INFLUENCE OF STEAM CURING ON ENGINEERING AND FLUID TRANSPORT PROPERTIES OF HIGH STRENGTH GREEN CONCRETE CONTAINING PALM OIL FUEL ASH

ABDULLAH MOHSEN AHMED ZEYAD

UNIVERSITI SAINS MALAYSIA

INFLUENCE OF STEAM CURING ON ENGINEERING AND FLUID TRANSPORT PROPERTIES OF HIGH STRENGTH GREEN CONCRETE CONTAINING PALM OIL FUEL ASH

By

ABDULLAH MOHSEN AHMED ZEYAD

Thesis submitted in fulfillment of the requirements

for the degree of

Doctor of Philosophy

UNIVERSITI SAINS MALAYSIA

July 2013

PENGARUH PENGAWETAN STIM TERHADAP SIFAT-SIFAT KEJURUTERAAN DAN PENGANGKUTAN BENDALIR KONKRIT HIJAU BERKEKUATAN TINGGI MENGANDUNGI ABU KELAPA SAWIT

By

ABDULLAH MOHSEN AHMED ZEYAD

Tesis yang diserahkan untuk

memenuhi keperluan bagi ijazah

Doktor Falsafah

Julai 2013

ACKNOWLEDGEMENT

In the name of Allah al Rahman al Raheem

At the onset, Alhamdulillah, I would like to thank Allah SWT for his blessing and kindness in providing me this opportunity and strength to carry out this important research.

I wish to express my sincere appreciation and gratitude to my supervisor, **Associate Professor Dr. Megat Azmi Megat Johari** for his encouragement, friendship and his guidance and continued support in completion of this work. I would also like to thank my co-supervisor **Dr. Norazura Muhamad Bunnori**, who assisted me during research and **Associate Professor Dr. Kamar Shah Ariffin** for assisting me during my laboratory experiments in the labs of School of Materials and Mineral Resources Engineering, and also for guiding me and providing valuable information related to my research work.

I wish to express my appreciation to the staff of Structural and Material Lab, **Mr. Shahril, Mr. Fauzi** and staffs at the Concrete Laboratory, School of Civil Engineering Universiti Sains Malaysia for their co-operation during the course of this research.

I would like to thank the School of Civil Engineering, Universiti Sains Malaysia and School of Materials and Mineral Resources Engineering, Engineering Campus Universiti Sains Malaysia for providing me necessary materials, facilities and technical assistance to conduct the laboratory work.

I would like to thank the Department of Civil Engineering, University of Science and Technology, Sana'a, Yemen for their support and encouragement without which it would to have been possible to carry out this work.

Lastly I would like to thank my father, my mother, my brothers and my sisters, Also special thanks to my wife, my daughters and my son for their support, care and encouragement that helped me to finish my thesis.

My earnest thanks to each and everyone who have directly or indirectly assisted me this work.

TABEL OF CONTENTS

ACKNOWLEDGMENT	i
TABLE OF CONTENTS	ii
LIST OF TABLES	X
LIST OF FIGURES	XV
LIST OF ABBREVIATIONS	xxvii
LIST OF SYMBOLS	XXX
ABSTRAK	xxxii
ABSTRACT	xxxiv
CHAPTER 1: INTRODUCTION	
1.1. Introduction	1
1.2. Research Problem Statements	7
1.3. Aim and Objectives	8
1.4. Scope of Research	9
1.5. Layout of Thesis	11
CHAPTER 2: LITERATURE REVIEW	
2.1. Introduction	13
2.2. Materials of High Strength Green Concrete	14
2.2.1. Cement	15
2.2.2. Mineral Admixture	17
2.2.2. (a) Pozzolanic Material	18
2.2.2.(b) Pozzolanic Activity	21
2.2.2. (c) Some of the Pozzolanic Material	23

	2.2.3.	Aggregate	28
	2.2	2.3. (a) Fine Aggregate (Sand)	29
	2.2	2.3.(b) Coarse Aggregate	30
	2.2.4.	Superplasticizers	35
	2.2	2.4. (a) Superplasticizer Activity	35
	2.2	2.4. (b) Application of Superplasticizer	36
2.3.	Palm Oi	l Fuel Ash	37
	2.3.1.	Effect of Chemical Compositions	41
	2.3.2.	Effect of Physical Properties	42
	2.3.3.	Effect of POFA on Properties of Fresh Concrete	43
	2.3.4.	Effect of Quantity of POFA on Strength of Concrete	44
	2.3.5.	Effect of Fineness of POFA on Strength of Concrete	47
	2.3.6.	Effect of Chemical Compositions of POFA on Strength	
		of Concrete	51
	2.3.7.	Effect of POFA on Transport Properties of Concrete	52
2.4.	Curing o	concrete	55
	2.4.1.	Effect of Temperature and Period Curing on Properties	
		concrete	56
	2.4.2.	Steam Curing at Atmospheric Pressure	58
	2.4.3.	Effect of Steam Curing on Strength of Concrete	
		Containing Pozzolan	59
	2.4.4.	Effect of Steam Curing Periods on Strength Concrete	59
	2.4.5.	Effect of Steam Curing Temperature on Strength of	65
		Concrete	
	2.4.6.	Effect of Steam Curing on Concrete Properties	68
2.5.	Microsta	ructural Characteristics	70
2.6.	Summar	у	72

CHAPTER 3: MATERIALS AND METHODS

3.1.	Introdu	ection	75
	3.1.1.	Flow chart of experimental work	75
3.2.	Materia	als	77
	3.2.1.	Palm Oil Fuel Ash (POFA)	77
	3.2.2.	Cement	81
	3.2.3.	Aggregate	81
	3.	2.3. (a) Fine Aggregate	81
	3.	2.3. (b) Coarse Aggregate	81
	3.2.4.	Water	82
	3.2.5.	Superplasticizers	82
3.3.	Charac	terization of Raw Materials	83
	3.3.1.	Chemical Compositions	83
	3.3.2.	Morphology with Chemical Compositions	86
	3.3.3.	Differential Thermal Analysis	87
	3.3.4	Physical Properties	88
3.4.	Mix Pr	oportioning of HSC	91
3.5.	Concre	te Mixing	95
3.6.	Casting	g of Concrete Samples	96
3.7.	Curing		97
3.8.	Testing	g Methods	100
	3.8.1.	Characteristics of Fresh Concrete	100
	3.8.2.	Compressive Strength	101
	3.8.3.	Initial Surface Absorption	102
	3.8.4.	Porosity and Water Absorption	104
	3.8.5.	Gas and Water Permeability	106
	3.8	3.5. (a) Gas Permeability	107
	3.8	8.5. (b) Water Permeability	108
	3.8.6.	Rapid Chloride Permeability	110
	3.8.7.	Rapid Chloride Migration	111

	3.	8.7. (b) Chloride Migration Coefficient	113
3.9.	Summ	ary	115
CHA	APTER	4: CHARACTERISTICS OF POFA AND PROPERTIES	S OF
		NCRETE	3 01
4.1.	Introdu	action	116
4.2.	Results	s and discussion	116
	4.2.1.	Treatment of POFA	116
	4.2.2.	Effect of Treatment on the Chemical and Mineral	
		Compositions of POFA	117
	4.2.3.	Effect of Treatment on the Physical Properties of POFA	122
	4.2.4.	Effect of Grinding on the Physical Properties of POFA	132
4.3.	Ceme	ent and Aggregate Properties	133
	4.3.1.	Cement	133
	4.3.2.	Aggregate	133
4.4.	HSC	Mix Proportions	135
4.5.	Effec	t of POFA on Properties of Fresh Concrete	136
	4.5.1.	Effect of POFA on Workability Based on Slump Test	136
	4.5.2.	Influence of POFA on Workability Based on Compacting	
		Factor Test	139
	4.5.3.	Effect of POFA on Workability Retention	140
	4.5.4.	Effect of POFA on Setting Times	142
4.6.	Sumr	nary	145
~			
CHA	APTER	5: COMPRESSIVE STRENGTH AND MICROSTRUCT	TURE
5.1.	Intro	luction	147
5.2.	Resu	ts and Discussion	149

3.8.7. (a) Chloride Penetration Resistance

112

	5.2.1.	Compr	essive Strength of HSGC with Normal Curing	151
		Regime		
	5.2.	1. (a)	Compressive Strength of HSGC Containing G-	
			POFA	151
	5.2.	1. (b)	Compressive Strength of HSGC Containing U-	
			POFA	152
	5.2.2.	Compr	essive Strength of HSGC with Steam Curing	
		Regim	nes	156
	5.2.	.2. (a)	Compressive Strength of HSGC Subjected to Steam	
			Curing Regime 1	157
	5.2	2.2. (b)	Compressive Strength of HSGC Subjected to	
			Steam Curing Regime 2	158
	5.2	2.2. (c)	Compressive Strength of HSGC Subjected to	
			Steam Curing Regime 3	159
	5.2	2.2. (d)	Compressive Strength of HSGC Subjected to	
			Steam Curing Regime 4	160
	5.2	2.2. (e)	Compressive Strength of HSGC Subjected to	
			Steam Curing Regime 5	162
	5.2.3.	Effect	of Steam Curing Temperature on Compressive	
		Streng	rth .	164
	5.2.4.	Effect	of Steam Curing Period on Compressive Strength	168
	5.2.5.	Optim	um Compressive Strength of HSGC Containing U-	
		POFA		171
5.3.	Result	ts and D	Discussion	174
	5.3.1.	Therm	nogravimetric Analysis	174
	5.3	3.1. (a)	Thermogravimetric Analysis of the HSGC with	
			Normal Curing	176
	5.3	3.1. (b)	Thermogravimetric Analysis of the HSGC with	
			Steam Curing	181
	5.3.2.	X-ray	Diffraction (XRD) Analysis	185
	5.3.	2. (a) X	X-ray Diffraction Analysis of the HSGC Under	
		r	normal Curing	185

	5.3.2	2. (b) X-ray Diffraction Analysis of the HSGC with Steam	
		Curing Regime	187
	5.3.3.	Morphology of the HSGC	191
	5.3.	3. (a) Morphology of the HSGC Under Normal Curing	191
	5.3.	3. (b) Morphology of the HSGC Under Steam Curing	200
		Regime	
5.4.	Summa	ry	209
	5.4.1.	Compressive Strength	209
	5.4.2.	Microstructural Characteristics	210
СН	APTER 6:	FLUID TRANSPORT PROPERTIES	
011			
6.1.	Introdu	action	211
6.2.	Results	and Discussion	212
	6.2.1.	Initial Surface Absorption	212
	6.2.	1. (a) Effect of POFA on Initial Surface Absorption	
		Under Normal Curing Regime	215
	6.2.	1. (b) Effect of Steam Curing Regimes on Initial Surface	
		Absorption of HSGC Containing U-POFA	221
	6.2.2.	Porosity and Water Absorption	226
	6.2.	2. (a) Effect of POFA on Porosity and Water Absorption	
		Under Normal Curing Regime	229
	6.2.	2. (b) Effect of Steam Curing Regimes on Water	
		Absorption and Porosity of HSGC Containing U-	233
		POFA	
	6.2.3.	Gas Permeability	239
	6.2.	3. (a) Effect of POFA on Gas Permeability	240
	6.2.	3. (b) Effect of Steam Curing Regimes on the Coefficient	
		of Gas Permeability of the HSGC Containing U-	
		POFA	244
	6.2.4.	Water Permeability	249

