

Summer 2011

# A Framework for Dynamic Traffic Monitoring Using Vehicular Ad-Hoc Networks

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**A FRAMEWORK FOR DYNAMIC TRAFFIC MONITORING USING  
VEHICULAR AD-HOC NETWORKS**

by

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A Dissertation Submitted to the Faculty of  
Old Dominion University in Partial Fulfillment of the  
Requirements for the Degree of

**DOCTOR OF PHILOSOPHY**

**COMPUTER SCIENCE**

**OLD DOMINION UNIVERSITY**

August 2011

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## ABSTRACT

### A FRAMEWORK FOR DYNAMIC TRAFFIC MONITORING USING VEHICULAR AD-HOC NETWORKS

Mohammad Hadi Arbabi  
Old Dominion University, 2011  
Director: Dr. Michele C. Weigle

Traffic management centers (TMCs) need high-quality data regarding the status of roadways for monitoring and delivering up-to-date traffic conditions to the traveling public. Currently this data is measured at static points on the roadway using technologies that have significant maintenance requirements. To obtain an accurate picture of traffic on any road section at any time requires a real-time probe of vehicles traveling in that section. We envision a near-term future where network communication devices are commonly included in new vehicles. These devices will allow vehicles to form *vehicular networks* allowing communication among themselves, other vehicles, and roadside units (RSUs) to improve driver safety, provide enhanced monitoring to TMCs, and deliver real-time traffic conditions to drivers.

In this dissertation, we contribute and develop a framework for dynamic traffic monitoring (DTMon) using vehicular networks. We introduce RSUs called *task organizers* (TOs) that can communicate with equipped vehicles and with a TMC. These TOs can be programmed by the TMC to task vehicles with performing traffic measurements over various sections of the roadway. Measurement points for TOs, or *virtual strips*, can be changed dynamically, placed anywhere within several kilometers of

the TO, and used to measure wide areas of the roadway network. This is a vast improvement over current technology.

We analyze the ability of a TO, or multiple TOs, to monitor high-quality traffic data in various traffic conditions (*e.g.*, free flow traffic, transient flow traffic, traffic with congestion, etc.). We show that DTMon can accurately monitor speed and travel times in both free-flow and traffic with transient congestion. For some types of data, the percentage of equipped vehicles, or the market penetration rate, affects the quality of data gathered. Thus, we investigate methods for mitigating the effects of low penetration rate as well as low traffic density on data quality using DTMon. This includes studying the deployment of multiple TOs in a region and the use of oncoming traffic to help bridge gaps in connectivity.

We show that DTMon can have a large impact on traffic monitoring. Traffic engineers can take advantage of the programmability of TOs, giving them the ability to measure traffic at any point within several *km* of a TO. Most real-time traffic maps measure traffic at midpoint of roads between interchanges and the use of this framework would allow for virtual strips to be placed at various locations in between interchanges, providing fine-grained measurements to TMCs. In addition, the measurement points can be adjusted as traffic conditions change. An important application of this is end-of-queue management. Traffic engineers are very interested in deliver timely information to drivers approaching congestion endpoints to improve safety. We show the ability of DTMon in detecting the end of the queue during congestion.

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For all from whom I learned.

## ACKNOWLEDGEMENTS

I would like to thank Dr. Michele C. Weigle for her guidance and constructive advice which led me to enthusiastically complete this dissertation. Her patience and ideas as well as her valuable comments and reviews have played an important role to conduct this research toward a proper direction. I would like to thank Dr. Stephan Olariu who has always been open for discussion and has inspired me in many ways. My sincere thanks to Dr. Michael Fontaine for being my external committee member and kind help in this project. I deeply appreciate his help, careful review, and constructive comments on my papers and this dissertation. I need to thank to Dr. Hussein Abdel-Wahab and Dr. Kurt J. Maly for being my committee members with their constructive criticism, feedback, and support.

I am very thankful to the best friends I have ever had, my lovely brothers, Dr. Reza Arbabi, Dr. Hossein Arbabi, and Dr. Javad Arbabi. They have taught me how to seek knowledge and pursue high goals. Their academic accomplishment, guidance, insight have always been a motivating track which I gladly follow. I thank my parents, who formed my current life, for their love and support.

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## CHAPTER 1

### INTRODUCTION

State transportation departments in the US must collect and probe various types of data for traffic monitoring purposes. Traffic management centers (TMCs) need high-quality data regarding the status of roadways for monitoring and delivering up-to-date information on traffic conditions to the traveling public. The most common of this traffic data are *traffic volume and flow rate, vehicle classification, traffic speed, traffic density, travel time, and delay*. Currently this data, such as speed and volume, is measured at static points on the roadway using technologies that have significant maintenance requirements. Systems that use this data to produce traffic reports are only as accurate as the quality of the collected data. To obtain an accurate picture of traffic on any road section at any time requires a real-time probe of vehicles traveling in that section. Current technologies can only monitor vehicles at fixed points of interest and require approximating some metrics, limiting the quality of the delivered data. Also fixed pointed detectors may miss locations where congestion occur since they are often located away from interchanges and other bottlenecks.

In the future, intelligent transportation systems - formed of combinational networks (*e.g.* wireless, mobile, vehicular, and sensor networks) - will use roadside units that can communicate with equipped vehicles (or mobile nodes) and with a TMC (a server). We envision a near-term future where network communication devices are commonly included in new vehicles. These devices will allow vehicles to form *vehicular networks*

allowing communication among themselves, other vehicles, and roadside units (RSUs) to improve driver safety, provide enhanced monitoring to TMCs, and deliver real-time traffic conditions to drivers. In this dissertation, we introduce a *Dynamic Traffic Monitoring* mechanism (DTMon). We introduce RSUs called *task organizers* (TOs) that can communicate with equipped vehicles and with a TMC. These TOs can be programmed by the TMC to task vehicles with performing traffic measurements over various sections of the roadway. Measurement points for TOs, or *virtual strips*, can be changed dynamically, placed anywhere within near or far distance from the TOs, and used to measure wide areas of the roadway network. This is a vast improvement over current technology, where specific measurement points must be decided in advance and hardware installed in those locations. Since vehicles are used to take traffic measurements, TOs can report speed, travel times, and delays without needing approximation even in low market penetration rates, and can report volume and density in high market penetration rates. In addition to reporting traffic measurements to a TMC, TOs can also be used to inform vehicles about the latest traffic conditions and other useful information from the TMC.

