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Gkantou, M, Theofanous, M and Baniotopoulos, C (2019) A NUMERICAL STUDY OF PRESTRESSED HIGH STRENGTH STEEL TUBULAR MEMBERS. Frontiers of Structural and Civil Engineering. ISSN 2095-2430

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A NUMERICAL STUDY OF PRESTRESSED HIGH STRENGTH STEEL TUBULAR MEMBERS

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Abstract: The structural behaviour of prestressed high strength steel (HSS) tubular members is investigated through the execution of advanced finite element modelling. Numerical models are developed and validated against published experimental data on HSS tubular members subjected to different levels of initial prestress and loaded either in tension or compression. The effect of the presence or absence of grouting on the strength and ductility of the members is also considered. To numerically replicate the structural response recorded in the tests, some key modelling features including the employed numerical solver, the adopted material models and the element types warrant careful consideration. Upon developing of the finite element models, the numerically generated ultimate loads, the corresponding failure modes and the full load-deformation curves are compared to the experimental ones, indicating a successful validation. As anticipated, prestressing enhances the load-bearing capacity for the tensile members, whereas it is detrimental for the compressive ones. A series of parametric studies is performed to assess the influence of key factors on the structural response of prestressed HSS members and the obtained results are discussed. Design guidance for tensile and compressive prestressed tubular members is also provided.

Keywords: Finite Element (FE), Prestressing, Tubular members, Grout, High Strength Steel (HSS)

1 INTRODUCTION

The application of prestressing or post-tensioning has been traditionally used in concrete structures to counteract the deflections and limit the tensile stresses generated by the applied loads during the in-service condition, thus leading to enhanced performance and more efficient design particularly for long-span applications. Work on prestressed steel structures dates back to 1950, when Magnel [1] discussed the potential material savings that could emerge from prestressing steel structures. The behaviour of prestressed load-bearing steel structures was later on analysed by Belenya [2], whilst a technique to erect arched trusses with sliding joints through prestressing of a cable, which also forms part of the load bearing structure, has been patented by Ellen [3]. Research on stressed-arch frames has been extensively carried out in Australia [4-5], whereas the behaviour of prestressed steel structures has been recently examined by a series of research studies [6-14]. Some of them [11-14] investigated the behaviour of steel structures, in which the prestressing cables are positioned within tubular sections, hence providing a practical means of prestressing the surrounding tubular member, whilst also protecting the prestressing cables. A series of long span steel tubular structures that exploit the advances offered by the use of prestress has already been developed by the Australian-based company S-Squared [15].

In order to allow for enhanced material savings, the gains in strength and stiffness due to the application of prestress could be combined with the benefits arising from the use of higher steel grades. In particular, steels with strength over 460 N/mm^2 could potentially allow for lighter structures, more economic design and profound sustainability benefits. The performance of high strength steels has been extensively studied [16-24].

Aiming to enhance the current understanding of the structural response of prestressed HSS tubular steel members and structures, the present study investigates the structural performance of prestressed members employing hot-finished square hollow sections (SHS) in high strength steel grades via advanced finite element (FE) modelling. Steel grades S460 and S690 are considered as representative of the low and high end of the strength range considered as high-strength respectively. The investigated members are SHS and are intended to be representative of typical structural components forming the bottom chord of tubular trusses. Since a bottom chord of a truss can be subjected to either tension or compression depending on the applied load (i.e. gravity loads and snow or uplift), both tension and compression of prestressed SHS is considered. Details on the execution of the finite element modelling are meticulously presented. Upon calibration of the models, a parametric study is performed to assess the influence of key factors and appropriate design recommendations are made.

2 NUMERICAL MODELLING

The structural performance of prestressed HSS tubular members subjected to either tension or compression is investigated by means of numerical modelling with the general-purpose FE package Abaqus [25]. This section provides a detailed description of the developed finite element (FE) models. An overview of the FE models along with the experimental programme [14] upon which the validation of the FE models was based is briefly discussed in Subsection 2.1. Subsections 2.2-2.5 state the adopted numerical modelling assumptions including the employed material models, element types, constraints, boundary conditions and interaction properties. The introduction of the initial geometric imperfections, the application of the prestress load and the type of analysis performed are discussed in Subsections 2.6, 2.7 and 2.8 respectively.

2.1 Overview of developed FE models

The initial stage of this research included the development of a nonlinear numerical model suitable for the simulation of tubular members subjected to different prestress levels and loaded either in tension or compression. The selection of the geometric and material properties of the simulated members was informed by a recent experimental study that reported tests on prestressed tubular members tested to failure under tensile or compressive loading [14]. The nomenclature employed herein is identical to the one employed in [14]. In the aforementioned study, the tubes employed 50×50×5 square hollow sections (SHS) in S460 and S690 steel grades and a 7-wire strand, with a nominal cross-sectional area of 150 mm² and a nominal ultimate tensile stress of 1860 N/mm², which was housed inside the SHS. The members subjected to tension were two meters long, whilst the ones subjected to compression were one meter long. In order to mitigate premature buckling of the tubular members under the application of the initial prestress load, steel collars were attached to the cables along the member length. The steel collars were marginally smaller than the internal SHS dimensions, hence they would come in contact with the surrounding tube upon initiation of buckling, thus activating the stiffness of the prestressed cable to restore the members to their straight configuration. To facilitate calibration of our models, the adopted overall and cross-sectional dimensions as well as the employed steel grades were identical to the ones reported in [14].

In line with [14], for the members subjected to tension (T), the following four prestress levels were studied for both steel grades:

- no cable (NG)
- a cable with a nominal level of prestress P_{nom} to eliminate slack prestress (NG0)
- a cable with a level of applied prestress equal to half of the optimal prestress level $0.5P_{opt}$ (NG1)
- a cable with an applied prestress level equal to the optimal prestress level P_{opt} prestress (NG2).

Note that the optimal prestress level P_{opt} is defined as the level of prestress for which both the tube and the cable yield simultaneously under in tension and is given from Eq. (1) [3]

$$P_{opt} = \left(\frac{A_c A_t}{A_t E_t + A_c E_c} \right) (f_{cy} E_t - f_{ty} E_c) \quad \text{but } P_{opt} \leq A_t f_{ty} \quad \text{and} \quad A_c f_{cy} \quad (1)$$

where A , E and f_y are the cross-sectional area, Young's modulus and yield strength; the subscripts c and t refer to the cable and surrounding steel tube respectively. Fracture of the wire strands combined with necking of the surrounding tube was the observed failure mode for all members subjected to tension.

For the steel members subjected to compression (C), the following two levels of prestress were investigated:

- no cable (NG)
- a cable with P_{opt} prestress (NG2).

The compressive members primarily failed by global buckling.