	6.2.4. (a) Effect of POFA on Water Permeability Under	
	Normal Curing Regime	250
	6.2.4. (b) Effect of Steam Curing Regimes on the Coefficient	
	of Water Permeability of the HSGC Containing U-	
	POFA	253
	6.2.5. Rapid Chloride Permeability	257
	6.2.5. (a) Effect of POFA on Rapid Chloride Permeability	
	Under Normal Curing Regime	259
	6.2.5. (b) Effect of Steam Curing Regimes on RCP of the	
	HSGC Containing U-POFA	264
	6.2.6. Rapid Chloride Migration	268
	6.2.6. (a) Effect of POFA on Resistance to Chloride	
	Migration Under Normal Curing Regime	270
	6.2.6. (b) Effect of Steam Curing Regimes on Resistance to	
	Chloride Migration of the HSGC Containing U-	
	POFA	277
6.3.	Summary	280
СНА	APTER 7: CONCLUSIONS AND RECOMMENDATIONS	
7.1.	General	282
7.2.	Effect of Treatment Processes on Properties of POFA	283
7.3.	Effect of POFA on Properties of Fresh Concrete	283
7.4.	Effect of G-POFA and U-POFA on Compressive Strength Under	
	Normal Curing Regime	284
7.5.	Effect of U-POFA on Compressive Strength Under Steam Curing	
	Regimes	284
7.6.	Effect of G-POFA and U-POFA on Microstructure	285
7.7.	Effect of G-POFA and U-POFA on Fluid Transport Properties	
	Under Normal Curing Regime	286
7.8.	Effect of U-POFA on Fluid Transport Properties Under Steam	
	Curing Regimes	286

7.9.	Recommendations for Further Research	287
REF	ERRNCES	288
APPI	ENDIX A	312
APPI	ENDIX B	316
APPI	ENDIX C	324
APPI	ENDIX D	332
LIST	OF PUBLICATION	336

LIST OF TABLES

3

Physical properties of OPC and POFA

CHAPTER 1

Table 1.1

Table 1.2	Chemical compositions of OPC and POFA	4
Table 1.3	Chemical requirements of classes N, F, and C based on ASTM	
	C618	5
Table 1.4	Physical requirements of classes N, F, and C based on ASTM	
	C618	5
CHAPTER	2	
Table 2.1	Chemical compositions of cement	15
Table 2.2	Compound compositions and fineness of cement	16
Table 2.3	ASTM C618 requirements for fly ash and natural pozzolans for	
	use as mineral admixtures in Portland cement concrete	19
Table 2.4	Classification, compositions, and particle characteristics of	
	mineral admixtures for concrete	20
Table 2.5	Chemical compositions of volcanic ash and volcanic pumice	24
Table 2.6a	Chemical compositions of silica fume	25
Table 2.6b	Physical properties of silica fume	25
Table 2.7	Chemical compositions of palm oil fuel ash	28
Table 2.8	Aggregate type, strength, and shore-hardness and Los Angeles	
	abrasion	31
Table 2.9:	Compressive strength of concrete (N/mm ²) made with different	
	types of aggregate at different curing time e	31
Table 2.10	Suggested-size coarse aggregate	31
Table 2.11	Mixture proportions of concrete with different MSA	33
Table 2.12	Volume of coarse aggregate per unit volume of concrete	
	recommended by ACI 211.4R	34

Table 2.13	Mixture proportions of concrete with different volume fraction	
	of aggregate	34
Table 2.14	Chemical compositions of POFA from different sources	40
Table 2.15	Physical properties of POFA from different sources	41
Table 2.16	Classification of palm oil fuel ash depending on the fineness	42
Table 2.17	Chemical compositions of SF, FA and MK	51
Table 2.18	Compressive strength of concrete with different delay period	
	for steam curing	61
Table 2.19	Cycle of steam curing from different sources	62
Table 2.20	Strength of normal curing and steam-cured concrete specimens	66
Table 2.21	Peaks detected on TGA curve and reaction identification from	72
	different sources	
CHAPTER	3	
Table 3.1	Required average compressive strength when data are not	
	available to establish sample standard deviation	92
Table 3.2	Suggested maximum-size of coarse aggregate	93
Table 3.3	Recommended volume of coarse aggregate per unit volume of	
	concrete	93
Table 3.4	Volume of sand from concrete component materials	95
Table 3.5	Regimes of steam curing	99
Table 3.6	Chloride ion penetrability based on total charge passed	111
Table 3.7	Test conditions for the chloride penetration	112
CHAPTER	4	
Table 4.1	Percentage of unburned carbon in POFA	118
Table 4.2	Chemical compositions of POFA	118

Table 4.3	Proportions of mineral in glassy and crystalline phases	
	(SIROQUANT)	121
Table 4.4	The silica oxide in the glassy phase	121
Table 4.5	Specific surface area of OPC and POFA	122
Table 4.6	Specific gravity and median particle size of OPC and POFA	124
Table 4.7	Chemical compositions of cement	133
Table 4.8	Mineral compositions of cement	133
Table 3.9	Properties of aggregate	134
Table 4.10	Grading of fine aggregate (sand)	134
Table 4.11	Grading of coarse aggregate	134
Table 4.12	Concrete mix proportions	135
CHAPTER	5	
Table 5.1	Regimes of steam curing	149
Table 5.2	Compressive strength test results (MPa)	150
Table 5.3	Loss of weight of the HSGC with NCR	175
Table 5.4	Loss of weight of the HSGC subjected to SCR-1 (50°C,	
	16hours)	176
Table 5.5	Elements of OPC paste under NCR at the age of 3 days from	
	SEM/EDX analysis	192
Table 5.6	Elements of U-POFA20 paste under NCR at the age of 3 days	
	from SEM/EDX analysis	193
Table 5.7	Elements of U-POFA40 paste under NCR at the age of 3 days	
	from SEM/EDX analysis	194
Table 5.8	Elements of U-POFA60 paste under NCR at the age of 3 days	
	from SEM/EDX analysis	195
Table 5.9	Elements of OPC paste under NCR at the age of 180 days from	
	SEM/EDX analysis	196
Table 5.10	Elements of U-POFA20 paste under NCR at the age of 180 days	
	from SEM/EDX analysis	197

Table 5.11	Elements of U-POFA40 paste under NCR at the age of 180 days	
	from SEM/EDX analysis	198
Table 5.12	Elements of U-POFA60 paste under NCR at the age of 180 days	
	from SEM/EDX analysis	199
Table 5.13	Elements of OPC paste under SCR-1 at the age of 3 days from	
	SEM/EDX analysis	201
Table 5.14	Elements of U-POFA20 paste under SCR-1 at the age of 3 days	
	from SEM/EDX analysis	202
Table 5.15	Elements of U-POFA40 paste under SCR-1 at the age of 3 days	
	from SEM/EDX analysis	203
Table 5.16	Elements of U-POFA60 paste under SCR-1 at the age of 3 days	
	from SEM/EDX analysis	204
Table 5.17	Elements of OPC paste under SCR-1 at the age of 180 days	
	from SEM/EDX analysis	205
Table 5.18	Elements of U-POFA20 paste under SCR-1 at the age of 180	
	days from SEM/EDX analysis	206
Table 5.19	Elements of U-POFA40 paste under SCR-1 at the age of 180	
	days from SEM/EDX analysis	207
Table 5.20	Elements of U-POFA60 paste under SCR-1 at the age of 180	
	days from SEM/EDX analysis	208
CHAPTER	6	
Table 6.1	ISAT at time intervals of 10 minutes	213
Table 6.2	ISAT at time intervals of 120 minutes	214
Table 6.3	Porosity of HSGC subjected to different steam curing regimes	227
Table 6.4	Water absorption of HSGC subjected to different steam curing	
	regimes	228
Table 6.5	Gas permeability coefficient of HSGC	239
Table 6.6	Water permeability coefficient of HSGC	249
Table 6.7	Total charge passed of HSGC	258

Table 6.8	Chloride migration coefficient of HSGC	269
Table 6.9	Chloride penetration rate of HSGC	270
CHAPTER 7		
Table 7.1	Loss of weight of the HSGC with NCR	242
Table 7.2	Loss of weight of the HSGC subjected to SCR-1	242

LIST OF FIGURES

CHAPTER 2

Figure 2.1.a	Ground Rice husk-bark ash	22
Figure 2.1.b	Ground bagasse ash	22
Figure 2.1.c	Fine fly ash	23
Figure 2.1.d	Ground palm oil fuel ash	23
Figure 2.1.e	Metakaolin	23
Figure 2.1.f	Silica fume	23
Figure 2.2	Illustration of crack extending inside concrete	33
Figure 2.3	The biggest producers of palm oil fuel ash	38
Figure 2.4	Effect of ash content on compressive strength concrete at 28	
	days	45
Figure 2.5	Relationship between compressive strength and cement	
	replacement by ground POFA	46
Figure 2.6	Effect of quantity of POFA on strength of	47
Figure 2.7	Effect of fineness of ash (POFA) on compressive strength of	
	concrete	48
Figure 2.8. a	Relationship between compressive strength and fineness OP	49
Figure 2.8.b	Relationship between compressive strength and fineness MP	49
Figure 2.8.c	Relationship between compressive strength and fineness SP	50
Figure 2.9	Compressive strength of mortars containing POFA of different	
	fineness	50
Figure 2.10	Pozzolanic activity for SF, FA and MK with fixed lime over	
	time	52
Figure 2.11	Water permeability of concrete with replacement ratios at 28	
	day	53
Figure 2.12	Influence of different pozzolanic materials on rapid chloride	~ 4
	permeability of concrete	54
Figure 2.13	Influence of different pozzolanic materials on chloride	~ ~
	penetration of concrete from rapid migration test.	55

