# Length of Need and Minimum System Length for F-Shape Portable Concrete Barrier 

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# LENGTH OF NEED AND MINIMUM SYSTEM LENGTH FOR F-SHAPE PORTABLE CONCRETE BARRIER 

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16. Abstract

Portable concrete barrier (PCB) systems are often used to redirect errant vehicles through a combination of inertial resistance, lateral friction loads, and tensile loads developed from the mass and friction of the barrier segments. State departments of transportation (DOTs) and other end users may wish to utilize minimal length PCB installations to shield a hazard or work zone or limit the number of barriers required on the upstream and downstream ends to reduce overall system length. However, concerns with the performance of shorter PCB installations include increased lateral deflections and working widths and barrier pocketing. Additionally, no impact testing has been performed near the upstream or downstream ends of the free-standing PCB system to determine the limits of the length of need (LON) of the system. These impacts may increase the potential for gating through the system, pocketing, rapid deceleration, and/or vehicle instability.

The objective of this research study was to investigate and evaluate the safety performance of a previously developed F-shape PCB system to determine minimum system length and the number of barriers required for the beginning and end of the LON. LS-DYNA simulation modeling was applied to determine potential beginning and end of LON points on reduced system lengths to select a configuration for full-scale testing and evaluation of a minimum length PCB system. A $100-\mathrm{ft}$ long PCB installation was selected, and full-scale crash testing was conducted on the beginning and end of LON of the reduced length system. Test no. NELON-1 was conducted to MASH test designation 3-35 criteria on the beginning of LON of the $100-\mathrm{ft}$ long PCB installation, and the vehicle was safely redirected. Test no. NELON-2 was conducted to modified MASH test designation no. 3-37 criteria on the end of LON of the 100-ft long PCB installation, but the test was deemed a failure as the vehicle demonstrated a roll angle in excess of 75 degrees. Review of the crash test results suggested that a nine barrier or 112.5 -ft long PCB installation would perform acceptably.

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## DISCLAIMER STATEMENT

This report was completed with funding from the Federal Highway Administration, U.S. Department of Transportation, the Midwest States Smart Work Zone Deployment Initiative (SWZDI), and the Nebraska Department of Roads. The contents of this report reflect the views and opinions of the authors who are responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of SWZDI, the Nebraska Department of Roads, the Federal Highway Administration, or the U.S. Department of Transportation. This report does not constitute a standard, specification, regulation, product endorsement, or an endorsement of manufacturers.


#### Abstract

ABOUT SWZDI Iowa, Kansas, Missouri, and Nebraska created the Midwest States Smart Work Zone Deployment Initiative in 1999 and Wisconsin joined in 2001. Through this pooled-fund study, researchers investigate better ways of controlling traffic through work zones. Their goal is to improve the safety and efficiency of traffic operations and highway work. The project is now administered by Iowa State University's Institute for Transportation.


## UNCERTAINTY OF MEASUREMENT STATEMENT

The Midwest Roadside Safety Facility (MwRSF) has determined the uncertainty of measurements for several parameters involved in standard full-scale crash testing and non-standard testing of roadside safety features. Information regarding the uncertainty of measurements for critical parameters is available upon request by the sponsor and the Federal Highway Administration.

## INDEPENDENT APPROVING AUTHORITY

The Independent Approving Authority (IAA) for the data contained herein was Ms. Karla Lechtenberg, Research Associate Engineer.

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The opinions, findings, and conclusions expressed in this publication are those of the authors and not necessarily those of the Iowa Department of Transportation or the U.S. Department of Transportation Federal Highway Administration.

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- Iowa (lead state)
- Kansas
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- Nebraska
- Wisconsin

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## EXECUTIVE SUMMARY

Portable concrete barrier (PCB) systems are often used to redirect errant vehicles through a combination of inertial resistance, lateral friction loads, and tensile loads developed from the mass and friction of the barrier segments. Unfortunately, recommendations on minimum PCB system lengths have generally been limited to the $200-\mathrm{ft}$ ( $61-\mathrm{m}$ ) length or longer in order to preserve the as-tested system deflections and impact behavior. In addition, guidance on the beginning and end of the length of need (LON) of these systems is typically given as a minimum of $100 \mathrm{ft}(30.5 \mathrm{~m})$ in order to preserve performance similar to existing crash tests.

State departments of transportation (DOTs) and other end users may wish to use shorter PCB installations to shield a hazard or work zone or limit the number of barriers required on the upstream and downstream ends to reduce overall system length. However, concerns with the performance of shorter PCB installations must be considered, including increased lateral deflections, working widths, and barrier pocketing, which could lead to vehicle instability or excessive decelerations. Additionally, no impact testing has been performed near the upstream or downstream ends of the free-standing PCB system to determine the limits of the LON of the system. Impacts at or near the barriers at the ends of a free-standing barrier system could produce very different barrier performance, and may include the potential for gating of the vehicle through the system, pocketing, rapid deceleration, and/or vehicle instability.

The objective of this research effort was to investigate and evaluate the safety performance of the previously developed F-shape PCB system in order to determine minimum system length and the number of barriers required for the beginning and end of the LON. LS-DYNA simulation was used to model MASH TL-3 impacts with a 2270P vehicle at varied locations along the PCB installation to determine the beginning and end of LON for a $200-\mathrm{ft}$ ( $61-\mathrm{m}$ ) long system. Next, models impacting the selected beginning and end of LON points were conducted on reduced system lengths to select a configuration for full-scale testing and evaluation. A $100-\mathrm{ft}$ ( $30.5-\mathrm{m}$ ) long PCB installation was selected, and full-scale crash testing was conducted at the beginning and end of LON of the reduced length system. Test no. NELON-1 was conducted according to MASH test designation no. $3-35$ on the beginning of LON of the $100-\mathrm{ft}(30.5-\mathrm{m})$ long PCB installation, and the vehicle was safely redirected. Test no. NELON-2 was conducted according to a modified MASH test designation no. 3-37 on the end of LON of the $100-\mathrm{ft}(30.5-\mathrm{m})$ long PCB installation, however, the test was deemed a failure as the vehicle demonstrated a roll angle in excess of 75 degrees. Review of the crash test results suggested that a nine barrier or $112.5-\mathrm{ft}$ ( $34-$ m ) long PCB installation would perform acceptably. Additional computer simulation modeling was conducted to provide guidance for deflections and working widths of intermediate length installations as well as for impacts at the $85^{\text {th }}$ percentile impact severity.

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## ACRONYMS, ABBREVIATIONS, AND SYMBOLS

| Acronym | Definition |
| :---: | :---: |
| AASHTO | - American Association of State Highway and Transportation Officials |
| AOS | - AOS Technologies AG |
| ASI | - Acceleration Severity Index |
| ASTM | - American Society for Testing and Materials |
| B.S.B.A. | - Bachelor of Science in Business Administration |
| c.g. | - center of gravity |
| CIP | - Critical Impact Point |
| CPU | - central processing unit |
| deg. | - degree |
| DOT | - Department of Transportation |
| DTS | - Diversified Technical Systems, Incorporated |
| E.I.T. | - Engineer in Training |
| FHWA | - Federal Highway Administration |
| ft | - foot |
| $\mathrm{ft} / \mathrm{s}$ | - feet per second |
| g's | - g-force, acceleration due to gravity at the Earth's surface |
| GP | - GoPro |
| GB | - gigabyte |
| h | - hour |
| Hz | - Hertz |
| i.e. | - id est (that is) |
| IAA | - Independent Approving Authority |
| in. | - inch |
| IS | - impact severity |
| JVC | - Victor Company of Japan, Limited |
| kg | - kilogram |
| kip-in. | - thousand pounds-force inches |
| kips | - thousand pounds-force |
| kJ | - kilojoules |
| km | - kilometer |
| km/h | - kilometers per hour |
| kN | - kilonewton |
| lb | - pound(s) |
| LED | - light-emitting diode |
| LON | - length of need |
| $\mathrm{m} / \mathrm{s}$ | - meters per second |
| m | - meter |
| mm | - millimeter |


| $\begin{aligned} & \text { MASH } \\ & \text { mm } \end{aligned}$ | - Manual for Assessing Safety Hardware <br> - millimeter |
| :---: | :---: |
| mph | - miles per hour |
| M.S.C.E. | - Master of Science in Civil Engineering |
| M.S.M.E. | - Master of Science in Mechanical Engineering |
| $m V$ | - millivolts |
| MwRSF | - Midwest Roadside Safety Facility |
| N | - Newton |
| NA | - not applicable |
| NCAC | - National Crash Analysis Center |
| NCHRP | - National Cooperative Highway Research Program |
| NDOR | - Nebraska Department of Roads |
| NHS | - National Highway System |
| no. | - number |
| nos. | - numbers |
| OIV | - occupant impact velocity |
| ORA | - occupant ridedown acceleration |
| PCB | - Portable Concrete Barrier |
| P.E. | - Professional Engineer |
| Ph.D. | - Doctor of Philosophy |
| PHD | - Post-Impact Head Deceleration |
| p.m | - post meridiem |
| R\&D | - research and development |
| RSVVP | - Roadside Safety Simulation Verification and Validation Program |
| RWD | - rear-wheel drive |
| s | - second |
| SAE | - Society of Automotive Engineers |
| SBP | - slope break point |
| sec | - second |
| SIM | - Sensor Input Module |
| THIV | - Theoretical Head Impact Velocity |
| TL | - Test Level |
| TTI | - Texas A\&M Transportation Institute |
| U.S. | - United States |
| US | - upstream |
| V | - volts |
| vs. | - versus |
| ${ }^{\circ} \mathrm{F}$ | - degrees Fahrenheit |
| , | - foot |
| " | - inch |
| \% | - percent |


| $\mu$ | - | vehicle-to-ground friction |
| :--- | :--- | :--- |
| $\sigma_{w}$ | - | yield strength of W-beam rail |
| $t_{w}$ | - | thickness of W-beam rail |
| $D_{b}$ | - | bolt diameter |
| $F_{v}$ | - | shear force |

## 1 INTRODUCTION

### 1.1 Problem Statement

Portable concrete barrier (PCB) systems redirect errant vehicles through a combination of various forces and mechanisms, including inertial resistance developed by the acceleration of several barrier segments, lateral friction loads, and the tensile loads developed from the mass and friction of the barrier segments upstream and downstream of the impacted region. Typically PCB designs are evaluated and tested using $200-\mathrm{ft}(61-\mathrm{m})$ long system lengths. It has generally been assumed that this length of system provides vehicle redirection, resulting system deflections, and working widths that are representative of longer PCB installations. Unfortunately, recommendations on minimum PCB system lengths have generally been limited to the $200-\mathrm{ft}$ (61$\mathrm{m})$ length or longer in order to preserve the as-tested system deflections and impact behavior. In addition, guidance on the beginning and end of the length of need (LON) of these systems is typically given as a minimum of $100 \mathrm{ft}(30.5 \mathrm{~m})$ (i.e., eight barrier segments of $12.5 \mathrm{ft}(3.8 \mathrm{~m})$ long) in order to preserve performance similar to existing crash tests.

Many instances exist where state departments of transportation (DOTs) and other end users wish to use shorter PCB installations to shield a hazard or work zone or limit the number of barriers required on the upstream and downstream ends to reduce the overall system length. Shorter barrier lengths are associated with lower accident frequencies and provide improved cost and safety benefits as long as they retain their ability to safely contain and redirect errant vehicles. However, concerns with the performance of shorter PCB installations must be considered. First, shorter PCB systems would be expected to have higher deflections and working widths than installations of 200 $\mathrm{ft}(61 \mathrm{~m})$ or more due to the reduction of upstream and downstream barrier mass and friction forces. Second, PCB systems may not develop sufficient longitudinal resistance at shorter system lengths and may form a pocket in front of an impacting vehicle, which could lead to vehicle instability or excessive decelerations. Finally, no impact testing has been performed near the upstream or downstream ends of free-standing PCB systems to determine the limits of the LON of the system. Impacts at or near the barriers at the ends of a free-standing barrier system may produce very different barrier performance than impacts near the center of the system, and the results may include the potential for gating of the vehicle through the system, pocketing, rapid deceleration, and/or vehicle instability.

Thus, a desire exists to install PCB systems shorter than $200 \mathrm{ft}(61 \mathrm{~m})$ and to more accurately define the beginning and end of the LON for these systems. Further study on the minimum effective length of PCB systems, their associated deflections and working widths, as well as a determination of the LON of these systems is warranted in order to provide more efficient and safe PCB installations.

Midwest Roadside Safety Facility (MwRSF) previously developed and full-scale vehicle crash tested a $12.5-\mathrm{ft}(3.8-\mathrm{m})$ long F-shape portable concrete barrier system for use in both freestanding and tie-down applications. This temporary barrier design is currently used by the Nebraska Department of Roads (NDOR). Full-scale crash testing of this barrier system was conducted under both the National Cooperative Highway Research Program (NCHRP) Report 350 [1] and Manual for Assessing Safety Hardware (MASH) [2] Test Level 3 (TL-3) safety requirements [3-4]. During the MASH TL-3 full-scale crash test, test no. 2214TB-2, the F-shape

PCB system exhibited a dynamic deflection of 79.6 in . ( $2,022 \mathrm{~mm}$ ) when impacted near the middle of a sixteen barrier segment test system with an overall length of $200 \mathrm{ft}(61 \mathrm{~m})$.

PCB installations shorter than the tested length would likely result in increased dynamic deflections as well as the potential for barrier pocketing. It is believed that the potential exists for shorter runs of free-standing F-shape PCBs to safely redirect errant vehicles. However, no existing research effort has been done to date to quantify the increased deflections and potential safety issues associated with shorter system lengths.

In order to effectively determine minimum system lengths and the required beginning and end of the LON for the free-standing F-shape PCB system, analysis of three main factors must be considered. These factors include the number of barriers required on the upstream end of the system, the number of barriers required on the downstream end of the system, and the overall system length. A minimum number of barrier segments are required on the upstream end of the system or beginning of LON to provide sufficient anchorage to safely redirect impacting vehicles with a reasonable dynamic deflection. Similarly, a minimum number of barrier segments is required on the downstream end of the system (i.e., end of the LON). However, the number of required barriers may be different on the upstream and downstream ends. In addition, the number of barrier segments required on the ends of the system will likely be affected by the overall length of the system. For example, the number of barrier segments required on the upstream end of a long PCB installation (i.e., higher downstream barrier resistance) may be different than the number of barriers required for a short system length that allows increased PCB movement downstream of the beginning of LON. Thus, determination of safe system lengths and beginning and end of the LON requirements for free-standing F-shape PCBs would require consideration of all of these factors.

### 1.2 Objectives

The objective of this research effort was to investigate and evaluate the safety performance of the previously developed F-shape PCB system in order to determine the minimum system length and the number of barriers required for the beginning and end of the LON. The minimum system length was evaluated through full-scale crash testing at the beginning and end of the LON. The full-scale crash testing was conducted and evaluated according to the TL-3 criteria set forth in MASH.

### 1.3 Scope

The research objective was achieved through completion of several tasks. First, a simple friction test to determine the coefficient of friction between the PCB and asphalt paving was conducted for comparison with previous PCB and concrete friction testing. Next, LS-DYNA computer simulation of the F-shape PCB system was conducted in order to analyze PCB system length and the beginning and end of the LON requirements. The simulation analysis provided guidance with respect to the potential minimum system length, number of barrier segments on the beginning and end of the LON, and critical impact points (CIP) for evaluation with full-scale crash testing. The proposed PCB system configuration was evaluated according to the MASH TL-3 safety criteria. Two full-scale crash tests were conducted to evaluate the system. The first test consisted of MASH test designation no. 3-35 to evaluate the effectiveness of the beginning of LON with a minimal system length. The second test consisted of a modified version of MASH test
designation no. 3-37 with the intent of assessing the end of the LON for the PCB system rather than maximizing vehicle snag and instability on a terminal or crash cushion. The full-scale vehicle crash tests were conducted, documented, and evaluated by MwRSF personnel in accordance with the MASH guidelines. Next, the test results were analyzed, evaluated, and documented. Following the full-scale crash testing, additional simulation analysis was conducted to provide guidance on PCB system deflections for intermediate system lengths. Finally, conclusions and recommendations were made that pertain to the safety performance of the LON for a free-standing, F-shape PCB system.

## 2 COMPONENT TESTING OF PCB FRICTION COEFFICIENTS

### 2.1 Purpose

Portable concrete barriers rely on friction between the bottom surface of the barrier and the roadway to develop resistance to longitudinal and lateral barrier motion and limit deflection. In previous research, Texas A\&M Transportation Institute (TTI) conducted basic component testing of PCB segments on flat ground to determine coefficients of friction for PCB segments on concrete [5]. The results of those component tests estimated the coefficient of friction for PCB segments on concrete to be 0.40 . MwRSF also conducted similar friction testing as part of a reduced deflection PCB study [6]. In that study, MwRSF identified static and kinetic coefficients of friction of 0.72 and 0.44 , respectively, for the F-shape PCB used in this study on a concrete tarmac.

For this study, NDOR requested additional component testing of the barrier-to-ground friction mechanism to quantify barrier-to-ground friction values for the PCB segment on asphalt paving. Thus, a quasi-static pull test of the concrete barrier segment on the asphalt paving was conducted for comparison with the previously determined values for the PCB segment when loaded on concrete. The details of the quasi-static pull test for determination of the static and kinetic coefficients of friction between the PCB segment and an asphalt road surface are provided in subsequent sections.

### 2.2 Scope

One quasi-static pull test was conducted on an F-shape PCB segment installed on asphalt paving in order to determine the static and kinetic coefficients of friction between the PCB segment and an asphalt road surface. The test setup is shown in Figure 1. An existing F-shape PCB segment used in a previous research effort was utilized for the quasi-static pull test. The PCB was installed on a $4-\mathrm{in}$. ( $102-\mathrm{mm}$ ) thick by $4-\mathrm{ft}(1.2-\mathrm{m})$ wide asphalt mow strip. The asphalt mow strip was constructed with a 52-34 grade binder typically utilized in highway shoulder construction in Nebraska.


Figure 1. Quasi-Static Pull Test Setup, Test No. TCBFA-1

### 2.3 Equipment and Instrumentation

Equipment and instrumentation utilized to collect and record data during the pull tests included a skid-steer, two tensile-load cells, standard-speed digital video, and a still camera.

### 2.3.1 Tensile-Load Cells

Two load cells were mounted in line with the pull cable to measure the tension in the cable for test no. TCBFA-1, as shown in Figure 2. The data from both load cells was processed and compared to ensure accuracy of the readings. The load cells were manufactured by Transducer Techniques and conformed to model no. TLL-50K with a load range up to 50 kips ( 222 kN ). During testing, output voltage signals were sent from the load cells to a National Instruments data acquisition board, acquired with LabView software, and stored permanently on a personal computer. The data collection rate for the load cells was 1,000 samples per second $(1,000 \mathrm{~Hz})$.


Figure 2. Load Cell Arrangement, Test No. TCBFA-1

### 2.3.2 Digital Photography

One GoPro digital video camera was used to document this test. The GoPro camera had a frame rate of 120 frames per second. The camera was placed laterally from the barrier test segment, with a view perpendicular to the direction of pull. A Nikon D3100 digital still camera was also used to document pre- and post-test conditions for this test.

### 2.4 Data Processing

For test no. TCBFA-1, force data was measured with the load cell transducers and filtered using the SAE Class 60 Butterworth filter conforming to the SAE J211/1 specifications [7]. The pertinent voltage signal was extracted from the bulk of the data signal similar to the acceleration data. The filtered voltage data was converted to load using the following equation:

$$
\text { Load }=\left[\frac{1}{\text { Gain }}\right] *\left[\frac{\text { Filtered Load Cell Data }}{\left(\frac{(\text { Calibration Factor })(\text { Excitation Voltage })}{\text { Full }- \text { Scale Load }}\right) *\left(\frac{1 \mathrm{~V}}{1000 \mathrm{mV}}\right)}\right]
$$

Details behind the theory and equations used for processing and filtering the load cell data are located in SAE J211/1. The gain and excitation voltage were recorded for each test. The calibration factor varied depending on the specific load cell being used. The load cell data was recorded in a data file and processed in a specifically-designed Excel spreadsheet. Force vs. time plots were created to describe the load imparted to the system.

## 3 FRICTION TESTING RESULTS AND DISCUSSION

Test no. TCBFA-1 was conducted to evaluate the barrier-to-ground friction coefficients for PCB segments on asphalt pavement. The component testing of the PCB segments sliding on concrete pavement was instrumented to estimate friction forces and coefficients. When the pulling force was initially applied to the barrier, a noticeable peak in the force vs. time graph was achieved. This peak force was used to calculate the static coefficient of friction between the surfaces by dividing the peak force by the weight of the barrier segment. Once the barrier began to slide on the pavement, the resistive force was reduced. The force readings taken while the barrier was in motion were averaged, and the average force was divided by the weight of the barrier segment to calculate the kinetic coefficient of friction.

### 3.1 Test No. TCBFA-1

In test no. TCBFA-1, a 4,796 lb ( $2,175 \mathrm{~kg}$ ) F-shape PCB segment was pulled on the asphalt pavement using a skid-steer. The corresponding force vs. time data is shown in Figure 3. A peak force of 3.07 kips ( 13.7 kN ) was measured prior to the onset of the PCB sliding. Once the PCB began to slide over the asphalt paving, an average force of $2.45 \mathrm{kips}(10.9 \mathrm{kN}$ ) was measured during barrier motion. Calculation of the friction coefficients for the barrier based on these forces and the mass of the barrier yielded static and kinetic coefficients of friction between the PCB and asphalt road surface of 0.64 and 0.51 , respectively.

The friction coefficients determined between the PCB and asphalt were similar to those obtained previously for the PCB on the concrete surface with the asphalt surface providing a slightly lower static coefficient of friction and a slightly higher dynamic coefficient of friction. This suggested that the design and evaluation of the PCB systems on concrete paving should provide relevant results for barriers installed on asphalt.


Figure 3. Force vs. Time, Test No. TCBFA-1

## 4 BASELINE MODEL OF F-SHAPE PCB SYSTEM

In order to evaluate impacts at the beginning and end of the LON and minimum system lengths, a baseline model of the free-standing, F-shape PCB system was created and compared to previous MASH TL-3 full-scale crash testing with the 2270P vehicle, test no. 2214TB-2 [4]. While previous simulation models of the F-shape PCB had been developed by the researchers, it was desired to further investigate the performance of the barrier model to promote improved results when analyzing the beginning and end of the LON and minimum system length. Thus, comparisons between the simulation model and the full-scale crash test were conducted based on dynamic barrier deflection, vehicle trajectory, and Roadside Safety Simulation Verification and Validation Program (RSVVP) analysis of vehicle transducer data [8]. Details for the baseline model development and the comparison with full-scale crash testing is detailed below.

### 4.1 PCB Model

The model of the F-shape portable concrete barrier was based on models developed previously at MwRSF for simulation of portable concrete barriers [9-6]. The model consisted of the F-shape barrier, the end connection loops, and the connection pins, as shown in Figure 4. The main body of the F-shape barrier model was created using shell elements with a rigid material definition. The rigid material definition allowed the proper mass and rotational inertias to be defined for the barrier even though it was essentially hollow. The barrier segments were assigned a mass of $4,976 \mathrm{lb}(2,257 \mathrm{~kg})$ based on measurements taken from actual barrier segments. The rotational inertias were determined based on SolidWorks models of the PCB segment. The SolidWorks models used tended to overestimate the mass and rotational inertia of the PCB segment as the solid model included the mass of the concrete body and the reinforcing steel, but did not account for the volume of concrete lost due to the reinforcing steel. Thus, the rotational inertias determined by the software were scaled down based on the ratio of the actual measured mass of the barrier segment to the software estimated mass of the segment. The use of the shell elements improved the overall contact of the barrier and the vehicle. In addition, the use of shell elements made it easier to fillet the corners and edges of the barrier. By rounding off the barrier edges, the edge contacts and penetrations were reduced, thus further improving the contact interface.

The loops in the barrier model consisted of two sets of three rebar loops. The connection loops were modeled with a rigid material as previous testing of the barrier in various configurations showed little to no deformation of the connection loops. The connection pin was modeled with the MAT_PIECEWISE_LINEAR_ PLASTICITY material in LS-DYNA with the appropriate properties for A36 steel. The baseline barrier system model incorporated a total of sixteen barrier segments for a total barrier length of $200 \mathrm{ft}(61.0 \mathrm{~m})$.

A critical component of the baseline model of the free-standing, F-shape PCB was the definition of the barrier-to-ground friction. PCB systems use a combination of inertial resistance and longitudinal tension to redirect impacting vehicles. The longitudinal tension in the barrier system is largely developed by barrier-to-ground friction. Previous research at TTI and MwRSF measured the kinematic friction coefficient for a concrete PCB segment sliding on a concrete surface to be between 0.40 and 0.44 [6-5]. Testing to measure the kinematic friction coefficient for a concrete PCB segment sliding on an asphalt surface detailed in the previous chapters of this report found a kinematic friction coefficient of 0.51 . The lower friction value of 0.40 was selected
for use in the analysis in order to better correlate with the road surface used in the full-scale testing and to maximize potential deflections. This friction value was applied in the LS-DYNA baseline model between the barrier segments and the shell element ground. In addition to providing appropriate friction coefficients, the barrier model needed to develop the correct weight or normal forces on the ground. This was accomplished by allowing the barriers in the simulation model to reach quasi-static equilibrium on the ground prior to being impacted. Damping was used to help the barriers reach a steady normal force on the ground and was turned off prior to vehicle impact.