In this chapter, we introduce the important traffic data and the vehicular ad-hoc networks which can be used to probe and monitor this traffic data. Knowing the importance of each type of traffic data, we propose DTMon, a dynamic traffic monitoring mechanism using vehicular ad-hoc networks, *TOs*, and *virtual strips*, to augment and improve the capability of the currently-used technologies. The thesis statement and contributions are listed at the end of this chapter.



## **1.1. TRAFFIC DATA**

The goals of monitoring traffic are directly tied to specific functional objectives of departments of transportation (DOTs) and the related service providers, so the type of data and its level of spatial or temporal aggregation vary depending on the ultimate use of the data [1]. For example, some data are collected to support real-time traveler information and traffic control, whereas other data are collected and used off-line to help characterize typical travel patterns and project future traffic conditions. Examples of some of the uses of traffic data include the following:

1. Predicting where roads should be built or expanded in the future
2. Designing bridges and pavements to withstand predicted traffic loads
3. Analyzing air quality in urban areas
4. Alerting drivers to congestion and accidents
5. Controlling traffic signals

Three basic variables, volume or flow rate, speed, and density, can be used to describe traffic on any roadway. In addition to these variables, travel time and delay are used to describe the traffic movement on any section of roadway [1, 2, 3].

### **1.1.1. Volume and Flow Rate**

Volume and flow rate are two measures that quantify the amount of traffic passing a point on a lane or roadway during a given time interval. These terms are defined as follows:

- *Volume* is the total number of vehicles that pass over a given point or section of a lane or roadway during a given time interval; volumes can be expressed in terms of annual, daily, hourly, or sub-hourly periods.
- *Flow rate* is the equivalent hourly rate at which vehicles pass over a given point or section of a lane or roadway during a given time interval of less than 1 hour, usually 15, 10, or 5 minutes.

Volume and flow rate are variables that usually quantify demand, that is, the number of vehicle occupants or drivers (usually expressed as the number of vehicles) who desire to use a given facility during a specific time period. Congestion can influence demand, and observed volumes sometimes reflect capacity constraints rather than true demand. For example, in congested conditions demand is greater than volume because not everyone that wants to use the road can access it.

The distinction between volume and flow rate is important. Volume is the number of vehicles observed or predicted to pass a point during a time interval. Flow rate represents the number of vehicles passing a point during a time interval less than 1 hour, but expressed as an equivalent hourly rate. A flow rate is the number of vehicles observed in a sub-hourly period, divided by the time (in hours) of the observation. For example, a volume of 100 vehicles observed in a 15-minute period implies a flow rate of  $100 \text{ veh}/0.25 \text{ hour}$ , or  $400 \text{ veh/h}$ . Also, flow rate can show the influence of sub-hourly fluctuations in traffic which could cause variation in congestion.

Volume and flow rate can be illustrated by the volumes observed for four consecutive 15-min periods. For example, the four counts are 1000, 1200, 1100, and 1000. The total

volume for the hour is the sum of these counts, or 4300 vehicles. The flow rate, however, varies for each 15-minute period. During the 15-minute period of maximum flow, the flow rate is 1200 veh/0.25 h, or 4800 veh/h. Note that 4800 vehicles do not pass the observation point during the study hour, but they do pass at that rate for 15 minutes.

### 1.1.2. Speed

Although traffic volumes provide a method for quantifying capacity values, speed is an important measure of the quality of the traffic data provided to the TMC. Speed is a measureable quantity of effectiveness, defining levels of service for many types of facilities, such as rural two-lane highways, urban streets, freeway weaving segments, and others. Speed can also be used to estimate travel time in some conditions.

*Speed* is defined as a rate of motion expressed as distance per unit of time, generally as kilometers per hour (km/h). In characterizing the speed of a traffic stream, a representative value must be used, because a broad distribution of individual speeds is observable in the traffic stream. Usually the average travel speed is used as the speed measure because it is easily computed from observation of individual vehicles within the traffic stream and is the most statistically relevant measure in relationship to other variables. Average travel speed is computed by dividing the length of the highway, street section, or segment under consideration by the average travel time of the vehicles traversing it. If travel times  $t_1, t_2, t_3, \dots, t_n$  (in hours) are measured for  $n$  vehicles traversing a segment of length  $L$ , the average travel speed is computed using Equation 1.

$$S = \frac{L}{t_a} \quad (1)$$

where

$S$  = average travel speed (km/h),

$L$  = length of the roadway segment (km),

$t_i$  = travel time of the  $i^{th}$  vehicle to traverse the segment (h),

$n$  = number of travel times observed, and

$t_a = \frac{1}{n} \sum_{i=1}^n t_i$  = average travel time over  $L$  (h).

The travel times in this computation include stopped delays due to fixed interruptions or traffic congestion. They represent the total travel times to traverse the defined roadway length. Several different speed parameters can be applied to a traffic stream. These include the following:

- *Average running speed* - A traffic stream measure based on the observation of vehicle travel times traversing a section of highway of known length. It is the length of the segment divided by the average running time of vehicles to traverse the segment. Running time includes only the time that vehicles are in motion.
- *Average travel speed* - A traffic stream measure based on travel time observed on a known length of highway. It is the length of the segment divided by the average travel time of vehicles traversing the segment, including all stopped delay times. It is also a *Space mean speed (SMS)*. It is called a space mean speed because the average travel time weights the average to the time each vehicle spends in the defined roadway segment or space.
- *Time mean speed (TMS)* - The arithmetic average of speeds of vehicles observed passing a point on a highway; also referred to as the average spot speed. The

individual speeds of vehicles passing a point are recorded and averaged arithmetically.