Figure 2.14	Effect of moist curing time on strength of concrete	56
Figure 2.15	Effect of curing temperature on strength concrete	57
Figure 2.16	Schematic representation of the heat treatment procedure	
	where the heating period is the same as the cooling period	60
Figure 2.17	Compressive strength with respect to treatment temperature	
	and time of treatment	64
Figures 2.18	Effects of time and temperature of steam curing at	
	atmospheric pressure on one-day compressive strength	64
Figures 2.19	Effects of time and temperature of steam curing at	
	atmospheric pressure on 28-day compressive strength	65
Figure 2.20.a	One-day compressive strength with respect to temperature	67
Figure 2.20.b	28-day compressive strength with respect to temperature	67
Figure 2.21	Effect of cementing materials and curing procedures on	
	steady-state migration results	68
Figure 2.22	Effects of curing condition and mineral admixture content on	
	sorptivity of concretes	70
Figure 2.23	Effects of curing condition and mineral admixture content on	
	chloride ion permeability of concretes	70
Figure 2.26	Porous characteristics measured by MIP	71
CHAPTER 3		
CHAI IER 3		
Figure 3.1	Flow chart for experimental work	76
Figure 3.2	Illustration of the functioning of the boiler	77
Figure 3.3	Schematic diagram of the stages of POFA treatment	78
Figure 3.4	Palm oil fuel ash in dumping ground near palm oil mill	79
Figure 3.5	The oven used for drying of POFA	79
Figure 3.6.a	The ball mill tank	80
Figure 3.6.b	The ball mill machine	80
Figure 3.7	G-POFA before treatment	81
Figure 3.8	T-POFA after treatment	81
Figure 3.9	XRF device used in chemical composition analysis	83

Figure 3.10	CHNS instrument used in the analysis of carbon content	84
Figure 3.11	BRUKER, D8 X-ray diffraction (XRD) instrument	85
Figure 3.12	Scanning electron microscope in combination with energy	
	dispersive x-ray spectroscopy (SEM/EDX) device	87
Figure 3.13	Thermal analysis apparatus	88
Figure 3.14	MASTERSIZER laser diffraction particle size analyzer	89
Figure 3.15	Instrument of nitrogen gas absorption test (BET)	90
Figure 3.16	Blaine surface area apparatus	90
Figure 3.17	Micromeritics AccuPyc 1330 helium pycnometer instrument	91
Figure 3.18	Proposed water/binder ratio depending on compressive	
	strength at 28 day relationship	94
Figure 3.19	Casting of concrete samples	97
Figure 3.20	Schematic representation of the steam curing regime (SCR-1)	98
Figure 3.21	Curing of samples by water immersion in plastic tank (normal	100
	curing)	
Figure 3.22	Curing of samples by steam in a stainless steel steam curing	
	tank (steam curing)	101
Figure 3.23	Concrete compression machine used in this study	102
Figure 3.24	Initial surface absorption (ISAT) apparatus	103
Figure 3.25	Vacuum saturation apparatus	105
Figure 3.26	Permeability apparatus	109
Figure 3.27	The rapid chloride permeability test (RCPT) Apparatus	111
Figure 3.28	The cells for the rapid chloride migration test	114
Figure 3.29	Penetration of chloride into concrete sample	115
CHAPTER 4		
Figure 4.1	Stages of POFA treatment	117
Figure 4.2	Chemical compositions (Pozzolanic material and carbon) of T-	
	POFA	119
Figure 4.3	X-ray diffraction analysis results of G-POFA and U-POFA	120
Figure 4.4	Particle size distribution of POFA and OPC	125

Figure 4.5	SEM micrograph of the POFA after sieving through 300 μm	
	sieve	127
Figure 4.6.a	EDX for the particle A of POFA after sieving through 300 μm	
	sieve	127
Figure 4.6.b	EDX for particle B of POFA after sieving through 300 μm	
	sieve	127
Figure 4.6.c	EDX for particle C of POFA after sieving through 300 μm	
	sieve	127
Figure 4.7	SEM micrograph of the G-POFA	128
Figure 4.8.a	EDX of the G-POFA	128
Figure 4.8.b	EDX of the G-POFA	128
Figure 4.8.c	EDX of the G-POFA	128
Figure 4.9	SEM micrograph of the T-POFA	129
Figure .10.a	EDX of the T-POFA	129
Figure .10.b	EDX of the T-POFA	129
Figure .10.c	EDX of the T-POFA	129
Figure 4.11	SEM micrograph of the U-POFA	130
Figure .12.a	EDX of the U-POFA	130
Figure .12.b	EDX of the U-POFA	130
Figure .12.c	EDX of the U-POFA	130
Figure 4.13	Thermogravimetric (TGA) and differential thermal analyses	
	(DTA) of POFA	131
Figure 4.14	Influence of G-POFA and U-POFA on workability based on	
	slump test	138
Figure 4.15	Influence of G-POFA and U-POFA on workability based on	
	compacting factor test	140
Figure 4.16	Influence of U-POFA on workability retention	142
Figure 4.17	Influence of G-POFA on workability retention	142
Figure 4.18	Influence of U-POFA on initial and final setting times	143
Figure 4.19	Influence of G-POFA on initial and final setting times	144

CHAPTER 5

Figure 5.1	Compressive strength of HSGC containing G-POFA	
	subjected to NCR	151
Figure 5.2	Compressive strength of HSGC containing U-POFA	
	subjected to NCR	154
Figure 5.3	Compressive strength of HSGC subjected to SCR-1	157
Figure 5.4	Compressive strength of HSGC subjected to SCR-2	159
Figure 5.5	Compressive strength of HSGC subjected to SCR-3	160
Figure 5.6	Compressive strength of HSGC subjected to SCR-4	161
Figure 5.7	Compressive strength of HSGC subjected to SCR-5	162
Figure 5.8	Compressive strength of OPCHSC exposed to different	
	steam curing temperatures (°C)	165
Figure 5.9	Compressive strength of U-POFA20 HSGC exposed to	
	different steam curing temperatures (°C)	165
Figure 5.10	Compressive strength of U-POFA40 HSGC exposed to	
	different steam curing temperatures (°C)	166
Figure 5.11	Compressive strength of U-POFA60 HSGC exposed to	
	different steam curing temperature (°C)	167
Figure 5.12	Compressive strength of OPC HSC exposed to different	
	steam curing periods (hour)	168
Figure 5.13	Compressive strength of U-POFA20 HSGC exposed to	
	different steam curing periods (hour)	169
Figure 5.14	Compressive strength of U-POFA40 HSGC exposed to	
	different steam curing periods (hour)	170
Figure 5.15	Compressive strength of U-POFA60 HSGC exposed to	
	different steam curing periods (hour)	171
Figure 5.16	Compressive strength of HSGC at the age of 1day	171
Figure 5.17	Compressive strength of HSGC at the age of 28days	172
Figure 5.18	Compressive strength of HSGC at the age of 360 days	173
Figure 5.19	Content of the gel (C-S-H and C-S-A-H) in the HSGC	
	containing the U-POFA under NCR	177

Figure 5.20	Content of Ca(OH) ₂ in the HSGC containing the U-POFA	
	under NCR	177
Figure 5.21	Thermogravimetric analysis of the pastes with NCR at the	
	age of 3 days	178
Figure 5.22	Thermogravimetric analysis of the pastes with NCR at the	
	age of 28 days	179
Figure 5.23	Thermogravimetric analysis of the pastes with NCR at the	
	age of 180 days	180
Figure 5.24	Content of the gel (C-S-H and C-S-A-H) in the HSGC	
	containing the U-POFA under SCR-1	181
Figure 5.25	Content of Ca(OH) ₂ in the HSGC containing the U-POFA	
	under SCR-1	182
Figure 5.26	Thermogravimetric analysis of the pastes with SCR-1 at the	
	age of 3 days	182
Figure 5.27	Thermogravimetric analysis of the pastes with SCR-1 at the	
	age of 28 days	184
Figure 2.28	Thermogravimetric analysis of the pastes with SCR-1 at the	
	age of 180 days	184
Figure 5.29	X-ray diffraction analysis of cement pastes at test age of 3	
	days under normal curing	186
Figure 5.30	X-ray diffraction analysis of cement pastes at test age of 28	
	days under normal curing	186
Figure 5.31	X-ray diffraction analysis of cement pastes at test age of	
	180 days under normal curing	187
Figure 5.32	X-ray diffraction analysis of cement pastes at test age of 3	
	days with steam curing SCR-1	188
Figure 5.33	X-ray diffraction analysis of cement pastes at test age of 28	
	days with steam curing SCR-1	188
Figure 5.34	X-ray diffraction analysis of cement pastes at test age of	
	180 days with steam curing SCR-1	189
Figure 5.35	SEM micrograph of OPC paste under NCR at the age of 3	
	days	192
Figure 5.35(A)	EDX of OPC paste under NCR at the age of 3 days	192

Figure 5.35(B)	EDX of OPC paste under NCR at the age of 3 days	192
Figure 5.36	SEM micrograph of U-POFA20 paste under NCR at the age	
	of 3 days	193
Figure 5.36(A)	EDX of U-POFA20 paste under NCR at the age of 3 days	193
Figure 5.36(B)	EDX of U-POFA20 paste under NCR at the age of 3 days	193
Figure 5.37	SEM micrograph of U-POFA40 paste under NCR at the age	
	of 3 days	194
Figure 5.37(A)	EDX of U-POFA40 paste under NCR at the age of 3 days	194
Figure 5.37(B)	EDX of U-POFA40 paste under NCR at the age of 3 days	194
Figure 5.38	SEM micrograph of U-POFA60 paste under NCR at the age	
	of 3 days	195
Figure 5.38(A)	EDX of U-POFA60 paste under NCR at the age of 3 days	195
Figure 5.38(B)	EDX of U-POFA60 paste under NCR at the age of 3 days	195
Figure 5.39	SEM micrograph of OPC paste under NCR at the age of 180	
	days	196
Figure 5.39(A)	EDX of OPC paste under NCR at the age of 180 days	196
Figure 5.39(B)	EDX of OPC paste under NCR at the age of 180 days	196
Figure 5.40	SEM micrograph of U-POFA20 paste under NCR at the age	
	of 180 days	197
Figure 5.40(A)	EDX of U-POFA20 paste under NCR at the age of 180 days	197
Figure 5.40(B)	EDX of U-POFA20 paste under NCR at the age of 180 days	197
Figure 5.41	SEM micrograph of U-POFA40 paste under NCR at the age	
	of 180 days	198
Figure 5.41(A)	EDX of U-POFA40 paste under NCR at the age of 180 days	198
Figure 5.41(B)	EDX of U-POFA40 paste under NCR at the age of 180 days	198
Figure 5.42	SEM micrograph of U-POFA60 paste under NCR at the age	
	of 180 days	199
Figure 5.42(A)	EDX of U-POFA60 paste under NCR at the age of 180 days	199
Figure 5.42(B)	EDX of U-POFA60 paste under NCR at the age of 180 days	199
Figure 5.43	SEM micrograph of OPC paste under SCR-1 at the age of 3	
	days	201
Figure 5.43(A)	EDX of OPC paste under SCR-1 at the age of 3 days	201
Figure 5.43(B)	EDX of OPC paste under SCR-1 at the age of 3 days	201