In order to ensure good bonding properties and corrosion resistance, grouting can be added within the tube of the members. For prestressed members subjected to tensile loading, the addition of concrete grouting is expected to have a minimal positive effect on the system's overall performance, since concrete is weak in tension. For prestressed members subjected to compression, the addition of concrete grouting is expected to lead to a significant enhancement of the compressive capacity due to the enhanced stiffness and strength of the resulting composite member. To evaluate the effect of grouting to the structural response of prestressed tubular members, two prestress levels, namely $0.5P_{opt}$ (G1) and P_{opt} (G2), were considered for the tensile grouted members (T) and three prestress levels, namely P_{nom} (G0), $0.5P_{opt}$ (G1) and P_{opt} (G2), for the compressive grouted members (C). Note that NG and G stands for non-grouted and grouted members respectively. A summary of the developed numerical models which were in line with the tested specimens in [14] is provided in Table 1.

Table 1. Summary of the developed numerical models.

Tensile members (T)
<ul style="list-style-type: none"> • no cable (NG) • a cable with P_{nom} prestress (NG0) • a cable with $0.5P_{opt}$ prestress (NG1) • a cable with P_{opt} prestress (NG2)
Tensile grouted members (TG)
<ul style="list-style-type: none"> • $0.5P_{opt}$ (G1) • P_{opt} (G2)
Compressive members (C)
<ul style="list-style-type: none"> • no cable (NG) • a cable with P_{opt} prestress (NG2)
Compressive grouted members (CG)
<ul style="list-style-type: none"> • P_{nom} (G0) • $0.5P_{opt}$ (G1) • P_{opt} (G2)

2.2 Material modelling

For the steel tubes, the cable and the collars, the von Mises yield criterion with isotropic hardening was employed to represent the material response. The uniaxial stress-strain response of the SHS and the cable was extracted from material coupon tests. For the steel tubes, the average material response as extracted from S460 and S690 tensile coupon tests [22] and shown in Figure 1(a) has been adopted. For the cables, material tests found an average yield stress and Young's modulus equal to 1703 N/mm^2 and 130000 N/mm^2 respectively [14]. A 2-stage Ramberg-Osgood model according to the equations provided in [26] has been employed to simulate the cable's response and the resulting stress-strain curve is shown in Figure 1(b). For the steel collars, an elastic-perfectly plastic response with yield stress 355 N/mm^2 has been assumed, given that these components were not expected to be subjected to high stresses.

To simulate the response of concrete in both tension and compression, the concrete damaged plasticity (CDP) constitutive model [25] available in the material library of ABAQUS has been used. CDP employs a Drucker-Prager type of yield surface and a non-associated flow rule to simulate the plastic response of concrete in compression and accounts for the effect of crushing in compression or cracking in tension via stiffness degradation. The values introduced for the angle of dilation θ , the eccentricity, the ratio of equibiaxial to uniaxial compressive stress f_{b0}/f_{c0} , the ratio of the second stress invariant on the tensile meridian to that on the compressive meridian at initial yield and the viscosity parameter were 40, 0.1, 1.16, 0.666 and 0 respectively, in line with [27]. The Young's modulus E and tensile strength f_{ct} were determined according to ACI from the following equations: $E = 4700 \times f_c^{0.5}$ and $f_{ct} = 0.33 \times f_c^{0.5}$ [28], where f_c the grout's compressive strength equal to 50 N/mm^2 for the present study [14]. The Poisson's ratio was assumed equal to 0.2. The compressive stress-strain response was formulated according to the recommendations provided in [27] as shown in Figure 1(c). The tension stiffening behaviour was defined in terms of tensile stress f_{ct} and fracture energy, the latter evaluated equal to 0.09, as a function of the cube's compressive strength and the aggregate's size. Note that the influence of the assumed values for the dilation angle and the fracture energy was preliminary studied, revealing minor variations in the attained response for different values of the aforementioned quantities.

Damage properties have not been included in any of the material models of the present study, since the applied loads were monotonic. The stress-strain curves were converted into the true stress-logarithmic plastic strain format before their input into the software.

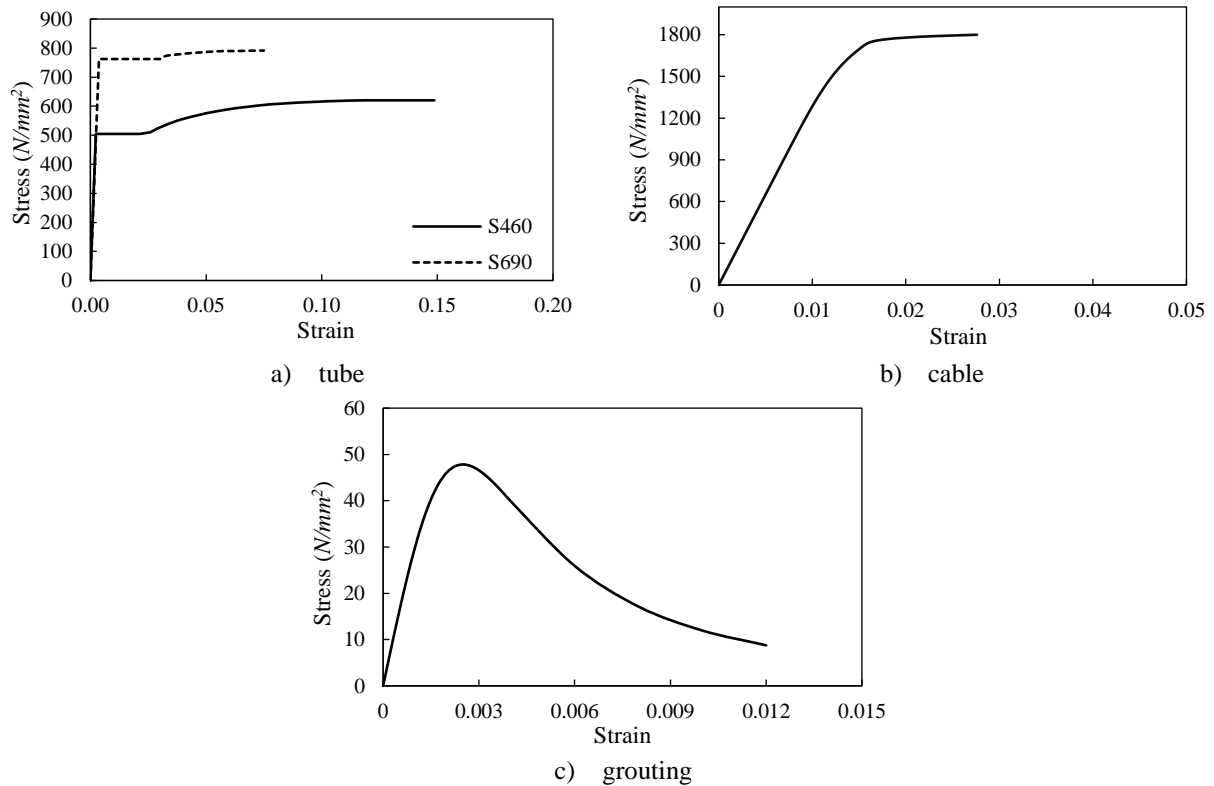


Figure 1. Material properties.