Figure 4. F-Shape PCB: (a) Actual and (b) Finite Element Model

### 4.2 Vehicle Models

MASH denotes that a TL-3 longitudinal barrier such as the F-shape PCB utilized in this research must be subjected to impacts with the 2270 P pickup truck and the 1100 C small car. However, the 2270P test vehicle was deemed more critical than the 1100C small car due to the likelihood of increased barrier deflections, impact loading, and barrier pocketing. Further, vehicle instabilities have been exhibited during full-scale crash tests involving 2270P pickup trucks with F-shape PCB systems due to vehicle climb.

The Chevrolet Silverado quad cab vehicle model was chosen for the research and simulation study. The Silverado vehicle model was originally created by the National Crash Analysis Center (NCAC) and later modified by MwRSF personnel for use in roadside safety applications. Three versions of the Chevrolet Silverado vehicle model were investigated as part of the analysis of the baseline model: Version 2 (V2), Version 3 (V3), and Version 3 - Reduced (V3r). All three versions of the vehicle model represented the same Chevrolet Silverado quad cab vehicle, but there were differences in the tires, steering, vehicle-to-ground friction, and mesh size, among other factors. These differences are summarized in Figure 5.

The V3 and V3r models of the truck incorporated steering for the front wheels while the V2 model did not. The V2 model had a tire stiffness that correlated with the stiffness of actual truck tires, while the V3 and V3r models used significantly stiffer tire models. The meshes for all three versions of the truck model were different, with the main variation being the larger, coarser mesh of the reduced model. The coarser mesh of the V3r model improved its CPU efficiency, but may have had other effects in terms of contacts and vehicle deformation. Finally, the V3 and V3r models used default tire-to-ground friction values that were over twice as high as the default value for the V2 model. As such, it was believed that these differences in the vehicle models could contribute to the accuracy of the baseline model. Thus, all three vehicle models were used and compared when simulating the baseline model of the F-shape PCB system. Additional variations to the truck model that had been implemented by MwRSF over time were also investigated. These included the use of additional weld attachments between the truck box and frame in Version 3 that had previously been shown to improve stability and disengagement of the front wheels to represent suspension failure.

### 4.3 Baseline Model Simulations

The baseline model of the sixteen, free-standing, F-shape PCBs was simulated with a 2270 P vehicle impacting the system at a speed of $62 \mathrm{mph}(100 \mathrm{~km} / \mathrm{h})$ and at an angle of 25 degrees. The vehicle impacted the system $4.3 \mathrm{ft}(1.3 \mathrm{~m})$ upstream of the center of the joint between the eighth and ninth barrier segments. In order to evaluate the barrier model, a series of simulations were conducted using variations of the three Chevrolet Silverado vehicle models noted previously. This included simulations of the V2, V3 and V3r models and variations of those models, including changes in tire-to-ground friction, the use of front wheel disengagement, and the application of additional weld connections on the back end of the vehicle. The various models were compared to test no. 2214TB-2 based on the high-speed video comparison, dynamic deflection of the barrier system, and RSVVP comparison of transducer data. A summary of the model runs is shown in Table 1.


| Version No. | Tire Stiffness | Steering | Vehicle-to- <br> Ground Friction | Mesh |
| :---: | :---: | :---: | :---: | :---: |
| V2 | Soft | No | $\mu=0.40$ | Fine |
| V3 | Hard | Yes | $\mu=0.90$ | Fine |
| V3r | Hard | Yes | $\mu=0.90$ | Coarse |

Figure 5. Chevy Silverado 2270P Truck Model Variations

### 4.3.1 Chevy Silverado V3 Simulations

Analysis of the simulation of the F-shape PCB with the standard Chevy Silverado V3 found that the V3 model did not provide the best correlation with test no. 2214TB-2. Comparison of the high-speed video, shown in Figures 6 and 7, found that the V3 model displayed increased vehicle roll and pitch as compared to the full-scale test. This was confirmed by comparison of the rate gyro data between the simulation and testing. Additionally, the front wheels of the V3 model tended to steer away from the barrier, which was opposite of the steering behavior in test no. 2214TB-2. Comparison of the vehicle transducer data using the RSVVP program found that the standard Chevy Silverado V3 did not meet the single channel or multiple channel metric comparisons. The dynamic deflection of the PCB system in the V3 model was found to be 75.3 in . $(1,912 \mathrm{~mm})$ which was slightly less than the 79.6 in . $(2,022 \mathrm{~mm})$ deflection measured in the full-scale crash test. Review of the model suggested that the discrepancies between the simulation model behavior and the full-scale crash test were largely due to the combination of the V3 model's increased tire stiffness, higher tire-to-ground friction values, and the differences in the vehicle steering behavior. Vehicle tail slap with the barrier was also observed to be an issue with the V3 model due to the rigid rear axle assembly used on the vehicle. During vehicle tail slap with the PCBs, the axle assembly seemed to increase the severity of the tail slap and produce excess yaw and high lateral accelerations as compared to the full-scale testing.

Table 1. Summary of F-shape PCB Baseline Model Simulations

|  |  | Wheel | Tire-Ground |  | Dynamic | RSVVP CFC 180 (single channel) |  |  |  |  |  | RSVVP Multiple Channel | Notes |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Run No. | Model | Disengagemen t | Friction | End Welds | Deflection (mm) | x -acceleration | Yacceleration | z-acceleration | Yaw | Roll | Pitch |  |  |
| Run 5 | V3 | No | 0.9 | No | 1912 | No | No | No | No | No | No | No | Did not meet RSVVP - no single channels or multi-channel. Vehicle tires initially steer away from barrier in model and towards barrie in test. |
| Run 6 | V3 | Yes | 0.9 | Yes | 1995 | NA | NA | NA | NA | NA | NA | NA | RSVVP not run. Truck trajectory has far too much roll and pitch motion near end of simulation. Better deflections. Vehicle tires initially steer away from barrier in model and towards barrier in test. |
| Run 7 | V3 | Yes | 0.9 | No | 1961 | NA | NA | NA | NA | NA | NA | NA | RSVVP not run. Truck trajectory has far too much roll and pitch motion near end of simulation. Better deflections. Extra back end welds reduced truck roll slightly. Vehicle tires initially steer away from barrier in model and towards barrier in test. |
| Run 8 | V3 | No | 0.9 | Yes | 1965 | No | No | No | No | No | No | No | Roll of vehicle increased compared to Run 5. Vehicle tires initially steer away from barrier in model and towards barrier in test. Extra back end welds increased truck roll slightly. |
| Run 9 | V3r | No | 0.9 | NA | 1554* | NA | NA | NA | NA | NA | NA | NA | RSVVP not run. Excessive body roll and model instability. |
| Run 10 | V3 | Yes | 0.4 | Yes | 2015 | No | No | No | No | No | No | No | Vehicle tires initially steer away from barrier in model and towards barrier in test. Roll and pitch motions near end of model much improved over Run 6. Much better RSVVP comparisons. Note that this and all previous models have good lateral acceleration comps but underestimate longitiudnal deceleration. Potentially low vehicle to barrier friction issue. |
| Run 11 | V2 | No | 0.4 | NA | 2061 | No | Yes | No | No | No | No | Yes | V2 truck steering much closer to test - does not steer away. V2 truck does not allow steering. Accelerations much less "noisy". V-V comparisons much improved. Yaw better than V3 truck. Acceleration much closer even with CFC 180 comps. Softer tires and steering response appear to be a major factor. Tail slap seems to be over represented in severity leading to excess yaw and high lateral accelerations as compared to the test. Note that single channel comparisons improve greatly with CFC 60 accelerations. Velocity curves unchanged, but accelerations compare better (i.e, long accelerations pass). |
| Run 12 | V2 | Yes | 0.4 | NA | 2057 | No | Yes | No | No | No | No | Yes | Disconnect of wheel tends to increase roll and decrease climb as compared to Run 11. Appears that keeping the tire attached is a better representation of test even though tire detached in test. |
| Run 13 | V3r | Yes | 0.9 | NA | 1895 | NA | NA | NA | NA | NA | NA | NA | RSVVP not run. V3r has much higher roll and vehicle instability than V3 or V2. |
| Run 14 | V3 | no | 0.4 | No | 1976 | No | No | No | No | No | No | No | Reduced pitch and roll motions as compared to Run 5. Vehicle tires initially steer away from barrier in model and towards barrier in test. Similar in improvement seen between Run 6 and Run 10. May suggest lower barrier to ground friction for all models. Still very early drop in yaw rate. Likely due to tailslap and potentially vehicle-barrier friction as noted above. |



Time $=0.200 \mathrm{sec}$


Time $=0.300 \mathrm{sec}$
Figure 6. Crash Sequence - Standard Chevy Silverado V3 Model and Test No. 2214TB-2

L
Time $=0.400 \mathrm{sec}$

2
Time $=0.500 \mathrm{sec}$

L
noopregan Ematino
noopregan Ematino
Time $=0.600 \mathrm{sec}$


Time $=0.700 \mathrm{sec}$
Figure 7. Crash Sequence - Standard Chevy Silverado V3 Model and Test No. 2214TB-2

Subsequent changes were made to the V3 model to investigate if the model performance improved. These changes included disengagement of the left-front wheel during impact with the PCBs, reduction of the tire-to-ground friction, and adding additional welds to connect the box to the truck frame. Disengagement of the left-front vehicle wheel was observed in full-scale crash test no. 2214TB-2, but adding similar wheel release to the V3 model impact with the F-shape PCB did not improve correlation. Wheel disengagement tended to further increase the vehicle roll and pitch motions. Reduction of the tire-to-ground friction improved the response of the V3 model impacting the F-shape PCB by providing decreased roll and pitch motions and slightly increasing lateral barrier deflections. However, the steering of the vehicle wheels still prevented the simulation from meeting the single channel and multiple channel RSVVP comparisons. Finally, analysis of the additional welds on the rear section of the vehicle found little to no effect on the results of the simulation of the F-shape PCB impact.

### 4.3.2 Chevy Silverado V3r Simulations

Another series of simulations was conducted using the Chevy Silverado V3r model impacting the F-shape PCB system. The reduced model of the Chevy Silverado displayed similar increased roll and pitch motions, reduced lateral deflections, and inaccurate steering behavior as the V3 model. Additionally, the V3r model developed instabilities during simulation that were likely due to the coarser mesh used in the model and corresponding problems with the contact algorithms. Based on these issues, the V3r version of the Chevy Silverado was not selected for use as part of the baseline analysis of the PCB system.

### 4.3.3 Chevy Silverado V2 Simulations

A final series of simulations was conducted using the Chevy Silverado V2 model impacting the F-shape PCB system. Recall that the V2 model of the Silverado had significantly softer tires and lower default tire-to-ground friction values, but it did not include steering of the front wheels like the V3 and V3r models. Simulation of the F-shape PCB system with the Chevy Silverado V2 model demonstrated better correlation with the full-scale test results than the previous simulations with the V3 and V3r vehicles. The softer tires and lower tire-to-ground friction resulted in vehicle climb and roll and pitch motions that corresponded well with test no. 2214TB-2. Additionally, the lack of steering in the V2 model provided better correlation with the motion of the front wheels in the full-scale test as it did not show the tires steering away from the barrier like the V3 and V3r models. Similarly, increased vehicle yaw and lateral accelerations during tail slap were observed with the V2 model as compared with the V3 and V3r models.

Comparison of the results from the Chevy Silverado V2 model impacting the F-shape PCB system are shown in sequential images in Figures 8 through 11. This comparison found good correlation between the V2 model simulation and test no. 2214TB-2 in terms of vehicle behavior and the barrier motions. The simulation of the PCB impact with the V2 model had a peak dynamic barrier displacement of 81.1 in . $(2,061 \mathrm{~mm})$ which was nearly identical to the 79.6 in . $(2,022 \mathrm{~mm})$ displacement observed in test no. 2214TB-2. Additionally, RSVVP comparisons of the vehicle acceleration and rotation data found that the V2 model provided the best correlation with the fullscale test as it passed the single channel correlations for the lateral acceleration and yaw rotation and met the multiple channel comparisons in RSVVP. The results of the RSVVP comparison are shown in Figure 12.

Additional simulations were conducted with the Chevy Silverado V2 model impacting the F-shape PCB system that included disengagement of the front wheel on the impact side as was observed in the test. The overall response of the Chevy Silverado V2 model with front wheel disengagement was very similar to the original V 2 simulation in terms of vehicle deceleration and barrier displacement. Disengagement of the front wheel increased vehicle roll and decreased vehicle climb of the barrier as compared to test no. 2214TB-2. Additionally, disengagement of the front wheel tended to produce instabilities in some impact configurations due to the interaction of the disengaged tire and wheel with the barrier and ground later in the impact event.

### 4.3.4 Baseline Model Conclusions

Review of the simulations of the TL-3 impacts with the various Chevy Silverado models into the F-shape PCB system led to several observations about the baseline simulation model. First, the stiff tires, steering, and tire-to-ground friction on the Chevy Silverado V3 and V3r models adversely affected the correlation of the model with the test results. The stiffer tires potentially improved simulation stability by deforming less under load, but the increased stiffness tended to over-exaggerate the tire interaction with the barrier. This led to increased roll and pitch motions and negatively affected vehicle accelerations. The inclusion of front-wheel steering in the V3 and V3r models did not improve model correlation even though it would seem to be more accurate to include vehicle steering. It is possible that the steering in the model may need to include the mechanical resistance to motion of an actual steering mechanism, reduce tire stiffness, or refine vehicle tire and wheel friction with the barrier segments in order to produce a more accurate steering response. The default tire-to-ground friction value also tended to degrade the model correlation with the full-scale crash test due to an observed increase in roll and pitch motions. Second, the tail slap event for all three of the vehicle models tended to be more severe than what is typically observed in physical crash tests with these types of barriers and caused increased vehicle yaw and lateral accelerations. It was noted that this could potentially be improved through the use of more deformable structures and connections in the current rigid rear axle assembly.

Disengagement of the front wheel was implemented with all three versions of the truck model. This tended to increase the instability in most cases and did not improve the correlation with the full-scale test. It was noted that wheel disengagement could be used to bracket the vehicle response if necessary later in the research effort.

Finally, review of the results from all three truck models found that the Chevy Silverado V2 model of the impact with the F-shape PCB produced the best correlation with full-scale crash test no. 2214TB-2. Vehicle and barrier motions correlated well with the full-scale test based on high-speed video comparisons, and the dynamic lateral barrier deflection of the model was within 2 percent of that observed in the full-scale test. RSVVP analysis of the vehicle transducer data from the model and the test met two of the single channel comparisons and the multiple channel comparison. Thus, the baseline model for the simulation of the beginning and end of LON impacts on the F-shape PCB was selected to use the Chevy Silverado V2 vehicle model with the previously developed F-shape barrier model.


Time $=0.000 \mathrm{sec}$


Time $=0.100 \mathrm{sec}$


Time $=0.200 \mathrm{sec}$


Time $=0.300 \mathrm{sec}$
Figure 8. Overhead Sequential Views, Chevy Silverado V2 Model and Test No. 2214TB-2


Time $=0.400 \mathrm{sec}$


Time $=0.500 \mathrm{sec}$


Time $=0.600 \mathrm{sec}$


Time $=0.700 \mathrm{sec}$
Figure 9. Overhead Sequential Views, Chevy Silverado V2 Model and Test No. 2214TB-2


Time $=0.000 \mathrm{sec}$


Time $=0.100 \mathrm{sec}$


Time $=0.200 \mathrm{sec}$


Time $=0.300 \mathrm{sec}$
Figure 10. Downstream Sequential Views, Chevy Silverado V2 Model and Test No. 2214TB-2


Time $=0.500 \mathrm{sec}$


Time $=0.600 \mathrm{sec}$


$$
\text { Time }=0.700 \mathrm{sec}
$$

Figure 11. Downstream Sequential Views, Chevy Silverado V2 Model and Test No. 2214TB-2

(a) Longitudinal Velocity

(c) Vertical Velocity

(e) Pitch Angle

(b) Lateral Velocity

(d) Yaw Angle

(f) Roll Angle

Figure 12. RSVVP Results, Chevy Silverado V2 Impact with F-Shape PCB Model

## 5 EVALUATION OF LENGTH OF NEED

With the baseline simulation model of the sixteen barrier, 200-ft ( $61-\mathrm{m}$ ) long F-shape PCB system successfully calibrated against full-scale crash test no. $2214 \mathrm{~TB}-2$, the researchers began to use the baseline model to investigate the limits of the LON for the barrier system. A series of models were simulated that impacted each of the sixteen barrier segments in the system at a target impact point $4.3 \mathrm{ft}(1.3 \mathrm{~m})$ upstream of the joint between the adjacent segments. Due to computation instabilities with the truck model as it impacted the first barrier joint downstream of impact, some models were run with an impact point 12 in . or 24 in . ( 305 mm or 610 mm ) farther upstream in order to allow the simulations to run to completion. Barrier no. 16 was impacted midway along its length as there was no joint downstream of impact. The impact conditions for each simulation consisted of the 2270P vehicle impacting the barrier at a speed of 62 mph ( 100 $\mathrm{km} / \mathrm{h}$ ) and at an angle of 25 degrees. This corresponded to the MASH TL-3 impact conditions for test designation no. 3-11. Each of the simulations were analyzed to investigate a variety of parameters that would indicate the potential for safe vehicle redirection at that point along the length of the barrier system. These factors included:

1. Vehicle redirection
2. Vehicle climb
3. Vehicle stability (roll, pitch, and yaw)
4. Vehicle parallel time
5. Occupant risk (ORA and OIV)
6. Barrier pocketing - determined by the angle of the barrier prior to the vehicle contacting it
7. Displacement of the end barriers
8. Barrier roll (rotation of the barrier about its longitudinal axis)
9. Joint loads and pin deformation

The simulation of the various impact points was separated into two parts. Simulations of impacts along the first eight barrier segments of the $200-\mathrm{ft}(61-\mathrm{m})$ long barrier system were conducted to evaluate the beginning of LON, while impacts along the last eight barrier segments were conducted to evaluate the end of LON. Details of that analysis are provided below.

### 5.1 Beginning of Length of Need Simulations

The results from all of the simulations impacting the first eight barrier segments of the sixteen barrier, $200-\mathrm{ft}(61-\mathrm{m})$ long F-shape PCB system were compared to evaluate a potential beginning of LON point. Sequential photographs comparing the behavior of the PCB system at all eight impact points are shown in Figures 13 through 20. Review of the simulations found that the performance of the F-shape PCB system changed significantly when impacted closer to the upstream end of the barrier system. All of the impacts resulted in vehicle redirection. This was largely due to the inertial resistance of the barriers being sufficient to supply the primary redirective forces necessary to prevent gating of the barrier. Similarly, the time required for the vehicle to parallel the barrier during the impacts, the occupant risk values, and the vehicle climb of the barrier were consistent through all eight impacts. Vehicle stability for all of the impacts was acceptable, but vehicle roll tended to increase as the impact point moved upstream.

Barrier motions and deflections were directly affected as the impact of the vehicle neared the upstream end of the system. Maximum lateral barrier deflections, shown in Figure 21,
displayed only minor variations from impacts on the fifth through the eighth barriers in the PCB system. Impacts on the first four barriers of the system showed increasing lateral deflections as the impact approached the end of the system. This was a cause for concern due to increased lateral deflections potentially affecting vehicle stability as well as requiring larger clear areas behind the barrier system.

The maximum longitudinal displacement of the end barriers of the PCB system was also collected during the simulations, as shown in Figures 22 and 23. Large displacements of the end barriers indicated that the barrier system was potentially not providing sufficient tension upstream and downstream of the impact point and that barrier performance may be degraded. Longitudinal displacement of barrier no. 1 on the upstream end was most affected as the vehicle impacts approached the upstream end. Displacement of this barrier tended to increase as the impact point moved upstream. These increases were less severe when impacting barrier nos. 4 through 8 , but became larger when impacting the first three barriers of the system. While there was no quantitative limit for the end barrier displacement, the displacements observed for the impacts on the first three barriers in the system were concerning as they effectively tripled the displacement of the end barrier observed for the baseline impact at the midspan of the system. Longitudinal displacement of barrier no. 16 was not as drastically affected, but it was noted that the displacement of this barrier decreased as the impact point of the vehicle moved upstream.

Pocketing of the barrier ahead of the vehicle was not noted even with the increased barrier deflections. This was largely due to the vehicle redirection occurring early in the impact event due to the inertial resistance of the barrier when barrier deflections were small.

Impacts near the upstream end of the system, particularly barrier nos. 1 through 3, produced high levels of deformation in the connecting pin between the barrier segments. A comparison of the connecting pin deformation for the baseline, midspan impact simulation, and the impact of the vehicle on barrier no. 1 is shown in Figure 24. The connection pin in the simulation of the impact on barrier no. 2 showed a large degree of deformation in the regions where it was loaded by the barrier connection loops. This level of deformation was not observed in the baseline, midspan simulation nor was it observed in full-scale crash testing. Thus, the deformation of the pin indicated that the loading of the barrier joints was increasing for impacts near the end of the system.

(g) impact US of joint between barrier nos. 7 and 8
(h) impact US of joint between barrier nos. 8 and 9

Figure 13. Simulation of Beginning of LON for 16-Barrier F-Shape PCB System, Overhead View, $t=0.000 \mathrm{sec}$

(g) impact US of joint between barrier nos. 7 and 8
(h) impact US of joint between barrier nos. 8 and 9

Figure 14. Simulation of Beginning of LON for 16-Barrier F-Shape PCB System, Overhead View, $t=0.400 \mathrm{sec}$

$E$
(a) impact US of joint between barrier nos. 1 and 2


L
(c) impact US of joint between barrier nos. 3 and 4 퉁

(e) impact US of joint between barrier nos. 5 and 6 $\underset{\substack{\text { Noor pceregining of LoN-Long System } \\ \text { Times }}}{\substack{\text { Ess }}}$

£

(d) impact US of joint between barrier nos. 4 and 5

$L_{\infty}$
(f) impact US of joint between barrier nos. 6 and 7


$E$


L
(b) impact US of joint between barrier nos. 2 and 3



L
-2.
(h) impact US of joint between barrier nos. 8 and 9
(g) impact US of joint between barrier nos. 7 and 8

Figure 15. Simulation of Beginning of LON for 16-Barrier F-Shape PCB System, Overhead View, $t=0.800 \mathrm{sec}$

(h) impact US of joint between barrier nos. 8 and 9

Figure 16. Simulation of Beginning of LON for 16-Barrier F-Shape PCB System, Overhead View, $t=1.100 \mathrm{sec}$


Figure 17. Simulation of Beginning of LON for 16-Barrier F-Shape PCB System, Downstream View, $t=0.000 \mathrm{sec}$

(h) impact US of joint between barrier nos. 8 and 9

Figure 18. Simulation of Beginning of LON for 16-Barrier F-Shape PCB System, Downstream View, $t=0.400 \mathrm{sec}$


Figure 19. Simulation of Beginning of LON for 16-Barrier F-Shape PCB System, Downstream View, $t=0.800 \mathrm{sec}$

(a) impact US of joint between barrier nos. 1 and 2

(b) impact US of joint between barrier nos. 2 and 3 $\underset{\substack{\text { Noor pcesegining or LoN-Long System } \\ \text { Times }}}{\substack{1155}}$

$L$
(c) impact US of joint between barrier nos. 3 and 4


1
(e) impact US of joint between barrier nos. 5 and 6



L

(g) impact US of joint between barrier nos. 7 and 8


(d) impact US of joint between barrier nos. 4 and 5

$E$
(f) impact US of joint between barrier nos. 6 and 7



L
(h) impact US of joint between barrier nos. 8 and 9

Figure 20. Simulation of Beginning of LON for 16-Barrier F-Shape PCB System, Downstream View, $\mathrm{t}=1.100 \mathrm{sec}$


Figure 21. Beginning of LON Simulations, Maximum Lateral Barrier Deflections


Figure 22. Beginning of LON Simulations, Maximum Longitudinal Displacement of Barrier No. 1


Figure 23. Beginning of LON Simulations, Maximum Longitudinal Displacement of Barrier No. 16

(a) Impact at Barrier No. 1

WI F-shape TCB - Free-Standing
Time $=1200$

(b) Impact at Barrier No. 8 (Midspan)

Figure 24. Beginning of LON Connection Pin Deformation Comparison

### 5.2 End of Length of Need Simulations

The results from all of the simulations impacting the last eight barrier segments of the sixteen barrier, $200-\mathrm{ft}(61-\mathrm{m})$ long F-shape PCB system were compared to evaluate a potential end of LON point. Sequential views comparing the behavior of the PCB system at all eight impact points are shown in Figures 25 through 32. Review of the simulations found that the performance of the F-shape PCB system changed significantly when impacted closer to the downstream end of the barrier system. Impacts on barrier nos. 9 through 14 resulted in vehicle redirection. This was largely due to the inertial resistance of the barriers being sufficient to supply the primary redirective forces necessary to prevent gating of the barrier. However, impact on barrier nos. 15 and 16 resulted in large deflections of the final barrier segment that represented more of a gatingtype behavior for the end of the system. Gating of the system was also observed with respect to vehicle impact on barrier no. 14, but the 2270P vehicle was still effectively redirected in that impact prior to the large displacement of the end barrier segment. The time required for the vehicle to become parallel to the barrier during the impacts was similar for impacts on barrier nos. 9 through 15, but impact on barrier no. 16 yielded a delayed time to parallel due to the lack of downstream barriers and the gating of the end of the system. Occupant risk values were generally consistent for all of the impacts except barrier no. 16, which had much lower deceleration values due to the system gating and not redirecting the vehicle. Vehicle climb of the barrier was consistent through all the impacts. Vehicle stability for all of the impacts was acceptable, but vehicle roll and yaw tended to increase as the impact point moved downstream. Impacts on barrier nos. 12 through 15 displayed increased vehicle roll, while impacts on barrier nos. 15 and 16 yielded a significant increase in vehicle yaw. These increases in yaw and roll of the vehicle potentially indicated a concern for vehicle stability in these impacts on the downstream end of the system.