Figure 5.44	SEM micrograph of U-POFA20 paste under SCR-1 at the	
	age of 3 days	202
Figure 5.44(A)	EDX of U-POFA20 paste under SCR-1 at the age of 3 days	202
Figure 5.44(B)	EDX of U-POFA20 paste under SCR-1 at the age of 3 days	202
Figure 5.45	SEM micrograph of U-POFA40 paste under SCR-1 at the	
	age of 3 days	203
Figure 5.45(A)	EDX of U-POFA40 paste under SCR-1 at the age of 3 days	203
Figure 5.45(B)	EDX of U-POFA40 paste under SCR-1 at the age of 3 days	203
Figure 5.46	SEM micrograph of U-POFA60 paste under SCR-1 at the	
	age of 3 days	204
Figure 5.46(A)	EDX of U-POFA60 paste under SCR-1 at the age of 3 days	204
Figure 5.46(B)	EDX of U-POFA60 paste under SCR-1 at the age of 3 days	204
Figure 5.47	SEM micrograph of OPC paste under SCR-1 at the age of	
	180 days	205
Figure 5.47(A)	EDX of OPC paste under SCR-1 at the age of 180 days	205
Figure 5.47(B)	EDX of OPC paste under SCR-1 at the age of 180 days	205
Figure 5.48	SEM micrograph of U-POFA20 paste under SCR-1 at the	
	age of 180 days	206
Figure 5.48(A)	EDX of U-POFA20 paste under SCR-1 at the age of 180	
	days	206
Figure 5.48(B)	EDX of U-POFA20 paste under SCR-1 at the age of 180	
	days	206
Figure 5.49	SEM micrograph of U-POFA40 paste under SCR-1 at the	
	age of 180 days	207
Figure 5.49(A)	EDX of U-POFA40 paste under SCR-1 at the age of 180	
	days	207
Figure 5.49(B)	EDX of U-POFA40 paste under SCR-1 at the age of 180	
	days	207
Figure 5.50:	SEM micrograph of U-POFA60 paste under SCR-1 at the	
	age of 180 days	208
Figure 5.50(A)	EDX of U-POFA60 paste under SCR-1 at the age of 180	
	days	208

	days	208
CHAPTER 6		
Figure 6.1	Initial surface absorption for the G-POFA HSGC at time	
	intervals of 10 minutes	215
Figure 6.2	Initial surface absorption for the G-POFA HSGC at time	
	intervals of 120 minutes	216
Figure 6.3	Initial surface absorption for the U-POFA HSGC at time	
	intervals of 10 minutes	217
Figure 6.4	Initial surface absorption for the U-POFA HSGC at time	
	intervals of 120 minutes	218
Figure 6.5	Initial surface absorption for the G-POFA and U-POFA	
	HSGC at time intervals of 10 minutes under NCR	219
Figure 6.6	Initial surface absorption for the G-POFA and U-POFA	
	HSGC at time intervals of 120 minutes under NCR	220
Figure 6.7	Initial surface absorption with SCR-1 at time intervals of 10	
	minutes	221
Figure 6.8	Initial surface absorption with SCR-2 at time intervals of 10	
	minutes	221
Figure 6.9	Initial surface absorption with SCR-3 at time intervals of 10	
	minutes	222
Figure 6.10	Initial surface absorption with SCR-1 at time intervals of 120	
	minutes	223
Figure 6.11	Initial surface absorption with SCR-2at time intervals of 120	
	minutes	223
Figure 6.12	Initial surface absorption with SCR-3 at time intervals of 120	223
	minutes	
Figure 6.13	Initial surface absorption with SCR-4 at time intervals of 10	
	minutes	224
Figure 6.14	Initial surface absorption with SCR-5 at time intervals of 10	
	minutes	224

EDX of U-POFA60 paste under SCR-1 at the age of 180

Figure 5.50(B)

Figure 6.15	Initial surface absorption with SCR-4 at time intervals of 120	
	minutes	225
Figure 6.16	Initial surface absorption with SCR-5 at time intervals of 120	
	minutes	225
Figure 6.17	Porosity of HSGC containing G-POFA	229
Figure 6.18	Water absorption of HSGC containing G-POFA	229
Figure 6.19	Porosity of HSGC containing U-POFA	230
Figure 6.20	Water absorption of HSGC containing U-POFA	230
Figure 6.21	Water absorption of HSGC containing G-POFA and U-POFA	
	under normal curing regime	232
Figure 6.22	Porosity of HSGC containing G-POFA and U-POFA under	
	normal curing regime	232
Figure 6.23	Water absorption of HSGC containing U-POFA with SCR-1	234
Figure 6.24	Water absorption of HSGC containing U-POFA with SCR-2	234
Figure 6.25	Water absorption of HSGC containing U-POFA with SCR-3	234
Figure 6.26	Porosity of HSGC containing U-POFA subjected to SCR-1	235
Figure 6.27	Porosity of HSGC containing U-POFA subjected to SCR-2	235
Figure 6.28	Porosity of HSGC containing U-POFA subjected SCR-3	235
Figure 6.29	Water absorption of the HSGC containing U-POFA with SCR-4	236
Figure 6.30	Water absorption of the HSGC containing U-POFA with	
U	SCR-5	236
Figure 6.31	Porosity of the HSGC containing U-POFA with SCR-4	237
Figure 6.32	Porosity of the HSGC containing U-POFA with SCR-5	237
Figure 6.33	Gas permeability of HSGC containing G-POFA	240
Figure 6.34	Gas permeability of HSGC containing U-POFA	241
Figure 6.35	Coefficient of gas permeability of G-POFA and U-POFA	
	HSGCs	243
Figure 5.36	Coefficient of gas permeability of the HSGC containing U-	
	POFA with SCR-1	244
Figure 6.37	Coefficient of gas permeability of the HSGC containing U-	
	POFA with SCR-2	244

Figure 6.38	Coefficient of gas permeability of the HSGC containing U-	
	POFA with SCR-3	245
Figure 6.39	Coefficient of gas permeability of the HSGC containing U-	246
	POFA with SCR-4	
Figure 6.40	Coefficient of gas permeability of the HSGC containing U-	246
	POFA with SCR-5	
Figure 6.41	Water permeability of HSGC containing G-POFA under NCR	250
Figure 6.42	Water permeability of HSGC containing U-POFA	251
Figure 6.43	Coefficient of water permeability of G-POFA and U-POFA	
	HSGC under NCR	252
Figure 6.44	Coefficient of water permeability of the HSGC containing U-	
	POFA with SCR-1	254
Figure 6.45	Coefficient of water permeability of the HSGC containing U-	
	POFA with SCR-2	254
Figure 6.46	Coefficient of water permeability of the HSGC containing U-	
	POFA with SCR-3	254
Figure 6.47	Coefficient of water permeability of the HSGC containing U-	
	POFA with SCR-4	256
Figure 6.48	Coefficient of water permeability of the HSGC containing U-	
	POFA with SCR-5	256
Figure 6.48	Rapid chloride permeability of the HSGC containing G-POFA	
	under NCR	259
Figure 6.49	Rapid chloride permeability of the HSGC containing U-POFA	261
Figure 6.50	Rapid chloride permeability of the HSGC containing G-POFA	
	and U-POFA under NCR	263
Figure 6.51	RCPT of the HSGC containing U-POFA with SCR-1	265
Figure 6.52	RCPT of the HSGC containing U-POFA with SCR-2	265
Figure 6.53	RCPT of the HSGC containing U-POFA with SCR-3	265
Figure 6.54	RCPT of the HSGC containing U-POFA with SCR-4	267
Figure 6.55	RCPT of the HSGC containing U-POFA with SCR-5	267
Figure 6.56	Chloride penetration rate of the HSGC containing G-POFA	
	under NCR	271

Figure 6.57	Chloride migration coefficient of the HSGC containing G-	
	POFA under NCR	271
Figure 6.58	Chloride migration coefficient of the HSGC containing U-	
	POFA under NCR	273
Figure 6.59	Chloride penetration rate of the HSGC containing U-POFA	
	under NCR	274
Figure 6.60	Chloride migration coefficient of the HSGC containing G-	
	POFA and U-POFA under NCR	275
Figure6.61	Chloride penetration rate of the HSGC containing G-POFA	
	and U-POFA under NCR	276
Figure 6.62	Chloride migration coefficient of the HSGC with SCR-1	277
Figure 6.63	Chloride migration coefficient of the HSGC with SCR-2	277
Figure 6.64	Chloride migration coefficient of the HSGC with SCR-3	278
Figure 6.65	Chloride penetration rate of the HSGC with SCR-1	278
Figure 6.66	Chloride penetration rate of the HSGC with SCR-2	278
Figure 6.67	Chloride penetration rate of the HSGC with SCR-3	279

LIST OF ABBREVIATIONS

AASHTO American association of state highway and transportation

officials

ACV Aggregate crushing value

ASTM American society for testing and materials

BET Surface area based on Brunauer-Emmett-Teller theory using

nitrogen gas absorption test

BSI British standard institution

C₂S Dicalcium silicate

C₃A Tricalcium aluminate

C₃S Tricalcium silicate

C₄AF Tetracalcium aluminoferrite

CEB-FIP Comite Euro-international du Beton (CEB) and the federation

International de la Precontrainte (FIP).