2.3 Element types

In FE modelling, the structural parts can be simulated with different element types. The most commonly applied elements are solid, shell and beam elements. Continuum or solid elements simply model small blocks of material in a component. Shell elements approximate a three-dimensional continuum with a surface model and are used to model structures in which the one dimension (the thickness) is significantly smaller than the other dimensions and the stresses in the thickness direction are negligible. In beam elements, one dimension (the length) is significantly greater than the other two dimensions and the longitudinal stress is the most important.

An initial study examined the behaviour of non-grouted prestressed members for the following three cases of employed FE element types: i) shell elements for the tube, beam elements for the cable, ii) shell elements for the tube, solid elements for the cable, solid elements for the collars; iii) solid elements for the tube, solid elements for the cable, solid elements for the collars. Similar response was achieved for all cases considered. Given that the concrete of grouted members could only be simulated with solid elements and in order to maintain consistency between grouted and non-grouted models, it was decided to further investigate model (iii). Three linear solid elements were used through the thickness of the steel tubes, to accurately capture localised bending of the tubular face should it occur. For the simulation of the cable, the 7-strand wire was simplified to a circular solid beam section with a cross-sectional area equal to the effective cable area of 150 mm^2 . An initial mesh convergence study was performed to ensure accuracy whilst keeping computational time to a minimum.

2.4 Constraints and boundary conditions

For both compressive and tensile members a rigid body was assigned in the end sections and the motion was constrained to a reference point (RP) to facilitate the application of the boundary conditions. For tensile members, all degrees of freedom were restrained at the RPs, except for the translational degree of freedom along the longitudinal axis which was left unrestrained in one of the two ends. For compressive members, the rotational degrees of freedom around the two transverse member's axes were also left unrestrained at the RPs of the end sections. In order to reduce the required computational time, the symmetry in geometry, applied load and obtained response was exploited. Half of the structural system was modelled along the longitudinal axis and appropriate symmetry boundary conditions were

assigned. Some typical FE models including the applied boundary conditions and constraints are shown in Figure 2.

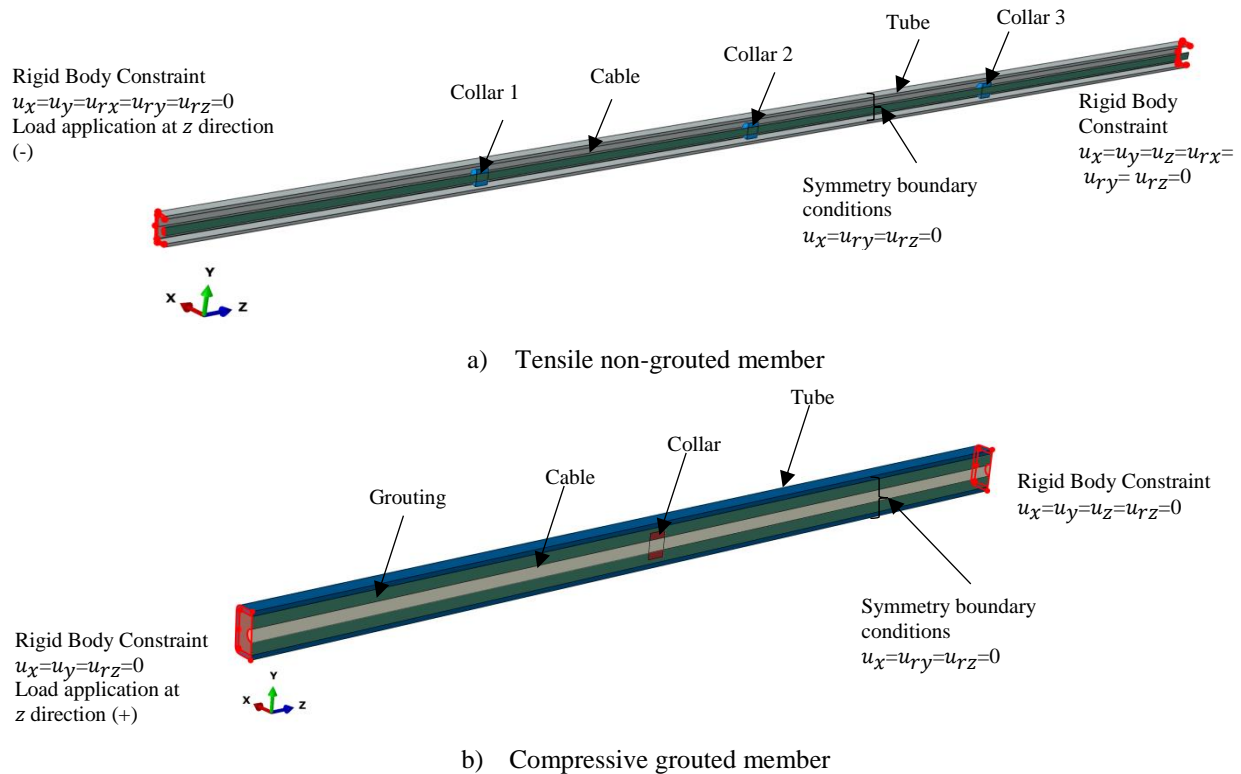


Figure 2. Typical finite element models.

2.5 Interaction properties

A tie constraint which equates the degrees of freedom of the constrained entities using a master-slave formulation was employed to simulate the attachment of the steel collars to the cables. For the contact between (i) tube and cable and (ii) tube and collar in non-grouted members, and between (i) grouting and collar (ii) grouting and tube and (iii) grouting and cable in grouted members, the general contact algorithm of Abaqus [25] was applied. For normal contact, the “hard” contact relationship that minimises the penetration has been adopted. A friction coefficient was employed to define the tangential behaviour over contacting surfaces. A preliminary parametric study demonstrated that the effect of the frictional coefficient on the overall performance is small for the given application. In line with [27], a friction coefficient equal to 0.3 was adopted for the tangential contact behaviour.