Barrier motions and deflections were also affected as the impact of the vehicle neared the downstream end of the system. Maximum lateral barrier deflections, shown in Figure 33, displayed only minor variations for impacts on barrier nos. 9 through 13 in the PCB system. Impacts on the last three barriers of the system showed much higher lateral deflections as the impact approached the end of the system. These lateral deflections were largely due to the gating behavior of the downstream end of the system noted previously. This was a cause for concern due to increased lateral deflections potentially affecting vehicle stability as well as requiring larger clear areas behind the barrier system.

The maximum longitudinal displacement of the end barriers of the PCB system was also collected during the simulations, as shown in Figures 34 and 35. Large displacements of the end barriers indicated that the barrier system was not potentially providing sufficient tension upstream and downstream of the impact point and that barrier performance may be degraded. Longitudinal displacement of barrier no. 16 on the downstream end was most affected as the vehicle impacts approached the downstream end. Displacement of this barrier tended to increase as the impact point moved downstream. These increases were less severe when impacting barrier nos. 9 through 12 , but became larger when impacting the last four barriers of the system. Impact on barrier nos. 14 through 16 resulted in gating of the end of the barrier, which generated large lateral deflections of the end barrier but not large longitudinal displacement. Longitudinal displacement of barrier no. 1 was not as drastically affected, but the displacement of this barrier decreased as the impact point of the vehicle moved upstream.


Figure 25. Simulation of End of LON for 16-Barrier F-Shape PCB System, Overhead View, $\mathrm{t}=0.000 \mathrm{sec}$


Figure 26. Simulation of End of LON for 16-Barrier F-Shape PCB System, Overhead View, $\mathrm{t}=0.400 \mathrm{sec}$


Figure 27. Simulation of End of LON for 16-Barrier F-Shape PCB System, Overhead View, $\mathrm{t}=0.800 \mathrm{sec}$


Figure 28. Simulation of End of LON for 16-Barrier F-Shape PCB System, Overhead View, $\mathrm{t}=1.100 \mathrm{sec}$


Figure 29. Simulation of End of LON for 16-Barrier F-Shape PCB System, Downstream View, $\mathrm{t}=0.000 \mathrm{sec}$


Figure 30. Simulation of End of LON for 16-Barrier F-Shape PCB System, Downstream View, $\mathrm{t}=0.400 \mathrm{sec}$


Figure 31. Simulation of End of LON for 16-Barrier F-Shape PCB System, Downstream View, $\mathrm{t}=0.800 \mathrm{sec}$


Figure 32. Simulation of End of LON for 16-Barrier F-Shape PCB System, Downstream View, $\mathrm{t}=1.100 \mathrm{sec}$


Figure 33. End of LON Simulations, Maximum Lateral Barrier Deflections


Figure 34. End of LON Simulations, Maximum Longitudinal Displacement of Barrier No. 1


Figure 35. End of LON Simulations, Maximum Longitudinal Displacement of Barrier No. 16

Pocketing of the barrier ahead of the vehicle was not noted even with the increased barrier deflections. This was largely the result of the vehicle redirection occurring early in the impact event due to the inertial resistance of the barrier when barrier deflections were small.

Finally, impacts near the downstream end of the system, particularly barrier nos. 14 and 15 , produced high levels of deformation in the connection pin between the barrier segments. A comparison of the connecting pin deformation for the simulation of the baseline model impacted at the midspan and the impact of the vehicle on barrier no. 15 is shown in Figure 36. The connection pin in the simulation of the impact on barrier no. 2 showed a large degree of deformation in the regions where it was loaded by the barrier connection loops. This level of deformation was not observed in the simulation of the baseline model impacted at the midspan, nor was it observed in full-scale crash testing. Thus, the deformation of the pin indicated that the loading of the barrier joints was increasing for impacts near the end of the system.

### 5.3 Selection of Beginning and End of LON for 16 PCB Simulations

Review of the data from the simulations of the beginning and end of LON for the F-shape PCB system with sixteen barrier segments raised concerns regarding impacts at the far upstream and downstream ends of the system. On the upstream end of the PCB system, impacts on the first three barrier segments produced increased lateral barrier deflections and longitudinal barrier displacements. While all of the simulated impacts on the upstream end of the system produced stable vehicle redirection, there was concern that the high levels of barrier displacement would put the PCB system at the limits of its performance and may induce vehicle stability issues not captured by the model. Simulations near the upstream end of the system displayed increased vehicle roll that supported this concern. Additionally, excessively large deflections may cause operational issues related to clear zones behind the displaced barrier segments. Deformations of the PCB connection pins were also increased for impacts on the first three barriers of the PCB system, which would indicate increased loading of the barrier joint. Based on these concerns, it was recommended that a minimum of three barrier segments be used to define the beginning of LON of the PCB system without further analysis prior to investigation of reduced system lengths.

Similarly, simulation of impacts on the downstream end of the system demonstrated potential concerns when impacting the final three barriers of the PCB system. Impacts on barrier nos. 14 through 16 caused the end of the barrier to gate and display significantly higher deflections as compared to impacts farther upstream. Additionally, impacts on the final three barriers had a combination of increased vehicle yaw and roll motions, which raised potential concerns for vehicle stability. Pin deformations indicate potentially increased loading of the barrier joint were also observed when impacting barrier nos. 14 and 15 . Based on these concerns and the improved performance of impacts farther upstream in the system, it was recommended that a minimum of three barrier segments be used to define the end of LON of the PCB system without further analysis prior to investigation of reduced system lengths.

NDOR PCB-Beginning of LON-Long System Time $=1200$

(a) Impact at Barrier No. 15

WI F-shape TCB - Free-Standing
Time $=1200$

(b) Impact at Barrier No. 8 (Midspan)

Figure 36. End of LON Connection Pin Deformation Comparison

## 6 EVALUATION OF REDUCED SYSTEM LENGTHS

Once beginning and end of LON locations were selected for the sixteen-barrier F-shape PCB system, the researchers investigated of reduced system length. It was recognized that the overall performance of the barrier system, especially when impacted at the beginning and end of LON, could change if system lengths were minimized. Thus, simulation models were conducted on reduced length PCB systems to determine the potential for the reduced length system to continue to perform safely and to recommend a system length for full-scale crash testing and evaluation. Based on the previous recommendations for the sixteen-barrier system of a minimum of three barriers to define beginning of LON and three barriers to define the end of LON, the researchers selected a seven-barrier long system for investigation. This length would provide the recommended three barrier segments on each end of the system and a single barrier in the middle of the system to provide a finite redirective length.

### 6.1 Seven Barrier F-Shape PCB System Simulations

Two simulations were conducted on a seven-barrier long F-shape PCB system with the 2270P vehicle under the MASH impact conditions for test designation no. 3-11. One simulation was run impacting $4.3 \mathrm{ft}(1.3 \mathrm{~m})$ upstream of the joint between barrier nos. 3 and 4 to evaluate the beginning of LON for the reduced length system, while a second simulation was run impacting 4.3 $\mathrm{ft}(1.3 \mathrm{~m})$ upstream of the joint between barrier nos. 4 and 5 to evaluate the end of LON for the reduced length system.

Simulation of the impact on the beginning of LON for the seven-barrier long system displayed acceptable results in terms of the barrier performance, as shown in Figures 37 and 38. The 2270 P vehicle was safely and smoothly redirected with vehicle stability that compared well with the baseline model of the original sixteen-barrier long PCB system. Occupant risk values for the simulation were well below the MASH limits. As would be expected, lateral and longitudinal barrier displacements increased significantly as compared to an impact near the midspan of the standard $200-\mathrm{ft}(61-\mathrm{m})$ system length used for full-scale crash testing. Peak lateral barrier deflections were found to be 95.3 in . $(2,420 \mathrm{~mm})$ at the downstream end of barrier no. 4 , while the longitudinal displacement of the barriers on the upstream and downstream ends of the system were found to be 27.3 in . 693 mm ) and $7.0 \mathrm{in} .(178 \mathrm{~mm})$, respectively. However, the peak lateral deflection was within 3 percent of the deflection of the standard length PCB system when impacted at the beginning of the LON.

It was noted that the reduced length and corresponding reduction in upstream and downstream tensile loads in the system altered the deflection of the PCB segments. Specifically, it was noted that a knee formed at the joint between barrier nos. 5 and 6 and impacted the rear, left-side door on the 2270 P vehicle as the vehicle traversed the joint, as shown in Figure 39. The formation of a knee between the barrier segments that impacted the side of the vehicle was not observed in simulations of the full-length systems nor had it been noted in full-scale testing. The impact of the knee on the rear, left-side door caused only moderate damage and did not affect vehicle stability or occupant risk values. As such, this was not believed to pose a serious degradation of the barrier performance. However, it did indicate that the reduced length of the system affected barrier behavior.


Figure 37. Simulation of Beginning of LON for 7-Barrier F-Shape PCB System, Overhead View

$亡$



L
(a) 0.000 sec
(b) 0.100 sec


$E$

(c) 0.200 sec


L
(d) 0.300 sec


L
(e) 0.400 sec



L
(f) 0.500 sec


L
(g) 0.600 sec


$L$
(h) 0.700 sec

Figure 38. Simulation of Beginning of LON for 7-Barrier F-Shape PCB System, Downstream View


Figure 39. Simulation of Beginning of LON for 7-Barrier F-Shape PCB System, Barrier Knee Impact at Barrier Nos. 5 and 6 Joint

Simulation of the impact on the end of LON for the seven-barrier long system raised potential concerns regarding the use of the shorter system length. Sequential images of the sevenbarrier F-shape PCB system impacted at the proposed end of LON are shown in Figures 40 and 41. The 2270 P vehicle was redirected, and occupant risk values for the simulation were below the MASH limits. Peak lateral barrier deflections were 96.5 in . $(2,451 \mathrm{~mm})$ at the upstream end of barrier no. 6, while the longitudinal displacement of the barriers on the upstream and downstream ends of the system were found to be 17.2 in . $(437 \mathrm{~mm}$ ) and 23.2 in . $(589 \mathrm{~mm}$ ), respectively. Of more concern was the vehicle interaction with the barrier as it reached the end of the system. At .630 s after impact, the vehicle was proceeding past the final barrier in the PCB system when the final barrier in the system rotated into the left-side door, as shown in Figure 42. The motion of the PCB segments downstream of impact in the reduced-length system changed as compared to the full length system simulated previously due to the difference in longitudinal resistance provided on the upstream end of the system. This resulted in more pronounced rotation of the end barrier that caused the end of the barrier segment to impact the left-side door. Impact of the end of the barrier with the door in the simulation caused significant damage to the door and raised concerns with respect to occupant compartment safety, occupant risk concerns, and potential degradation of vehicle stability. Review of these results with the project sponsor verified these concerns, and it was desired to mitigate the potential for impact of the end barrier segment on the vehicle.


Figure 40. Simulation of End of LON for 7-Barrier F-Shape PCB System, Overhead View


Figure 41. Simulation of End of LON for 7-Barrier F-Shape PCB System, Downstream View

NDOR PCB-Reduced Length-7 Barriers Time $=655$


Figure 42. Simulation of End of LON for 7-Barrier F-Shape PCB System, Final Barrier Impact on Driver-Side Door

### 6.2 Eight-Barrier F-Shape PCB System Simulations

Based on the concerns with the door impact observed in the seven-barrier PCB system simulations, the researchers conducted additional simulation models on an eight-barrier long PCB system. In this system, three PCB segments were used for the beginning of LON, four PCB segments were used for the end of LON, and a single barrier segment was placed between the regions to provide a finite redirective length. It was believed that the use of an additional PCB segment in the end of LON region would mitigate the door impact observed in the seven PCB system simulation.

Two simulations were conducted on an eight-barrier long, F-shape PCB system with the 2270P vehicle under the MASH impact conditions for test designation no. 3-11. One simulation was run impacting $4.3 \mathrm{ft}(1.3 \mathrm{~m})$ upstream of the joint between barrier nos. 3 and 4 to evaluate the beginning of LON for the reduced length system, while a second simulation was run impacting 4.3 $\mathrm{ft}(1.3 \mathrm{~m})$ upstream of the joint between barrier nos. 4 and 5 to evaluate the end of LON for the reduced length system.

Simulation of the impact on the beginning of LON for the eight-barrier long system displayed acceptable results in terms of the barrier performance, as shown in Figures 43 and 44. The 2270 P vehicle was safely and smoothly redirected, and occupant risk values for the simulation were below the MASH limits. Peak lateral barrier deflections were 94.8 in . ( $2,408 \mathrm{~mm}$ ) at the downstream end of barrier no. 4, while the longitudinal displacement of the barriers on the upstream and downstream ends of the system were found to be 28.7 in . ( 729 mm ) and 2.9 in . (74 mm ), respectively. The reduced length of the barrier system again allowed formation of a knee at the joint between barrier nos. 5 and 6 that impacted the side of the 2270 P vehicle and produced similar damage as the previous simulation of the beginning of LON impact with a seven-barrier PCB system, as shown in Figure 45.

Simulation of the impact on the end of LON for the eight-barrier long system displayed improved performance as compared to the seven-barrier long system. Sequential images of the eight F-shape PCB system impacted at the proposed end of LON are shown in Figures 46 and 47. The 2270P vehicle was redirected, and occupant risk values for the simulation were below the MASH limits. Peak lateral barrier deflections were 90.0 in . $(2,286 \mathrm{~mm})$ at the downstream end of barrier no. 5, while the longitudinal displacement of the barriers on the upstream and downstream ends of the system were found to be 18.0 in . ( 458 mm ) and 12.5 in . ( 318 mm ), respectively. The use of an additional barrier on the end of the system mitigated the impact of the free-end of the final barrier segment with the side of the 2270 P vehicle. However, it was noted that a knee formed at the joint between barrier nos. 6 and 7 and impacted the left side of the vehicle, as shown in Figure 48. The impact of the knee formed between these barrier segments posed less concern as the severity and damage associated with the vehicle contact with the knee appeared to be significantly less than the damage observed due to the rotation of the free end of the system into the door observed in the seven-barrier PCB system simulation.


Figure 43. Simulation of Beginning of LON for 8-Barrier F-Shape PCB System, Overhead View


Figure 44. Simulation of Beginning of LON for 8-Barrier F-Shape PCB System, Downstream View


Figure 45. Simulation of Beginning of LON for 8-Barrier F-Shape PCB System, Barrier Knee Impact at Barrier Nos. 5 and 6 Joint


Figure 46. Simulation of End of LON for 8-Barrier F-Shape PCB System, Overhead View


Figure 47. Simulation of End of LON for 8-Barrier F-Shape PCB System, Downstream View


Figure 48. Simulation of End of LON for 8-Barrier F-Shape PCB System, Barrier Knee Impact at Barrier Nos. 6 and 7 Joint

### 6.3 Selection of System Length for Full-Scale Testing

The simulations of the reduced length F-shape PCB systems found that a seven-barrier long system was capable of redirecting the 2270P vehicle under the MASH TL-3 impact conditions, albeit with an increase in barrier deflections over those observed in midspan impacts with the standard sixteen-barrier long system evaluated previously in full-scale testing. However, impact near the end of LON of the seven barrier system showed a potential for the final barrier in the system to rotate and impact the left-side door, and raised concerns for the overall safety performance of the seven-barrier long system. To address this issue, an additional barrier was placed on the end of the system which increased the total system length to eight barriers. Simulation of the eight-barrier long PCB system demonstrated an improved response as the vehicle was safely redirected in both simulated impacts, and the rotation of the free end of the final barrier of the system was no longer able to impact the side of the vehicle. It was noted that a knee formed at the joint between barrier nos. 7 and 8 and still impacted the side of the vehicle. Similar knee formation and impact with the side of the vehicle was also observed in the beginning of LON impacts on both the seven and eight-barrier long systems. While the impact of the knee with the side of the 2270 P vehicle caused moderate concern, the contact appeared to be less severe than the contact from the free barrier end in the seven-barrier long system. As such, it was decided to proceed with evaluation of an eight-barrier long F-shape PCB system under the MASH TL-3 criteria.

## 7 DESIGN DETAILS

The barrier system test installations were comprised of eight $12-\mathrm{ft} 6-\mathrm{in}$. ( $3.81-\mathrm{m}$ ) long, rebar reinforced, F-shape portable concrete barriers. As the barrier system was identical for test nos. NELON-1 and NELON-2, only the design drawing depicting the targeted impact point is shown for NELON-2. The barrier system components for test nos. NELON-1 and NELON-2 are shown in Figures 51 through 55 and the barrier system layouts for test nos. NELON-1 and NELON-2 are shown in Figures 49 and 50, respectively. Photographs of the test installations are shown in Figures 56 and 57. Material specifications, mill certifications, and certificates of conformity for the system materials are shown in Appendix A.

The F-shape PCB segments were 12-ft 6-in. (3.81-m) long F-shape PCBs and constructed with a $5,000 \mathrm{psi}(34.5 \mathrm{MPa})$ minimum compressive strength concrete. The barrier segments were $22 \frac{1}{2} \mathrm{in}$. ( 572 mm ) wide at the base and 8 in . $(203 \mathrm{~mm})$ wide at the top. Each of the barrier segments were connected by $1 \frac{1}{4}-\mathrm{in}$. ( $32-\mathrm{mm}$ ) diameter A36 steel connection pins and connection pin plates placed between $3 / 4-\mathrm{in}$. ( $19-\mathrm{mm}$ ) diameter, epoxy coated reinforcing bar loops extending from the end of the barrier sections. The connection loop bar material was A709 Grade 70 or A706 Grade 60 steel. All PCB segments were installed on the concrete tarmac at the MwRSF outdoor test facility.


PLAN VIEW

elevation view

Notes: (1) Test shall be performed according to test designation no. 3-35 of MASH criteria.
(2) Impact point is $513 / 16^{\prime \prime}(1.3 \mathrm{~m})$ upstream from the centerline of
the joint between barrier nos. 3 and 4 .
(3) The ends of the barrier should be pulled until there is no slack in the joints.


Figure 49. System Layout, Test No. NELON-1

## ELEVATION VIEW

Notes: (1) Test shall be performed according to test designation no. 3-35 of MASH criteria.
(2) Impact point is $513 / 16^{\prime \prime}(1.3 \mathrm{~m})$ upstream from the centerline of the joint between barrier nos. 4 and 5 .
(3) The ends of the barrier should be pulled until there is no slack in the joints.


Figure 50. System Layout, Test No. NELON-2


Figure 51. Portable Concrete Barrier, Test Nos. NELON-1 and NELON-2


Figure 52. Portable Concrete Barrier Profile Detail, Test Nos. NELON-1 and NELON-2


Figure 53. Bill of Bars - Portable Concrete Barriers, Test Nos. NELON-1 and NELON-2


Figure 54. Connection Pin Detail, Test Nos. NELON-1 and NELON-2

| Item No. | QTY. | Description | MaterialSpec | Hardware Guide |
| :---: | :---: | :---: | :---: | :---: |
| d1 | 8 | NDOR Portable Concrete Barrier | min f 'c=5000 psi [34.5 MPa] | SWC09 |
| d2 | 7 | 1 1/4" [32] Dia., 28" [711] Long Connector Pin | ASTM A36 | FMWO2 |
| d3 | 96 | 1/2" [13] Dia., 72" [1829] Long Form Bar | ASTM A615 Grade 60 | - |
| d4 | 16 | 1/2" [13] Dia., 146" [3708] Long Longitudinal Bar | ASTM A615 Grade 60 | - |
| d5 | 24 | 5/8" [16] Dia., 146" [3708] Long Longitudinal Bar | ASTM A615 Grade 60 | - |
| d6 | 48 | 3/4" [19] Dia., 36" [914] Long Anchor Loop Bar | ASTM A615 Grade 60, Epoxy Coated or Galvanized | - |
| d7 | 16 | 3/4" [19] Dia., 102" [2591] Long Connection Loop Bar | ASTM A709 Grade 70 or A706 Grade 60, Epoxy Coated or | - |
| d8 | 16 | 3/4" [19] Dia., 91" [2311] Long Connection Loop Bar | ASTM A709 Grade 70 or A706 Grade 60, Epoxy Coated or | - |
| d9 | 16 | 3/4" [19] Dia., 101" [2565] Long Connection Loop Bar | ASTM A709 Grade 70or A706 Grade 60, Epoxy Coated or <br> Galvanized | - |




Figure 56. Test Installation Photographs, Test No. NELON-1


## 8 TEST REQUIREMENTS AND EVALUATION CRITERIA

### 8.1 Test Requirements

Terminals and redirective crash cushions, such as the free-standing, F-shape PCB system, must satisfy impact safety standards in order to be declared eligible for federal reimbursement by the Federal Highway Administration (FHWA) for use on the National Highway System (NHS). For new hardware, these safety standards consist of the guidelines and procedures published in MASH [2]. According to the TL-3 safety performance criteria of MASH, terminals and redirective crash cushions must be subjected to nine full-scale vehicle crash tests. However, since this investigation did not involve a crash cushion or terminal and was solely focused on evaluating the beginning and end of the shortest length of need of the PCB system, only three full-scale vehicle crash tests were valid for evaluation of the system, as summarized in Table 2.

Table 2. MASH TL-3 Crash Test Conditions for Terminals and Crash Cushions

| Test Article | Test <br> Designation No. | Test Vehicle | Vehicle Weight, lb (kg) | Impact Conditions |  | Evaluation Criteria ${ }^{1}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | $\begin{gathered} \text { Speed, } \\ \mathrm{mph} \\ (\mathrm{~km} / \mathrm{h}) \end{gathered}$ | Angle, deg. |  |
| Terminals and Redirective Crash Cushions | 3-34 | 1100C | $\begin{gathered} 2,425 \\ (1,100) \\ \hline \end{gathered}$ | $\begin{gathered} 62 \\ (100) \\ \hline \end{gathered}$ | 25 | A,D,F,H,I |
|  | 3-35 | 2270P | $\begin{gathered} 5,000 \\ (2,270) \\ \hline \end{gathered}$ | $\begin{gathered} 62 \\ (100) \\ \hline \end{gathered}$ | 25 | A,D,F,H,I |
|  | 3-37 | 2270P | $\begin{gathered} \hline 5,000 \\ (2,270) \\ \hline \end{gathered}$ | $\begin{gathered} 62 \\ (100) \\ \hline \end{gathered}$ | 25 | A,D,F,H,I |

${ }^{1}$ Evaluation criteria explained in Table 3.

The first test would consist of MASH test designation no. 3-35. This test involves an impact with a 2270 P vehicle at a speed of $62 \mathrm{mph}(100 \mathrm{~km} / \mathrm{h})$ and at an angle of 25 degrees on the beginning of the LON. This test would evaluate the effectiveness of the beginning of LON with a minimal system length. The second test would consist of a modified version of MASH test designation no. 3-37 with the intent of assessing the end of the LON for the PCB system rather than maximizing vehicle snag and instability on a terminal or crash cushion. This test involves an impact with a 2270 P vehicle at a speed of $62 \mathrm{mph}(100 \mathrm{~km} / \mathrm{h})$ and at an angle of 25 degrees on a critical impact point near the downstream end of the system. The system length and number of barrier segments on the beginning and end of the LON for both tests were based on the guidance determined during the simulation effort. The critical impact points were selected based on Table 2-6 of MASH and the beginning and end of LON. Thus, the impact point for test designation no. $3-35$ would be $4.3 \mathrm{ft}(1.3 \mathrm{~m})$ upstream of the joint between the third and fourth barrier segments, while the impact point for test designation no. 3-37 would be $4.3 \mathrm{ft}(1.3 \mathrm{~m})$ upstream of the joint between the fourth and fifth barrier segments.