CH Calcium hydroxide

C-S-A-H Calcium silicate aluminates hydrates

C-S-H Calcium silicate hydrates

DRUW Dry-rodded unit weight

DTA Differential thermal analysis

EDX/EDS Energy dispersive x-ray spectroscopy

FA Fly ash

FVCA Fraction volume of oven dry-rodded coarse aggregate

Aggregate

GGBS Ground granulated blast furnace slag

G-POFA Ground palm oil fuel ash

HPC High performance concrete

HRWR High range water reducer

HSC High strength concrete

HSGC High strength green concrete

ISAT Initial surface absorption test

LOI Loss on ignition

MK Metakaolin

MPa Mega Pascal

MSA Maximum size aggregate

NCR Normal curing regime

NSC Normal strength concrete

O.D. Oven-dry weight of coarse aggregate

OCCA Optimum content of coarse aggregate

OPC Ordinary Portland cement

POFA Palm oil fuel ash

RCMT Rapid chloride migration test

RCPT Rapid chloride permeability test

RH Relative humidity

RHA Rice husk ash

SCR Steam curing regime

SEM Scanning electron microscope

SF Silica fume

SP Superplasticizer

TGA Thermo-gravimetric analyses

T-GPOFA Treated palm oil fuel ash

U-POA Ultra-fine palm oil waste ash

VA Volcanic ash

VP Volcanic pumice

W-POFA Original palm oil fuel ash

XRD X-ray diffraction

XRF X-ray fluorescence

LIST OF SYMBOLS

% Percentage

μ Viscosity of the gas (Ns/m²)

μm Micrometer

C_{CH} Content of calcium hydroxide in sample (%)

 C_{PR} Chloride penetration rate (m/(v.hr))

D_{nssm} Non-steady-state migration coefficient (m²/s)

f Flow (mm/m²/s)

Fc' Compressive strength (N/mm²)

Frc Designated compressive strength (N/mm²)

K Coefficient of gas permeability

Kw Coefficient of water permeability

M_{cristobalite} Mass percentage of cristobalite

M _{Crystalline:} Total mass percentage of crystalline phases

M glassy Total mass percentage of glassy phases

 M_{K-Al-P} Mass percentage of potassium aluminum phosphate

Mass percentage of quartz

Mc The total mass of sample at 100°C

Mcc Loss in mass at between 410 to 520°C

 $P_{\rm in}$ Pressure at inlet (bar)

 P_{out} Pressure at outlet (bar)

Q Volume flow rate (m^3/s)

Te Average value of the initial and final temperatures in the solution

U Absolute value of the applied voltage

w/b Water to binder ratio

w/c Water to cement ratio

 X_D Average value of the penetration depths

ρ Density of water

PENGARUH PENGAWETAN STIM TERHADAP SIFAT-SIFAT KEJURUTERAAN DAN PENGANGKUTAN BENDALIR KONKRIT HIJAU BERKEKUATAN TINGGI MENGANDUNGI ABU KELAPA SAWIT

ABSTRAK

Kajian-kajian terdahulu mengenai penggunaan abu sisa bahan bakar kelapa sawit (POFA) dalam konkrit berkekuatan tinggi jelas menunjukkan keberkesanan POFA yang terhad, di mana hanya tahap penggantian yang kecil boleh digunakan dan kekuatan awal konkrit menurun dengan ketara terutama pada kandungan POFA yang tinggi. Adalah dijangkakan bahawa kecekapan POFA boleh dipertingkatkan melalui rawatan mekanikal dan haba. Ini akan membolehkan POFA digunakan dalam kuantiti yang besar dalam konkrit berkekuatan tinggi tanpa menjejaskan sifat-sifat dan prestasi konkrit. Oleh yang demikian, objektif utama penyiasatan adalah untuk mengkaji pengaruh abu sisa bahan api kelapa sawit terawat (POFA) terhadap sifatsifat kejuruteraan dan pengangkutan bendalir konkrit hijau berkekuatan tinggi (HSGC). Di samping itu, pengaruh pelbagai regim pengawetan stim keatas sifat-sifat kejuruteraan dan pengangkutan bendalir HSGC yang mengandungi sehingga 60% POFA terawatt juga dikaji. Kesan perawatan POFA terhadap ciri-ciri fizikal dan kimianya telah diselidiki berasaskan ujian-ujian seperti teknik XRD, analisa XRF, SEM/EDX, analisa terma dan analisa saiz zarah. POFA yang diperolehi dari kilang sawit telah dirawat menggunakan rawatan haba untuk menghapuskan lebihan kandungan karbon dan dikisar kepada saiz zarah median kurang lebih 2 µm. POFA terawatt halus (U-POFA) yang terhasil telah digunakan untuk menghasilkan HSGC dengan tahap penggantian POFA 0, 20, 40 dan 60% daripada jisim simen Portland biasa (OPC). Keputusan kajian menunjukkan bahawa rawatan yang dijalankan terhadap POFA menghasilkan pozzolan yang sangat cekap. HSGC mengandungi U-

POFA mempamerkan kebolehkerjaan dan pengekalan kebolehkerjaan yang unggul serta masa pemejalan yang terencat berbanding HSGC yang mengandungi POFA yang tidak dirawat dan juga HSC-OPC kawalan. Bagi kes kekuatan mampatan, penggunaan U-POFA mengurangkan kekuatan awal pada umur 1, 3 dan 7 hari, tetapi meningkatkan kekuatan pada 28, 90, 180 dan 360 hari untuk semua HSGC mengandungi POFA, dimana kekuatan melebihi 95 MPa dicapai untuk semua POFA-HSGC pada umur 28 hari. Manakala, sifat-sifat pengangkutan bendalir yang dinilai melalui ujian keliangan, penyerapan permukaan awal, penyerapan air, kebolehtelapan klorida pesat, penghijrahan klorida pesat, kebolehtelapan gas dan kebolehtelapan air menunjukkan peningkatan yang ketara dengan penggunaan U-POFA, di mana HSGC yang mengandungi 60% U-POFA mempamerkan peningkatan terbesar pada jangka pendek (28 hari) dan jangka panjang (360 hari). Pengaplikasian regim pengawetan stim menyumbang terhadap peningkatan kekuatan awal pada 1, 3 dan 7 hari, di mana peningkatan di dalam sifat-sifat konkrit bergantung kepada kadar penggantian U-POFA serta suhu dan juga tempoh pengawetan stim. Di samping itu, U-POFA memainkan peranan dalam mengurangkan kesan negatif yang mungkin terhasil ekoran daripada penggunaan regim pengawetan stim yang berbeza. Oleh itu, keputusan keseluruhan kajian menunjukkan U-POFA mempunyai potensi yang signifikan sebagai bahan tambah mineral pozzolan yang efisyen untuk penghasilan HSGC dengan menjanjikan sifat-sifat kejuruteraan dan pengangkutan bendalir yang unggul apabila dikenakan pengawetan biasa dan regim pengawetan stim.

INFLUENCE OF STEAM CURING ON ENGINEERING AND FLUID TRANSPORT PROPERTIES OF HIGH STRENGTH GREEN CONCRETE CONTAINING PALM OIL FUEL ASH

ABSTRACT

Previous studies on the utilization of palm oil fuel ash (POFA) in high strength concrete indicated clear limitation in term of the efficiency of the POFA, whereby only small replacement can be used and early strength is significantly reduced in particular at high POFA content. It is envisaged that the efficiency of the POFA could be improved via mechanical and heat treatments. This will enable the POFA to be used in greater volume in high strength concrete, without jeopardizing the properties and performance of the concrete. Thus, the main objective of this investigation is to study the influence of treated palm oil fuel ash (POFA) on the engineering and fluid transport properties of high-strength green concrete (HSGC). In addition, the influence of application of various regimes of steam curing on the engineering and fluid transport properties of the HSGC containing up to 60% of treated POFA is also studied. Effect of treated POFA on its chemical and physical properties has been studied based on tests such as XRD techniques, XRF analysis, SEM/EDX, thermal analysis and particle size analysis. POFA obtained from a palm-oil industry waste was treated via heat treatment to remove excess carbon and ground to a median particle size of about 2 µm. The ultrafine POFA (U-POFA) obtained was then utilized in the production of HSGCs with POFA replacement levels of 0, 20, 40 and 60% by mass of ordinary Portland cement (OPC). The results show that the treatment processes undertaken on the POFA result in a highly efficient pozzolan mineral admixture for HSC and will enable it to be utilized higher quantity. The HSGC containing the U-POFA exhibits superior workability and workability retention as

well as extended setting times when compared with HSGC containing untreated POFA and the control OPC HSC. In the case of compressive strength, the inclusion of the U-POFA reduces early age strength of the HSGC at 1, 3 and 7 days, but enhances the strength at 28, 90, 180 and 360 days for all HSGCs containing POFA, where strength exceeding 95 MPa was achieved for all the POFA-HSGCs at the age of 28 days. Whereas the transport properties assessed via initial surface absorption, porosity, water absorption, rapid chloride permeability, rapid chloride migration, gas permeability and water permeability tests show significant improvement with the inclusion of the ultrafine POFA, with the HSGC containing 60 % U-POFA exhibiting the greatest improvement at short-term (28 days) and long-term (360 days). The application of steam curing regimes contributes towards improving the compressive strength and fluid transport properties of the HSGC at the early ages of 1, 3 and 7 days, whereby this improvement in the properties of concrete depends largely on replacement rates of cement by the U-POFA and the temperature as well as the period of steam curing. In addition, the U-POFA plays a role in lessening the negative effect that may result from the employment of the different steam curing regimes. Thus, the overall results show that the U-POFA possesses significant potential as an efficient pozzolanic mineral admixture for the production of HSGC with promisingly superior engineering and fluid transport properties when subjected to normal and steam curing regimes.

CHAPTER 1

INTRODUCTION

1.1. Introduction

Concrete is an integral part of the construction of all building structures around the world. Presently, the major thrust is on developing newer concrete in order to adhere to the latest specifications for satisfying modern construction requirements. One such new and modern type of concrete is the "high-strength green concrete" (HSGC). It is a combination of green concrete and high strength concrete. Protecting the environment is the main concern of green concrete (environmental friendly concrete). This protection is accomplished by the ability of this type of concrete to include industrial and agricultural wastes as part of the binder components. This process has three main benefits: 1) reducing the resurgence of carbon dioxide through minimization of cement usage; 2) mitigating the hazardous effects of wastes by their usage in concrete industries; this usage will completely remove wastes from being a source of environmental pollution and; 3) improving properties of high strength concrete which results in reduction of overall cost of concrete production.

High strength concrete (HSC) has undergone significant development over the past few years. The definition of HSC has been changing with the passage of time. A concrete having a compressive strength of about 34 MPa easily fitted the definition of HSC in the late 1950's, whereby a concrete having compressive strength of more than 41 MPa was a rarity with its usage limited to only few applications. According to CEB-FIP (1990) state-of-the-art report, things changed in 1990 onwards with compressive strength of HSC defined at 60 MPa. Presently, a concrete having a compressive strength exceeding 80 MPa fits the specifications of HSC (Persson,

1996; Neville, 2012). Moreover, the applications of HSC have increased, and it has now been used in many parts of the world. HSC can be manufactured using several methods of mix design. In particular, its composition consists of strong coarse aggregates, high content of Portland cement and lower water/(cement or binder) ratio. Nowadays, high range water reducer (superplasticizer) is extensively utilized in HSC production, in addition to mineral admixtures. The production of HSC necessitates the use of low water/binder ratio (w/b), typically in the range of 0.22 to 0.35. Therefore, the use of high range water reducing admixture or superplasticiser is required to obtain this low water/binder ratio as well as to achieve adequate workability. Mineral admixtures such as silica fume are usually used in HSC mixture, to achieve compressive strength above 80 MPa at the age of 28 days. Sometimes, other pozzolanic mineral admixtures such as ground granulated blast-furnace slag and fly ash are used in order to achieve the desired HSC. However they may have limited strength especially at high replacement level and also at early ages (Neville & Aitcin, 1993; Megat Johari, 2000).