- *Free-flow speed* - The average speed of vehicles on a given facility, measured under low-volume conditions, when drivers tend to drive at their desired speed and are not constrained by control delay or congestion.

For most of the procedures using speed as a measure of effectiveness in this dissertation, SMS and TMS are the defining parameters. For uninterrupted-flow facilities like a segment of a highway or a street, the average travel speed is equal to the average running speed.

SMS is always less than TMS, but the difference decreases as the absolute value of speed increases. Based on the statistical analysis of observed data, this relationship is useful because TMS often is easier to measure in the field than SMS [1, 2, 3]. To calculate SMS accurately, average travel time must be calculated in an accurate way which requires the observation of two constructing points of the segment rather than one.

It is possible to calculate both time mean and space mean speeds from a sample of individual vehicle speeds. For example, suppose three vehicles are recorded with speeds of 40, 60, and 80 km/h. The time to traverse 1 km is 1.5 min, 1.0 min, and 0.75 min, respectively. The time mean speed is 60 km/h, calculated as  $(40 + 60 + 80)/3$ . The space mean speed is 55.4 km/h, calculated as  $(60)[3 \div (1.5 + 1.0 + 0.75)]$ .

### **1.1.3. Travel Time and Delay**

*Travel time* is the amount of time that takes a vehicle to travel between two points on the road. It is the piece of data most understandable for the driving public and, thus, is the

most desired data for traffic engineers. Historically, gathering travel times has been challenging. Travel time is a variable used in calculation of average running speed and average travel speed and also space mean speed.

*Delay*, or *expected delay*, is also a piece of data useful for the driving public. This is the time difference between the observed travel time of the road segment and the travel time usually measured directly by knowing the length of the segment and the desired speed on that segment. For example, the travel time for a 5 km segment of a roadway with a speed limit (desired speed) of 80 km/h is 0.0625 h ( $5 \text{ km} \div 80 \text{ km/h}$ ). If the observed average travel time of the vehicles that have passed the same segment is 0.09 h, then the average delay can be estimated as 0.0275 h (1.65 min). This delay can be assumed as the average delay that the drivers have encountered during their journey through the segment or can be used as an estimate for the expected delay, the delay that drivers may expect to encounter upon entering the same segment.

#### **1.1.4. Vehicle Classification**

*Vehicle classification* data records traffic volume with respect to the type of vehicle that passes a point on the road. The Federal Highway Administration (FHWA) has defined a set of 13 vehicle classes that are commonly used by most states [1, 3]. In addition to vehicle classification, TMC needs the percentage of vehicles in different classes which pass a point on the road. The classification scheme is separated into categories depending on whether the vehicle carries passengers or commodities. Non-passenger vehicles are further subdivided by number of axles and number of units, including both power and

trailer units. The most common vehicle types are motorcycles, buses, passenger cars (including all sedans, coupes, and station wagons), and trucks.

#### 1.1.5. Density

*Density* is the number of vehicles occupying a given length of a lane or roadway at a particular instant. For the computations in this dissertation, density is usually averaged over time and is usually expressed as vehicles per kilometer (veh/km).

Direct measurement of density in the field is difficult, requiring a vantage point for photographing, videotaping, observing significant lengths of highway, or surrogating based on other metrics. Density can be computed, however, from the average travel speed and flow rate, which are measured more easily. Equation 2 is used for under-saturated traffic conditions.

$$D = \frac{v}{S} \quad (2)$$

where

$v$  = flow rate (veh/h),

$S$  = average travel speed (km/h), and

$D$  = density (veh/km).

A highway segment with a rate of flow of 1000 veh/h and an average travel speed of 50 km/h would have a density of

$$D = \frac{1000 \text{ veh/h}}{50 \text{ km/h}} = 20 \text{ veh/km}$$

Density is a critical parameter because it characterizes the quality of traffic operations. It describes the proximity of vehicles to one another and reflects the freedom to maneuver within the traffic stream.

Roadway occupancy is frequently used as a surrogate for density in control systems because it is easier to measure. *Occupancy in space* is the proportion of roadway length covered by vehicles, and *occupancy in time* identifies the proportion of time a roadway cross section is occupied by vehicles.

#### **1.1.6. Headway and Spacing**

*Spacing*, or space headway, is the distance between successive vehicles in a traffic stream, measured from the same point on each vehicle (*e.g.*, front bumper, rear axle, etc.). *Headway*, or time headway, is the time between successive vehicles as they pass a point on a lane or roadway, also measured from the same point on each vehicle.

These characteristics are microscopic, since they relate to individual pairs of vehicles within the traffic stream. Within any traffic stream, both the spacing and the headway of individual vehicles are distributed over a range of values, generally related to the speed of the traffic stream and prevailing conditions. In the aggregate, these microscopic parameters relate to the macroscopic flow parameters of density and flow rate.

Spacing is a distance, measured in meters. It can be determined directly by measuring the distance between common points on successive vehicles at a particular instant. This generally requires complex techniques, so that spacing is usually derived from other direct measurements. Headway, in contrast, can be easily measured with time observations as vehicles pass a point on the roadway. Since headway and spacing are related, knowing the headway allows the spacing to be determined as shown in Equation 3.



The relationship between average spacing and average headway in a traffic stream depends on speed, as indicated in Equation 3.

$$\text{Headway (s/veh)} = \frac{\text{Spacing (m/veh)}}{\text{Speed (m/s)}} \quad (3)$$

This relationship also holds for individual headway and spacing between pairs of vehicles. The speed is that of the second vehicle in a pair of vehicles.