Test designation no. 3-34 with the 1100 C vehicle would not be necessary based on comparison of barrier geometry with previous concrete barrier systems and the intended rationale for the test. With respect to previous testing, in test no. 7069-3, a rigid, F-shape bridge rail was
successfully impacted by a small car weighing $1,800 \mathrm{lb}(816 \mathrm{~kg})$ at $60.1 \mathrm{mph}(96.7 \mathrm{~km} / \mathrm{h})$ and 21.4 degrees according to the American Association of State Highway and Transportation Officials (AASHTO) Guide Specifications for Bridge Railings [11-12]. In the same manner, test nos. CMB5 through CMB-10, CMB-13, and 4798-1 showed that rigid New Jersey safety shape barriers struck by small cars meet safety performance standards [13-14]. In addition, in test no. 2214NJ-1, a New Jersey safety shape barrier was impacted by a passenger car weighing $2,579 \mathrm{lb}(1,170 \mathrm{~kg})$ at $60.8 \mathrm{mph}(97.8 \mathrm{~km} / \mathrm{h})$ and 26.1 degrees according to the TL-3 standards set forth in MASH [15]. Furthermore, temporary New Jersey safety shape concrete median barriers have experienced only slight barrier deflections when impacted by small cars and behave similarly to rigid barriers, as seen in test no. 47 [16].

Additionally, test designation no. 3-34 is intended to evaluate the impact performance of terminals and crash cushions at the critical impact point where the behavior of the device changes from gating or capturing to redirection. Vehicle trajectory and occupant risk are the main concerns for this test. However, the PCB system evaluated herein does not use a fixed anchorage or other element to provide redirective forces at the beginning or end of LON, but rather relies on the inertia of the PCB segments and membrane tensile forces generated by the mass and corresponding friction of adjacent barrier segments. Additionally, the potential for gating or excessive deflection of the beginning or end of LON for the PCB system was expected due to the heavier 2270P vehicle rather than the lower weight 1100 C vehicle. Thus, the critical impact point for the system as defined for test designation no. 3-34 would likely be upstream of the beginning of LON defined by the 2270 P test. As the scope of this study did not extend into determining proper termination of the system outside of the LON, test designation no. 3-34 was believed to be unnecessary to evaluate the F-shape PCB minimum length of need and reduced system length.

It should be noted that the test matrix detailed herein represents the researchers' best engineering judgement with respect to the MASH safety requirements and their internal evaluation of critical tests necessary to evaluate the crashworthiness of the barrier system. However, the recent switch to new vehicle types as part of the implementation of the MASH criteria and the lack of experience and knowledge regarding the performance of the new vehicle types with certain types of hardware could result in unanticipated barrier performance. Thus, any tests within the evaluation matrix deemed non-critical may eventually need to be evaluated based on additional knowledge gained over time or revisions to the MASH criteria.

### 8.2 Evaluation Criteria

Evaluation criteria for full-scale vehicle crash testing are based on three appraisal areas: (1) structural adequacy; (2) occupant risk; and (3) vehicle trajectory after collision. Criteria for structural adequacy are intended to evaluate the ability of the portable concrete barrier to contain and redirect impacting vehicles. In addition, controlled lateral deflection of the test article is acceptable. Occupant risk evaluates the degree of hazard to occupants in the impacting vehicle. Post-impact vehicle trajectory is a measure of the potential of the vehicle to result in a secondary collision with other vehicles and/or fixed objects, thereby increasing the risk of injury to the occupants of the impacting vehicle and/or other vehicles. These evaluation criteria are summarized in Table 3 and defined in greater detail in MASH. The full-scale vehicle crash tests were conducted and reported in accordance with the procedures provided in MASH.

In addition to the standard occupant risk measures, the Post-Impact Head Deceleration (PHD), the Theoretical Head Impact Velocity (THIV), and the Acceleration Severity Index (ASI) were determined and reported on the test summary sheet. Additional discussion on PHD, THIV, and ASI is provided in MASH.

Table 3. MASH Evaluation Criteria for Terminals and Crash Cushions

| Structural <br> Adequacy | A. | Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable. |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Occupant Risk | D. | Detached elements, fragments or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone. Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.3 and Appendix E of MASH. |  |  |
|  | F. | The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees. |  |  |
|  | H. | Occupant Impact Velocity (OIV) (see Appendix A, Section A5.3 of MASH for calculation procedure) should satisfy the following limits: |  |  |
|  |  | Occupant Impact Velocity Limits |  |  |
|  |  | Component | Preferred | Maximum |
|  |  | Longitudinal and Lateral | $\begin{gathered} \hline 30 \mathrm{ft} / \mathrm{s} \\ (9.1 \mathrm{~m} / \mathrm{s}) \\ \hline \end{gathered}$ | $\begin{gathered} 40 \mathrm{ft} / \mathrm{s} \\ (12.2 \mathrm{~m} / \mathrm{s}) \\ \hline \end{gathered}$ |
|  | I. | The Occupant Ridedown Acceleration (ORA) (see Appendix A, Section A5.3 of MASH for calculation procedure) should satisfy the following limits: |  |  |
|  |  | Occupant Ridedown Acceleration Limits |  |  |
|  |  | Component | Preferred | Maximum |
|  |  | Longitudinal and Lateral | 15.0 g's | 20.49 g's |

## 9 TEST CONDITIONS

### 9.1 Test Facility

The testing facility is located at the Lincoln Air Park on the northwest side of the Lincoln Municipal Airport and is approximately 5 miles ( 8.0 km ) northwest of the University of NebraskaLincoln.

### 9.2 Vehicle Tow and Guidance System

A reverse-cable, tow system with a $1: 2$ mechanical advantage was used to propel the test vehicle. The distance traveled and the speed of the tow vehicle were one-half that of the test vehicle. The test vehicle was released from the tow cable before impact with the barrier system. A digital speedometer was used on the tow vehicle to increase the accuracy of the test vehicle's impact speed.

A vehicle guidance system that was developed by Hinch [17] was used to steer the test vehicle. A guide flag, attached to the right-front wheel and the guide cable, was sheared off before impact with the barrier system. The $3 / 8$-in. $(9.5-\mathrm{mm})$ diameter guide cable was tensioned to approximately $3,500 \mathrm{lb}(15.6 \mathrm{kN})$ and supported both laterally and vertically every $100 \mathrm{ft}(30.5 \mathrm{~m})$ by hinged stanchions. The hinged stanchions stood upright while holding up the guide cable. As the vehicle was towed down the line, the guide flag struck and knocked each stanchion to the ground.

### 9.3 Test Vehicles

For test no. NELON-1, a 2008 Dodge Ram was used as the test vehicle. The curb, test inertial, and gross static vehicle weights were $4,833 \mathrm{lb}(2,192 \mathrm{~kg}), 4,991 \mathrm{lb}(2,264 \mathrm{~kg})$, and 5,148 $\mathrm{lb}(2,335 \mathrm{~kg})$, respectively. The test vehicle is shown in Figure 58, and vehicle dimensions are shown in Figure 59.

For test no. NELON-2, a 2008 Dodge Ram was also used as the test vehicle. The curb, test inertial, and gross static vehicle weights were $5,036 \mathrm{lb}(2,284 \mathrm{~kg}), 5,005 \mathrm{lb}(2,270 \mathrm{~kg})$, and 5,161 $\mathrm{lb}(2,341 \mathrm{~kg})$, respectively. The test vehicle is shown in Figure 60, and vehicle dimensions are shown in Figure 61.

The longitudinal component of the center of gravity (c.g.) was determined using the measured axle weights. The Suspension Method [18] was used to determine the vertical component of the c.g. for the pickup truck. This method is based on the principle that the c.g. of any freely suspended body is in the vertical plane through the point of suspension. The vehicle was suspended successively in three positions, and the respective planes containing the c.g. were established. The intersection of these planes pinpointed the final c.g. location for the test inertial condition. The location of the final c.g. is shown in Figures 59 and 61. Data used to calculate the location of the c.g. and ballast information is shown in Appendix B.


Figure 58. Test Vehicle, Test No. NELON-1


Figure 59 Vehicle Dimensions, Test No. NELON-1


Figure 60. Test Vehicle, Test No. NELON-2


Figure 61. Vehicle Dimensions, Test No. NELON-2

Square, black-and white-checkered targets were placed on the vehicle for reference to be viewed from the high-speed digital video cameras and aid in the video analysis, as shown in Figures 62 and 63. Round, checkered targets were placed on the c.g. on the left-side door, the rightside door, and the roof of the vehicle.

The front wheels of the test vehicle were aligned to vehicle standards except the toe-in value was adjusted to zero so that the vehicles would track properly along the guide cable. A 5B flash bulb was mounted under the vehicle's left-side windshield wiper and was fired by a pressure tape switch mounted at the impact corner of the bumper. The flash bulb was fired upon initial impact with the test article to create a visual indicator of the precise time of impact on the highspeed videos. A remote controlled brake system was installed in the test vehicle so the vehicle could be brought safely to a stop after the test.

### 9.4 Simulated Occupant

For test nos. NELON-1 and NELON-2, A Hybrid II 50 ${ }^{\text {th }}$-Percentile, Adult Male Dummy, equipped with clothing and footwear, was placed in the left-front seat of the test vehicle with the seatbelt fastened. The dummy, which had a final weight of $156 \mathrm{lb}(70 \mathrm{~kg})$ for test no. NELON-1 and $157 \mathrm{lb}(71 \mathrm{~kg})$ for test no. NELON-2, was represented by model no. 572 , serial no. 451 , and was manufactured by Android Systems of Carson, California. As recommended by MASH, the dummy was not included in calculating the c.g. location.

### 9.5 Data Acquisition Systems

### 9.5.1 Accelerometers

Two environmental shock and vibration sensor/recorder systems were used to measure the accelerations in the longitudinal, lateral, and vertical directions. All of the accelerometers were mounted near the c.g. of the test vehicles. The electronic accelerometer data obtained in dynamic testing was filtered using the SAE Class 60 and the SAE Class 180 Butterworth filter conforming to the SAE J211/1 specifications [7].

The two systems, the SLICE-1 and SLICE-2 units, were modular data acquisition systems manufactured by Diversified Technical Systems, Inc. (DTS) of Seal Beach, California. The acceleration sensors were mounted inside the bodies of custom built SLICE 6DX event data recorders and recorded data at $10,000 \mathrm{~Hz}$ to the onboard microprocessor. Each SLICE 6DX was configured with 7 GB of non-volatile flash memory, a range of $\pm 500 \mathrm{~g}$ 's, a sample rate of 10,000 Hz , and a $1,650 \mathrm{~Hz}$ (CFC 1000) anti-aliasing filter. The "SLICEWare" computer software program and a customized Microsoft Excel worksheet were used to analyze and plot the accelerometer data.

### 9.5.2 Rate Transducers

Two identical angle rate sensor systems mounted inside the bodies of the SLICE-1 and SLICE-2 event data recorders were used to measure the rates of rotation of the test vehicle. Each SLICE MICRO Triax ARS had a range of 1,500 degrees/sec in each of the three directions (roll, pitch, and yaw) and recorded data at $10,000 \mathrm{~Hz}$ to the onboard microprocessors. The raw data measurements were then downloaded, converted to the proper Euler angles for analysis, and
plotted. The "SLICEWare" computer software program and a customized Microsoft Excel worksheet were used to analyze and plot the angular rate sensor data.

### 9.5.3 Retroreflective Optic Speed Trap

The retroreflective optic speed trap was used to determine the speed of the vehicle before impact. Five retroreflective targets, spaced at approximately $18-\mathrm{in}$. ( $457-\mathrm{mm}$ ) intervals, were applied to the side of the vehicle. When the emitted beam of light was reflected by the targets and returned to the Emitter/Receiver, a signal was sent to the data acquisition computer, recording at $10,000 \mathrm{~Hz}$, as well as the external LED box activating the LED flashes. The speed was then calculated using the spacing between the retroreflective targets and the time between the signals. LED lights and high-speed digital video analysis are only used as a backup in the event that vehicle speeds cannot be determined from the electronic data.

### 9.5.4 Digital Photography

Five AOS high-speed digital video cameras, eight GoPro digital video cameras, and two JVC digital video cameras were utilized to film test nos. NELON-1 and NELON-2. Camera details, camera operating speeds, lens information, and schematics of the camera locations relative to the systems are shown in Figures 64 and 65.

The high-speed videos were analyzed using ImageExpress MotionPlus and RedLake MotionScope software programs. Actual camera speed and camera divergence factors were considered in the analysis of the high-speed videos. A Nikon D3200 digital still camera was also used to document pre- and post-test conditions for all tests.


Figure 62. Target Geometry, Test No. NELON-1


Figure 63. Target Geometry, Test No. NELON-2


| No. | Type | Operating Speed <br> (frames/sec) | Lens | Lens Setting |
| :---: | :---: | :---: | :---: | :---: |
| AOS-5 | AOS X-PRI Gigabit | 500 | Vivitar 135 mm Fixed | - |
| AOS-6 | AOS X-PRI Gigabit | 500 | Fujinon 50 mm | - |
| AOS-7 | AOS X-PRI Gigabit | 500 | Sigma 28-70 DG | - Sigma 28-70 |
| AOS-8 | AOS S-VIT 1531 | 500 | Kowa 12 mm Fixed | 30 |
| AOS-9 | AOS TRI-VIT 2236 | 500 |  | - |
| GP-3 | GoPro Hero 3+ | 120 |  |  |
| GP-4 | GoPro Hero 3+ | 120 |  |  |
| GP-5 | GoPro Hero 3+ | 120 |  |  |
| GP-6 | GoPro Hero 3+ | 120 |  |  |
| GP-7 | GoPro Hero 4 | 120 |  |  |
| GP-8 | GoPro Hero 4 | 120 |  |  |
| GP-9 | GoPro Hero 4 | 120 |  |  |
| GP-10 | GoPro Hero 4 | 120 |  |  |
| JVC-3 | JVC - GZ-MG27u (Everio) | 29.97 |  |  |
| JVC-4 | JVC - GZ-MG27u (Everio) | 29.97 |  |  |

Figure 64. Camera Locations, Speeds, and Lens Settings, Test No. NELON-1


Figure 65. Camera Locations, Speeds, and Lens Settings, Test No. NELON-2

## 10 FULL-SCALE CRASH TEST NO. NELON-1

### 10.1 Weather Conditions

Test no. NELON-1 was conducted on March 3, 2016 at approximately 1:30 p.m. The weather conditions as per the National Oceanic and Atmospheric Administration (station 14939/LNK) were reported and are shown in Table 4.

Table 4. Weather Conditions, Test No. NELON-1

| Temperature | $46^{\circ} \mathrm{F}$ |
| :--- | :--- |
| Humidity | $52 \%$ |
| Wind Speed | 15 mph |
| Wind Direction | $0^{\circ}$ from True North |
| Sky Conditions | Cloudy |
| Visibility | 10 Statute Miles |
| Pavement Surface | Dry |
| Previous 3-Day Precipitation | 0 in. |
| Previous 7-Day Precipitation | 0 in. |

### 10.2 Test Description

The 4,991-lb (2,264-kg) pickup truck impacted the portable concrete barrier system at a speed of $62.1 \mathrm{mph}(100.0 \mathrm{~km} / \mathrm{h})$ and at an angle of 24.8 degrees. A summary of the test results and sequential photographs are shown in Figure 66. Additional sequential photographs are shown in Figures 67 and 68. Documentary photographs of the crash test are shown in Figures 69 and 70.

Initial vehicle impact was to occur $51^{3 / 16}$ in. $(1,300 \mathrm{~mm})$ upstream from the centerline of the joint between barrier nos. 3 and 4, as shown in Figure 71. The impact point was selected based on LS-DYNA simulation of the beginning of the LON for the reduced length PCB system and MASH guidance for the critical impact point on PCB systems. The actual point of impact was $48^{11 / 16}$ in. $(1,237 \mathrm{~mm})$ upstream from the centerline of the joint between barrier nos. 3 and 4 . A sequential description of the impact events is contained in Table 5. Following the initial impact, the 2270P vehicle was captured and safely redirected by the barrier system. The vehicle came to rest $191 \mathrm{ft}-9$ in ( 58.4 m ) downstream of the initial impact point and $9 \mathrm{ft}-8 \mathrm{in}$. ( 2.9 m ) in front of the front face of the barrier system. The vehicle trajectory and final position are shown in Figures 66 and 72 , respectively.

Table 5. Sequential Description of Impact Events, Test No. NELON-1

| TIME <br> $(\mathrm{sec})$ | EVENT |
| :---: | :--- |
| 0.000 | Vehicle's left-front bumper contacted barrier no. 3. |
| 0.002 | Vehicle's left-front bumper deformed, and vehicle's left-front tire contacted <br> barrier no. 3. |


| 0.008 | Vehicle's left headlight deformed. |
| :---: | :---: |
| 0.010 | Vehicle's left quarter panel deformed. |
| 0.016 | Vehicle's left-front tire lost contact with ground, and downstream end of barrier no. 3 deflected backward. |
| 0.022 | Vehicle's left-front door flexed away from frame at upper rear corner. |
| 0.024 | Vehicle's grille and engine hood deformed. |
| 0.028 | Upstream end of barrier no. 4 deflected backward. |
| 0.038 | Downstream end of barrier no. 4 deflected forward. |
| 0.042 | Vehicle pitched upward. |
| 0.044 | Vehicle's left-rear door flexed away from frame at upper rear corner, and vehicle yawed away from barrier. |
| 0.048 | Upstream end of barrier no. 5 deflected forward. |
| 0.054 | Vehicle rolled toward barrier system, and vehicle's left headlight shattered and disengaged from vehicle. |
| 0.056 | Downstream end of barrier no. 5 deflected backward. |
| 0.058 | Airbags deployed. |
| 0.062 | Barrier no. 5 deflected upstream. |
| 0.064 | Barrier no. 3 deflected downstream. |
| 0.066 | Barrier no. 3 rotated counterclockwise. |
| 0.068 | Barrier no. 2 deflected downstream. |
| 0.072 | Barrier no. 6 rotated clockwise. |
| 0.074 | Vehicle's right-front tire became airborne. |
| 0.078 | Downstream end of barrier no. 2 deflected forward. |
| 0.084 | Barrier no. 1 deflected downstream. |
| 0.118 | Barrier no. 6 deflected upstream. |
| 0.120 | Barrier no. 7 deflected downstream. |
| 0.134 | Upstream end of barrier no. 3 deflected backward. |
| 0.144 | Upstream end of barrier no. 6 deflected forward. |
| 0.164 | Upstream end of barrier no. 2 deflected forward. |
| 0.194 | Vehicle was parallel to system at a speed of $50.3 \mathrm{mph}(80.9 \mathrm{~km} / \mathrm{h})$. |
| 0.200 | Vehicle's right-rear tire became airborne. |
| 0.202 | Downstream end of barrier no. 6 deflected backward. |
| 0.204 | Downstream end of barrier no. 1 deflected forward. |
| 0.232 | Upstream end of barrier no. 7 deflected backward. |
| 0.272 | Vehicle's left-rear quarter panel contacted barrier no. 4 and deformed, and vehicle's left taillight contacted barrier no. 4 and deformed. |
| 0.278 | Left taillight disengaged from vehicle. |


| 0.298 | Vehicle pitched downward. |
| :--- | :--- |
| 0.312 | Barrier no. 4 deflected downstream. |
| 0.378 | Vehicle's left-front tire regained contact with ground. |
| 0.424 | Vehicle's left-front door impacted knee formed by joint between barrier nos. 5 <br> and 6. |
| 0.454 | Vehicle's left-rear door impacted knee formed by joint between barrier nos. 5 and <br> 6. |
| 0.542 | Vehicle lost contact with the system at a speed of $44.8 \mathrm{mph}(72 \mathrm{~km} / \mathrm{h})$ and a 12.3 <br> degree angle. |
| 0.550 | Vehicle's left rear tire was airborne. |
| 0.692 | Vehicle pitched upward. |
| 0.768 | Vehicle rolled away from barrier. |
| 1.066 | Vehicle's right-front tire regained contact with ground. |
| 1.180 | Vehicle's right-rear tire regained contact with ground. |
| 1.230 | Vehicle's left-rear tire regained contact with ground. |
| 3.154 | Vehicle came to rest 191 ft -9 in. $(58.4 \mathrm{~m})$ downstream and $9 \mathrm{ft}-8 \mathrm{in} .(2.9 \mathrm{~m})$ <br> laterally in front of the barrier system. |

### 10.3 Barrier Damage

Damage to the barrier system was moderate, as shown in Figures 73 through 79. Barrier system damage consisted of contact marks on the front face of the concrete barriers, spalling and gouging of the concrete, and concrete cracking and fracture. The length of vehicle contact along the barrier system was approximately $29 \mathrm{ft}-3 \mathrm{in}$. ( 8.9 m ), which spanned from 14 in . ( 356 mm ) upstream of the targeted impact point to the downstream edge of barrier no. 5.
 downstream end toe on the back side of barrier no. 1 . A $71 / 2-\mathrm{in}$. ( $191-\mathrm{mm}$ ) wide x $2-\mathrm{in}$. ( $51-\mathrm{mm}$ ) thick piece of concrete disengaged from the upstream toe corner on the back side of barrier no. 2 . Contact marks began 14 in . ( 356 mm ) upstream from the targeted impact point near the groundline and extended the length of barrier no. 3. Barrier no. 3 had gouging that started $2 \frac{1}{2} \mathrm{in}$. ( 64 mm ) downstream from the targeted impact point and 13 in . $(330 \mathrm{~mm})$ from the groundline and extended 6 in. ( 152 mm ) upward and 16 in . ( 406 mm ) downstream. A 4-in. ( $102-\mathrm{mm}$ ) wide x $101 / 2-\mathrm{in}$. ( $267-$ mm ) tall concrete piece disengaged from the downstream corner of the front side of barrier no. 3, beginning $191 / 2 \mathrm{in}$. ( 495 mm ) from the ground. A $61 / 4-\mathrm{in}$. ( $159-\mathrm{mm}$ ) wide x $61 / 4-\mathrm{in}$. ( $159-\mathrm{mm}$ ) tall piece disengaged from the downstream corner of the toe on the front side of barrier no. 3. A crack began 15 in . ( 381 mm ) upstream of the impact point and extended around both faces of barrier no. 3.

Cracking was found on the upstream end of barrier no. 4 that started 21 in ( 533 mm ) from the ground and extended $103 / 4 \mathrm{in}$. ( 273 mm ) upward and onto the barrier's front face and ended 5 in. $(127 \mathrm{~mm})$ downstream. A 1-in. ( $25-\mathrm{mm}$ ) wide gouge started 12 in . 305 mm ) from the ground and extended $181 / 2 \mathrm{in}$. ( 470 mm ) upward on the corner of the upstream end and continued onto the front face of barrier no. 4. A $6-\mathrm{in}$. wide x 7 -in. tall x $31 / 2-\mathrm{in}$. deep ( $152-\mathrm{mm} \times 178-\mathrm{mm} \times 89-\mathrm{mm}$ )
piece of concrete disengaged from the upstream corner of the front side on the toe of barrier no. 4. Barrier no. 4 also had a crack on the front face 36 in . ( 914 mm ) downstream from the upstream end that extended to the back side of the barrier. On the front face of barrier no. 4, a 47½-in. (1,207$\mathrm{mm})$ long x $10-\mathrm{in}$. tall $(254-\mathrm{mm})$ piece of concrete disengaged 26 in . $(660 \mathrm{~mm})$ from the upstream end at the groundline. Concrete that measured $18 \mathrm{in} . \times 10 \mathrm{in}$. x 5 in . ( $457 \mathrm{~mm} \times 254 \mathrm{~mm} \times 127$ mm ) disengaged from the bottom of the toe on the back face of barrier no. 4 starting at $521 / 2 \mathrm{in}$. $(1,334 \mathrm{~mm})$ downstream from the upstream end of the barrier.

Barrier no. 5 damage included cracking, gouging, spalling, and contact marks. Multiple cracks were found on the upstream face with one beginning 2 in . 51 mm ) from the front face and the other beginning 4 in . $(102 \mathrm{~mm})$ from the front face. Both cracks extended from the top of the barrier to the connection loop on the side of the barrier. Cracking was also present starting 12 in . ( 305 mm ) downstream from the center extending vertically around both sides and the top of the barrier. Gouges on the upstream front corner of the barrier began at the top of the barrier and extended 8 in . ( 203 mm ) downward. A $6-\mathrm{in}$. wide x $6-\mathrm{in}$. tall x $2-\mathrm{in}$. deep ( $152-\mathrm{mm} \times 152-\mathrm{mm} x$ $51-\mathrm{mm}$ ) concrete piece disengaged from the front upstream toe corner of barrier no. 5. A 7 in .wide x $7-\mathrm{in}$. tall x $2-\mathrm{in}$. deep ( $178-\mathrm{mm} \times 178-\mathrm{mm} \times 51-\mathrm{mm}$ ) concrete piece disengaged from the downstream front corner at the top of barrier no. 5 . Contact marks were found 2 in . ( 51 mm ) from the top of the barrier and began $6 \mathrm{in} .(152 \mathrm{~mm})$ upstream from the center and extended to the downstream end.