2.6 Geometric imperfections and residual stresses

In order to incorporate the initial geometric imperfections in the structural system, a linear buckling analysis was initially performed and the lowest buckling mode shape corresponding to the observed failure mode was introduced in the subsequent analysis. It is noteworthy that only tie constraints have been used in the linear buckling analysis step for the extraction of the lowest elastic buckling mode shapes of the tubes, which were subsequently utilised as an initial geometric imperfection pattern. Residual stresses were of low magnitude for the hollow sections considered in the present study as documented in [22] and thus they were not explicitly modelled.

2.7 Initial prestress

Upon introduction of geometric imperfections, the stresses arising due to prestress were introduced through an initial static analysis. The extracted stress state was inserted as a predefined stress field in the subsequent nonlinear analysis. The introduction of the prestress resulted in a magnification of the initial geometric imperfection for the members under compression and conversely in a reduction of imperfections for the members in tension. The method

of introducing the prestress as a predefined temperature field has also been found in literature [12-13] but the simpler, nevertheless efficient, method of executing an initial static analysis was preferred.

2.8 Analysis techniques

Implicit solvers and incremental-iterative procedures (e.g. Newton-Raphson) are commonly employed to determine the load-displacement equilibrium of a structure in nonlinear static problems. Even though this method appeared suitable for the compressive non-grouted specimens, the combined material, geometric and contact nonlinearities made convergence difficult for the grouted members subjected to tension. This is because the material response of concrete in tension is not monotonic and hence the stiffness matrix is not positive definite unless a technique like static stabilisation or viscous regularisation is employed. For this reason and in order to maintain consistency within the whole study the explicit solver, in which the response is obtained incrementally by explicitly advancing the kinematic state from the previous increment, was applied for the analyses hereafter. The explicit solver is meant to be used for dynamic problems as it relies on a truly dynamic algorithm for time integration of the equilibrium equations, which necessitates the use of several hundreds small but computationally inexpensive increments. Given that the problem considered was static, a quasi-static analysis (i.e. a dynamic analysis over a time large enough to render inertia effects negligible) was performed. The chosen analysis period, mass scaling and smooth time-history of the applied load led ensured that the kinetic energy (ALLKE in Abaqus history output) was only a very small fraction (less than 3%) of the internal energy (ALLIE in Abaqus history output) over the largest part of the analysis. The analyses were executed at the BlueBEAR Cluster possessed by the University of Birmingham. In order to mitigate dynamic oscillations after the occurrence of member buckling, large values of time periods needed to be applied for the compressive members.

3 VALIDATION OF THE FINITE ELEMENT MODELS

Adopting the modelling assumptions described in Section 2, the numerical results were validated against the test data reported in [14]. Subsection 3.1 provides the comparison between the experimental and the numerical response of the tensile members. Subsection 3.2 focuses on the compressive members and reports an imperfection study to determine the magnitude that leads to the closest agreement with the test data. The obtained failure modes are presented in Subsection 3.3.

3.1 Validation of model for tensile members

The numerically obtained ultimate loads $N_{u,FE}$ for the simulated members in tension were compared to the experimental ones $N_{u,Exp}$. The results are reported in Table 2, where an excellent agreement with the experimental results can be observed. The load-deformation response of some typical cases is shown in Figure 3, where a very good replication of the overall load-deformation response can be observed. It is noteworthy that the continuous drops in the experimental curve of Figure 3(c) are due to the successive fracture of wires comprising the cable. This feature could be captured numerically with explicit modelling of the strands, as was done in [29], but would necessitate the explicit modelling of the actual cable geometry, the adoption of a suitable damage initiation and propagation criterion calibrated against experimental results, a very fine mesh to capture the localised fracture and significantly more computational time. All these were beyond the scope of the present study, as shortly upon the fracture of one wire the rest follow.

Table 2. Tensile specimens - Comparison of numerical and experimental [14] ultimate loads.

Specimen	$N_{u,FE}/N_{u,Exp}$
T460NG	1.02
T460NG0	1.06
T460NG1	0.98
T460NG2	1.04
T460G1	0.98
T460G2	0.95
T690NG	0.94
T690NG0	0.98
T690NG1	0.94
T690NG2	0.97
T690G1	0.93
T690G2	0.94
Mean	0.98
COV	0.04

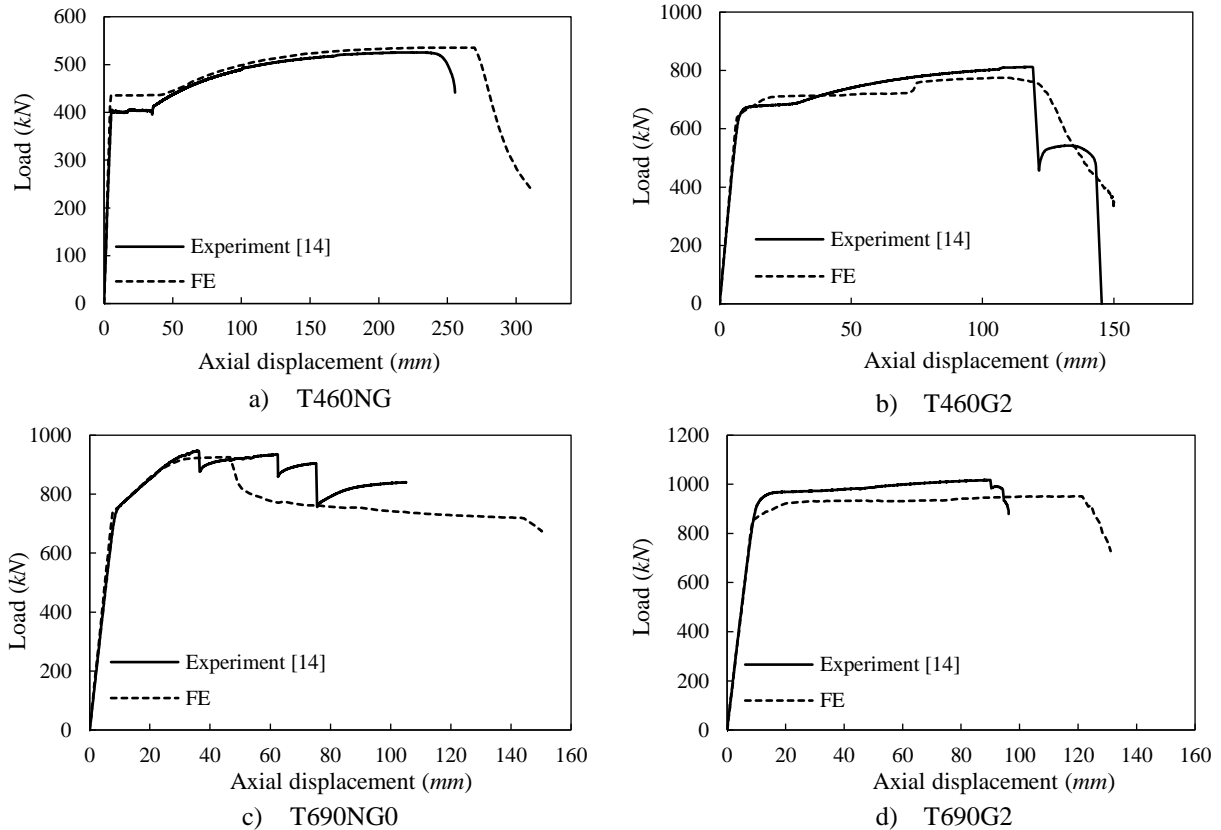


Figure 3. Tensile specimens - Comparison of numerical and experimental [14] load-deformation responses.