The average vehicle spacing in a traffic stream is directly related to the density of the traffic stream, as determined by Equation 4.

$$\text{Density (veh/km)} = \frac{1000}{\text{Spacing (m/veh)}} \quad (4)$$

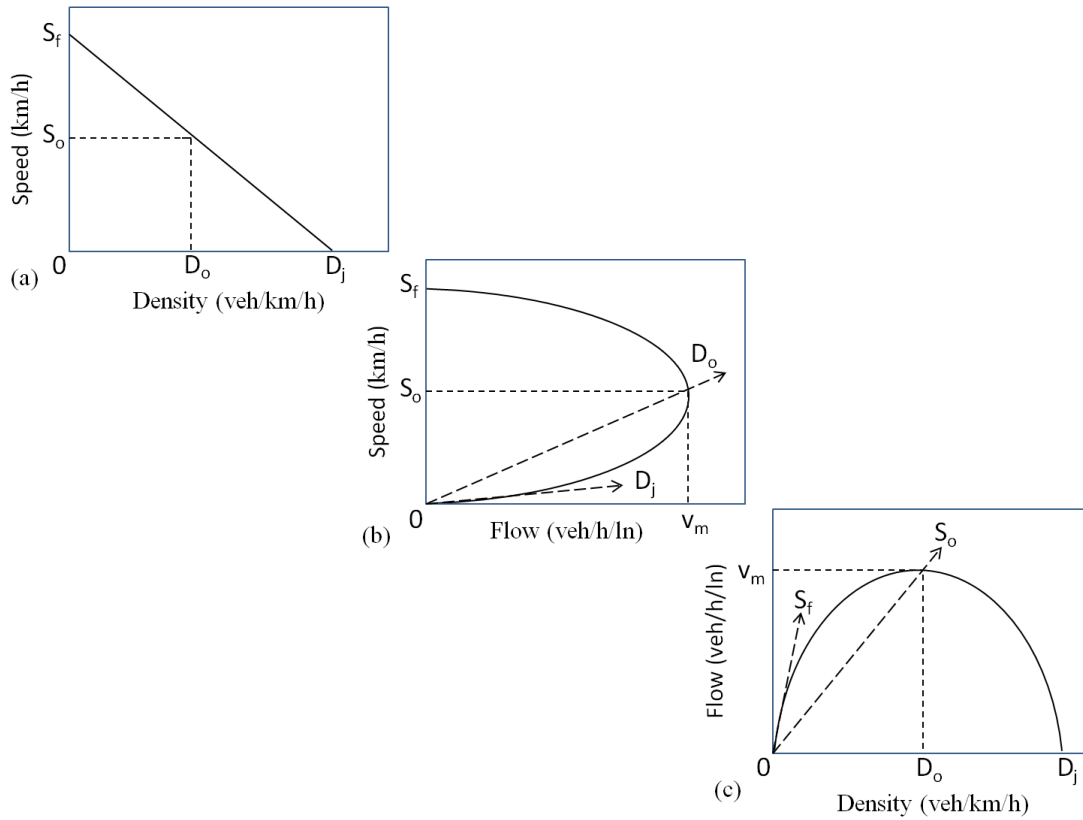
Flow rate is related to the average headway of the traffic stream as shown in Equation 5.

$$\text{Flow rate (veh/h)} = \frac{3600}{\text{Headway (s/veh)}} \quad (5)$$

#### 1.1.7. Relationship Among Basic Parameters

Equation 2 cites the basic relationship among the parameters density, speed, and flow rate, describing an uninterrupted traffic stream. Although the equation  $v = S * D$  algebraically allows for a given flow rate to occur in an infinite number of combinations of speed and density, there are additional relationships restricting the variety of flow conditions at a location.

**Figure 1** shows a generalized representation of these relationships, which are the basis for the capacity analysis of uninterrupted-flow facilities like in highways and streets where relationship between speed and density is assumed to be linear. Speed is space mean speed.



**Figure 1. Generalized relationship among speed, density, and flow rate (based on [2]).**

The form of these functions depends on the prevailing traffic and roadway conditions on the segment under study and on its length in determining density. Although the diagrams in **Figure 1** show continuous curves, it is unlikely that the full range of the functions would appear at any particular location. Survey data usually show discontinuities, with part of these curves not present.

The curves of **Figure 1** illustrate several significant points. First, a zero flow rate occurs under two different conditions. One is when there are no vehicles on the roadway, thus both density and flow rate are zero. Speed is theoretical for this condition and would

be selected by the first driver (presumably at a high value). This speed (the free flow speed) is represented by  $S_f$  in **Figure 1**.

The second case is when density becomes so high that all vehicles must stop—the speed is zero, and the flow rate is zero, because there is no movement and vehicles cannot pass a point on the roadway. The density at which all movement stops is called *jam density*, denoted by  $D_j$  in **Figure 1**.

Between these two extreme points, the dynamics of traffic flow produce a maximizing effect. As flow increases from zero, density also increases, since more vehicles are on the roadway. When this happens, speed declines because of the interaction of vehicles. This decline is negligible at low and medium densities and flow rates. As density increases, these generalized curves suggest that speed decreases significantly before capacity is achieved. Capacity is reached when the product of density and speed results in the maximum flow rate. This condition is shown as *optimum speed*  $S_o$  (often called critical speed), *optimum density*  $D_o$  (sometimes referred to as critical density), and maximum flow  $v_m$ .

The slope of any ray line drawn from the origin of the speed-flow curve represents the inverse of density, based on Equation 2. Similarly, a ray line in the flow-density graph represents speed. As examples, **Figure 1** shows the average free-flow speed and speed at capacity, as well as optimum and jam densities. The three diagrams are redundant, since if any one relationship is known, the other two are uniquely defined.

As shown in **Figure 1**, any flow rate other than capacity can occur under two different conditions, one with a high speed and low density and the other with high

density and low speed. The high-density, low-speed side of the curves represents oversaturated flow. Sudden changes can occur in the state of traffic (*e.g.*, in speed, density, and flow rate). We emphasize that these are theoretical relationships based on a linear speed-density relationship. Empirical data differs from these somewhat.

## **1.2. Vehicular Ad-hoc Networks**

A Vehicular Ad-Hoc Network, or VANET, is a technology that uses vehicles (usually moving vehicles) as nodes in a network to create a mobile network. Therefore, VANET is a form of mobile ad-hoc network [4, 5]. Vehicles in VANETs are able to communicate with nearby vehicles and between vehicles and nearby fixed equipments. The main goal of VANETs is to provide safety and comfort for passengers. There are several applications for VANETs such as incident management, collision warning, vehicle tracking, improved driving, resource awareness, etc. [5]. There may also be multimedia and Internet connectivity facilities for drivers (passengers), all provided within the wireless coverage of each vehicle. Automatic payment for parking lots and toll collection are other examples of possibilities using VANETs.