The damage on barrier nos. 6 and 7 was limited to spalling and gouging. A gouge started at the top of barrier no. 6 and extended 10 in . $(254 \mathrm{~mm}$ ) down on the front-upstream corner. A 6in. wide x 7 in.-tall x 2 in.-deep ( $152-\mathrm{mm} \times 178-\mathrm{mm} \times 51-\mathrm{mm}$ ) piece of concrete at the upstreamback corner at the bottom disengaged from barrier no. 6 . A $3^{1 ⁄ 2} 2$-in. wide x $5^{1 / 2}-\mathrm{in}$. tall x 1 -in. deep ( $89-\mathrm{mm} \times 140-\mathrm{mm} \times 25-\mathrm{mm}$ ) piece of concrete disengaged from the downstream back corner at the bottom of barrier no. 6. Barrier no. 7 had two pieces disengage from the barrier. A 12-in. wide x $7-\mathrm{in}$. tall x $11 / 2-\mathrm{in}$. deep ( $305-\mathrm{mm} \times 178-\mathrm{mm} \times 38-\mathrm{mm}$ ) piece of concrete disengaged 40 in . ( 1,016 mm ) downstream from the center of barrier no. 7 . A 13 -in. wide x 6 -in. tall x $11 / 2-\mathrm{in}$. deep (330$\mathrm{mm} \times 152-\mathrm{mm} \times 38-\mathrm{mm}$ ) piece of concrete disengaged from the upstream-back corner at the bottom of barrier no. 7 .

Multiple connection pins within the PCB system experienced deformations during the impact. The connection pins between barrier nos. 3 and 4, as well as between barrier nos. 4 and 5 bent slightly near the location of the lower connection loops.

The permanent set of the barrier system was 128 in . $(3,251 \mathrm{~mm})$, as measured in the field. The longitudinal barrier displacement of the barriers on the upstream and downstream ends of the barrier system were found to be $471 / 2 \mathrm{in} .(1,207 \mathrm{~mm})$ and $4 \mathrm{in} .(102 \mathrm{~mm})$, respectively, as measured in the field. The maximum lateral dynamic barrier deflection was 128.3 in . ( $3,259 \mathrm{~mm}$ ), as determined from high-speed digital video analysis. The working width of the system was found to be 150.8 in . ( $3,830 \mathrm{~mm}$ ), also determined from high-speed digital video analysis.

### 10.4 Vehicle Damage

The damage to the vehicle was moderate, as shown in Figures 80 through 82. The maximum occupant compartment deformations are listed in Table 6 along with the deformation limits established in MASH for various areas of the occupant compartment. Note that none of the
established MASH deformation limits were violated. Complete occupant compartment and vehicle deformations and the corresponding locations are provided in Appendix C.

Table 6. Maximum Occupant Compartment Deformations by Location

| LOCATION | MAXIMUM <br> DEFORMATION <br> in. (mm) | MASH ALLOWABLE <br> DEFORMATION <br> in. (mm) |
| :---: | :---: | :---: |
| Wheel Well \& Toe Pan | $11 / 2(38)$ | $\leq 9(229)$ |
| Floor Pan \& Transmission Tunnel | $1 / 2(13)$ | $\leq 12(305)$ |
| Side Front Panel (in Front of A-Pillar) | $5 / 8(16)$ | $\leq 12(305)$ |
| Side Door (Above Seat) | $1 / 2(13)$ | $\leq 9(229)$ |
| Side Door (Below Seat) | $1 / 2(13)$ | $\leq 12(305)$ |
| Roof | $1 / 2(13)$ | $\leq 4(102)$ |
| Windshield | $1 / 2(13)$ | $\leq 3(76)$ |

The majority of damage was concentrated on the left-front corner and left side of the vehicle where the impact occurred. The left side of the front bumper was crushed inward and back $16 \mathrm{in} .(406 \mathrm{~mm})$. The left headlight housing assembly disengaged. The grille was fractured around the left-side headlight assembly and had a $1-\mathrm{in} .(25-\mathrm{mm})$ long crack in the center. The front bumper had a $1-\mathrm{in} .(25-\mathrm{mm})$ crease on the bottom edge $3 \mathrm{in} .(76 \mathrm{~mm})$ left of center. The left-front fender was pushed upward and inward in front of the left-front wheel. The left-front tire disengaged from its bead and was deflated with significant tearing on the sidewall. The left-front rim was deformed significantly with a $16-\mathrm{in}$. ( $406-\mathrm{mm}$ ) long dent on the bottom of the rim. Denting and scraping were observed on the entire left side of the vehicle with the most significant being a $62-\mathrm{in}$. long x $30-\mathrm{in}$. tall x $4-\mathrm{in}$. deep ( $1,575-\mathrm{mm} \times 762-\mathrm{mm} \times 102-\mathrm{mm}$ ) dent beginning at the rear of the left-front door and extending rearward to the left-rear wheel well. The left-rear door was dented and was ajar approximately $11 / 2 \mathrm{in}$. $(38 \mathrm{~mm})$ at the top of the door, but the door remained latched. There was a $11 / 2$-in. ( $38-\mathrm{mm}$ ) long buckle on the C-pillar at the top of the bed. The left-rear wheel assembly disengaged from the vehicle at the axle shaft. The tire was found deflated, and a $9-\mathrm{in}$. ( $229-\mathrm{mm}$ ) long buckle was present on the outside of the wheel. The left-rear brake line was sheared off and leaked brake fluid. The left taillight disengaged from the vehicle. A $3-\mathrm{in}$. ( $76-\mathrm{mm}$ ) gap was found between the front edge of the right-front fender and the corner of the hood. A $1 / 8-\mathrm{in}$. ( $3-\mathrm{mm}$ ) gap was found between the top of the right-rear quarter panel and the top of the tailgate. Both airbags deployed.

### 10.5 Occupant Risk

The calculated occupant impact velocities (OIVs) and maximum $0.010-\mathrm{sec}$ average occupant ridedown accelerations (ORAs) in both the longitudinal and lateral directions are shown in Table 7. Note that the OIVs and ORAs were within suggested limits, as provided in MASH. The calculated THIV, PHD, and ASI values are also shown in Table 7. The results of the occupant risk analysis, as determined from the accelerometer data, are summarized in Figure 66. The
recorded data from the accelerometers and the rate transducers are shown graphically in Appendix D.

Table 7. Summary of OIV, ORA, THIV, PHD, and ASI Values, Test No. NELON-1

| Evaluation Criteria |  | Transducer |  | MASH <br> Limits |
| :---: | :---: | :---: | :---: | :---: |
|  |  | SLICE-1 | SLICE-2 |  |
| OIV <br> $\mathrm{ft} / \mathrm{s}(\mathrm{m} / \mathrm{s})$ | Longitudinal | -14.57 (-4.44) | -13.75 (-4.19) | $\pm 40$ (12.2) |
|  | Lateral | 15.68 (4.78) | 16.93 (5.16) | $\pm 40$ (12.2) |
| $\begin{gathered} \text { ORA } \\ \text { g's } \end{gathered}$ | Longitudinal | -6.63 | -6.92 | $\pm 20.49$ |
|  | Lateral | 16.76 | 15.20 | $\pm 20.49$ |
| MAX. <br> ANGULAR DISPL. deg. | Roll | -30.56 | -26.93 | $\pm 75$ |
|  | Pitch | -12.96 | -15.00 | $\pm 75$ |
|  | Yaw | 53.51 | 52.23 | not required |
| $\begin{aligned} & \text { THIV } \\ & \mathrm{ft} / \mathrm{s}(\mathrm{~m} / \mathrm{s}) \end{aligned}$ |  | 20.0 (6.08) | 20.7 (6.32) | not required |
| $\begin{gathered} \text { PHD } \\ \text { g's } \end{gathered}$ |  | 16.84 | 15.23 | not required |
| ASI |  | 1.12 | 1.10 | not required |

### 10.6 Discussion

The analysis of the test results for test no. NELON-1 showed that the beginning of the LON for the free-standing, F-shape PCB system adequately contained and redirected the 2270P vehicle with controlled lateral displacements of the barrier. There were no detached elements nor fragments which showed potential for penetrating the occupant compartment nor presented undue hazard to other traffic. Deformations of, or intrusions into, the occupant compartment that could have caused serious injury did not occur. The test vehicle did not penetrate nor ride over the barrier and remained upright during and after the collision. Vehicle roll, pitch, and yaw angular displacements, as shown in Appendix D, were deemed acceptable, because they did not adversely influence occupant risk nor cause rollover. After impact, the vehicle exited the barrier at an angle of 12.3 degrees and its trajectory did not violate the bounds of the exit box. Therefore, test no. NELON-1 was determined to be acceptable according to the MASH safety performance criteria for test designation no. 3-35.

0.000 sec
0.120 sec
0.232 sec


- Test Number......................................................................................................................................................................................................... Daten-1
- MASH Test Designation $\qquad$ ..3-35
- Test Article........................................................................Free-standing, F-Shaped PCB
- Total Length $.102 \mathrm{ft}-4 \mathrm{in} .(31.2 \mathrm{~m})$
- Key Component - Portable Concrete Barrier
Length ....................................................................................... 150 in. ( $3,810 \mathrm{~mm}$ )
Height.......................................................................... 32 in. $(813 \mathrm{~mm})$
Number of Barriers..................................................................................................
- Key Component - Connecting Pin
$\qquad$ 28 in. (711 mm)
Diameter.......................................................................................... $1^{11 / 4}$ in. ( 32 mm )
- Vehicle Make /Model............................................................................ 2008 Dodge Ram
 Test Inertial. $\qquad$ ,991 lb (2,264 kg) Gross Static.
- Impact Condition
Speed $\qquad$ $.62 .1 \mathrm{mph}(100.0 \mathrm{~km} / \mathrm{h})$
Angle
Impact Location........................................................................................ 48 11/16 in. (1,237 mm) US of Joint between Barrier Nos. 3 and 4
- Impact Severity (IS) .................................. 113.6 kip-ft ( 154.0 kJ ) > $106 \mathrm{kip}-\mathrm{ft}(144.0 \mathrm{~kJ})$
- Exit Conditions
Speed.
$44.8 \mathrm{mph}(72.0 \mathrm{~km} / \mathrm{h})$
Angle
12.3 deg

- Vehicle Stability $191 \mathrm{ft}-9 \mathrm{in} .(58.4 \mathrm{~m})$ downstream $9 \mathrm{ft}-8 \mathrm{in}$. ( 2.9 m ) laterally in front
- Vehicle Damage $\quad$ Moderate
VDS [19] .11-LFQ-3
CDC [20] $\qquad$ .11-LYEW-2
Maximum Interior Deformation $\qquad$ $11 / 2$ in. $(38.1 \mathrm{~mm})$

Figure 66. Summary of Test Results and Sequential Photographs, Test No. NELON-1


Figure 67. Additional Sequential Photographs, Test No. NELON-1


Figure 68. Additional Sequential Photographs, Test No. NELON-1


Figure 69. Documentary Photographs, Test No. NELON-1


Figure 70. Documentary Photographs, Test No. NELON-1


Figure 71. Impact Location, Test No. NELON-1


Figure 72. Vehicle Final Position and Trajectory Marks, Test No. NELON-1


Figure 73. System Deflection and Damage, Test No. NELON-1



Figure 75. Barrier No. 3 Damage, Test No. NELON-1




Figure 78. Barrier No. 6 Damage, Test No. NELON-1





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Figure 80. Vehicle Damage, Test No. NELON-1



Figure 82. Occupant Compartment Damage, Test No. NELON-1

## 11 FULL-SCALE CRASH TEST NO. NELON-2

### 11.1 Weather Conditions

Test no. NELON-2 was conducted on March 16, 2016 at approximately 1:00 p.m. The weather conditions as per the National Oceanic and Atmospheric Administration (station 14939/LNK) were reported and are shown in Table 8.

Table 8. Weather Conditions, Test No. NELON-2

| Temperature | $59^{\circ} \mathrm{F}$ |
| :--- | :--- |
| Humidity | $27 \%$ |
| Wind Speed | 18 mph |
| Wind Direction | $290^{\circ}$ from True North |
| Sky Conditions | Sunny |
| Visibility | 10 Statute Miles |
| Pavement Surface | Dry |
| Previous 3-Day Precipitation | 0.51 in. |
| Previous 7-Day Precipitation | $0.58 \mathrm{in}$. |

### 11.2 Test Description

The $5,005-\mathrm{lb}(2,270-\mathrm{kg})$ pickup truck impacted the portable concrete barrier system at a speed of $63.0 \mathrm{mph}(101.4 \mathrm{~km} / \mathrm{h})$ and at an angle of 24.5 degrees. A summary of the test results and sequential photographs are shown in Figure 84. Additional sequential photographs are shown in Figures 85 and 86. Documentary photographs of the crash test are shown in Figures 87 and 88.

Initial vehicle impact was to occur $51^{3 / 16} \mathrm{in}$. $(1,300 \mathrm{~mm})$ upstream from the centerline of the joint between barrier nos. 4 and 5, as shown in Figure 89. The impact point was selected based on LS-DYNA simulation of the end of the LON for the reduced length PCB system and MASH guidance for the critical impact point on PCB systems. The actual point of impact was 63 in . (1,600 mm ) upstream from the downstream edge of barrier no. 4. A sequential description of the impact events is contained in Table 9. During the impact, the 2270 P vehicle was captured and redirected, however, the left-front door unlatched and opened when the vehicle rolled onto its left side before rolling back and exiting the system. The vehicle came to rest $165 \mathrm{ft}-10$ in ( 50.5 m ) downstream of the initial impact point and $28 \mathrm{ft}-11 \mathrm{in}$. $(8.8 \mathrm{~m})$ in front of the front face of the barrier system. The vehicle trajectory and final position are shown in Figures 84 and 90, respectively.

Table 9. Sequential Description of Impact Events, Test No. NELON-2

| TIME <br> $(\mathrm{sec})$ | EVENT |
| :---: | :--- |
| 0.000 | Vehicle's left-front bumper contacted downstream end of barrier no. 4. |
| 0.002 | Vehicle's front bumper deformed. |


| 0.006 | Vehicle's left headlight deformed, and vehicle's left fender contacted barrier no. <br> 4. |
| :--- | :--- |
| 0.008 | Vehicle's left fender deformed. |
| 0.020 | Downstream end of barrier no. 4 deflected backward. |
| 0.024 | Vehicle's engine hood deformed. |
| 0.026 | Vehicle's grille deformed, and vehicle's left-front door flexed away from frame <br> at upper rear corner. |
| 0.036 | Upstream end of barrier no. 5 deflected backward. |
| 0.046 | Vehicle's airbags deployed. |
| 0.052 | Vehicle yawed away from barrier system, vehicle rolled toward barrier system, <br> and vehicle's left-rear door flexed away from frame at upper rear corner. |
| 0.056 | Upstream end of barrier no. 6 deflected forward. |
| 0.090 | Barrier no. 5 cracked at center. |
| 0.092 | Vehicle's right-front tire became airborne, and downstream end of barrier no. 3 <br> deflected backward. |
| 0.102 | Vehicle pitched upward. |
| 0.118 | Barrier no. 3 deflected downstream, and barrier no. 2 deflected downstream. |
| 0.120 | Barrier no. 1 deflected downstream. |
| 0.132 | Upstream end of barrier no. 6 deflected backward. |
| 0.142 | Vehicle's left headlight detached. |
| 0.172 | Vehicle's right-rear tire became airborne. |
| 0.182 | Upstream end of barrier no. 7 deflected forward. |
| 0.194 | Vehicle was parallel to system at a speed of 51.2 mph (82.3 km/h). |
| 0.264 | Vehicle's left quarter panel contacted barrier no. 5 and deformed. |
| 0.268 | Vehicle's left taillight deformed. |
| 0.272 | Vehicle's left-front tire contacted ground. |
| 0.280 | Vehicle pitched downward, and vehicle's rear bumper deformed. |
| 0.282 | Upstream end of barrier no. 8 deflected backward, and barrier no. 2 deflected <br> forward. |
| 0.302 | Vehicle's left taillight detached. |
| 0.346 | Upstream end of barrier no. 7 cracked. |
| 0.376 | Upstream end of barrier no. 8 deflected forward. |
| 0.390 | Vehicle's left-front door contacted downstream knee formed at joint between <br> barrier nos. 6 and 7. <br> 0.404 |
| 0.420 | Downstream end of barrier no. 6 cracked. |
| 0.438 | Vehicle's left-rear door contacted downstream knee formed at joint between <br> barrier nos. 6 and 7. |


| 0.448 | Vehicle's left side mirror contacted barrier system. |
| :--- | :--- |
| 0.454 | Upstream end of barrier no. 7 deflected backward. |
| 0.524 | Vehicle's left-front door opened. |
| 0.528 | Vehicle lost contact with system at a speed of $39.4 \mathrm{mph}(63.4 \mathrm{~km} / \mathrm{h})$ and an angle <br> of 10.4 degrees. |
| 0.602 | Vehicle's tailgate deformed. |
| 0.724 | Downstream end of barrier no. 2 deflected backward. |
| 0.762 | Upstream end of barrier no. 8 deflected backward. |
| 0.858 | Vehicle's open left-front door contacted ground. |
| 1.336 | PCB system deflection came to a stop. |
| 1.544 | Vehicle's left-rear tire contacted ground. |
| 1.602 | Vehicle's left quarter panel contacted ground. |
| 1.636 | Vehicle's rear bumper contacted ground. |
| 1.692 | Vehicle rolled away from barrier system. |
| 2.160 | Vehicle's left quarter panel contacted ground. |
| 2.866 | Vehicle's right-front tire regained contact with ground. |
| 2.886 | Vehicle's right-rear tire regained contact with ground. |
| 3.208 | Vehicle came to rest $165 \mathrm{ft}-10$ in. (50.5 m) downstream and $28 \mathrm{ft}-11 \mathrm{in}. \mathrm{(8.8}$ <br> m) laterally in front of barrier system. |

### 11.3 Barrier Damage

Damage to the barrier system was moderate, as shown in Figures 91 through 96. Barrier system damage consisted of contact marks on the front face of the concrete barriers, spalling and gouging of the concrete, and concrete cracking and fracture. The length of vehicle contact along the barrier system was approximately $30 \mathrm{ft}-2 \mathrm{in}$. ( 9.2 m ), which spanned from 18 in . ( 457 mm ) downstream from the center target of barrier no. 4 to 9 in . ( 229 mm ) downstream from the upstream edge of barrier no. 7 .

A $1-\mathrm{in}$. wide $\mathrm{x} 3-\mathrm{in}$. tall $\mathrm{x} 1 / 4-\mathrm{in}$. deep ( $25-\mathrm{mm} \times 76-\mathrm{mm} \times 6-\mathrm{mm}$ ) concrete portion disengaged from barrier no. 2 at the downstream corner on the back face of the toe at the groundline. A $4-\mathrm{in}$. wide x $2^{11 / 4}-\mathrm{in}$. tall $\mathrm{x}^{1 / 4}-\mathrm{in}$. deep ( $102-\mathrm{mm} \times 57-\mathrm{mm} \times 6-\mathrm{mm}$ ) piece of concrete disengaged from the upstream corner of the back face of barrier no. 3 on the bottom of the toe. Gouges started 17 in. ( 432 mm ) from the groundline and $24 \mathrm{in} .(610 \mathrm{~mm}$ ) downstream from the center target on barrier no. 4 and extended a total length of $211 / 4 \mathrm{in}$. ( 540 mm ). A 4 -in. wide x 151/4in. tall x $2^{1 / 2}-$ in. deep ( $102-\mathrm{mm} \times 387-\mathrm{mm} \times 64-\mathrm{mm}$ ) concrete piece located at the downstream edge $151 / 2 \mathrm{in}$. ( 394 mm ) above the groundline disengaged from barrier no. 4 . A $11 / 2$-in. wide $\mathrm{x} 83 / 4$-in. tall x $11 / 2-$ in. deep ( $38-\mathrm{mm} \times 222-\mathrm{mm} \times 38-\mathrm{mm}$ ) piece of concrete disengaged from the downstream corner of the front face on the bottom of the toe of barrier no. 4 .

A 7-in. (178-mm) long crack was found on the upstream side of barrier no. 5 that started 2 in. $(51 \mathrm{~mm})$ from the front face and $21 / 2 \mathrm{in}$. $(64 \mathrm{~mm})$ from the top of the barrier. On the upstream
edge of barrier no. 5 , a $23 / 4-\mathrm{in}$. wide $\times 15-\mathrm{in}$. tall $\times 1 / 8-\mathrm{in}$. deep ( $70-\mathrm{mm} \times 381-\mathrm{mm} \times 3-\mathrm{mm}$ ) portion of concrete disengaged $21 / 2 \mathrm{in}$. ( 64 mm ) from the ground on the front face. Two concrete portions disengaged from the front face of the toe at the bottom of barrier no. 5; the first was located at the upstream edge and was 8 in . wide $\times 81 / 2 \mathrm{in}$. tall $\times 3 \mathrm{in}$. deep ( $203 \mathrm{~mm} \times 216 \mathrm{~mm} \times 76 \mathrm{~mm}$ ) and the second began $391 / 2 \mathrm{in}$. ( $1,003 \mathrm{~mm}$ ) downstream from the upstream edge and extended $511 / 2 \mathrm{in}$. $(1,308 \mathrm{~mm})$ downstream. A $171 / 4-\mathrm{in}$. wide x 11 -in. tall x $41 / 2$-in. deep ( $438-\mathrm{mm} \times 279-\mathrm{mm} \times 114-$ mm ) piece of concrete disengaged from the toe on the back face of barrier no. 5 beginning $52 \frac{1}{2} \mathrm{in}$. $(1,334 \mathrm{~mm})$ from the upstream edge. A crack was located $611 / 2 \mathrm{in}$. ( $1,562 \mathrm{~mm}$ ) downstream of the upstream edge of barrier no. 5 and extended across both faces of the barrier. Another large crack was located $311 / 4 \mathrm{in}$. $(794 \mathrm{~mm}$ ) downstream from the center target and extended vertically across the back face and the width of barrier no. 5 at the top. Cracking was found 14 in . ( 356 mm ) downstream from the center target that extended across both faces of barrier no. 5 .

Gouges started at the top of barrier no. 6 and extended 16 in . ( 406 mm ) downward with a maximum width of $23 / 4 \mathrm{in}$. ( 70 mm ) on the front face at the upstream edge. A $12 \frac{1}{2} 2$-in. wide $\times 81 / 4-$ in. tall x 4 -in. deep ( $318-\mathrm{mm} \times 210-\mathrm{mm} \times 102-\mathrm{mm}$ ) concrete portion disengaged from the bottom of the toe on the upstream side of barrier no. 6. A $22 \frac{1}{2}-\mathrm{in}$. wide x 7 -in. tall x $2-\mathrm{in}$. deep ( $572-\mathrm{mm}$ x $178-\mathrm{mm} \times 51-\mathrm{mm}$ ) concrete portion that began 52 in . ( $1,321 \mathrm{~mm}$ ) downstream from the center of the barrier at the bottom of the toe disengaged from the downstream edge of the back face. A 4in. wide x $13-\mathrm{in}$. tall x $31 / 2-\mathrm{in}$. deep ( $102-\mathrm{mm} \times 330-\mathrm{mm} \times 89-\mathrm{mm}$ ) portion of concrete disengaged from barrier no. 6 on the downstream edge of the front face.

A $2^{1 / 2}$-in. wide x $91 / 2$-in. tall $\times 1 / 2$-in. deep ( $64-m m \times 241-m m \times 13-m m$ ) concrete portion disengaged from the upstream edge on the front face of barrier no. 7 at 24 in . ( 610 mm ) from the ground. A $6-\mathrm{in}$. wide x $8-\mathrm{in}$. tall x $2-\mathrm{in}$. deep ( $152-\mathrm{mm} \times 203-\mathrm{mm} \times 51-\mathrm{mm}$ ) concrete piece disengaged from the upstream edge on the back face at the bottom of the toe. A $51 / 4-\mathrm{in}$. wide $x 5^{1 / 2-}$ in. tall x $2-\mathrm{in}$. deep ( $133-\mathrm{mm} \times 140-\mathrm{mm} \times 51-\mathrm{mm}$ ) concrete portion disengaged from the downstream edge on the back face at the bottom of the toe. A $91 / 2-\mathrm{in}$. wide $\mathrm{x} 71 / 2-\mathrm{in}$. tall $\times 2$-in. deep ( $241-\mathrm{mm} \times 191-\mathrm{mm} \times 51-\mathrm{mm}$ ) concrete portion disengaged from the upstream edge on the back face of the toe at the bottom of barrier no. 8 .

Multiple connection pins between the PCBs were deformed during the impact. The connecting pin between barrier nos. 4 and 5 had a slight bend at the location of the lower connection loops. The connecting pin between barrier nos. 5 and 6 had a slight bend at the location of the upper connection loops.