3.2 Validation of model for compressive members

The compressive specimens are expected to fail by global buckling and hence the amplitude of the initial geometric imperfections affects the ultimate response. In order to find the imperfection magnitude that results in the best agreement with the experimental results, the following six magnitudes were investigated: (i) measured imperfection magnitudes, as given in [14], (ii) $L/3000$, (iii) $L/2000$, (iv) $L/1500$, (v) $L/1000$ and (vi) $L/750$, where L the member's length. The numerically generated ultimate loads were compared to the experimental ones, as shown in Table 3. As can be seen, best agreement with a mean value of $N_{u,FE}/N_{u,Exp}$ 0.97 and relatively large but still acceptable coefficient of variation, was achieved for the magnitude $L/1000$. This magnitude has been employed in the subsequent parametric

studies. The load-deformation response for the considered imperfection magnitudes was also recorded, demonstrating overall good agreement with the test response, as shown for typical cases in Figure 4.

Table 3. Compressive specimens - Comparison of numerical and experimental [14] ultimate loads.

Specimen	$N_{u,FE}/N_{u,Exp}$					
	Imperfection Magnitude					
	Measured	L/3000	L/2000	L/1500	L/1000	L/750
C460NG0	1.02	1.08	1.06	1.05	1.00	0.96
C460NG2	0.90	1.12	1.01	1.01	0.95	0.91
C460G0	1.10	1.16	1.14	1.11	1.04	1.00
C460G1	1.17	1.33	1.28	1.25	1.19	1.15
C460G2	1.17	1.30	1.17	1.17	1.14	1.06
C690NG0	0.95	0.96	0.95	0.94	0.92	0.91
C690NG2	0.88	0.91	0.89	0.88	0.86	0.83
C690G0	0.91	0.91	0.90	0.88	0.86	0.84
C690G1	0.93	1.02	1.00	0.99	0.96	0.94
C690G2	0.88	0.83	0.80	0.80	0.78	0.76
Mean	0.99	1.06	1.02	1.01	0.97	0.93
COV	0.12	0.16	0.14	0.14	0.13	0.12

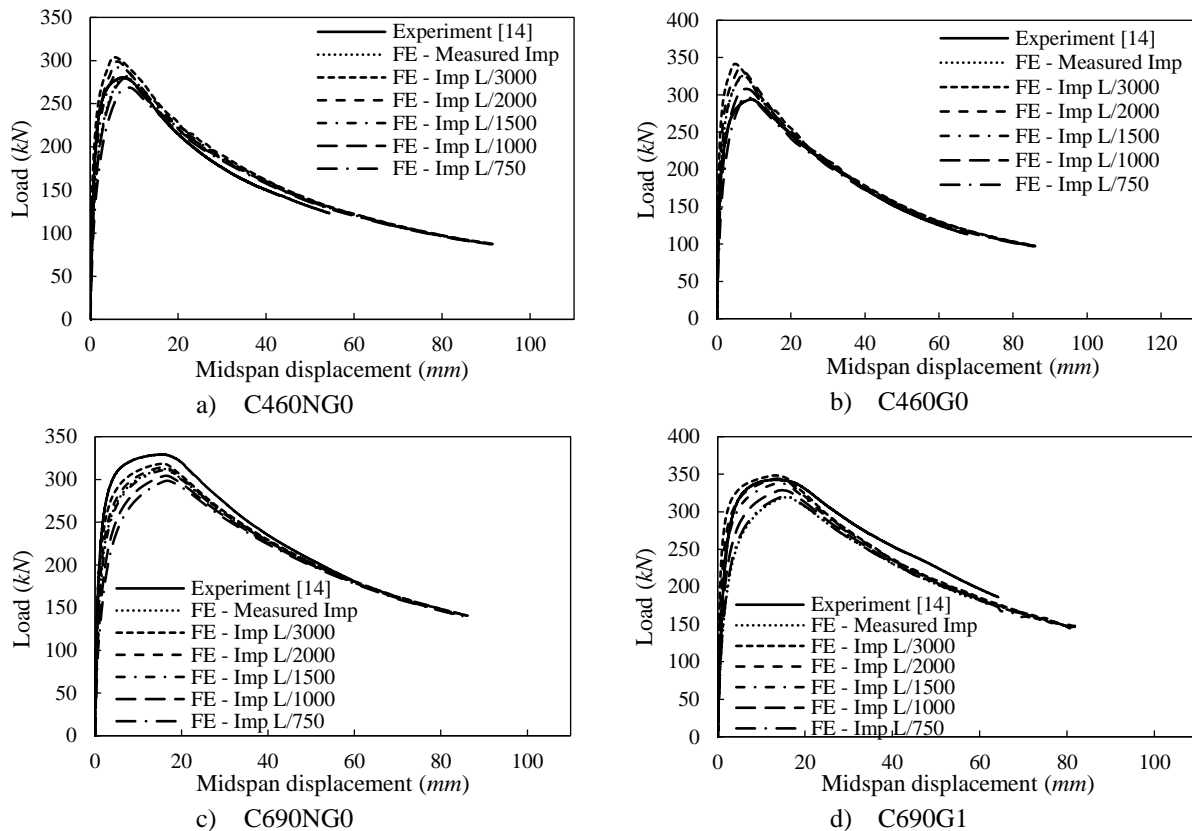
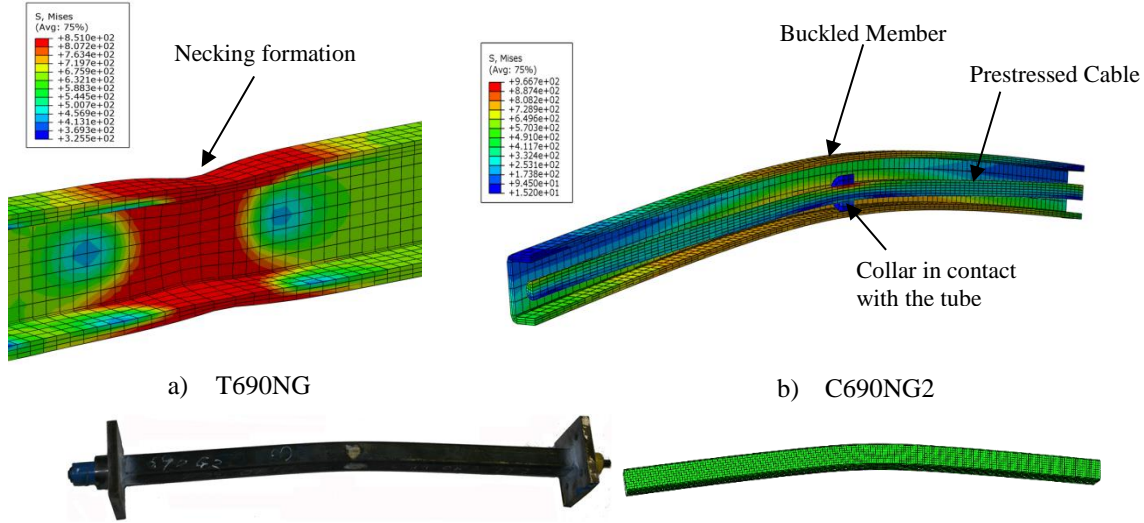