An on-board unit (OBU) consists of a wireless transceiver in a vehicle that provides the communication with nearby road-side units (RSU) using Dedicated Short Range Communication (DSRC). DSRC [6] provides short-to-medium range wireless communication channels (5.9 GHz up to range of 300 meters) specifically designed for automotive use. There are a corresponding set of protocols and standards (IEEE 1609 [7], IEEE 802.11p [8]) associated with DSRC. Vehicles are also able to store useful data

during their trip for later delivery. The information gathered by OBUs and RSUs can be mined to derive traffic data and its derivatives.

The main advantages of using VANETs in monitoring traffic compared with other technologies are as follows:

1. VANETs can be used to collect and monitor real time traffic speeds and travel times.
2. VANETs can be used to collect and monitor real time information on congestion.
3. VANETs have the potential to be used to dynamically change traffic signal timings.
4. There are two-way communications between vehicles and roadside units in VANETs.
5. VANETs have the potential to provide up-to-date traveler information in addition to safety messages.
6. RSUs are nonintrusive.

The initial disadvantages are as follows:

1. VANET is still in its infancy, and the technology and requirements have not been thoroughly tested and evaluated.
2. Its reliability and accuracy must be ensured.
3. Like other probe vehicle-based methods, VANET suffers when the market penetration rate and sample size are low.
4. Its cost is still unknown. (OBU, RSU, operating, maintenance, etc.)

In Chapter 3, we will explain the use of VANETs in our proposed framework (DTMon) and its components in more detail.

### 1.3. Traffic Management Centers

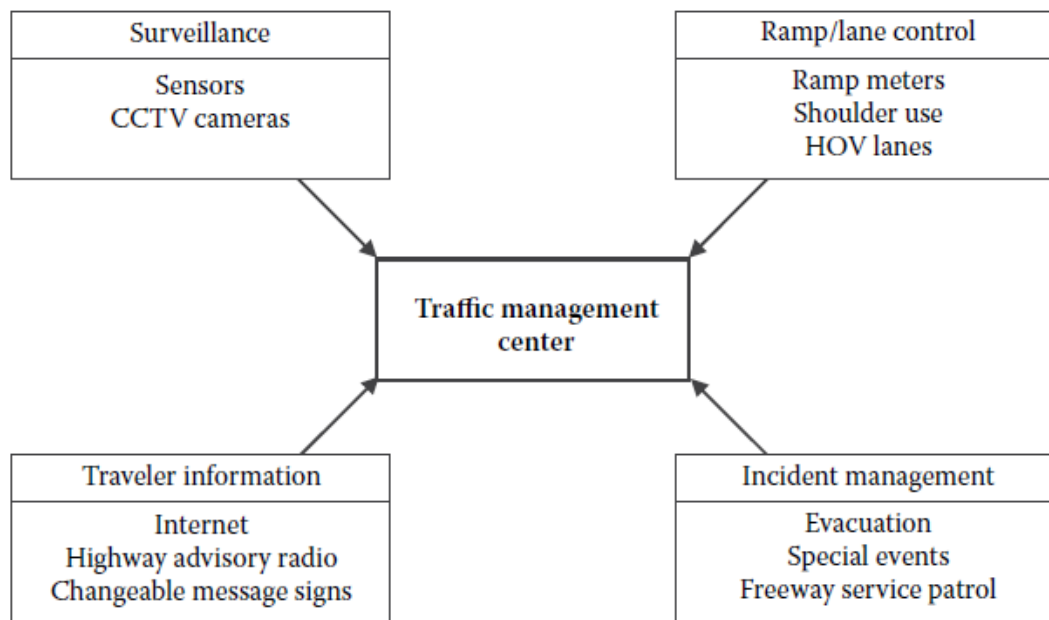
The TMC serves as the focal point for the management of the roadway transportation system in an area [3]. It integrates data from a variety of different sensor sources and provides a means for operators to manage traffic and inform the public from a centralized point. Many TMCs are also co-located with emergency responders to help facilitate coordination when a crash or other emergency arises. The workstations offer functionality to perform tasks like changing messages on dynamic message signs (DMSs), viewing police dispatch reports, and controlling closed-circuit cameras.

Sensors and other detection devices can usually communicate and transfer data with the TMCs through wired, wireless, or by portable data storage [3, 9, 10]. Wired mechanism involves regular cable, fiber optic cables (higher bandwidth), or telephone lines. Wireless transfer of data can be done through cell phones or radio frequencies specified for this purpose. Portable data storage devices are usually located nearby sensors or attached to them. They store data which will be transferred to TMCs for archiving or off-line data processing.

The major functions of the TMC are the following (see **Figure 2**):

1. *Surveillance*. The surveillance function involves the collection of data on traffic flow conditions on the roadways being monitored.

2. *Traveler information.* In the traveler information function, the TMC provides information on current conditions to the public, enabling them to change routes or times of departure to avoid congestion.
3. *Incident detection and management.* Incident detection and management includes the timely detection of sources of congestion and developing strategies to mitigate their impact.
4. *Ramp and lane control.* This involves dynamically changing traffic control devices on ramps and main freeway lanes to improve traffic flow.



**Figure 2. Functions of a TMC. Copyright©2011 from Chapter 1: “Traffic Monitoring” by M. Fontaine in *Vehicular Networks From Theory to Practice*, edited by S. Olariu and M. Weigle. Reproduced by permission of Taylor and Francis Group, LLC, a division of Informa plc.**

#### 1.4. THESIS STATEMENT AND CONTRIBUTIONS

There are several sensors/detectors and technologies currently used to monitor traffic data (see Chapter 2 Sections 2.1 and 2.2). These traffic data are usually monitored by technologies such as inductive loop detectors (ILDs), video detection systems, acoustic tracking systems, and microwave radar sensors (MRS). In addition, wireless technologies are used in systems such as automatic vehicle identification (AVI), automatic vehicle location (AVL), and wireless location technology (WLT).