Pozzolanic materials are mostly originated from industrial or/and agricultural wastes. Therefore, the utilization of these pozzolanic waste materials to produce concrete is necessary and is beneficial from environmental point of view. Moreover, from an economic perspective, the use of these pozzolanic materials considerably decreases the quantity of Portland cement used in the production of concrete, which in turn leads to potential reduction in overall cost of producing concrete. Furthermore, these additives can contribute towards improving the engineering properties of concrete such as strength and transport properties, which consequently could lead to enhancement in durability performance. The employment of pozzolanic materials with Portland cement to manufacture high-strength concrete helps at improving the

properties of HSC. The efficiency of pozzolanic materials depends on the reactions between the amorphous silica (SiO₂) and the calcium hydroxide Ca(OH)₂ to produce calcium silicate hydrate (C-S-H) gel. Also the fineness and shape of the pozzolanic materials make the concrete more homogeneous and denser, due to the filling and refining of the pores (Sellevold, 1987; Detwiler & Mehta, 1989; Siddique, 2011b).

Palm oil fuel ash (POFA) is a new addition to the list of pozzolanic materials. Palm oil is a popular crop in Malaysia and it constitutes one of the major agro industries in the region. Raw materials in the form of fresh fruit bunches are supplied to the palm oil industry which generates a large amount of solid waste materials in the form of empty fruit bunches, fibers and kernel shells. These solid waste materials in turn are used as fuel to heat boiler to produce steam and generate electricity required in the palm oil extraction process. After combustion, the raw materials produce around 5% POFA by weight from the overall solid wastes. Due to the limited utilization of POFA, it has to be disposed off as landfill materials, leading to a potential future environmental problem. However, many researchers (Tay, 1990; Tay & Show, 1996; Awal & Hussin, 1997) found that POFA had pozzolanic properties and could be used as a partial replacement of cement in concrete. Tables 1.1 and 1.2 present the physical and chemical properties of ordinary Portland cement (OPC) and POFA.

Table 1.1: Physical properties of OPC and POFA (Awal & Hussin, 1997)

Physical properties	OPC	POFA	
Surface area (m²/kg)	314	519	
Specific gravity	3.28	2.22	

Table 1.2: Chemical compositions of OPC and POFA (Awal & Hussin, 1997)

Component	OPC (%)	POFA (%)
Silicon dioxide (SiO ₂)	20.2	43.6
Aluminium oxide (Al ₂ O ₃)	5.7	11.4
Ferric oxide (Fe ₂ O ₃)	3.0	4.7
Calcium oxide (CaO)	62.5	8.4
Magnesium oxide (MgO)	2.6	4.8
Sulphur trioxide (SO ₃)	1.8	2.8
Sodium oxide (Na ₂ O)	0.16	0.39
Potassium oxide (K ₂ O)	0.87	3.5
Loss on ignition (LOI)	2.7	18.0

According to Abdul Awal & Hussin (1996), POFA fulfills the requirements of ASTM C618 (ASTM, 2005) as a good pozzolanic material and can be group in between class C and F pozzolan. Tables 1.3 and 1.4 respectively show the chemical and physical requirements of classes N, F and C of pozzolanic mineral admixtures according to ASTM C618. Based on the POFA chemical compositions in Table 1.2 and the ASTM C618 chemical requirements in Table 1.3, it is clear that the POFA does not satisfy the ASTM C618 chemical requirement with regards to LOI. The listed LOI value of 18% in Table 1.2 is much higher than the maximum allowable value of 12% for class F mineral admixture for material with acceptable performance records.

Table 1.3: Chemical requirements of classes N, F, and C based on ASTM C618 (ASTM, 2005)

Туре	Mineral admixture class			
	N	F	С	
$SiO_2+Al_2O_3+Fe_2O_3$, min (%)	70	70	50	
SO ₃ , max (%)	4	5	5	
Moisture content, max (%)	3	3	3	
Loss on ignition, max (%)	10	6	6	

Table 1.4: Physical requirements of classes N, F, and C based on ASTM C618 (ASTM, 2005)

T	Mineral admixture class			
Туре	N	F	С	
Fineness; Amount retained when wetsieved on 45 µm, max, (%)	34	34	34	
Strength activity index. With Portland cement, at 7 days, min, percent of control	75	75	75	
Strength activity index. With Portland cement, at 28 days, min, percent of control	75	75	75	
Water requirement, max, percent of control	115	105	105	
Density , max variation from average, (%)	5	5	5	
Percentage retained on 45 μm, max variation, percentage points from average	5	5	5	

According to Tonnayopas *et al.* (2006) pozzolanic reaction between the POFA and cement matrix can affect the development of concrete strength. In addition, it has been found that POFA had pozzolanic properties and could be used as a partial replacement of cement in concrete (Tay, 1990; Awal & Hussin, 1997; Sukantapree *et al.*, 2002). The test results on the performance of POFA revealed that it had a good

potential in reducing the expansion due to alkali silica reaction (Awal & Hussin, 1997). Nonetheless, the large size of POFA particle and its porous structure make the POFA to have low pozzolanic potential. According to Abdul Awal & Hussin (1996) and Sukantapree *et al.*(2002), grinding the pozzolanic material can improve the reactivity of POFA.

The amount of gel resulting from the process of hydration of cement is important in determining the properties of concrete. Curing of concrete has a significant effect on the properties of hardened concrete. The objective of concrete curing is to keep concrete saturated to promote the hydration of cement. In general, proper curing of concrete is expected to improve its properties such as strength, abrasion resistance, durability, water tightness, volume stability, and resistance to freezing and thawing. Hydration of cement continues for several years as long as the mixture contains unhydrated cement particles, the presence of water and under a favorable temperature conditions but the rate keeps decreasing over time. Curing method, time period as well as temperature directly affects the properties of concrete. The normal curing method involves immersing concrete samples in water for specific time duration. Among the other methods is the steam curing method accomplished under atmospheric pressure conditions. This type of curing is used to improve some of the engineering properties of concrete. The steam curing is often used to accelerate the hydration process which will eventually lead to an increase in the compressive strength of concrete at early age. The steam curing method is mostly used in the production of precast concrete (Liu et al., 2005; Yazici et al., 2005). The advantage of using steam curing method is that the initial rate of strength development is accelerated due to the elevated temperature of steam in addition to high humidity. In faster production lines, steam curing results in achieving the desired early strength

with short active curing time (Higginson, 1961; Turkel & Alabas, 2005). Most of the steam curing plants uses twenty-four hours cycle in order to speed up the production and to reduce the costs of concrete casting. Typical steam-curing cycle at the atmospheric pressure consists of four stages. The first stage is the initial delay period prior to steaming process, the second stage is a period of increase in the temperature inside the steam curing tank, in the third stage, the peak temperature for steam curing remains constant, the fourth stage is a period for decrease in the steam curing temperature.

1.2. Research Problem Statements

The recent spur in the development of activities in the field of construction and housing has resulted in increased demand for cement in Malaysia. Therefore, reducing the consumption of cement obviously helps in decreasing the construction costs. In addition, the increase in cement production to cater for the escalating demand causes greater damage to the environment due to the production of carbon dioxide during its manufacture. Hence, attempts at reducing the consumption of cement help to preserve the environment. In Malaysia, local materials such as POFA can be used as a partial substitute for cement to produce "GREEN CONCRETE" that greatly assists in reducing the environmental impact of wastes, as well as reduces the cost of construction with improvement in the performance of concrete. As a whole, extensive utilization of by-product based pozzolanic mineral admixture such as POFA could contribute towards better sustainability of the concrete industry.

Accordingly, the sufficiently amorphous POFA contains high amount of silica (SiO₂) which makes it possible to be used as a pozzolanic material in concrete. Nonetheless, previous research has shown that the maximum quantity of POFA that can be used in

HSC is about 30% from the mass of cement, when the POFA is used as partial cement replacement material. This limitation could be attributed to the coarser particle size and high unburned carbon of the POFA used. Therefore, in the present study, POFA was treated to lower the unburned carbon and to reduce the particle size and then used in the production of HSC by partially substituting ordinary Portland cement (OPC) at replacement levels of 0, 20, 40 and 60% on mass-for-mass basis. It was envisaged that the treatment process undertaken will enhance the efficiency of the POFA as pozzolanic mineral admixture for HSC and will enable it to be utilized higher quantity.

The available literature also indicates that the application of POFA as a partial substitute for cement in concrete production led to a decrease in compressive strength at early ages, with improvements in later ages. This problem somehow puts a restriction on the use of POFA in the concrete industry. This issue can probably be solved by the use of steam curing, which can raise the potential performance of concrete containing POFA at early ages. This will further enhance the POFA usage in HSC industry, especially for the precast and pre-stressed concrete. The application of POFA in concrete, especially in HSC requires further investigations. This is more pertinent when the POFA is treated for better efficiency, thus, its effects on the engineering and transport properties of HSC at early and later ages should be investigated.

1.3. Aim and Objectives

The primary aim of this study is to utilize agro-industry waste material, POFA that is locally available in Malaysia to produce HSC, which is environmentally friendly (high-strength green concrete, HSGC), as well as a concrete that has high durability

to sustain the internal and external effects of aggressive environment. Thus the research intends to achieve the following specific objectives:

- To quantify the effect of mechanical and heat treatment processes of POFA via physical and chemical characterizations.
- 2) To study the effect of POFA inclusion on engineering and fluid transport properties of HSGC.
- 3) To investigate the influence of steam curing regimes on engineering, microstructure and fluid transport properties of HSGC containing POFA.

1.4. Scope of Research

The engineering and transport properties of high-strength green concrete containing POFA are influenced by many factors. Among these factors, characteristics of the POFA used, rates of replacement of the cement with POFA, and type of curing are considered crucial.

In this research, different POFA replacement parentages namely 0, 20, 40 and 60% are examined in order to assess the influence of POFA content on the relevant properties of HSGC. In addition, the effect of different steam curing temperatures of 50°C, 65°C, and 80°C with steam curing period of 16 hours is studied. Similarly, the influence steam curing periods of 6 hours, 11 hours and 16 hours are studied at steam curing temperature of 80°C. The production of HSGC requires high quality materials, optimum mix design and preparation of trial mix. Therefore, low w/b ratio is considered, and superplasticizer is added to give good fresh concrete properties. Furthermore, in order to improve the properties of POFA, it is important to optimize the properties of HSGC, which requires the treatment process (mechanical and heat treatment) the waste of palm oil fuel ash, to improve its efficiency while

concomitantly conforming the requirements of relevant standards in particular ASTM C618 (ASTM, 2005).