Figure 4. Compressive specimens - Comparison of numerical and experimental [14] load-deformation responses.

3.3 Failure modes

In addition to the comparison of the experimental to FE ultimate loads and load-deformation curves, the failure modes were also successfully replicated. Figures 5(a) and 5(b) depicts typical numerically obtained failure modes for the tensile (i.e. necking formation) and compressive (i.e. global buckling) members. The true Von Mises stresses are

also displayed. Note that even though no material damage has been introduced in the models, necking has been successfully captured, demonstrating that the formation of necking is a geometric and not a material instability as was demonstrated in [30]. In Figure 5(c), the experimental failure mode [14] is compared with the numerically generated one, showing a good agreement.



c) Comparison of experimental [14] and numerical failure mode
Figure 5. Numerically obtained failure modes.

4 PARAMETRIC STUDIES

Based on the numerically obtained results, a series of parametric studies are performed in order to study the effect of key factors on the performance of prestressed tubular members. The parametric studies are described in Subsection 4.1, whilst the effect of the investigated parameters on the structural response are discussed in Subsections 4.2-4.6.

4.1 Overview

Upon successful validation of the FE models, a series of parametric studies were carried out in order to evaluate the effects of key parameters on the structural performance. The same modelling assumptions with those of the validated models have been adhered to, while initial geometric imperfections with a magnitude equal to $L/1000$ where L the member's length, have been adopted in all cases. The effects of the following parameters were considered: i) the prestress level (no cable, P_{nom} , $0.5P_{opt}$, P_{opt}); ii) the effect of the presence of grouting (NG, G); iii) the strength class of the grouting (C25, C50, C70); iv) the tube's steel grade (S460, S690); v) the ratio of the cross sectional area of the prestressed cable over the cross-sectional area of the tube (A_c/A_t).

4.2 Effect of the level of applied prestress

In order to evaluate the effect of the applied level of prestress level, Figure 6 presents the numerically obtained ultimate loads plotted against the prestress level, which is given as a percentage of the cable's yield load ($A_c f_{yc}$). Note that 0% prestress in the horizontal axis of Figure 6(a) corresponds to a tensile member without any cable. As can be seen in Figure 6(a), the introduction of a prestressed cable increases considerably the ultimate performance of the tensile members. A further increase is achieved with the addition of grouting. Figure 6(b) shows that the introduction of initial compressive stresses to the tube during the prestressing of the cable decreases the system's ultimate load capacity under compression. A noticeable improvement in the performance can be achieved with the addition of concrete, owing to its enhanced compressive resistance.

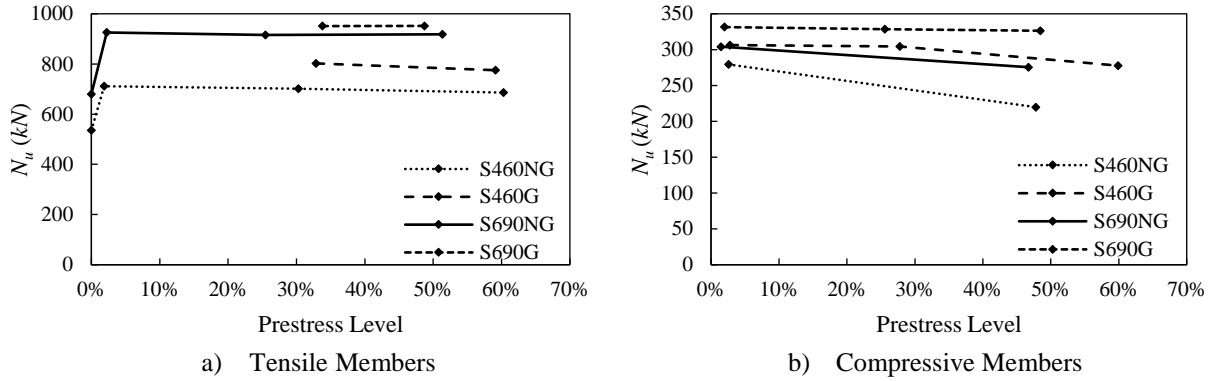


Figure 6. Effect of the prestress level.

4.3 Grouting's effect

The grouting's effect on the overall structural behaviour can be seen in Figure 7. The response of the grouted tensile members is more ductile compared to their counterparts without any grouting, as can be seen in Figure 7(a). This is presumably due to delaying the formation of necking as the presence of grouting restricts the inwards deformation of the cross-section. For the compressive members, improvement in the performance can be achieved with the addition of concrete, owing to its enhanced compressive resistance and stiffness, as can be observed in Figure 7(b). The presence of concrete delays the occurrence of global buckling as the member is stiffer. The results shown in Figure 7 are for specimens in Grade S690 prestressed at optimal level, while similar conclusions are also drawn for the S460 steel grade.