Among these, ILDs are the most prevalent, generally have the highest accuracy, and can monitor all of the fundamental traffic data except travel times. However, they are prone to failure. Maintenance, installation, and replacement can be problematic, and because of this, large portions of an ILD network may not be returning quality data at any given time. Video detection systems have issues in inclement weather (*e.g.*, fog, rain, snow) and especially with occlusion. Occlusion is also a major problem for acoustic tracking and microwave radar systems. The ability of AVI-based systems to provide useful data is directly linked to the number of probes on the road. Therefore, these systems require the installation of significant roadside infrastructure to detect equipped vehicles. AVL systems are susceptible to the same sample size limitations as AVI systems. Also, trucking companies have been reluctant to share their AVL data with others due to concerns about losing competitive advantages in the marketplace, so data sources are not widely available. Note that AVL devices can also be installed in municipal buses or taxis. WLT systems are based on the presence of cellular phones in



vehicles for monitoring traffic. But, these systems do not have the same location precision as GPS and cannot distinguish between different phones in the same vehicle.

**Thesis Statement:** Vehicular networks and in particular DTMon, a dynamic traffic monitoring system consisting of *task organizers* (programmable road side units) and *virtual strips* (dynamic points of interest), can be used to efficiently provide to TMCs high-quality, fine-grained traffic measurements covering a large area of the roadway.

This thesis is a framework for designing various systems that could be deployed in different configurations based on highway topology and traffic conditions. We will analyze the capabilities of DTMon and TOs to monitor high-quality traffic data in various traffic conditions (free-flow and congested states). For some types of data, the percentage of equipped vehicles, or the penetration rate, affects the quality of data gathered. We will investigate methods for mitigating the effects of low penetration rate as well as low traffic density on data quality. This may include deploying multiple TOs in a region or using oncoming traffic to help bridge gaps in connectivity.

Our framework, and in particular DTMon, can have a large impact on traffic monitoring. Traffic engineers can take advantage of the programmability of TOs, giving them the ability to measure traffic at any point within up to reasonably far distances from TO. Since the location of virtual strips can be changed at any time, measurement points can be adjusted as traffic conditions change. An important example of this is end-of-queue management. Traffic engineers are very interested in monitoring the endpoints of congestion and would like to deliver timely information to drivers approaching these points to improve safety. Another important example of the usefulness of dynamic

measurement is an evacuation scenario. As drivers evacuate, they may take roadways that are not normally monitored. A TO could be placed temporarily in those locations as conditions mandate to measure traffic and provide important information to drivers. Most real-time traffic maps measure traffic at midpoint of roads between interchanges and the use of this framework would allow for virtual strips to be placed at various locations in between interchanges, providing fine-grained measurements to TMCs. Maybe most importantly, a single TO could be used to monitor traffic up to reasonably far distances, resulting in lower monitoring costs with less delay than is currently possible in providing real-time information from/to drivers.

**Contributions:**

1. *A method for using probe vehicles to perform spatial sampling of traffic conditions* – Probe vehicles can provide real-time measurements of speed and travel time. Using spatial sampling allows for these measurements to be made at specific locations of interest on the roadway. This avoids the need for interpolation and estimation that is required when temporal sampling of probe vehicles is performed.
2. *An analysis of the factors that can impact the quality of monitored traffic data when using vehicular networks* – These factors are market penetration rate, traffic conditions, communication range, distance between communicating entities, methods of message delivery, message reception rate, and message delay. We have performed an analysis of these factors and how each contributes to the

quality of traffic data that can be reported. We note that both traffic conditions and market penetration rate affect the distance between communicating entities.

3. *An evaluation of the impact of different methods of message delivery on the quality of traffic data that can be gathered by vehicular networks* – We compared four different message delivery methods (regular forwarding, dynamic transmission range, store-and-carry, and a hybrid approach) by measuring message reception rate and message delay in different traffic conditions and different market penetration rates. We found that a hybrid approach (regular forwarding coupled with store-and-carry and using vehicles traveling in the opposite direction) can significantly improve the performance of DTMon in poorly connected traffic conditions. We also showed that when the market penetration rate is high, the message delay can be reduced significantly.
4. *An evaluation of the effectiveness of DTMon as compared with current technologies such as inductive loop detectors (ILD) and automatic vehicle location (AVL)* – We show that DTMon can be used to report travel times that are not statistically different from the actual travel times. In comparison, we show that technologies such as ILD cannot measure travel times with this level of accuracy.
5. *A demonstration of the usefulness of DTMon's monitoring approach for monitoring congested traffic conditions* – We have shown the ability of DTMon to allow a TMC to dynamically place additional monitoring points (virtual strips) in locations where congestion is building up. This allows the TMC to detect

transitions in traffic flow using speeds and travel times, without having to rely on flow rate information. We have shown that DTMon can be used to detect and track the end-of-the-queue in traffic with congestion.

6. *Highway mobility modules for the ns-3 network simulator* – We have contributed the first highway mobility modules designed to produce realistic vehicle mobility and communications in ns-3. The mobility model has been validated against the well-known vehicular mobility models, and the networking components use ns-3, which has been validated against wireless models. These modules have been released to the ns-3 community and are now being used by other researchers around the world.

## 1.5. OUTLINE

The dissertation is organized as follows:

- Chapter 2 presents the background and related work for the proposed DTMon and implemented integrated VANET simulator. We also present the background and related work regarding traffic monitoring using technologies such as sensors/detectors and probe vehicle-based systems.
- Chapter 3 introduces and explains the components of DTMon for dynamic traffic monitoring using VANETs. We describe the main components of the proposed framework such as proposed task organizers, proposed virtual strips and segment, equipped vehicles and methods/techniques to monitor traffic data. We analyze the effect of the market penetration rate and the traffic density on DTMon's performance. We explain various scenarios and strategies for deploying TOs. We

also describe the advantages of TOs and dynamically defined virtual strips in augmenting the current in-use systems for monitoring traffic.