In the fresh concrete state, the engineering properties such as setting times, workability and workability retention are evaluated. In addition, the hardened concrete state, the compressive strength and fluid transport properties are evaluated. For this study, steam curing cycle of not exceeding 24 hours is applied. The samples after being subjected to the steam curing regimes are immersed in a water tank, immediately after 24 hours until the testing ages. The concrete samples are then taken from the curing water tank to conduct tests at the ages of 1, 3, 7, 28, 90, 180 and 360 days. The influence of the POFA as partial cement replacement with steam curing regimes on engineering and transport properties of the HSGC are then evaluated. Besides compressive strength, fluid transport properties; namely initial surface absorption, porosity, water absorption, gas permeability, water permeability, rapid chloride permeability and rapid chloride migration are studied. In addition, the changes in the microstructure of the HSGC due to U-POFA inclusion and steam curing are also studied using scanning electron microscope with energy dispersive Xray microanalysis (SEM/EDX), X-ray diffraction (XRD) and thermal analysis (TGA/DT).

1.6. Layout of Thesis

Chapter one gives the background of the study, presenting an overview of the present research. It also discusses the issues concerning the research problems, aims and objectives

Chapter two presents an introduction to the materials used for the production of HSGC. It also presents the background of the characteristics of pozzolanic materials, in addition to characteristics of the palm oil fuel ash used. It highlights the performance of the HSGC containing the pozzolanic materials, and also the effect of application of steam curing regimes on the performance of the HSGC.

Chapter three presents the materials used and methods adopted throughout the experimental work. The procedures used in the treatment of the POFA for improving its properties and increasing its efficiency are presented. In addition, the mix design and mixture proportions used for the production of HSGC, mixing, casting as well as curing procedures adopted are also described. The last part of the chapter explains the detailed experimental procedures undertaken in the research.

Chapter four presents the results and discussion on the process of treatment of the POFA and the effects on its chemical and physical characteristics. It also presents the results and discussion on the effect of using treated and untreated POFA (G-POFA and U-POFA) as partial replacement of cement by rates up to 60%, on the characteristics of fresh high-strength green concrete, namely setting times, workability, and workability retention.

Chapter five presents the results and discussion on the effect of using the G-POFA and U-POFA as partial replacement by rates up to 60% of cement mass, on the strength of the high-strength green concrete. In addition, the influence of application of steam curing regime adopted using several temperatures (50°C, 65°C and 80°C) at 16 hours and also with different periods (6 hours, 11 hours and 16 hours) at 80°C, on the properties of HSGC containing U-POFA are discussed. In addition, the results and discussion also portray the effect of using the U-POFA as partial cement replacement by rates up to 60% of cement mass, on the microstructure of the HSGC binders based on the thermogravimetric, X-ray diffraction and scanning electron microscopy analyses.

Chapter six presents the results and discussion on the effect of using the G-POFA and U-POFA as partial replacement by rates up to 60% of cement mass, on fluid transport properties of the HSGC. Moreover, the influence of application of steam curing regimes adopted on fluid transport properties of the HSGC containing U-POFA are evaluated via the initial surface absorption, porosity, water absorption, gas permeability, water permeability, rapid chloride permeability and rapid chloride migration tests.

Chapter seven presents the conclusions and recommendations for future research.

CHAPTER 2

LITERATURE REVIEW

2.1. Introduction

The rapid development in all areas of civil engineering has led to increased demand for concrete. Consequently, many researchers have focussed their attention towards developing concrete that suits the present construction requirements and are capable of withstanding the harsh surrounding environment. This has led to the development of the concept of high-strength concrete (HSC) over the years. HSC is generally employed in the construction of large structures or high-rise buildings. Their use can reduce the size of structural elements and construction weight, eventually reducing the overall cost of the building. In the context of Malaysia, where many structures are built in aggressive environment, it is very important to use a good quality HSC. Therefore, in order to minimize the damage caused by the harsh environment, mineral additives are added as part of the binder in the production of concrete. There are a large number of additives which could bring about positive impact on durability performance of concrete.

HSC could somehow induce a negative impact on the environment, because the production of HSC consumes a large amount of cement, and the process of manufacturing of cement leads to the emission of large quantity of CO₂, causing harm to the environment. It has been estimated that about a ton of carbon dioxide is generated for every ton of Portland cement produced (Mehta, 1994; Bouzoubaa *et al.*, 1998; Mehta, 2002; Turanli *et al.*, 2005). The use of cement replacement materials which partially substitutes the Portland cement in HSC helps to reduce the

environmental pollution and minimize the cost of construction (Domone & Illston, 2010).

Most of the supplementary cementitious materials that are commonly used include wastes from industrial processes, thereby having significant economic advantages. Moreover, the added advantage of these additives is that they can be used to enhance the existing properties of HSC. The use of industrial and agricultural waste for the production of HSC, can therefore produce high strength green concrete (HSGC) or environment-friendly HSC. In this chapter, an introduction to the materials used for the production of HSGC is presented, and existing methods of curing concrete, which improves HSGC properties, and other properties related to this research are reviewed. Particular attention is given to the effect of the supplementary cementitious materials (pozzolanic material) and steam curing regimes, on the relevant properties of HSGC.

2.2. Materials for High Strength Green Concrete

HSC used in the construction industry usually utilizes the same materials employed for the production of normal strength concrete (NSC). However, unlike in the case of NSC, superplasticizer (sp) is an essential element of HSGC. Furthermore, for higher strength range, silica fume (SF) is normally added to concrete and other mineral admixtures such as fly ash (FA) and ground granulated blast slag (GGBS) also can be used. In addition, the use of good quality and stronger aggregate will be an added advantage to the strength of the concrete (Megat Johari, 2000; Lothenbach *et al.*, 2011). As well, pozzolanic materials in the form of waste ash originating from agriculture-based industry such as rice husk ash, palm oil fuel ash and bagasse ash can be used to partially replace the Portland cement, in order to enhance the strength

and durability performance of concrete (Tangchirapat *et al.*, 2009a; Kishore *et al.*, 2011; Rukzon & Chindaprasirt, 2011).

2.2.1. Cement

Ordinary Portland cement (Type I) is normally used to produce HSC, and in situations where high early strength is required such as in precast concrete industry, rapid-hardening Portland cement (Type III) can be used. The strength development, strength potential and other properties of HSC depend on the type of cement. The strength development characteristics of HSC may not be identical. This can be due to the allowed variation in the compositions and fineness of cement, in accordance with the standard specifications, owing to variation in manufacturing practices of a certain type of cement. The major compound compositions in ordinary Portland cement (OPC) are tricalcium silicate (C₃S), dicalcium silicate (C₂S), tricalcium aluminate (C₃A) and tetracalcium aluminoferrite (C₄AF) (Neville, 2012). The silicates, C₃S and C₂S in cement are the most important component responsible for producing C-S-H gel as a result of cement hydration. Table 2.1 shows the chemical compositions, while Table 2.2 shows the compound compositions and fineness of the ordinary Portland cement.

Table 2.1: Chemical compositions of cement (Kosmatka et al., 2002)

Portland	Chemical compositions (%)						
cement	SiO ₂	Al ₂ O ₃	Fe ₂ O ₃	CaO	MgO	SO ₃	Na ₂ O
Min Max.	18.7-22	4.7-6.3	1.6-4.4	60.6-66.3	0.7-4.2	1.8-4.6	0.11-1.2
Mean	20.5	5.4	2.6	63.9	2.1	3.0	0.61

Table 2.2: Compound compositions and fineness of cement (Kosmatka *et al.*, 2002)

Portland	Compound compositions (%)				
cement	C_2S	C ₃ S	₃ S C ₃ A C ₄ AF Blai		Blaine fineness (m²/kg)
MinMax.	40-63	9-31	6-14	5-13	300-421
Mean	54	18	10	8	369

The C₃S is responsible for early strengths at 3, 7, and 28 days and the early strength would be higher if the cement contains relatively large amounts of C₃S. On the other hand, the C₂S contributes to later age strength with slow rate of hydration at early age. Therefore, its participation increases the long-term strength, normally over the age of 28 days. Thus, cement with higher relative proportion of C₂S normally exhibits lower early strength, but higher long-term strength. Meanwhile, C₃A liberates high heat of hydration in the first few days, and it contributes significantly towards setting, with slight contribution towards the development of early strength (Taylor, 1997; Tennis & Jennings, 2000; Kosmatka et al., 2002). By altering the cement compositions, its strength development characteristics can be modified. Moreover, adjusting the cement fineness facilitates easy control over rate of early strength development. Also, by making a change in the surface area of the cement from 320 to 450 m²/kg (Blaine fineness), it is possible to increase the compressive strengths of cement mortar, between 50% to 100%, 30% to 60% and 15% to 40% at test ages of 1, 3, and 7 days, respectively (Mehta & Monteiro, 2006). A higher specific surface area will result in rapid hydration kinetics, however it may reduce the later age strength development (Megat Johari, 2000; Neville, 2012).

Production of HSC requires high content of cement; this would increase the shrinkage and creep in concrete. Therefore, it is not recommended to use big amount of cement exceeding 550 kg/m³ to control shrinkage. The preferred content of cement in the

production of HSC varies between 400 to 550 kg/m³. Nevertheless, it is possible to use a total binder content (cement and SF) of up to 650 kg/m³(Burg & Ost, 1992; Neville, 2012).

2.2.2. Mineral Admixture

Mineral admixture denotes minute granular materials, added to concrete and cement mortar to obtain specific engineering properties, which can be used as a partial replacement, or as an additive to cement. It is economically advantageous and promotes recycling of industrial and other waste by-products making it environmental friendly cement. Mostly, pozzolanic materials are used as mineral admixtures. Other than being pozzolanic, some of these admixtures have additional self cementitious properties. In the past, natural pozzolans from volcanic earths, tuffs, and shales were used. However, recently silica fume, fly ash, ground granulated blast furnace slag, rice husk ash and other industrial waste have become the main source of mineral admixtures (Megat Johari, 2000; Neville, 2012; Ozer & Ozkul, 2004). In ASTM C618 standard specification, pozzolan is defined as "siliceous or siliceous and aluminous material which in itself possesses little or no cementitious value but will, in finely divided form and in the presence of moisture, chemically react with calcium hydroxide at ordinary temperatures to form compounds possessing cementitious properties" (ASTM, 2005). The uses of mineral admixtures, pozzolanic or/and cementitious materials could affect the properties of concrete both in the fresh and hardened states. In fresh concrete, mix proportions, water requirement, setting characteristics, rheological characteristics including workability, bleeding and cohesiveness, heat evolution rate, total heat of hydration and several other properties are influenced by the use of mineral admixtures. In the case of hardened concrete, the rate of strength development and ultimate strength, permeability characteristics,

sulphate and chloride resistance, alkali silica reaction, carbonation, thermal cracking and shrinkage are influenced by the use of mineral admixtures (Chandra, 1996; Neville, 2012; Sata *et al.*, 2007; Chandara *et al.*, 2010).