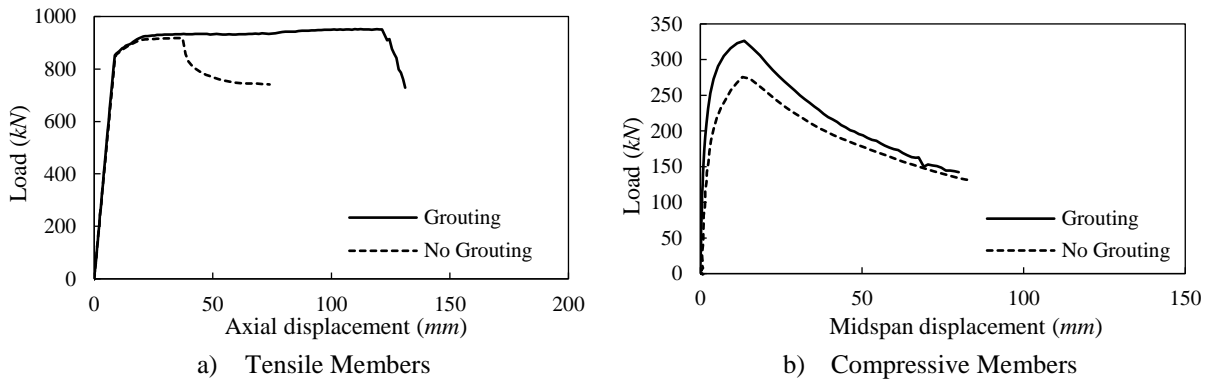


Figure 7. Grouting's effect.

4.4 Effect of the strength class of the grouting

Upon examining the influence of the grouting on the performance of steel members under axial loading, the effect of employing grouting of different strengths has also been studied. Three types of grouting with compressive strengths of 25 N/mm^2 , 50 N/mm^2 and 70 N/mm^2 , the stress-strain responses in compression of which are shown in Figure 8(a), have been considered. It is noteworthy that the strength class of the grouting had no noticeable effect for the tensile specimens, as the increase of the system's ductility is independent from the tensile strength of the grouting which is in any case negligible and owes to the material's presence inside the tubular member that delays the necking of the specimen. The results shown in Figure 8(b) correspond to S460 compressive grouted members prestressed at optimal level. It can be seen that as the grouting's grade increases, there is a corresponding small increase in the system's ultimate load capacity.

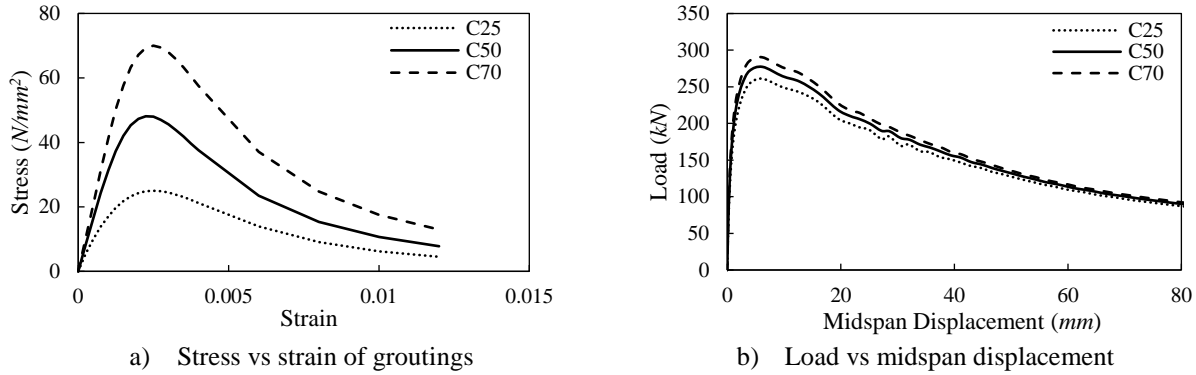


Figure 8. Effect of the grouting's grade in compressive specimens.

4.5 Effect of the tube's steel grade

Typical results corresponding to grouted tensile members prestressed at optimal level and non-grouted compressive members prestressed at nominal level are shown in Figure 9. In order to allow the evaluation of the steel grade's effect, the applied loads are presented in a non-dimensional format. In tensile specimens, the loads are normalised by the tube's yield strength, whilst for the compressive ones by the tube's buckling strength, according to [31]. The more pronounced strain-hardening properties of the S460 steel grade compared to that of the S690 steel grade is evident in Figure 9(a). In Figure 9(b), it can be seen that the S460 specimen has stiffer response and larger normalised ultimate load compared to its counterpart in S690 steel grade. Note that similar results regarding the steel grade effect have been observed for all studied members.

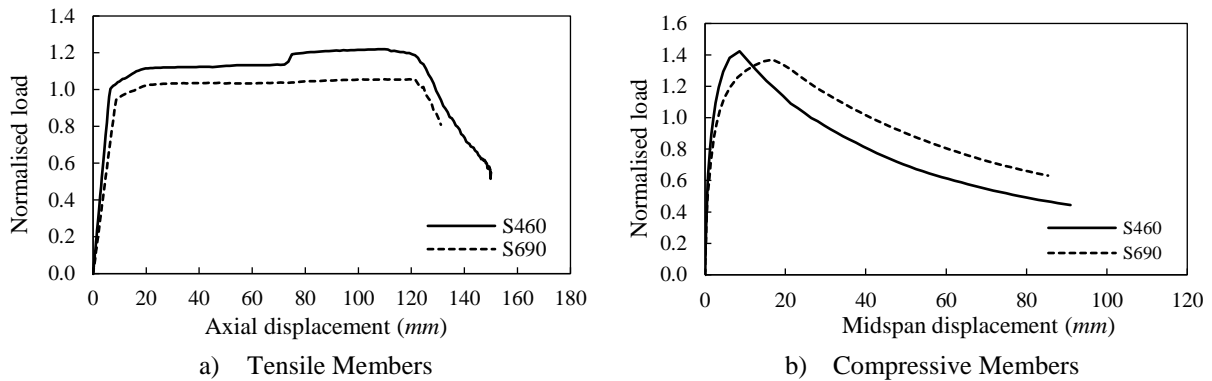


Figure 9. Effect of the tube's steel grade.

4.6 Effect of A_c/A_t

In order to evaluate the influence of the cable on the system's tensile response, the effect of the ratio of the cross sectional area of the prestressed cable over the cross-sectional area of the tube (A_c/A_t) has been examined. Keeping the tube's cross-section constant, cables in three different geometries, $A_c = 90, 150$ and 260 mm^2 , leading to the respective A_c/A_t ratios of 0.10, 0.17 and 0.30, have been studied. As shown in Figure 10(a), as the total cross-sectional area increases, the ultimate tensile performance increases as well. In Figure 10(b), the ultimate loads of the system normalised with the tube's yield strength are plotted against the A_c/A_t ratios, revealing a linear relationship between them.