The permanent set of the barrier system was 126 in . (3,200 mm), as measured in the field. The longitudinal displacement of the barriers on the upstream and downstream ends of the system were found to be $281 / 2 \mathrm{in}$. ( 724 mm ) and $227 / 8 \mathrm{in}$. ( 581 mm ), respectively, as measured in the field. The maximum lateral dynamic barrier deflection was 127.8 in . ( $3,246 \mathrm{~mm}$ ), as determined from high-speed digital video analysis. The working width of the system was found to be 150.3 in . $(3,818 \mathrm{~mm})$, also determined from high-speed digital video analysis.

### 11.4 Vehicle Damage

The damage to the vehicle was moderate, as shown in Figures 97 and 98. The maximum occupant compartment deformations are listed in Table 10 along with the deformation limits established in MASH for various areas of the occupant compartment. Note that none of the
established MASH deformation limits were violated. Complete occupant compartment and vehicle deformations and the corresponding locations are provided in Appendix C.

Table 10. Maximum Occupant Compartment Deformations by Location

| LOCATION | MAXIMUM <br> DEFORMATION <br> in. (mm) | MASH ALLOWABLE <br> DEFORMATION <br> in. (mm) |
| :---: | :---: | :---: |
| Wheel Well \& Toe Pan | $3 / 8(9)$ | $\leq 9(229)$ |
| Floor Pan \& Transmission Tunnel | $1 / 8(3)$ | $\leq 12(305)$ |
| Side Front Panel (in Front of A-Pillar) | $0(0)$ | $\leq 12(305)$ |
| Side Door (Above Seat) | $2(51)$ | $\leq 9(229)$ |
| Side Door (Below Seat) | $11 / 4(32)$ | $\leq 12(305)$ |
| Roof | $1 / 2(13)$ | $\leq 4(102)$ |
| Windshield | $1 / 2(13)$ | $\leq 3(76)$ |

The majority of damage was concentrated on the left-front corner and the left side of the vehicle where the impact occurred. There was a 5 -in. long x $1-\mathrm{in}$. tall x $3 / 4-\mathrm{in}$. deep ( $127-\mathrm{mm} \times 25-$ $\mathrm{mm} \times 19-\mathrm{mm}$ ) buckle on the radiator core support below the radiator. There was also a $10-\mathrm{in}$. (254mm ) long scrape on the radiator core support behind the left headlight housing. Buckling occurred on the left side of the front bumper that was $133 / 8 \mathrm{in}$. long x 8 in . tall $\times 31 / 2 \mathrm{in}$. deep ( $340 \mathrm{~mm} \times 203$ $\mathrm{mm} \times 89 \mathrm{~mm}$ ). The front bumper had a kink on the bottom at the centerline and scraping on the left side. The left headlight assembly disengaged from the vehicle. The left-front fender was pushed upward and inward in front of the left-front wheel. The left-front steel wheel was deformed significantly with a $15-\mathrm{in}$. long x $71 / 2-\mathrm{in}$. wide ( $381-\mathrm{mm} \times 191-\mathrm{mm}$ ) buckle on the hubcap. The leftfront tire was deflated and had $41 / 8$ in. long x $27 / 8 \mathrm{in}$. wide ( $105 \mathrm{~mm} \times 73 \mathrm{~mm}$ ) and $81 / 2 \mathrm{in}$. long x $31 / 8$ in. wide ( $216 \mathrm{~mm} \times 79 \mathrm{~mm}$ ) tears in the sidewall. The right-front tire bead disengaged from the wheel and was deflated and there was a $31 / 2-\mathrm{in}$. ( $89-\mathrm{mm}$ ) long kink on the wheel. Scraping measuring $11 / 2 \mathrm{in}$. ( 38 mm ) long was found on the bottom of both lower control arms as well as indications that the bump stops on both control arms came into contact with the frame of the vehicle. The left side motor mount was fractured on the engine side of the mount. The front grille disengaged from the vehicle and was located on the ground approximately 10 feet ( 3 m ) downstream from the front of the final position of the vehicle. A 4-in. long x 1114-in. tall ( $102-\mathrm{mm}$ $x$ 32-mm) tear was found in the sheet metal at the midspan of the left-front door. A 9-in. long $x$ $2^{1 / 4}-\mathrm{in}$. wide by $1 / 8-\mathrm{in}$. deep ( $229-\mathrm{mm} \times 57-\mathrm{mm} \times 3-\mathrm{mm}$ ) gouge was found in the middle of the leftfront door. The rear of the left-front door was ajar 2 in . $(51 \mathrm{~mm}$ ) and the top of the left-rear door was ajar $23 / 4 \mathrm{in}$. $(70 \mathrm{~mm})$. A $1-\mathrm{in}$. $(25-\mathrm{mm})$ deep dent on the lower portion of the front of the leftrear door was $91 / 2 \mathrm{in}$. long x 8 in . tall ( $241 \mathrm{~mm} \times 203 \mathrm{~mm}$ ). A large buckle in the middle of the front of the left-rear door was 11 in . long x $31 / 4 \mathrm{in}$. wide ( $279 \mathrm{~mm} \times 83 \mathrm{~mm}$ ). Denting, scraping, and gouging were observed on the entire left side of the vehicle with the most significant being a 105in. $(2,667-\mathrm{mm})$ long gouge that began at the front of the left-front door and extended rearward to the left-rear wheel well. The left-rear wheel assembly had cracking and a $133 / 4-\mathrm{in}$. long x $71 / 4-\mathrm{in}$. tall ( $349-\mathrm{mm} \times 184-\mathrm{mm}$ ) buckle on the hubcap as well as scrape marks on the steel wheel. A gouge on the quarter panel began above the left-rear wheel and extended $52 \frac{1}{2} \mathrm{in}$. $(1,334 \mathrm{~mm})$ to the rear
of the vehicle. A dent on the left-rear quarter panel above the fuel door measured $61 / 2 \mathrm{in}$. long $x 8^{1 / 2}$ in. tall $\times 1 / 8 \mathrm{in}$. deep ( $165 \mathrm{~mm} \times 216 \mathrm{~mm} \times 3 \mathrm{~mm}$ ). The left taillight of the vehicle disengaged and there was scraping around the taillight housing. The left side of the rear bumper was scraped and had a $2-\mathrm{in}$. long x $2^{11 / 4}-\mathrm{in}$. wide ( $51-\mathrm{mm} \times 57-\mathrm{mm}$ ) kink. The tailgate disengaged from its hinges but remained attached to its support cables. Both airbags deployed.

In test no. NELON-2, an onboard GoPro camera view indicated significant deformation of the left side B-pillar due to impact with the knee formed at the joint between barrier nos. 6 and 7. Due to this deformation, attempts were made to measure and report the displacement of the Bpillar. B-pillar deformation measurements for test no. NELON-2 consisted of both physical measurements of the maximum B-pillar deformation and film analysis measurements utilizing the GoPro cameras mounted inside of the vehicle. The measurements were reviewed and only the physical measurements were deemed appropriate for the final report:

1. Three different film analysis attempts were made and all three yielded different data. There were concerns that the motion of the camera, the alignment of the camera, and lens correction issues influenced the results. As such, these were not deemed appropriate for reporting purposes.
2. The permanent set deformations taken by the field staff were measured at two locations on the B-pillar. These measurements were taken by measuring the distance from one side of the vehicle to the other on an undamaged Dodge Ram and then measuring the same distance on the test vehicle. The difference was the measured lateral permanent set deflection of the B-pillar. The values obtained are shown in Table 11.

Table 11. B-Pillar Deformation, Test No. NELON-2

| B-Pillar Measurement <br> Location <br> in. (mm) | Undamaged Vehicle <br> Measurement <br> in. (mm) | NELON-2 Vehicle <br> Measurement <br> in. (mm) | Lateral Permanent <br> Set B-Pillar <br> Deformation <br> in. (mm) |
| :---: | :---: | :---: | :---: |
| Lower B-Pillar, $61 / 4(159)$ <br> above floorpan | $647 / 8(1,648)$ | $611 / 4(1,556)$ | $35 / 8(92)$ |
| Mid B-Pillar, $161 / 2(419)$ <br> above floorpan | $643 / 4(1,645)$ | $60(1,524)$ | $43 / 4(121)$ |



Figure 83. B-Pillar Deformation, Test No. NELON-2

### 11.5 Occupant Risk

The calculated occupant impact velocities (OIVs) and maximum 0.010-sec average occupant ridedown accelerations (ORAs) in both the longitudinal and lateral directions are shown in Table 12. Note that the OIVs and ORAs were within suggested limits, as provided in MASH. The calculated THIV, PHD, and ASI values are also shown in Table 12. The results of the occupant risk analysis, as determined from the accelerometer data, are summarized in Figure 84. The recorded data from the accelerometers and the rate transducers are shown graphically in Appendix E.

Table 12. Summary of OIV, ORA, THIV, PHD, and ASI Values, Test No. NELON-2

| Evaluation Criteria |  | Transducer |  | MASH <br> Limits |
| :---: | :---: | :---: | :---: | :---: |
|  |  | SLICE-1 | SLICE-2 |  |
| OIV <br> $\mathrm{ft} / \mathrm{s}(\mathrm{m} / \mathrm{s})$ | Longitudinal | -12.86 (-3.92) | -11.94 (-3.64) | $\pm 40$ (12.2) |
|  | Lateral | 15.49 (4.72) | 17.59 (5.36) | $\pm 40$ (12.2) |
| $\begin{gathered} \text { ORA } \\ \text { g's } \end{gathered}$ | Longitudinal | -5.73 | -6.45 | $\pm 20.49$ |
|  | Lateral | 13.48 | 11.02 | $\pm 20.49$ |
| MAX. ANGULAR DISPL. deg. | Roll | -86.06 | -82.28 | $\pm 75$ |
|  | Pitch | -20.30 | -20.17 | $\pm 75$ |
|  | Yaw | 49.78 | 48.29 | not required |
| $\begin{gathered} \text { THIV } \\ \mathrm{ft} / \mathrm{s}(\mathrm{~m} / \mathrm{s}) \end{gathered}$ |  | 18.7 (5.71) | 21.5 (6.55) | not required |
| $\begin{gathered} \text { PHD } \\ \text { g's } \end{gathered}$ |  | 13.74 | 11.39 | not required |
| ASI |  | 1.01 | 1.11 | not required |

### 11.6 Discussion

The analysis of the test results for test no. NELON-2 showed that the end of LON for the reduced length, free-standing, F-shape PCB system adequately contained and redirected the 2270P vehicle with controlled lateral displacements of the barrier. There were no detached elements nor fragments which showed potential for penetrating the occupant compartment nor presented undue hazard to other traffic. Deformations of, or intrusions into, the occupant compartment that could have caused serious injury did not occur. However, the left-front door of the vehicle became unlatched and opened during the impact. The cause for the door latch release was not determined. Examination of the door latch did not reveal damage or fracture that would have caused the latch to disengage, but motion of the dummy limbs or the impact of the door into the barrier may have potentially activated the latch mechanism. While this behavior is not specifically outlined as violating the safety requirements in MASH, there was potential concern that the opening of the door exposed the vehicle occupant. The test vehicle did not penetrate nor ride over the barrier and remained upright during and after the collision. Vehicle pitch and yaw angular displacements, shown in Appendix E, were deemed acceptable because they did not adversely influence occupant risk, however, vehicle roll did exceed the occupant risk safety criteria of 75 degrees established in MASH. After impact, the vehicle exited the barrier at an angle of 10.4 degrees, and its trajectory did not violate the bounds of the exit box. Therefore, due to the excessive roll of the vehicle, test no. NELON-2 was determined to be unacceptable according to the MASH safety performance criteria for test designation no. 3-37.

0.000 sec
0.046 sec


- Test Number $\qquad$
- Date ASH Test Designation No...........................................................
$\qquad$
- Test Article. $\qquad$
$\qquad$ Free-standing, F-Shaped PCB
- Total Length

Length
Height.
Number of Barriers.. $\qquad$

- Key Component - Connecting Pin

Length ..
Diameter $\qquad$ .28 in. $(711 \mathrm{~mm})$

- Vehicle Make /Model... $\ldots .1^{1 / 4} \mathrm{in} .(32 \mathrm{~mm})$
$\qquad$ 2008 Dodge Ram

Test Inertial. ,005 lb (2,284 kg)
Gross Static. $5,161 \mathrm{lb}(2,341 \mathrm{~kg})$

- Impact Conditions

Speed. $\qquad$ $.63 .0 \mathrm{mph}(101.4 \mathrm{~km} / \mathrm{h})$
Angle .......................................................................................................... 24.5 deg
Impact Location......... $85 \frac{1}{2}$ in. $(2,172 \mathrm{~mm})$ US of Joint between Barrier Nos. 4 and 5

- Impact Severity (IS) .113 .8 kip-ft $(154.3 \mathrm{~kJ})>106$ kip-ft $(144.0 \mathrm{~kJ})$
- Exit Conditions

Speed $\qquad$ $39.4 \mathrm{mph}(63.4 \mathrm{~km} / \mathrm{h})$ Angle ... 3
xit Box Criterion $\qquad$ ..................Pass

- Vehicle Stability .Unsatisfactory
- Vehicle Stopping Distance . $\qquad$ $28 \mathrm{ft}-11$ in $(8.8 \mathrm{~m})$ downstream
- Vehicle Damage ..Moderate VDS [19]
$\qquad$ .01-LYEW-2
Maximum Interior Deformation $\qquad$ 2 in. ( 51 mm )
- Test Article Damage ..............572] $\begin{gathered}221 / 2^{\prime \prime} \\ \text { - }\end{gathered}$
- Maximum Test Article Deflections

Permanent Set ............................................................................. 126 in. ( $3,207 \mathrm{~mm}$ )
Dynamic. $.127 .8 \mathrm{in} .(3,247 \mathrm{~mm})$ Working Width
$\qquad$ $127.8 \mathrm{in} .(3,247 \mathrm{~mm})$
$150.3 \mathrm{in} .(3,818 \mathrm{~mm})$

- Transducer Data

| Evaluation Criteria |  | Transducer |  | MASH <br> Limit |
| :---: | :---: | :---: | :---: | :---: |
|  |  | SLICE-1 | $\begin{gathered} \hline \text { SLICE-2 } \\ \text { (Primary) } \\ \hline \end{gathered}$ |  |
| $\begin{gathered} \text { OIV } \\ \mathrm{ft} / \mathrm{s} \\ (\mathrm{~m} / \mathrm{s}) \end{gathered}$ | Longitudinal | -12.86 (-3.92) | -11.94 (-3.64) | $\begin{gathered} \pm 40 \\ (12.2) \\ \hline \end{gathered}$ |
|  | Lateral | 15.49 (4.72) | 17.59 (5.36) | $\begin{gathered} \pm 40 \\ (12.2) \end{gathered}$ |
| $\begin{gathered} \text { ORA } \\ \text { g's } \end{gathered}$ | Longitudinal | -5.73 | -6.45 | $\pm 20.49$ |
|  | Lateral | 13.48 | 11.02 | $\pm 20.49$ |
| MAX ANGULAR DISP. deg. | Roll | -86.06 | -82.29 | $\pm 75$ |
|  | Pitch | -20.30 | -20.17 | $\pm 75$ |
|  | Yaw | 49.78 | 48.29 | $\begin{gathered} \text { not } \\ \text { required } \\ \hline \end{gathered}$ |
| THIV - ft/s (m/s) |  | 18.7 (5.71) | 21.5 (6.55) | $\begin{gathered} \text { not } \\ \text { required } \end{gathered}$ |
| PHD - g's |  | 13.74 | 11.39 | $\begin{aligned} & \text { not } \\ & \text { required } \end{aligned}$ |
| ASI |  | 1.01 | 1.11 | $\begin{gathered} \text { not } \\ \text { required } \\ \hline \end{gathered}$ |

Figure 84. Summary of Test Results and Sequential Photographs, Test No. NELON-2


Figure 85. Additional Sequential Photographs, Test No. NELON-2


Figure 86. Additional Sequential Photographs, Test No. NELON-2


Figure 87. Documentary Photographs, Test No. NELON-


Figure 88. Documentary Photographs, Test No. NELON-2


Figure 89. Impact Location, Test No. NELON-2


Figure 90. Vehicle Final Position and Trajectory Marks, Test No. NELON-2


Figure 91. System Deflection and Damage, Test No. NELON-2



Figure 93. Barrier No. 4 Damage, Test No. NELON-2


Figure 94. Barrier No. 5 Damage, Test No. NELON-2



Figure 95. Barrier No. 6 Damage, Test No. NELON-2


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MwRSF Report No. MRP-03-337-17
Figure 96. Barrier Nos. 7 and 8 Damage, Test No. NELON-2






## 12 DISCUSSION OF FULL-SCALE TEST RESULTS

The researchers reviewed the results of full-scale crash test nos. NELON-1 and NELON-2 to assess the performance of the reduced-length F-shape PCB system when impacted at the beginning and end of LON. In test no. NELON-1, the 2270P vehicle impacted the beginning of LON on the eight-barrier long F-shape PCB system and was safely redirected. This correlated well with the behavior predicted by the LS-DYNA simulation models, including the impact from the knee that formed at the joint between barrier nos. 5 and 6 into the side of the vehicle and the corresponding damage to the front and rear doors. The maximum dynamic barrier deflection for test no. NELON-1 was 128.3 in . $(3,259 \mathrm{~mm})$, which was a 35 percent increase over the dynamic deflection predicted by the LS-DYNA computer simulation prior to the test. The increase in barrier deflection was more than anticipated, but did not cause issues with respect to the safe redirection of the vehicle.

It was theorized that the increased deflection was likely due to a combination of factors. First, simplifications in the PCB model may have reduced barrier deflections. The PCB model uses non-deformable, rigid elements for the PCB body and the connection loop rebar. The rigid element body of the barrier does not allow for fracture of the barrier toes or other barrier damage that would allow increased joint rotations and increased barrier motions. The inability to fracture the barrier toes may have had a significant effect as the loss of the barrier toes allows the barriers to deflect more prior to the corners of the barriers locking up and transmitting tension to adjacent barrier segments. This could have significantly increased the deflections, as observed in the full-scale testing. Similarly, the use of rigid connection loops may make the PCB connection stiffer and further reduce deflections. Differences between the simulated and actual vehicles used in the analysis may have also contributed to the difference in deflection.

Test no. NELON-2 initially performed similarly to test no. NELON-1 as the vehicle was captured and redirected. Peak lateral barrier deflections were similar to NELON-1 and were again larger than those predicted by the LS-DYNA simulation. However, test no. NELON-2 was deemed unacceptable according to the MASH safety requirements due to vehicle roll that exceeded 75 degrees after it exited the barrier system. Review of the test suggested potential factors that may have contributed to the vehicle rollover. First, the increased barrier deflections observed in test no. NELON-2 due to reduced system length and impact at the downstream LON point may have adversely affected the vehicle trajectory. Comparison of the barrier deflections and vehicle trajectories from test nos. NELON-1, NELON-2, and 2214TB-2 are shown in Figures 100 through 103. Review of these three tests showed that the reduced length system tests displayed higher deflections of barrier segments near the impact of the vehicle and less gradual deflection of adjacent barrier segments as compared to the full-length PCB system. These differences were likely due to both the reduced upstream and downstream tensile forces developed by shorter systems, as evidenced by the increased longitudinal displacement of the ends of the system, and increased barrier damage, as noted previously. The increased deflection of the reduced length systems clearly affected vehicle trajectory. Test nos. NELON-1 and NELON-2 exhibited higher exit angles of 12.3 degrees and 10.4 degrees, respectively, as compared to 7.9 degrees for test no. 2214TB-2. Similarly, comparison of vehicle roll angles during the first 0.500 sec of the vehicle redirection exhibited significantly higher roll for the reduced length systems, as shown in Figure 104. Thus, it was believed that the effect of reduced system length on the PCB deflections contributed to vehicle instability.

A second factor that potentially contributed to the vehicle instability observed in test no. NELON-2 was the impact from a knee formed at the joint between barrier segment nos. 6 and 7. The knee extended forward laterally from the original barrier line and impacted the left-front door of the 2270 P vehicle at approximately 0.390 sec after initial impact. The impact of the knee into the door may have further increased vehicle instability.

NELON-1

0.000 sec

0.100 sec

0.200 sec

0.300 sec

Figure 100. PCB System Comparison, Overhead View

NELON-1
NELON-2
2214TB-2


Figure 101. PCB System Comparison, Overhead View

## NELON-1


0.000 sec

0.100 sec

0.200 sec

0.300 sec

Figure 102. PCB System Comparison, Downstream View

NELON-1

0.400 sec

0.600 sec

0.700 sec

Figure 103. PCB System Comparison, Downstream View


Figure 104. Vehicle Roll Angle Comparison for PCB Testing
While the results from test nos. NELON-1 and NELON-2 found that an eight-barrier long PCB system was not adequate to safely redirect vehicles under MASH TL-3 criteria, it was believed that a nine-barrier long system would be sufficient for safe barrier performance. A nine barrier system would be comprised of three PCB segments before the beginning of LON, one a barrier segment for a finite redirective length, and five barrier segments following the end of the LON, as shown in Figure 105. Test no. NELON-1 demonstrated that three barrier segments prior to the beginning of LON was sufficient for an eight-barrier long PCB system. The addition of a fifth barrier segment to the downstream end of the system provides the same number of downstream barriers for an impact at the end of LON as were utilized in NELON-1. An impact on the end of length of need for a nine-barrier long system would be expected to perform similarly to test no. NELON-1. Thus, it is recommended that the minimum system length for the free-standing, F-shape PCB system be set at nine barrier segments.


Figure 105. Nine Barrier Segment Reduced Length PCB System

## 13 ANALYSIS OF BARRIER DEFLECTIONS

A final component of the research study was an investigation of the PCB system's lateral deflections when used with intermediate system lengths. With the potential for system lengths less than the standard sixteen-barrier long PCB system, it was desired to estimate potential barrier deflections for systems between nine and sixteen barriers long at the midspan and beginning and end of LON for the system.

### 13.1 Simulation Calibration with Full-Scale Crash Test

Although additional crash tests could be conducted to determine the deflection of the reduced length PCB systems, the cost would be extremely high. Instead, LS-DYNA computer simulation of the reduced length systems was used to estimate the deflection of the barrier segments. LS-DYNA was used to model the behavior of the barrier system when subjected to fullscale crash testing. After the model was calibrated to accurately predict barrier deflections for the high-energy crash test conditions, the impact conditions were revised and the barrier deflections were estimated for the lower energy crash.

In order to calibrate the simulation model of the reduced length PCB system, a simulation model of test no. NELON-1 was created and simulated under the test impact conditions. Initial simulations of test no. NELON-1 demonstrated significantly lower deflections than the full-scale test. The discrepancy between the physical test and the model was largely attributed to the concrete damage and fracture observed in the test which was not reproduced in the rigid PCB model.

The researchers discussed applying a LS-DYNA concrete material model in order to capture the concrete damage seen in the physical test. However, this was rejected because of the researchers' limited confidence in the ability of the concrete material model to capture the damage in the full-scale test and a lack of previous experience applying the material model to the simulation of PCB segments. As such, a significant amount of additional component level simulation and modeling would have been required to accurately model a PCB segment using the concrete material model. Additionally, the concrete damage that contributed to the deflections in test no. NELON-1 was distributed through several barrier segments. Thus, capturing the damage would require modeling of fully-reinforced PCB segments with the concrete material model at a fine enough mesh size to capture the barrier segment damage. It was believed that this would be very computationally expensive. Based on these considerations the PCB system deflection was modeled without the concrete material model.

As a compromise, the simulation model of test no. NELON-1 was modified to reduce the barrier to ground friction level until the simulation model reproduced the dynamic barrier deflections observed in the full-scale test. While this was not the optimal solution, it provided a conservative baseline with which to create simulations using the reduced impact conditions. It was believed that the reduction in barrier friction would produce conservative estimates of the deflection of the barrier system. The concrete damage in the simulation model, for which the reduced friction was acting as a surrogate, would not be as large of a factor for impacts involving larger system lengths, as those systems tend to display less barrier damage. Thus, the reduction in friction would likely generate larger estimated deflections than explicit modeling of concrete damage and provide a conservative result.

A simulation model of the reduced deflection PCB system tested in test no. NELON-1 was simulated using a reduced barrier-to-ground friction coefficient of 0.27 . The results from this model estimated a dynamic lateral barrier deflection of 126.9 in . ( $3,223 \mathrm{~mm}$ ). This value correlated well with the 128.3 in. ( $3,259 \mathrm{~mm}$ ) dynamic lateral barrier deflection from test no. NELON-1. Comparison of sequential images from the simulation and crash test also demonstrated good correlation, as shown in Figures 106 through 109. Thus, the model with a reduced friction coefficient was used to simulate deflections for the intermediate system lengths.