2.2.2. (a) Pozzolanic Materials

In general, pozzolanic materials possess different properties and react differently with Ca(OH)₂ in the presence of water at normal temperature to form compounds possessing cementing properties. The pozzolanic materials, having good characteristics can be used to improve the properties of concrete in terms of long-term strength and durability performance. They can be used either as partial replacement or as additive, to cement. According to ASTM C618, depending upon the compositions and particle characteristics, pozzolanic materials can be classified into three different classes; N, C and F, as shown in Table 2.3 (Toutanji *et al.*, 2004; ASTM, 2005; Elahi *et al.*, 2010; Lothenbach *et al.*, 2011).

Table 2.3: ASTM C618 requirements for fly ash and natural pozzolans for use as mineral admixtures in Portland cement concrete (ASTM, 2005)

Chamical manufacture and a	Mineral admixture class			
Chemical requirements	N	F	C	
Silicon dioxide (SiO ₂) plus aluminum oxide (Al ₂ O ₃) plus iron oxide (Fe ₂ O ₃), min, %	70.0	70.0	50.0	
Sulfur trioxide (SO ₃), max, %	4.0	5.0	5.0	
Moisture content, max, %	3.0	3.0	3.0	
Loss on ignition, max, %	10.0	6.0	6.0	
Available alkalies, as equivalent, as Na ₂ O, max, %	1.5	1.5	1.5	
Physical requirements				
Fineness: Amount retained when wet-sieved on 45 µm (No.325) sieve, max, %	34	34	34	
Strength activity index: With Portland cement, at 7 and 28 days, min, percent of control	75	75	75	
Pozzolanic activity index, with lime at 7 days, min. (MPa)	5.5	5.5	-	
Water requirement, max, percent of control	115	105	105	
Autoclave expansion or contraction, max, %	0.8	0.8	0.8	
Density, max variation from average, %	5	5	5	
Percent retained on 45-µm (No. 325), max variation, percentage points from average (%)	5	5	5	

Note: The use of Class F pozzolan containing up to 12.0 % loss on ignition may be approved by the user if either acceptable performance records or laboratory test results are made available.

In addition, there are also some pozzolans which are non-crystalline or poorly crystalline but contain sufficient calcium to form compounds which possess cementitious properties after interaction with water. According to Malhotra & Mehta (1996), they can be classified as in Table 2.4

Table 2.4: Classification, compositions, and particle characteristics of mineral admixtures for concrete (Malhotra & Mehta, 1996)

Classification	Chemical and mineralogical compositions	Particle characteristics
Cementitious and pozzolanic Ground granulated blast- furnace slag (cementitious)	Mostly silicate glass containing mainly calcium, and silica, crystalline compounds of melilite group may be present in small quantity.	Unprocessed material is of sand size and contains 10-15% moisture. Before use it is dried and ground to particles less than 45µm (usually about 500 m²/kg blaine). Particles have rough texture.
High–calcium fly ash (cementitious and pozzolanic)	Mostly silicate glass containing mainly calcium, and alkalis. The small quantity of crystalline matter present generally consists of quartz and C ₃ A; free lime and periclase may be present; CS and C ₄ A ₃ S may be present in the case of high-sulfur coals. Un-burnt carbon is usually less than 2%.	Powder corresponding to 10-15% particles larger than 45 µm (usually about 300-400 m²/kg blaine). Most particles are solid spheres less than 20 µm in diameter. Particle surface is generally smooth but not as clean as in low calcium fly ash.
High active pozzolans Condensed silica fume	Consists essentially of pure silica in non-crystalline form.	Extremely fine powder consisting of solid spheres of 0.1 µm average diameter (about 20,000 m²/kg surface area by nitrogen adsorption).
Rice husk ash	Consists essentially of pure silica in non-crystalline form.	Particles are generally less than 45 µm but they are highly cellular (about 60,000 m²/kg surface area by nitrogen adsorption).
Normal pozzolans Low-calcium fly ash	Mostly silicate glass containing aluminium, iron, and alkalis. The small quantity of crystalline matter presents generally consists of quartz, mullet, sillimanite, hematite, and mangnetite.	Powder corresponding to 15-30% particles larger than 45 µm (usually about 200-300 m²/kg blaine). Most particles are solid spheres less than 20 µm.

There are many types of mineral admixtures, in addition to new types discovered that can be classified as pozzolanic materials. Pozzolans can be classified into two types namely, natural or artificial. The natural pozzolans are formed from volcanic activity. Artificial pozzolans are produced by the combustion of traditional materials such as calcined clay, silica stone and metakaolin. Agricultural ashes such as rice husk ash, palm oil fuel ash and bagasse ash and industrial wastes such as ground granulated blast furnace slag, silica fume and many types of fly ash also fall under this category (Tay & Show, 1996; Targan *et al.*, 2003; Rukzon & Chindaprasirt, 2011).

2.2.2. (b) Pozzolanic Activity

The pozzolanic activity is the ability of pozzolanic material to react with calcium hydroxide in the availability of water to produce compounds containing cementitious properties. Interaction of active silica or non-crystalline silica glass (amorphous silica) with lime is more rapid when compared to that with crystalline silica which has lower interaction properties (Antiohos & Tsimas, 2006). New chemical compounds such as, calcium silicate hydrate (C-S-H) and calcium hydroxide (Ca(OH)₂) also written as (CH) are released in the hydration process of two major compounds of cement namely tricalcium silicate (C_3S) and dicalcium silicate (C_2S). Consequently the pozzolanic reaction will take place only when CH is released, such that the pozzolanic material in mortar or concrete mix, interacts to produce C-S-H along with calcium aluminate hydrate C-S-A-H, which in general terms are known as cement gels, formed from hardened cement paste (Isaia et al., 2003; Lothenbach et al., 2011). In general, the pozzolanic activity depends not only on the pozzolanic reaction with CH, but also on the filler effect or physical effect through distribution of the particles in the mixture. This decreases the pores of hardened paste matrix and makes it denser and more homogeneous leading to improvement in the microstructures and properties

of concrete. Figures 2.1(a, b, c, d, e and f) show the different forms of particulate material in the pozzolans. Consequently, many researchers have reported that, the pozzolanic activity depends on several factors such as: 1) percentage of pozzolanic materials (SiO₂+Al₂O₃+Fe₂O₃) in mineral admixtures, 2) content of non-crystalline (amorphous) silicon dioxide (SiO₂), 3) distribution of particle size, 4) specific surface area and 5) particles shape (spherical, cuboid, plate etc. (Cordeiro *et al.*, 2011; Megat Johari *et al.*, 2011). Therefore, improving these factors will help increase the effectiveness of pozzolanic material which in turn would lead to improved properties of concrete. Equations 2.1 and 2.2 illustrate the hydration of cement and pozzolanic reaction:

$$(C_2S \text{ or } C_3S) + (H_2O) \rightarrow \text{primary } C-S-H + Ca(OH)_2$$
 (2.1)

$$Ca(OH)_2 + pozzolan \rightarrow secondary C-S-H$$
 (2.2)

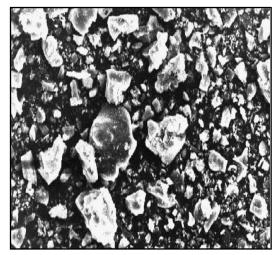


Figure 2.1 (a): Ground Rice husk-bark ash (Sata *et al.*, 2007)

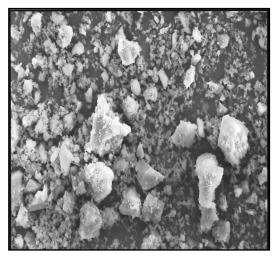


Figure 2.1 (b): Ground bagasse ash (Somna *et al.*, 2011)

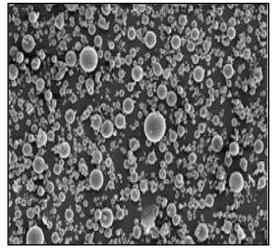


Figure 2.1 (c): Fine fly ash (Chindaprasirt *et al.*, 2008a)

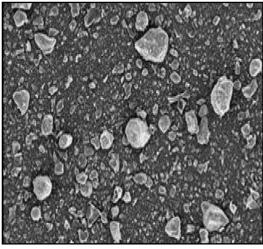


Figure 2.1 (d): Ground palm oil fuel ash (Chindaprasirt *et al.*, 2008a)

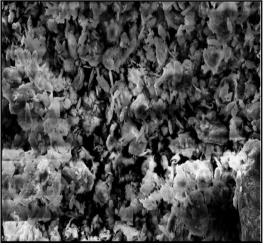


Figure 2.1 (e): Metakaolin (Chindaprasirt *et al.*, 2008a)

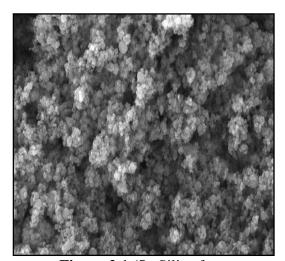


Figure 2.1 (f): Silica fume (Chindaprasirt *et al.*, 2008a)

2.2.2. (c) Some of the Pozzolanic Materials

a) **Volcanic materials**: such as volcanic ash (VA) and volcanic pumice (VP) are found abundantly in volcanic areas around the world. These volcanic materials can be used to produce low cost construction materials that are sustainable. Volcanic materials are also classified as pozzolanic materials because they satisfy the requirements of ASTM C618 (ASTM, 2005). Moreover they react with calcium hydroxide (Ca(OH)₂), producing cementious materials (Hossain, 2003; Siddique,

2011a). A study by Hossain (2005) showed that VA and VP are made up of silica and have similar composition (refer Table 2.5).

Table 2.5: Chemical compositions of volcanic ash and volcanic pumice (Hossain, 2005)

Components	VA (%)	VP (%)
SiO ₂	59.32	60.82
Al ₂ O ₃	17.54	16.71
Fe ₂ O ₃	7.06	7.04
CaO	6.1	4.44
MgO	2.55	1.94
Na ₂ O	3.80	5.42
K ₂ O	2.03	2.25
SO ₃	0.71	0.14
Loss on ignition (LOI)	1.03	1.52

- b) **Metakaolin** (MK) is classified as a pozzolanic material: the calcination of kaolinitic clay (Al₂Si₂O₅(OH)₄) at a temperature ranging between 500°C and 800°C, produces metakaolin (Al₂Si₂O₇). It usually contains chemical compositions in the range of 50-55% of SiO₂ and 40-45% of Al₂O₃. MK is highly reactive with calcium hydroxide and can accelerate cement hydration (Zhang & Malhotra, 1995; Shekarchi *et al.*, 2010). It contains alumina (calcium aluminate hydrates and alumosilicate hydrates) which interact with Ca(OH)₂ to produce alumina-containing phases C₂ASH₈, C₄AH₁₃ and C₃AH₆ (Siddique & Klaus, 2009).
- c) Silica fume (SF) is a by-product in the manufacture of silicon metal and ferrosilicon metal alloys. SF composes primarily of silicon dioxide (SiO₂) as can be seen