: the DPSE was spent in overcoming the nonlinear
 436 viscoelastic effect only; and
 437 (2) Damage stage: the DPSE was spent in overcoming the nonlinear viscoelastic
 438 effect and in developing damages such as cracking and permanent deformation in
 439 the asphalt mixture.

440 Based on the formulations of the pseudostrain and DPSE, the DPSEs of all loading
441 cycles were determined for every RDT test at each strain level. The following characteristics
442 of the DPSEs were observed in the three stages:

- 443 (1) Linear viscoelastic stage: the DPSE of any loading cycle was zero;
- 444 (2) Nonlinear viscoelastic stage:
- 445 a. As the number of loading cycles increased, the DPSE of each loading
446 cycle remained approximately the same, which indicated that the same
447 amount of energy was spent overcoming the nonlinear viscoelasticity in
448 every cycle;
- 449 b. When the strain level of the RDT test increased to a higher level within the
450 nonlinear viscoelastic stage, the DPSE of every loading cycle increased to
451 a larger value and stayed unchanged as the number of loading cycles
452 increased; this fact indicated that more energy was spent overcoming the
453 larger nonlinear viscoelasticity at a higher strain level.
- 454 (3) Damage stage: with the growth of the number of loading cycles, the DPSE of the
455 loading cycle was increasing, which indicated that a larger amount of DPSE was
456 spent developing damages including cracking and permanent deformation in the
457 asphalt mixtures.

458 The findings of this study are capable of strictly differentiating the linear
459 viscoelasticity from the nonlinear viscoelasticity of asphalt mixtures. Therefore, the
460 pseudostrains and the DPSEs of the nonlinear viscoelastic stage and damage stage can be
461 rigorously determined while eliminating the sole linear viscoelastic effect of the asphalt
462 mixtures. This is critical for the accurate determination of the DPSE spent in overcoming the
463 nonlinear viscoelasticity and in developing damages in the asphalt mixtures. The test and
464 analysis methods developed in this study provide clarity, simplicity and accuracy to the
465 characterization of material properties that are used in the design and construction of asphalt
466 pavements. Based on the definitions and formulations established in this study, the DPSE for
467 overcoming the nonlinear viscoelasticity is further distinguished from the DPSE for driving
468 the damage development in an ongoing investigation for the purpose of establishing energy-
469 based models for predicting damage evolution in asphalt mixtures.

470

471 **Acknowledgements**

472 The authors acknowledge the financial support of the “973 Program” of the Ministry of
473 Science and Technology of China (Project No. 2015CB060100) and the Research Fund for

474 the Doctoral Program of Higher Education of China (Project No. 20120143110004). Special
 475 thanks are to the 1,000-Youth Elite Program of China for the start-up funds for purchasing
 476 the laboratory equipment that is crucial to this research.

477

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521

522 Appendix: Derivation of Equation 8

523 For a sinusoidal strain history $\varepsilon_2(t) = \varepsilon_0 \cos(\omega t - \varphi_m)$, the corresponding viscoelastic
 524 stress $\sigma_{VE2}(t)$ is derived as follows.

$$\begin{aligned}
 \sigma_{VE2}(t) &= \int_0^t E(t-\tau) \frac{\partial \varepsilon_2(\tau)}{\partial \tau} d\tau \\
 525 \quad &= \int_0^t E(t-\tau) \frac{\partial \varepsilon_0 \cos(\omega \tau - \varphi)}{\partial \tau} d\tau \quad (A.1) \\
 &= -\varepsilon_0 \omega \int_0^t E(t-\tau) \sin(\omega \tau - \varphi) d\tau
 \end{aligned}$$

526 Let $\xi = t - \tau$, then $\tau = t - \xi$, and when $\tau \in [0, t]$, $\xi \in [t, 0]$. Equation A.1 is then re-arranged
 527 as:

$$\begin{aligned}
 \sigma_{VE2}(t) &= -\varepsilon_0 \omega \int_0^t E(\xi) \sin[\omega(t-\xi) - \varphi] d(t-\xi) \\
 &= \varepsilon_0 \omega \int_t^0 E(\xi) \sin[(\omega t - \varphi) - \omega \xi] d\xi \\
 528 \quad &= \varepsilon_0 \omega \int_0^t E(\xi) [\sin(\omega \xi) \cos(\omega t - \varphi) - \cos(\omega \xi) \sin(\omega t - \varphi)] d\xi \quad (A.2) \\
 &= \varepsilon_0 \omega \left[\int_0^t E(\xi) \sin(\omega \xi) d\xi \right] \cos(\omega t - \varphi) - \varepsilon_0 \omega \left[\int_0^t E(\xi) \cos(\omega \xi) d\xi \right] \sin(\omega t - \varphi)
 \end{aligned}$$

529 According to the linear viscoelastic theory [13]–[15], the storage modulus $E'(\omega)$ and
 530 the loss modulus $E''(\omega)$ of the complex modulus $E^*(\omega)$ can be expressed as follows:

$$531 \quad E'(\omega) = \omega \int_0^t E(\xi) \sin(\omega\xi) d\xi = \left| E^* \right|_{LVE} \cos \varphi_{LVE} \quad (\text{A.3})$$

$$532 \quad E''(\omega) = \omega \int_0^t E(\xi) \cos(\omega\xi) d\xi = \left| E^* \right|_{LVE} \sin \varphi_{LVE} \quad (\text{A.4})$$

533 Therefore, Equation A.2 is further formulated as:

$$\begin{aligned}
 \sigma_{VE2}(t) &= \varepsilon_0 E'(\omega) \cos(\omega t - \varphi) - \varepsilon_0 E''(\omega) \sin(\omega t - \varphi) \\
 &= \varepsilon_0 \left| E^* \right|_{LVE} \cos \varphi_{LVE} \cos(\omega t - \varphi) - \varepsilon_0 \left| E^* \right|_{LVE} \sin \varphi_{LVE} \sin(\omega t - \varphi) \\
 534 \quad &= \varepsilon_0 \left| E^* \right|_{LVE} \left[\cos \varphi_{LVE} \cos(\omega t - \varphi) - \sin \varphi_{LVE} \sin(\omega t - \varphi) \right] \\
 &= \varepsilon_0 \left| E^* \right|_{LVE} \cos(\omega t - \varphi + \varphi_{LVE})
 \end{aligned} \quad (\text{A.5})$$

535 To summarize, the viscoelastic stress $\sigma_{VE2}(t)$ corresponding to the strain history

536 $\varepsilon_2(t) = \varepsilon_0 \cos(\omega t - \varphi_m)$ is determined to be:

$$537 \quad \sigma_{VE2}(t) = \varepsilon_0 \left| E^* \right|_{LVE} \cos(\omega t - \varphi + \varphi_{LVE}) \quad (\text{A.6})$$