- Chapter 4 evaluates the proposed methods in free-flow traffic and addresses the effect of traffic flow, speed, density, and market system penetration rate on monitoring high quality traffic data. We show the performance of various methods of message delivery on information and message reception rate and delay in monitoring high quality traffic data. In addition to free flow traffic, we evaluate the proposed methods in traffic with congestion and addresses the effect of traffic flow, speed, density, and market system penetration rate on monitoring high quality traffic data. We show the performance of various methods of message delivery and compare the results with sensors/detectors and probe vehicle-based systems. We also show advantage of using our dynamic monitoring in detecting congestion.
- We conclude and summarize our research and achievements in Chapter 5. We discuss future research directions in Chapter 5 as well.
- Appendix A presents the first implementation of a well-known vehicle mobility model in ns-3, the next generation of the popular ns-2 networking simulator. We show the need and motivation for a quality integrated VANET simulator which was used to evaluate our proposed dynamic traffic monitoring system.

## CHAPTER 2

### BACKGROUND AND RELATED WORK

In the United States and numerous other parts of the world, road traffic is a critical part of a region's economic activity. Numerous measures are taken to address problems arising from traffic flow, its transition, and congestion. An essential step in this direction is the creation of a traffic monitoring system with capabilities to estimate traffic data with significant accuracy and reliability. Historically, dedicated infrastructure has been used for monitoring traffic data, however, the corresponding sensors have limited coverage, high installation and maintenance costs, and their fixed position does not enable optimized and adaptive sampling as traffic conditions change. Therefore, there is a need for a mechanism which can augment these monitoring systems or even replace them.

In this chapter, we present traffic monitoring methods and technologies. We explain the advantages and disadvantages of these technologies.

#### 2.1. TRAFFIC MONITORING METHODS AND TECHNOLOGIES

There are several sensors and detectors, methods, and technologies that can be used to collect, probe, and monitor traffic data [3, 9, 10]. The sensor technologies can be broadly classified into two categories: *intrusive* and *nonintrusive* sensors.

- *Intrusive detectors* require intrusion into the pavement to perform installation or maintenance activities.
- *Nonintrusive detectors* are mounted either above or beside the roadway.

An example of an intrusive detector would be an inductive loop detector (ILD) installed in the pavement surface. Intrusive detectors in comparison to nonintrusive detectors have several problems:

1. Travel lanes may have to be closed in order to perform maintenance on the sensor.
2. Timely maintenance, particularly in a congested urban area, is a significant barrier where it may be difficult to close lanes during the day due to the impact on traffic.
3. They need to be replaced every time a road is repaved.

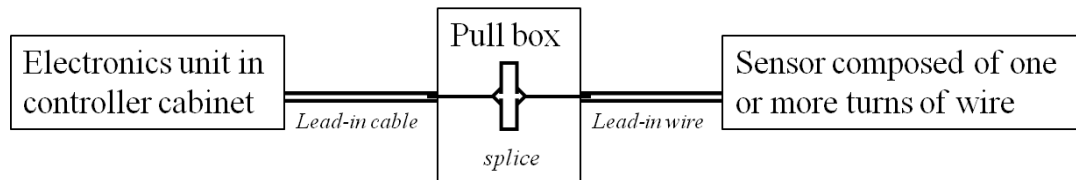
In addition to sensors, a number of new monitoring methods that rely on the use of probe vehicle data have been deployed as a way to gather traffic data mostly relating to travel time and speed. *Probe vehicle-based systems* track the movements of a subset of the vehicle population in order to estimate the travel characteristics of all vehicles on the road. Probe vehicle systems can generate estimates of speed and travel time on a section of road, but they generally do not produce estimates of the volume or density of traffic. By using this approach, estimates of SMS (rather than TMS) are generated for the road. Thus, the speed estimates more fully characterize what drivers actually experience [2, 3, 9]. One method to probe traffic data is the use of wireless technology such as cell phones.

In this section, we introduce these commonly used sensors, methods, and technologies in addition to their connectivity to TMC.

### **2.1.1. Inductive Loop Detectors**

Inductive loops are intrusive detectors, consisting of a coiled wire that is cut into the pavement. The ILD senses the presence of metal objects that pass over the coiled wire.

When a vehicle passes over or stops on top of an ILD, it reduces the loop inductance of the wire. Currents are induced in the vehicle, reducing loop inductance. The reduction in loop inductance is then translated by a controller into a detection of the presence of a vehicle (see **Figure 3**). Lengths of vehicles can be estimated, but the number of axles is not explicitly counted.



**Figure 3. Inductive loop detector system.**

ILDs can be installed as either a single or double loop configuration. Single loop detectors comprise just one loop of coiled wire installed in the pavement and are generally used to provide traffic volume and density information. With double loops, two loops are installed in a lane of road one after another, with a short space between the two ILDs. Double loops are necessary to generate speed estimates. As the two ILDs are placed a known distance apart, the speed of a vehicle can be estimated by examining the time that elapses between the activations of the two loops.

There are also several advantages to using ILDs:

1. The technology is mature, and there is a large experience base with the sensors.
2. All of the fundamental traffic data including volume, occupancy, TMS, and vehicle classification can be provided by ILDs.



3. ILDs are robust to inclement weather such as fog, rain, and snow.
4. ILDs generally have the highest accuracy of commonly used sensors, with accuracy reported to be between  $-1$  and  $+2\%$  across a variety of studies [11, 12].
5. ILDs can produce high quality data (*i.e.*, flow rate and volume).

Disadvantages of ILDs are as follows:

1. ILDs require regular tuning to ensure that speeds and vehicle classification data are of high quality.
2. They are intrusive sensors, therefore maintenance, installation, and replacement of ILDs can be problematic, particularly in congested urban areas.
3. Multiple loops are usually required to monitor a location.
4. Detection accuracy may decrease when design requires detection of a large variety of vehicle classes.
5. They are prone to failure, and large portions of an ILD network may not be returning quality data at any given time because of ILD failure rates and maintenance difficulties.