NELON-1

0.000 sec

0.100 sec

0.200 sec

0.300 sec

Figure 106. Test No. NELON-1 vs. LS-DYNA Simulation, Overhead View

NELON-1

0.400 sec

0.600 sec

0.700 sec

Figure 107. Test No. NELON-1 vs. LS-DYNA Simulation, Overhead View

NELON-1
LS-DYNA Simulation

0.100 sec
0.000 sec


$$
0.200 \mathrm{sec}
$$

Figure 108. Test No. NELON-1 vs. LS-DYNA Simulation, Downstream View

NELON-1


Figure 109. Test No. NELON-1 vs. LS-DYNA Simulation, Downstream View

### 13.2 TL-3 PCB Deflections for Intermediate System Lengths

In order to estimate the lateral barrier deflections for intermediate system lengths, a series of simulations were conducted on the F-shape PCB system with varying lengths at the beginning of LON, the midspan of the system, and the end of LON. System lengths of 9, 10, 12, 14, and 16 barriers were simulated. Note that no midspan simulation was conducted for the nine-barrier long system as this location would have been outside of the LON of the barrier. As noted previously, a reduced barrier-to-ground friction coefficient of 0.27 was used for the simulations to better correlate with the full-scale testing conducted near the ends of the system. While simulating the barriers with the reduced friction value may overestimate barrier deflections, it was believed that a conservative approach was warranted when estimating potential system deflections. The simulation model of the midspan impact on the sixteen barrier F-shape PCB system used the original barrier-to-ground friction coefficient of 0.40 , as this model had previously been validated against test no. 2214 TB -2. All simulations were conducted using the MASH TL-3 impact conditions of $62 \mathrm{mph}(100 \mathrm{~km} / \mathrm{h})$ and an angle of 25 degrees with the 2270P vehicle model.

The lateral barrier deflection results from the simulations of intermediate systems lengths are shown in Figure 110. Lateral barrier deflections were plotted versus the number of barriers in the system for the beginning of LON, end of LON, and midspan impacts. The beginning of LON impacts demonstrated the highest lateral deflections as the impact was closer to the end of the system than the other conditions. The lateral barrier deflection values for the beginning of LON impacts ranged from 125.5 in . ( $3,188 \mathrm{~mm}$ ) to 131.6 in . $(3,343 \mathrm{~mm}$ ) and tended to increase slightly as the number of barriers in the system increased. The lateral deflections did not vary significantly due to the proximity of the impact to the free end of the system having a greater effect than the length of the system. Similarly, barrier deflections did not decrease as system length increased, as would typically be expected, because the increased system length provided more anchorage at the downstream end of the system and created increased loading and deflection of the upstream end of the barriers.

Lateral barrier deflections for impacts at the end of LON displayed similar behavior. The lateral barrier deflection values for the end of LON impacts ranged from 111.7 in . $(2,837 \mathrm{~mm})$ to 121.9 in . $(3,096 \mathrm{~mm})$ and tended to increase slightly as the number of barriers in the system increased. Deflection magnitude decreased as compared to the beginning of LON impacts due to the impact being two barrier segments farther from the free end of the system. However, a similar trend toward an increase in barrier deflections with increased barrier system length was noted.

Finally, midspan impacts on intermediate length F-shape PCB systems demonstrated the lowest lateral barrier deflections. The lateral barrier deflection values for the midspan impacts ranged from $114.6 \mathrm{in} .(2,911 \mathrm{~mm})$ for a 10 -barrier long system to 81.1 in . $(2,060 \mathrm{~mm})$ for a $16-$ barrier long system. For the midspan impacts, lateral barrier deflection tended to decrease as system length increased.


Figure 110. Lateral Barrier Deflections for Intermediate PCB System Lengths, MASH TL-3

### 13.3 85 $^{\text {th }}$ Percentile Impact Severity PCB Deflections for Intermediate System Lengths

Previous research at MwRSF investigated PCB deflection limits for less critical PCB installations [21]. This research argued that when temporary concrete barriers are used on the edge of a bridge, the risk of the entire line of barriers falling off the deck requires that deflection limits be selected to preclude such behavior in almost all impact scenarios. Hence, it was recommended that at the edge of a bridge deck, design deflection limits should be selected to contain more than 95 percent of all crashes. In all other barrier applications, the consequences of a barrier exceeding the design deflection criteria are not severe. In these situations, a more modest deflection limit criterion based on an 85th percentile impact severity was deemed more appropriate. The sponsor of this research effort requested that a similar analysis be performed on the low-deflection PCB system developed herein in order to provide deflection limits for less critical installations.

A number of research studies have shown that the impact severity (IS), as defined below, is a good indicator of the degree of loading and the lateral deflections of longitudinal barriers [2224].

$$
\frac{1}{2} m(v \sin \theta)^{2}
$$

where:

$$
\begin{aligned}
& \mathrm{m}=\text { mass of impacting vehicle } \\
& \mathrm{v}=\text { velocity of impacting vehicle } \\
& \theta=\text { angle of impact }
\end{aligned}
$$

IS incorporates the effect of the mass of the impacting vehicle to provide a good measure of the severity of impact and the magnitude of the resulting barrier deflections. In order to determine appropriate IS values for this study, data was taken from the results of the NCHRP 2217 project [25]. NCHRP 22-17 was used to generate the impact conditions for MASH and represented the most applicable data set to draw from. While the NCHRP 22-17 data was biased toward severe and fatal crashes, it was believed that the dataset would provide a conservative basis for the analysis that correlated with the impact conditions specified in MASH.

Figure 111 shows the IS distribution for freeways from NCHRP 22-17. As shown in Figure 111 , the $95^{\text {th }}$ percentile IS value was $127.6 \mathrm{kip}-\mathrm{ft}(173.0 \mathrm{~kJ})$. It was reasonable to utilize the deflections measured during full-scale crash testing no. 2214TB-2 when selecting barrier deflection limits for use near the edge of a bridge deck or drop-off or other critical installations. However, the $85^{\text {th }}$ percentile IS value, which is more appropriate for all other applications of temporary concrete barriers, was $78.3 \mathrm{kip}-\mathrm{ft}(106.2 \mathrm{~kJ})$. An IS value of $78.3 \mathrm{kip}-\mathrm{ft}(106.2 \mathrm{~kJ})$ would correspond to an impact velocity of $51.2 \mathrm{mph}(82.4 \mathrm{~km} / \mathrm{h})$ for a $5,000-\mathrm{lb}(2,268-\mathrm{kg})$ pickup truck impacting the barrier at an angle of 25 degrees. Barrier deflections under this impact condition would be less than those observed under the MASH TL-3 criteria.

Thus, a second series of computer simulations were conducted on the F-shape PCB system to estimate lateral dynamic barrier deflections for the $85^{\text {th }}$ percentile IS value. Simulations were conducted on the F-shape PCB system with $9,10,12,14$, and 16 barriers and at the beginning of LON, the midspan of the system, and the end of LON. Note that no midspan simulation was conducted for the nine-barrier long system as this location would have been outside of the LON of the barrier. As noted previously, a reduced barrier-to-ground friction coefficient of 0.27 was used for the simulations to better correlate with the full-scale testing conducted near the ends of the system. The simulation model of the midspan impact on the sixteen barrier F-shape PCB system used the original barrier-to-ground friction coefficient of 0.40 , as this model had previously been validated against test no. 2214TB-2. All simulations where conducted using the $85^{\text {th }}$ percentile IS impact conditions of $51.2 \mathrm{mph}(82.4 \mathrm{~km} / \mathrm{h})$ and an angle of 25 degrees with the 2270P vehicle model.

The lateral barrier deflections from the simulations of intermediate systems lengths using $85^{\text {th }}$ percentile IS impact conditions are shown in Figure 112. Lateral barrier deflections were plotted versus the number of barriers in the system for the beginning of LON, end of LON, and midspan impacts. The beginning of LON impacts demonstrated the highest lateral deflections as the impact was closer to the end of the system than the other conditions. The lateral barrier deflection values for the beginning of LON impacts ranged from 86.9 in . $(2,207 \mathrm{~mm})$ to 96.8 in . $(2,459 \mathrm{~mm})$ and tended to increase slightly as the number of barriers in the system increased. The lateral deflections did not vary significantly due to the proximity of the impact to the free end of the system having a greater effect than the system length. Similarly, barrier deflections did not decrease as system length increased, as would typically be expected, because the increased system length provide more anchorage of the downstream end of the system and created increased loading and deflection of the upstream end of the barriers.


Figure 111. NCHRP 22-17 IS Distribution for Freeways


Figure 112. Lateral Barrier Deflections for Intermediate PCB System Lengths, $85^{\text {th }}$ Percentile IS

Lateral barrier deflections for impacts at the end of LON displayed similar behavior. The lateral barrier deflection values for the end of LON impacts ranged from 81.0 in . ( $2,057 \mathrm{~mm}$ ) to $84.2 \mathrm{in} .(2,139 \mathrm{~mm})$ and tended to increase slightly as the number of barriers in the system increased. The deflection magnitude decreased as compared to the beginning of LON impacts due to the impact being two barrier segments farther from the free end of the system. However, a similar trend toward an increase in barrier deflections with increased barrier system length was noted.

Finally, midspan impacts on intermediate length F-shape PCB systems demonstrated the lowest lateral barrier deflections, which ranged from 81.3 in . ( $2,065 \mathrm{~mm}$ ) for a 10 -barrier long system to 67.7 in . ( $1,720 \mathrm{~mm}$ ) for a 16-barrier long system. For the midspan impacts, lateral barrier deflection tended to decrease as system length increased.

### 13.4 Discussion

Determination of guidance for lateral barrier deflections for varying system lengths under TL-3 and $85^{\text {th }}$ percentile IS impact conditions was dependent on several factors:

1. The estimated lateral barrier deflections taken from the simulation models
2. The MASH TL-3 full-scale crash test deflections of the F-shape PCB
3. The effect of the location along the length of the barrier

The estimated lateral barrier deflections observed in the simulation models of intermediate system lengths were discussed in detail in the previous sections. The available full-scale crash test data consisted primarily of test no. 2214TB-2. In this test, a 2270 P vehicle impacted the F-shape PCB used in this study and exhibited a dynamic deflection of 79.6 in . $2,021 \mathrm{~mm}$ ) when impacting near the middle of a 16 -barrier test system with an overall length of $200 \mathrm{ft}(61 \mathrm{~m})$. Test nos. NELON-1 and NELON-2 were not directly considered for the deflection guidance as they were conducted on eight-barrier long systems, which was below the recommended minimum system length. However, it was noted that the lateral barrier deflections for test nos. NELON-1 and NELON-2 of 128.3 in . ( $3,259 \mathrm{~mm}$ ) and 127.8 in . $(3,247 \mathrm{~mm})$, respectively, were significantly higher than the values for an impact near the midspan of a longer system.

The third factor that was taken into consideration was the impact location along the barrier length. The initial simulations used to locate potential beginning and end of LON locations on a barrier system with sixteen F-shape PCBs indicated that lateral and longitudinal barrier deflections increased for impacts along several barrier segments adjacent to the beginning and end of LON locations. Thus, similar behavior would be expected for barrier systems with varying lengths. Review of the simulations for the beginning of LON showed that the combination of lateral and longitudinal barrier deflections began to increase significantly when the barrier was impacted upstream of barrier segment no. 5 or two barrier segments downstream of the beginning of LON. This would suggest that barrier deflections in the region between the beginning of LON and two barriers downstream of the beginning of LON would be similar and greater than impacts closer to the midspan of the system. Similarly, review of simulated impacts near the end of LON found that the combination of lateral and longitudinal barrier deflections appeared to increase more significantly downstream of barrier segment no. 10 or two barrier segments upstream of the end of LON.

In order to provide guidance for deflections for varying PCB system lengths, it was proposed to divide the barrier system into three separate deflection regions, as shown in Figures 113 and 114. Region A was defined as the beginning of LON following the end of the third barrier in the system to two barriers downstream of the beginning of LON. Region C was defined as the end of LON to two barriers upstream of the end of LON. Regions A and C were expected to have increased barrier deflections associated with their proximity to the beginning and end of LON. Region B was defined as a region comprised of the remaining middle section of the barrier system that would have deflections that corresponded with impacts at the midspan of the PCB system length.

Deflection guidance for each region based on PCB system length is provided in Figures 113 and 114. Figure 113 displays the barrier deflection guidance for MASH TL-3 impact conditions and Figure 114 displays the barrier deflection guidance for the $85^{\text {th }}$ percentile IS impact condition. For simplicity and ease of implementation, the lateral barrier deflections in regions A and C were assumed to be the same. The magnitude of the lateral deflection was selected in a conservative manner due to the use of computer simulation to determine the values. Thus, the deflection for regions A and C were selected as the maximum lateral deflection predicted by the simulations over the range of system lengths for both the beginning or end of the LON point. The deflection of region $B$ was based on the simulated lateral deflections for a midspan impact on the various system lengths. Note that for system lengths of 12 barriers or less, region B does not exist and the deflection values for regions A and C are used throughout the LON. System lengths greater than or equal to 16 barrier segments are assumed to have similar lateral deflections in all regions. Also, deflection guidance was not provided for the areas outside of the beginning and end of the LON point as the performance of the PCB system in these areas is unknown.

It is recommended that installations in non-critical locations use the estimated lateral deflection values for the $85^{\text {th }}$ percentile IS impact provided in Figure 114 until further full-scale crash testing at reduced IS values or in-service evaluation of system damage for lower severity impacts indicate that lower deflection estimates are more appropriate. For critical installations adjacent to drop-offs or bridge deck edges, the MASH TL-3 system deflections provided in Figure 113 should be applied.


| PCB System Length <br> (No. of Barrier Segments) | MASH TL-3 Estimated Lateral Barrier Deflection in. (mm) |  |  |
| :---: | :---: | :---: | :---: |
|  | Region A | Region B | Region C |
|  | - | $132(3,353)$ |  |
|  | $132(3,353)$ | - | $132(3,353)$ |
|  | $132(3,353)$ | - | $132(3,353)$ |
|  | 12 | $132(3,353)$ | $91(2,311)$ |

Figure 113. F-Shape PCB Lateral Deflection Guidance, MASH TL-3


|  | PCB System Length (No. of Barrier Segments) | $85^{\text {th }}$ Percentile IS Estimated Lateral Barrier Deflection in. (mm) |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Region A | Region B | Region C |
|  | 9 | $97(2,464)$ |  | $97(2,464)$ |
|  | 10 | $97(2,464)$ | - | $97(2,464)$ |
|  | 12 | $97(2,464)$ | - | $97(2,464)$ |
| $\underset{\sim}{4}$ | 14 | $97(2,464)$ | $78(1,981)$ | $97(2,464)$ |
|  | $\geq 16$ | $97(2,464)$ | $68(1,727)$ | $97(2,464)$ |

Figure 114. F-Shape PCB Lateral Deflection Guidance, $85^{\text {th }}$ Percentile IS

## 14 SUMMARY, CONCLUSIONS, AND RECOMMENDATIONS

This study consisted of analysis, full-scale crash testing, and evaluation of the LON for a minimum length, free-standing, F-shaped PCB system. LS-DYNA computer simulation was the primary tool used to analyze the PCB system. A baseline model of the F-shape PCB system was developed and verified, and impacts along the entire length of the PCB system were simulated to determine potential beginning and end of LON points for the barrier at its standard length. The simulation results found that three barriers on the upstream end of the system were sufficient to define beginning of LON and three barriers on the downstream end of the system were sufficient to define end of LON.

A second series of LS-DYNA simulations were conducted on reduced length PCB systems to evaluate if the selected beginning and end of LON points remained viable for shorter systems. Simulation of a seven barrier segment PCB system suggested that vehicle redirection with the reduced length was possible, but concerns about the impacts at the end of LON of the system arose due to rotation of the final barrier segment into the doors of the impacting vehicle. Simulation of an eight-barrier long F-shape PCB system with one additional barrier segment added downstream of the end of LON mitigated the rotation of the end of the PCB segment into the side of the vehicle, but impact of a knee between two barrier segments on the door was noted.

In order to further evaluate the selected beginning and end of LON and the reduced system length, full-scale crash testing was performed on an eight-barrier long F-shape system. Two full scale crash tests were performed according to the TL-3 safety performance criteria defined in MASH, test designation no. 3-35 and a modified version of test designation no. 3-37. Test no. NELON-1 evaluated the effectiveness of the beginning of LON for a system with a minimal length and test no. NELON-2 assessed the end of LON for a system with a minimal length.

Test no. NELON-1 consisted of a 4,991-lb (2,264-kg) pickup truck impacting the PCB at a speed of $62.1 \mathrm{mph}(100.0 \mathrm{~km} / \mathrm{h})$ and at an angle of 24.8 degrees, resulting in an impact severity of 113.6 kip-ft ( 154.0 kJ ). The vehicle was successfully contained and smoothly redirected with moderate damage to the barrier and the vehicle. All vehicle decelerations fell within the recommended safety limits established in MASH. Thus, test no. NELON-1 passed the safety criteria of MASH test designation no. 3-35.

Test no. NELON-2 consisted of a 5,005-lb (2,270-kg) pickup truck impacting the PCB at a speed of $63.0 \mathrm{mph}(101.4 \mathrm{~km} / \mathrm{h})$ and at an angle of 24.5 degrees, resulting in an impact severity of $113.8 \mathrm{kip}-\mathrm{ft}(154.3 \mathrm{~kJ})$. The vehicle was successfully contained and redirected with moderate damage to the barrier and the vehicle. All vehicle decelerations fell within the recommended safety limits, however, the vehicle's maximum roll exceeded the 75 degree limit established in MASH. Thus, test no. NELON-2 did not pass the safety requirements for MASH test designation no. 3-37. A summary of the safety performance evaluation for both tests is provided in Table 13, and a comparison of test results is provided in Table 14.

Review of the results from both crash tests suggested that reduced length and impacts near the beginning and end of LON of the PCB system affected the performance of the barrier. Barrier deflections increased significantly and the vehicle stability was reduced. However, the successful result from test no. NELON-1 led to the recommendation that a nine-barrier long PCB system could meet the MASH TL-3 safety requirements. Thus, a minimum system length of nine barriers
was recommended with three barriers upstream of the beginning of LON and five barriers downstream of the end of LON. It should be noted that the recommended minimum length of nine barriers would pertain to a roadside PCB installation with potential impacts restricted to oncoming traffic. If the PCB installation is adjacent to narrow, opposing two lane traffic or was a median installation where the potential for impacts in opposing directions on the PCB system exist, a minimum of five barriers is required on each end of LON of the system to account for impacts in both directions of travel. This would set the minimum system length at eleven barriers for these types of installations.

The final task undertaken in this research was evaluation of the estimated lateral displacements of the reduced length F-shape PCB system under both MASH TL-3 and $85^{\text {th }}$ percentile IS impact conditions. Previous research at MwRSF suggested that it was feasible to use deflection limits for PCB systems in non-critical areas based on the estimated deflection of the PCB system when impacted at the $85^{\text {th }}$ percentile IS value, as determined from accident data. Computer simulation analysis was performed on the F-shape PCB with lengths ranging from 9 to 16 PCBs and estimated lateral barrier deflections were provided for the barrier system for both MASH TL-3 and the $85^{\text {th }}$ percentile IS based on PCB system length. The recommended lateral barrier deflections varied relative to the location of the impact along the LON of the barrier system. The MASH TL-3 barrier deflection guidance was recommended for critical PCB installations, while the $85^{\text {th }}$ percentile IS barrier deflection guidance was recommended for general PCB use in non-critical areas.

Determination of the beginning and end of LON and minimum system length for the Fshape PCB required to meet MASH TL-3 provides users with the option to use shorter PCB installations than have been previously recommended. Shorter length PCB systems have installation advantages in terms of flexibility and the reduction of the number of impacts. Additionally, longer installations can define the beginning and end of LON using three and five barrier segments, respectively, rather than the eight barriers previously recommended.

Table 13. Summary of Safety Performance Evaluation Results, Test Nos. NELON-1 and NELON-2

| Evaluation Factors | Evaluation Criteria |  |  | $\begin{gathered} \hline \text { Test No. } \\ \text { NELON-1 } \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Test No. } \\ \text { NELON-2 } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Structural <br> Adequacy | A. Test article should contain and stop; the vehicle should not pe controlled lateral deflection of | ct the vehicle or br , underride, or ove st article is accept | vehicle to a controlled installation although | S | S |
| Occupant Risk | D. Detached elements, fragments or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone. Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.3 and Appendix E of MASH. |  |  | S | S |
|  | F. The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees. |  |  | S | U |
|  | H. Occupant Impact Velocity (OIV) (see Appendix A, Section A5.3 of MASH for calculation procedure) should satisfy the following limits: |  |  | S | S |
|  | Occupant Impact Velocity Limits |  |  |  |  |
|  | Component | Preferred | Maximum |  |  |
|  | Longitudinal and Lateral | $30 \mathrm{ft} / \mathrm{s}(9.1 \mathrm{~m} / \mathrm{s})$ | $40 \mathrm{ft} / \mathrm{s}(12.2 \mathrm{~m} / \mathrm{s})$ |  |  |
|  | I. The Occupant Ridedown Acceleration (ORA) (see Appendix A, Section A5.3 of MASH for calculation procedure) should satisfy the following limits: |  |  | S | S |
|  | Occupant Ridedown Acceleration Limits |  |  |  |  |
|  | Component | Preferred | Maximum |  |  |
|  | Longitudinal and Lateral | 15.0 g's | 20.49 g 's |  |  |
| MASH Test Designation |  |  |  | 3-35 | 3-37 <br> (modified) |
| Final Evaluation (Pass or Fail) |  |  |  | Pass | Fail |

S - Satisfactory U - Unsatisfactory NA - Not Applicable

Table 14. Comparison of Test Results, Test Nos. NELON-1 and NELON-2

| Test No. |  | NELON-1 |  | NELON-2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| MASH Test Designation |  | 3-35 |  | 3-37 (modified) |  |
| Vehicle Weight $\mathrm{lb}(\mathrm{kg})$ |  | $\begin{gathered} 4,991 \\ (2,264) \\ \hline \end{gathered}$ |  | $\begin{gathered} 5,005 \\ (2,270) \\ \hline \end{gathered}$ |  |
| Impact Severity kip-ft (kJ) |  | $\begin{array}{r} 113.6 \\ (154.0) \\ \hline \end{array}$ |  | $\begin{gathered} 113.8 \\ (154.3) \\ \hline \end{gathered}$ |  |
| Contact Length ft (m) |  | $\begin{gathered} 29 \mathrm{ft}-3 \mathrm{in} . \\ (8.9) \end{gathered}$ |  | $\begin{gathered} 30 \mathrm{ft} .-2 \mathrm{in} . \\ (9.2) \end{gathered}$ |  |
| $\begin{gathered} \text { ORA } \\ \text { g's } \end{gathered}$ | Lateral | -6.63 | -6.92 | -5.73 | -6.45 |
|  | Longitudinal | 16.76 | 15.20 | 13.48 | 11.01 |
| OIV <br> $\mathrm{ft} / \mathrm{s}(\mathrm{m} / \mathrm{s})$ | Lateral | -14.57 (-4.44) | -13.73 (-4.18) | -12.86 (-3.92) | -11.94 (-3.64) |
|  | Longitudinal | 15.68 (4.78) | 16.92 (5.16) | 15.49 (4.72) | 17.59 (5.36) |
| Exit Time (sec) |  | . 526 |  | . 528 |  |
| Exit Velocity mph (km/h) |  | $\begin{gathered} 44.8 \\ (72.0) \\ \hline \end{gathered}$ |  | $\begin{gathered} 39.4 \\ (63.4) \\ \hline \end{gathered}$ |  |
| Exit angle (degrees) |  | 12.3 |  | 10.4 |  |
| Permanent Set in. (mm) |  | $\begin{gathered} 128 \\ (3,251) \\ \hline \end{gathered}$ |  | $\begin{gathered} 126 \\ (3,207) \\ \hline \end{gathered}$ |  |
| Dynamic Deflectionin. (mm) |  | $\begin{gathered} 128.3 \\ (3,259) \\ \hline \end{gathered}$ |  | $\begin{gathered} 127.8 \\ (3,247) \\ \hline \end{gathered}$ |  |
| Working Width in. (mm) |  | $\begin{gathered} 150.8 \\ (3,831) \\ \hline \end{gathered}$ |  | $\begin{gathered} \hline 150.3 \\ (3,818) \\ \hline \end{gathered}$ |  |
| Final Evaluation |  | Pass |  | Fail |  |

### 14.1 Recommendations

Several recommendations should be made regarding the research described herein. First, while the use of a nine-barrier long F-shape PCB system was deemed acceptable under MASH TL-3, end users should be cognizant of the increased lateral barrier deflections for these shorter installations and should account for correspondingly increased clear areas behind the PCBs to account for these deflections. Similarly, PCB installations should account for larger clear areas behind the PCBs near the ends of the barrier length to account for increased deflection observed with vehicle impacts near the ends of the system.

It may be desired to use the research developed herein to establish minimum system lengths and beginning and end of LON guidance for other PCB systems. However, the behavior of any PCB system can be significantly affected by differences in barrier segment length, barrier reinforcement and structural capacity, barrier shape, and the connection design. Due to the potential effect of these differences on barrier performance and the fact that the tests evaluated herein were near the limits of the barrier performance, the reduced system lengths and LON definitions developed are not recommended for use with other PCB systems without further research and evaluation.