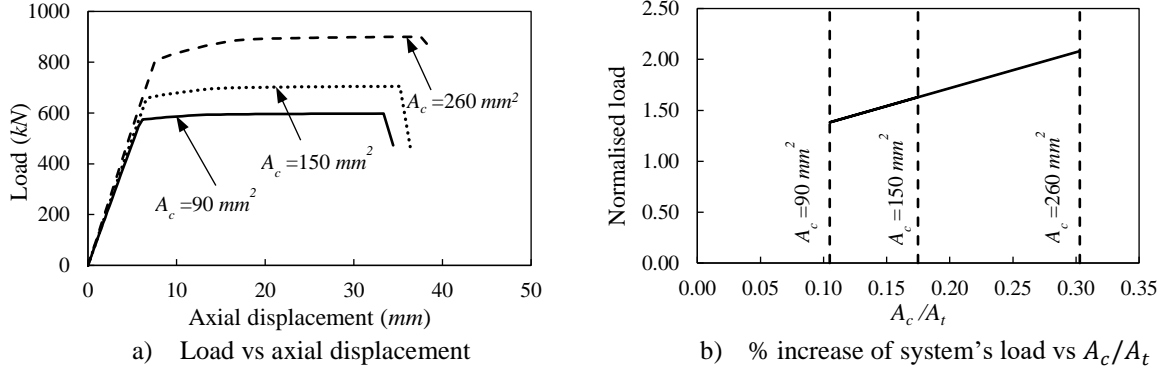


Figure 10. Effect of A_c/A_t in tensile specimens.

5 DESIGN RECOMMENDATIONS

Upon comprehensive numerical study of the prestressed members, the present section provides design guidance on tensile (Subsection 5.1) and compressive (Subsection 5.2) members prestressed at P_{opt} level.

5.1 Design of tensile prestressed members

When designing a prestressed member in tension, the design tensile load $N_{Ed,t}$ should not exceed the design plastic resistance of the chord $N_{pl,Rd}$. When a P_{opt} cable is to be inserted, the cross-sectional area of the tube and the cable can be determined according to Eq. (2).

$$N_{Ed,t} \leq N_{pl,Rd} = (A_t f_{ty} + A_c f_{cy}) / \gamma_{M0} \quad (2)$$

where γ_{M0} the partial factor for resistance of cross-sections with a recommended value of 1.0. [31] and the rest as previously defined. For tensile grouted members, the contribution of grouting can be considered negligible.

5.2 Design of compressive prestressed members

When designing a prestressed member in compression, the design compressive load $N_{Ed,c}$ should not exceed the buckling resistance of the chord, as stated in Eq. (3)

$$N_{Ed,c} \leq N_{b,Rd,p} \quad (3)$$

where $N_{b,Rd,p}$ is the design buckling resistance of the chord, taking due account of the prestress force P_i . $N_{b,Rd,p}$ may be determined using the modified Perry-Robertson approach, proposed by [12]. Employing the same framework with Eurocode [31], the method is able to account for the effect of the prestressing cables, through Eqs. (4)-(9), where the symbols are consistent with those used in [31]. In order to account for member instability, the compressive resistance of the system is expressed as the plastic cross-sectional resistance multiplied by a reduction factor χ_p .

$$N_{b,Rd,p} = \chi_p N_{pl} / \gamma_{M1} \quad (4)$$

where γ_{M1} the partial safety factor for resistance of members to instability with a recommended value of 1.0 and χ_p depends on the member slenderness ($\bar{\lambda}$) through Eqs. (6)-(7)

$$N_{pl} = A_t f_{ty} \quad (5)$$

$$\chi_p = \frac{(1 - P_i/N_{pl})}{\alpha_k \left[\phi_p + \sqrt{\phi_p^2 - \frac{(1 - P_i/N_{pl})}{\alpha_k} \bar{\lambda}^2} \right]} \quad (6)$$

$$\phi_p = \frac{(1 - P_i/N_{pl}) \bar{\lambda}^2 + [\alpha_k + \alpha(\bar{\lambda} - 0.2)]}{2\alpha_k} \quad (7)$$

where $\bar{\lambda}$ is the member slenderness given in Eq. (8), α_k is defined in Eq. (9) and α is the imperfection factor, selected from Table 6.2 of [31], but on the basis of an effective yield strength of the tube, defined as $f_{ty,eff} = f_{ty} - P_i/A_t$. A buckling curve a with $\alpha=0.21$ is proposed for hot-finished hollow sections with $f_{ty,eff} < 460 \text{ N/mm}^2$, whilst buckling

curve a_0 with $\alpha=0.13$ should be used for $f_{ty,eff} \geq 460 \text{ N/mm}^2$. Note that prestressing leads to a reduced yield strength for members under compression, thus rendering the sensitivity to geometric imperfections and effect of the residual stresses (expressed as a proportion of the yield strength) considerable for the member buckling resistance.

$$\bar{\lambda} = \sqrt{\frac{N_{pl}}{N_{cr}}} \quad (8)$$

$$\alpha_k = \frac{K_t}{K_c + K_t} \quad (9)$$

where N_{cr} the elastic critical load and K_c and K_t are the axial stiffness of the cable and the tube ($A_c E_c/L$ and $A_t E_t/L$), respectively.

As proposed in [12], the same design equations can be used for grouted specimens, while the grouting's contribution should be accounted for in the cross-section capacity and the slenderness (λ_g) evaluation, as set out in [32] from Eqs. (10)-(11)

$$N_{pl,g} = (A_t f_{ty} + A_g f_{gk}) \quad (10)$$

where A_g and f_{gk} are the cross-sectional area and the characteristic compressive cylinder strength of the grouting respectively.

$$\lambda_g = \frac{L_{cr}}{\pi} \sqrt{\frac{N_{pl}}{E_t I_t + 0.6 E_{gm} I_g}} \quad (11)$$

where E_t and I_t are the Young's modulus and the second moment of area of the tube, E_{gm} and I_g are the secant modulus and the second moment of area of the grouting respectively and L_{cr} the effective buckling length of the tube.

6 CONCLUSIONS

This paper investigated numerically the ultimate response of SHS tubular members in grade S460 and S690 prestressed by a 7-wire strand embedded within the surrounding tubular members. The studied members resemble parts of the bottom chord of a prestressed truss configuration, which has been successfully employed in structural applications [3]. Both tensile and compressive loads were considered, whilst the grouting's effect on the structural response, commonly adopted to mitigate corrosion of the prestressing cables, was also studied. The generated finite element models were comprehensively discussed. The numerically obtained ultimate load and load-deformation response were compared to the experimental ones, revealing a successful validation. The experimentally observed failure modes were also successfully replicated. A series of parametric studies were carried out and the influence of key factors, such as applied prestress level, ratio of cable cross-section to steel cross-section, selected steel grade and grouting strength on the structural response was extensively discussed. Overall it was shown that prestressing improves the structural performance of tensile members, while the addition of grouting leads to a more ductile behaviour. For members subjected to compressive axial loading, the introduction of initial compressive stresses to the tube causes the onset of global buckling at a lower load, while the latter is moderated with the addition of grouting.

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