Finally, the research effort has indicated that system lengths may be reduced significantly as compared to current guidance. The current research indicates that three and five barriers will be sufficient to define the beginning and end of LON, respectively, and safely redirect vehicles impacting between both points. This would shorten PCB installations approximately 44 percent as compared to current guidance. However, impacts between the beginning and end of LON and the ends of the system have not been evaluated. Computer simulations have indicated that vehicle impacts outside the LON may produce large barrier deflections, vehicle instability, increased barrier loading, and other hazards. Thus, research is needed to further investigate the potential hazards associated with impacts outside the proposed LON and to develop methods to safely terminate the PCB system in order to make effective use of reduced system lengths. Potential methods could include anchored system ends, flared barrier system ends, and/or shielded system ends. There is also the potential to evaluate critical impacts outside of the LON and determine if the system is crashworthy in areas beyond the LON that are outside the scope of the current study.

## 15 REFERENCES

1. Ross, H.E., Sicking, D.L., Zimmer, R.A., and Michie, J.D., Recommended Procedures for the Safety Performance Evaluation of Highway Features, National Cooperative Highway Research Program (NCHRP) Report 350, Transportation Research Board, Washington, D.C., 1993.
2. Manual for Assessing Safety Hardware (MASH), American Association of State Highway and Transportation Officials (AASHTO), Washington, D.C., 2009.
3. Faller, R.K., Rohde, J.R., Rosson, B.T., Smith, R.P., and Addink, K.H., Development of a TL-3 F-Shape Temporary Concrete Median Barrier, Final Report to the Midwest States Regional Pooled Fund Research Program, Report No. TRP-03-64-96, Midwest Roadside Safety Facility, University of Nebraska-Lincoln, Lincoln, Nebraska, December 1996.
4. Polivka, K.A., Faller, R.K., Sicking, D.L., Rohde, J.R., Bielenberg, B.W., Reid, J.D., and Coon, B.A., Performance Evaluation of the Free-Standing Temporary Barrier - Update to NCHRP 350 Test No. 3-11 with 28" C.G. Height (2214TB-2), Final Report to the National Cooperative Highway Research Program (NCHRP), Report No. TRP-03-174-06, Midwest Roadside Safety Facility, University of Nebraska-Lincoln, Lincoln, Nebraska, October 2006.
5. R. P. Bligh, N. M. Sheikh, W. L. Menges, and R. R. Haug. Development of Low-Deflection Precast Concrete Barrier. Report No. 0-4162-3. Texas Transportation Institute, College Station, TX, January 2005.
6. Bielenberg, R.W., Quinn, T.E., Faller, R.K., Sicking, D.L., and Reid, J.D., Development of a Retrofit, Low-Deflection, Temporary Concrete Barrier System, Final Report to the Wisconsin Department of Transportation, Report No. TRP 03-295-14, Midwest Roadside Safety Facility, University of Nebraska-Lincoln, Lincoln, Nebraska, March 31, 2014.
7. Society of Automotive Engineers (SAE), Instrumentation for Impact Test - Part 1 Electronic Instrumentation, SAE J211/1 MAR95, New York City, NY, July, 2007.
8. Mongiardini, M., Ray, M.H., Plaxico, C.A., Anghileri, M., Procedures for Verification and Validation of Computer Simulations Used for Roadside Safety Applications, Final Report to the National Cooperative Highway Research Program (NCHRP), NCHRP Report No. W179, Project No. 22-24, Worcester Polytechnic Institute, March 2010.
9. Bielenberg, B.W., Faller, R.K., Rohde, J.R., Reid, J.D., Sicking, D.L., and Holloway, J.C., Development of Tie-Down and Transition Systems for Temporary Concrete Barrier on Asphalt Road Surfaces, Final Report to the Midwest States Regional Pooled Regional Pooled Fund Program, Report No. TRP 03-180-06, Midwest Roadside Safety Facility, University of Nebraska-Lincoln, Lincoln, Nebraska, February 23, 2007.
10. Gutierrez, D.A., Bielenberg, R.W., Faller, R.K., Reid, J.D., and Lechtenberg, K.A., Development of a Mash TL-3 Transition Between Guardrail and Portable Concrete Barriers, Final Report to the Nebraska Department of Roads, Report No. TRP-03-300-14, Midwest Roadside Safety Facility, University of Nebraska-Lincoln, Lincoln, Nebraska, June 26, 2014.
11. Buth, C. E., Hirsch, T. J., and McDevitt, C. F., Performance Level 2 Bridge Railings, Transportation Research Record No. 1258, Transportation Research Board, National Research Council, Washington, D.C., 1990.
12. Guide Specifications for Bridge Railings, American Association of State Highway and Transportation Officials (AASHTO), Washington, D.C., 1989.
13. Bronstad, M. E., Calcote, L. R., and Kimball Jr, C. E., Concrete Median Barrier ResearchVol. 2 Research Report, Report No. FHWA-RD-77-4, Submitted to the Office of Research and Development, Federal Highway Administration, Performed by Southwest Research Institute, San Antonio, TX, March 1976.
14. Buth, C. E., Campise, W. L., Griffin III, L. I., Love, M. L., and Sicking, D. L., Performance Limits of Longitudinal Barrier Systems-Volume I: Summary Report, FHWA/RD-86/153, Final Report to the Federal Highway Administration, Office of Safety and Traffic Operations R\&D, Performed by Texas Transportation Institute, Texas A\&M University, College Station, TX, May 1986.
15. Polivka, K.A., Faller, R.K., Sicking, D.L., Rohde, J.R., Bielenberg, B.W., Reid, J.D., and Coon, B.A., Performance Evaluation of the Permanent New Jersey Safety Shape Barrier - Update to NCHRP 350 Test No. 3-10 (2214NJ-1), Final Report to the National Cooperative Highway Research Program, Report No. TRP-03-117-06, Midwest Roadside Safety Faciliy, University of Nebraska-Lincoln, Lincoln, Nebraska, October 13, 2006.
16. Fortuniewicz, J. S., Bryden, J. E., and Phillips, R. G., Crash Tests of Portable Concrete Median Barrier for Maintenance Zones, Report No. FHWA/NY/RR-82/102, Final Report to the Office of Research, Development, and Technology, Federal Highway Administration, Performed by the Engineering Research and Development Bureau, New York State Department of Transportation, December 1982.
17. Hinch, J., Yang, T.L., and Owings, R., Guidance Systems for Vehicle Testing, ENSCO, Inc., Springfield, Virginia, 1986.
18. Center of Gravity Test Code - SAE J874 March 1981, SAE Handbook Vol. 4, Society of Automotive Engineers, Inc., Warrendale, Pennsylvania, 1986.
19. Vehicle Damage Scale for Traffic Investigators, Second Edition, Technical Bulletin No. 1, Traffic Accident Data (TAD) Project, National Safety Council, Chicago, Illinois, 1971.
20. Collision Deformation Classification - Recommended Practice J224 March 1980, Handbook Volume 4, Society of Automotive Engineers (SAE), Warrendale, Pennsylvania, 1985.
21. Sicking, D.L., Reid, J.D., and Polivka, K.A., Deflection Limits for Temporary Concrete Barriers, Revised Final Report to the Midwest States Regional Pooled Fund Program, Report No. TRP-03-113-03, Midwest Roadside Safety Facility, University of NebraskaLincoln, Lincoln, Nebraska, June 18, 2003.
22. Bronstad, M.E., and Michie, J.D., Multiple Service-Level Highway Bridge Railing Selection Procedures, National Cooperative Highway Research Program (NCHRP) Report No. 239, Transportation Research Board, Washington, D.C., November 1981.
23. Sicking, D.L., Guidelines for Positive Barrier Use in Construction Zones, Transportation Research Record No. 1035, Transportation Research Board, National Research Council, Washington D.C., 1985.
24. Mak, K.K., and Sicking, D.L., Evaluation of Performance Level Selection Criteria for Bridge Railings, Final Report, NCHRP Project 22-8, Texas Transportation Institute, Texas A\&M University, September 1993.
25. Mak, K.M., Sicking, D.L., Benicio, F.D., and Coon, B.A., NCHRP Report 665 Identification of Vehicular Impact Conditions Associated with Serious Run-Off-Road Crashes, Final Report to the National Cooperative Highway Research Program (NCHRP) NCHRP Report No. 665, Project No. 17-22, University of Nebraska, 2010.

## 16 APPENDICES

## Appendix A. Material Specifications

Table 15. Bill of Materials, Test Nos. NELON-1 and NELON-2

| Item <br> No. | QTY. | Description | Material Spec | Hardware Guide | Reference |
| :---: | :---: | :---: | :---: | :---: | :---: |
| d1 | 8 | Portable Concrete Barrier | $\begin{gathered} \min \mathrm{f}^{\prime} \mathrm{c}=5000 \mathrm{psi}[34.5 \\ \mathrm{MPa}] \end{gathered}$ | - | See Test Report, NDOR LON Barriers R\#160198, page 11 |
| d2 | 7 | $\begin{gathered} 1 \text { 1/4" [32] Dia., 28" } \\ \text { [711] Long } \\ \text { Connector Pin } \end{gathered}$ | ASTM A36 | FMW02 | H\#737194 |
| d3 | 96 | 1/2" [13] Dia., 72" [1829] Long Form Bar | ASTM A615 Grade 60 | - | H\#581898 |
| d4 | 16 | $\begin{gathered} \hline 1 / 2^{\prime \prime} \text { [13] Dia., 146" } \\ \text { [3708] Long } \\ \text { Longitudinal Bar } \end{gathered}$ | ASTM A615 Grade 60 | - | H\#62133981/02 |
| d5 | 24 | $\begin{gathered} \text { 5/8" [16] Dia., 146" } \\ \text { [3708] Long } \\ \text { Longitudinal Bar } \end{gathered}$ | ASTM A615 Grade 60 | - | H\#58022182/02 |
| d6 | 48 | 3/4" [19] Dia., 36" <br> [914] Long Anchor Loop Bar | ASTM A615 Grade 60, Epoxy Coated or Galvanized | - | H\#57147246/02 |
| d7 | 16 | $\begin{gathered} 3 / 4 " \text { [19] Dia., 102" } \\ \text { [2591] Long } \\ \text { Connection Loop Bar } \end{gathered}$ | ASTM A709 Grade 70 or A706 Grade 60, Epoxy Coated or Galvanized | - | H\#KN15101113 |
| d8 | 16 | 3/4" [19] Dia., 91" [2311] Long Connection Loop Bar | ASTM A709 Grade 70 or A706 Grade 60, Epoxy Coated or Galvanized | - | H\#KN15101113 |
| d9 | 16 | $\begin{gathered} \text { 3/4" [19] Dia., 101" } \\ \text { [2565] Long } \\ \text { Connection Loop Bar } \end{gathered}$ | ASTM A709 Grade 70 or A706 Grade 60, Epoxy Coated or Galvanized | - | H\#KN15101113 |



| MECHANICAL PROPERTIES |  |
| :---: | :---: |
| Elygg. | BendTest |
| 15.60 | OK |


| GEOMETRIC CHARACTERISTICS |  |  |  |
| :---: | :---: | :---: | :---: |
| 8Leph | Det Het | Def Gap | Deefsace |
| -2.00 | 0.334 | 0.117 | 03 |

COMMENTS / NOTES
Material $100 \%$ meted and rolled in the USA. Manufaceuring processes for this steel, which may inchude scrap melied in an electric arc furrace and hot rolling, has been performed at Gerdau Se. Paul Mill, 1678 Red Rock Rd. St. Paul. Mimesoas, USA. Al produces produced from strand cass billers. Silicom kiiked (deoxidized) seect. No weld repaimecut performed. Sweel not exposed to mercury or any liguid alloy which is niguvid at ambient temperasures during processing or while in Gerdau St. Paul Mill's possession. Any modification to this cerstication as orovided by Gerdan - St. Paui Mill without the expressed writen consent of Gerdau St. Paul Mill negates the validity of chis tess report. This responslbie for the inabbility of this material to meet specific applicarione
Rell batch 62:33981002 roll did $2 / 1772014$

## The above figures are certified chemical and physical test records as comtained in the permanent records of company. This material, including the billets, was melted and hanactured in the USA. CMIR complies with EN 102043 3.l

Whaekery bhaskar yalamanchili mality director

Figure A-1. $1 / 2$-in. (13-mm) Dia., 146-in. (3,708-mm) Long Longitudinal Steel Bars, Test Nos. NELON-1 and NELON-2


| Heat Number | Sample No. | MECHANICALPROPERTIES |  |  |  | (Tensiles test date 09/11/15) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{aligned} & \text { Yicld } \\ & \text { (Psi) } \end{aligned}$ | Untimate <br> (Psi) | Elengation (\%) | Reductisa <br> (\%) | Bend | Wtht |
| \$81898 | 01 |  | 68691 | 94670 | 16.0 |  | Ок | 0.666 |
|  |  | (MPs) | 473.6 | 652.7 |  |  |  |  |
| 581898 | 02 |  | 67769 | 93410 | 14.3 |  | OK | 0.664 |
|  |  | (MPa) | 467.3 | 644.0 |  |  |  |  |

All melting and manuffacturing processes of the material subject to this lest certificate occurred in the United States of America-
ERMS also centifies this material to be free from Mercury cootomination.
This materiat has been produced, tested and conforros to the
requirements of the applicable specitications. We hereby certify that the
sbove test resulis represent those contained in the records of the Company.
Methods used: ASTM A370, A510, A615, A706.
Material test report shall not be reproduced except in full, without approval of the company.

Figure A-2. $1 / 2$-in. (13-mm) Dia., 72-in. ( $1,828-\mathrm{mm}$ ) Long Form Bar, Test Nos. NELON-1 and NELON-2

## ABC COATING CO. OF MINNESOTA, INC.

3200 COMO AVENUE SE MINNEAPOLIS, MN 55414
(612) 378-1855

FAX (612) 378-3262

| DATE SHIPPED : |  | CONTR: | CONCRETE INDUSTRIES |
| :---: | :---: | :---: | :---: |
| ABC JOB NO. : | NE 461 | COUNTY: | LINCOLN,NE |
| CUSTOMER: | CONCRETE INDUSTRIES | PROJECT: | 8006PIPE |
| P.O. \# | 8006PIPE | RELEASE : | 6.8 E |
|  |  |  | BARRIER CURB BARS |

WE CERTIFY THAT THE FOLLOWING DESCRIBED BAR MATERIAL HAS BEEN CLEANED, COATED WITH 3 M $* 413$ OR O'BRIEN 7-2719 OR VALSPAR \# 720A009 POWDER. INSPECTED IN ACCORDANCE WITH AND MEETS THE SPECIFICATION REQUIREMENTS OF THE NEBRASKA DEPARTMENT OF TRANSPORTATION AND ASTM A775-07b ,AASHTO M-284-06, ASTM D3963-01. MANUFACTURES CERTIFICATIONS FOR THE BAR MATERIAL ARE ON FILE


Figure A-3. 3/4-in. (19-mm) Dia. Connection Loop Bar, Test Nos. NELON-1 and NELON-2

# ABC Coating Company, Inc. (An Acuĩa Co. ) 

 Epoxy Coated Reinforcing Steel Test Record Minneapolis, MNAxalte 7-2719 - Powder Lot \#H H
Epoxy Coated in accordance with ASTM A 775 / A 775M - 07b

$$
\text { Date: } \quad 11-3-15
$$

Preheat Temperature Line 2: $1463^{\circ}-475^{\circ}$
Gel Time Line 2: 10 Seconds
Holidays Line 2: 145
Shift: Day (2) Night ()
w, $x$
$3 / 4 \quad A 7 C 6$ Bend Test



Certilied By :
Plant Operator Danicl Parra
Figure A-4. 3/4-in. (19-mm) Dia. Connection Loop Bar, Test Nos. NELON-1 and NELON-2


The above figures are certified chemical and physical test records as contained in the permanent records of company. We certify that these data are correct and in compliance with
specified requirements. This material, including the billets, was melted and manufactured in the USA. CMTR complies with EN 102043.1

$$
\text { Whaek ony buskar yalamancimu } \quad \text { Quauty director }
$$

Figure A-5. 5/8-in. (16-mm) Dia. Longitudinal Bar, Test Nos. NELON-1 and NELON-2


The above figures are certified chemical and physical test reconds as contained in the permanent records of company. We certify that these data are correct and in compliance with specified requirements. This material, including the billets, was melted and manufactured in the USA. CMTR complies with EN 102043.1 .

$$
\begin{aligned}
& \text { Thaekeory bhaskar yalamanchili foak Chumnebti usachurnetski } \\
& \text { QUAUTY ASSURANCEMCR }
\end{aligned}
$$

Figure A-6. $3 / 4$-in. (19-mm) Dia. Anchor Loop Bar, Test Nos. NELON-1 and NELON-2

# GENERAL TESTING LABORATORIES 

TELEPHONE (402)434-1891
FAX (402)434-2161
P.O. BOX 29529

LINCOLN, NEBRASKA 68529
March 9, 2016
Dave Borchers
Concrete Industries, Inc.
6300 Cornhusker Hwy.
Lincoln, NE 68507

Dear Dave,

Below are the strength values to date for the UNL Barrier Curbs produced at Concrete Industries.

| Cast Date | Release | 7 Day | 28 Day |
| :--- | ---: | :--- | ---: |
| $11 / 6 / 2015$ | 5717 | 6685 | 8012 |
| $11 / 9 / 2015$ | 4102 | 6956 | 8265 |
| $11 / 10 / 2015$ | 4478 | 8115 | 9492 |
| $11 / 11 / 2015$ | 3642 | 7417 | 8765 |
| $11 / 12 / 2015$ | 4666 | 7888 | 8887 |

Figure A-7. Concrete Strength Values, Test Nos. NELON-1 and NELON-2

```
06/13/2006 FrOM: MOREOLK IRON & METAL CO. TO: CONCRETE INDUSTRIES INC
PO 吾 :59101 N Inv 专:01410860
```

$06 / 08 / 2006$ FrOm: NORFOLK IRON \& METAL CO. TO: NORFOLK IRON \& METAL
Inv \#:04009576
BL\#-0238492 P.0.-04009324
Date: $5 / 01 / 06 \quad$ Heat Number: 737194
Post Office Box 309 Noxfolk, Nebraska 68702 Phone (402) 644-0200
Mill Certification
Chemical Testing
Certificate: 0780-01 Chemical Analysis
Acciapmes] Expires: 11/30/06
Test conform to ASTM A29, ASTM E415 and ASTM E1019-resulphurized grades
Spec: A36 ASTM A.36-01 Size: $\frac{1}{1 / 4}$ Rounds
ASME SA-36 E98 1.2500

| C | .17 | p | .02 | Mo | .03 |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Mn | .72 | Cu | .26 | V | .001 |
| Si | .19 | Cr | .13 | Nb | .001 |
| S | .02 | Ni | .10 |  |  |

Physical Properties

| Physical Properties |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Imperial |  | Metric |  |  |  |  |  |  |
| Yield | 48,126 | 46,440 |  | psi | 332 | 320 |  | MPA |  |
| Tensile | 68,999 | 69,609 |  | psi | 476 | 480 |  | MPA |  |
| \% Elongation | - 26 | 24 | \% | in 8" | 26 | 24 | \% | in 2 | 203.3 |
| Str | Cast | Reduct |  | Ratio: | 6:1 |  |  |  |  |

NORFOLK IRON \& METAL CO
1701 E . SOUTH AVE
EMPORIA, KS 66801


All Manufacturing processes, including melting have been performed in the U.S.A. Mercury in any form, has not been used in the production or testing of this material Welding or weld repair was not performed on this material. This matand the specifications described on this document and may not erial conforms to the speci Nucor Corporation. This reproduced except in full, without written approval of in of the NAFTA rules of origin. product is NAFTA certified under Paragraph FORM: 10 F002

Figure A-8. 1¼-in. (32-mm) Dia., 28-in. (71-mm) Long Connector Pin, Test Nos. NELON-1 and NELON-2

## Appendix B. Vehicle Center of Gravity Determination



Figure B-1. Vehicle Mass Distribution, Test No. NELON-1


Figure B-2. Vehicle Mass Distribution, Test No. NELON-2

## Appendix C. Vehicle Deformation Records



Figure C-1. Floor Pan Deformation Data - Set 1, Test No. NELON-1


Figure C-2. Floor Pan Deformation Data - Set 1, Test No. NELON-2


Figure C-3. Floor Pan Deformation Data - Set 2, Test No. NELON-1


Figure C-4. Floor Pan Deformation Data - Set 2, Test No. NELON-2


Figure C-5. Occupant Compartment Deformation Data - Set 1, Test No. NELON-1


Figure C-6. Occupant Compartment Deformation Data - Set 1, Test No. NELON-2


Figure C-7. Occupant Compartment Deformation Data - Set 2, Test No. NELON-1


Figure C-8. Occupant Compartment Deformation Data - Set 2, Test No. NELON-2


Figure C-9. Exterior Vehicle Crush (NASS) - Front, Test No. NELON-1


Figure C-10. Exterior Vehicle Crush (NASS) - Front, Test No. NELON-2


Figure C-11. Exterior Vehicle Crush (NASS) - Side, Test No. NELON-1


S
Figure C-12. Exterior Vehicle Crush (NASS) - Side, Test No. NELON-2

## Appendix D. Accelerometer and Rate Transducer Data Plots, Test No. NELON-1



Figure D-1. 10-ms Average Longitudinal Deceleration (SLICE-1), Test No. NELON-1


Figure D-2. Longitudinal Occupant Impact Velocity (SLICE-1), Test No. NELON-1


Figure D-3. Longitudinal Occupant Displacement (SLICE-1), Test No. NELON-1


Figure D-4. 10-ms Average Lateral Deceleration (SLICE-1), Test No. NELON-1


Figure D-5. Lateral Occupant Impact Velocity (SLICE-1), Test No. NELON-1

Figure D-6. Lateral Occupant Displacement (SLICE-1), Test No. NELON-1


Figure D-7. Vehicle Angular Displacements (SLICE-1), Test No. NELON-1


Figure D-8. Acceleration Severity Index (SLICE-1), Test No. NELON-1


Figure D-9. 10-ms Average Longitudinal Deceleration (SLICE-2), Test No. NELON-1


Figure D-10. Longitudinal Occupant Impact Velocity (SLICE-2), Test No. NELON-1


Figure D-11. Longitudinal Occupant Displacement (SLICE-2), Test No. NELON-1


Figure D-12. 10-ms Average Lateral Deceleration (SLICE-2), Test No. NELON-1


Figure D-13. Lateral Occupant Impact Velocity (SLICE-2), Test No. NELON-1


Figure D-14. Lateral Occupant Displacement (SLICE-2), Test No. NELON-1


Figure D-15. Vehicle Angular Displacements (SLICE-2), Test No. NELON-1

## Acceleration Severity Index (ASI) - SLICE-2

NELON-1


Figure D-16. Acceleration Severity Index (SLICE-2), Test No. NELON-1

## Appendix E. Accelerometer and Rate Transducer Data Plots, Test No. NELON-2



Figure E-1. 10-ms Average Longitudinal Deceleration (SLICE-1), Test No. NELON-2


Figure E-2. Longitudinal Occupant Impact Velocity (SLICE-1), Test No. NELON-2


Figure E-3. Longitudinal Occupant Displacement (SLICE-1), Test No. NELON-2


Figure E-4. 10-ms Average Lateral Deceleration (SLICE-1), Test No. NELON-2


Figure E-5. Lateral Occupant Velocity (SLICE-1), Test No. NELON-2


Figure E-6. Lateral Occupant Displacement (SLICE-1), Test No. NELON-2


Figure E-7. Vehicle Angular Displacements (SLICE-1), Test No. NELON-2


Figure E-8. Acceleration Severity Index (SLICE-1), Test No. NELON-2

## Longitudinal CFC-180 10-msec Extracted Average Acceleration - SLICE-2

NELON-2

Figure E-9. 10-ms Average Longitudinal Deceleration (SLICE-2), Test No. NELON-2


Figure E-10. Longitudinal Occupant Impact Velocity (SLICE-2), Test No. NELON-2


Figure E-11. Longitudinal Occupant Displacement (SLICE-2), Test No. NELON-2


Figure E-12. 10-ms Average Lateral Deceleration (SLICE-2), Test No. NELON-2

## Lateral Change in Velocity - SLICE-2

NELON-2


CFC-180 Extracted Lateral change in velocity ( $\mathrm{m} / \mathrm{s}$ )
Figure E-13. Lateral Occupant Impact Velocity (SLICE-2), Test No. NELON-2


Figure E-14. Lateral Occupant Displacement (SLICE-2), Test No. NELON-2

Euler Angular Displacements -SLICE-2
NELON-2


Figure E-15. Vehicle Angular Displacements (SLICE-2), Test No. NELON-2

## Acceleration Severity Index (ASI) - SLICE-2

NELON-2


Figure E-16. Acceleration Severity Index (SLICE-2), Test No. NELON-2

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