For example, in 2005 the Virginia Department of Transportation Traffic Management Center in Hampton Roads estimated that about 40% of their ILDs were not returning quality data [13]. The difficulties in maintaining ILDs have been a significant reason why alternative detection technologies have been pursued.

### **2.1.2. Video Detection System**

Video detection systems utilize cameras and image processing software to collect data on traffic flow. A camera is pointed at a roadway, and image processing software analyzes

changes in pixels between successive frames. The processing software identifies when a vehicle has entered the frame, and then translates the movement of the vehicle on the video into traffic flow parameters [9, 10].

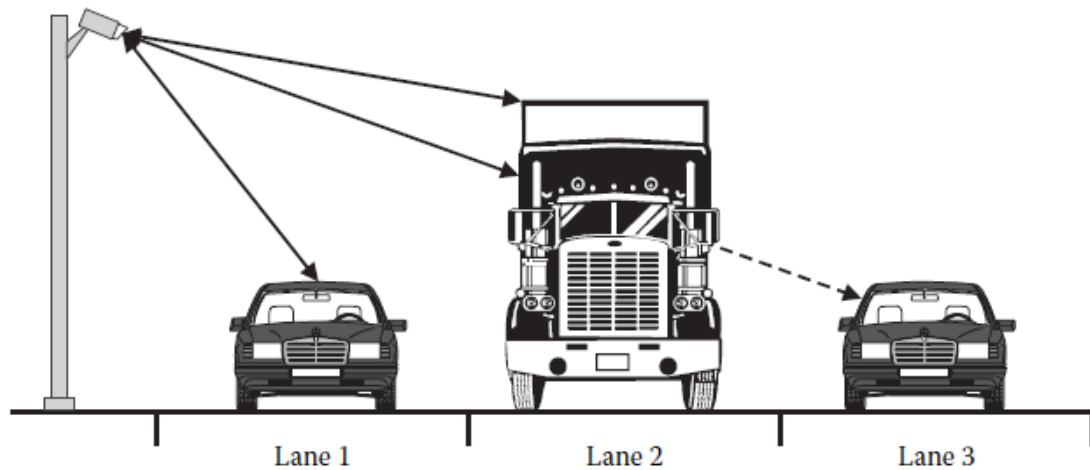
Video detection offers several advantages over ILDs:

1. Video detection systems are nonintrusive detectors which can collect all of the same traffic flow parameters as inductive loops.
2. A single controller and camera combination can be used to detect multiple lanes on an approach.
3. Wide-area detection can be provided when information gathered from multiple camera locations are linked together.
4. Video detection system is sometimes more cost effective solution for traffic signal detection over the lifecycle of the equipment.

The problems with video detection systems are as follows:

1. Some periodic maintenance, such as cleaning camera lenses, requires shutting down lanes.
2. Video detection systems have issues in inclement weather (*i.e.*, fog, rain, snow) which can create problems with the video processing software because they reduce the contrast in the image between vehicles and the background.
3. The field of view of the camera can be affected by high winds.
4. Reliable nighttime signal actuation requires street lighting.
5. Occlusion is a major potential problem with video detection systems (see **Figure 4**) [3, 4].

*Occlusion* occurs when a vehicle obscures another vehicle within the camera's field of view. Occlusion can cause undercounting of traffic volumes or poor speed estimation.



**Figure 4. Occlusion and its impact on video detection.** Copyright©2011 from Chapter 1: “Traffic Monitoring” by M. Fontaine in *Vehicular Networks From Theory to Practice*, edited by S. Olariu and M. Weigle. Reproduced by permission of Taylor and Francis Group, LLC, a division of Informa plc.

### 2.1.3. Microwave Radar Sensors

Microwave radar sensors (MRS) are devices for transmitting high frequency electromagnetic signals and receiving echoes from objects of interest within their volume of coverage.

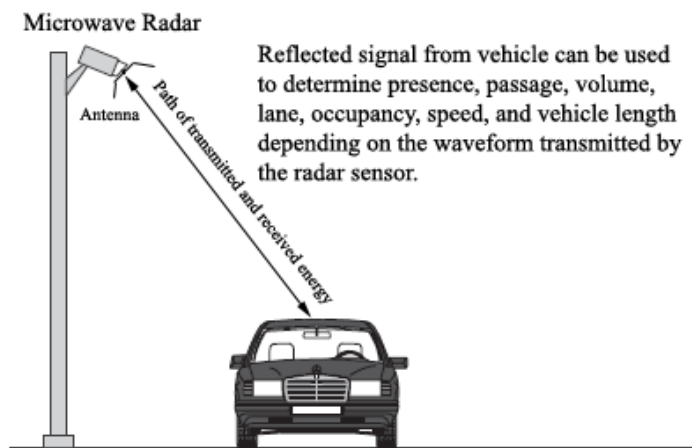
MRS have flexibility in where they can be placed. The sensors are typically mounted on existing structures that pass over the highway (such as sign structures or bridges) or on

posts adjacent to the roadway, therefore MRS are another alternative option to ILDs, and have the following advantages:

1. MRS are nonintrusive.
2. They can directly measure speed.
3. They can provide volume, occupancy, TMS, and vehicle classification data.
4. They can be mounted to collect data for a single lane or to collect data across multiple lanes.
5. They are typically insensitive to inclement weather at relatively short ranges.

MRS have disadvantages:

1. The accuracy of MRS is not as good as well-functioning ILDs.
2. MRS suffer from many of the same occlusion problems as video detection.
3. They cannot detect stopped vehicles and vehicles with low speed.



**Figure 5. Microwave radar operation.**

The most common application of microwave sensors is to supplement data collected from ILDs on major freeway facilities. Infrared, ultrasonic, and acoustic radars are fairly similar technologies that are used for detection of vehicles, but these technologies do not perform as accurately as MRS in various conditions and are not used as widely as ILDs, cameras, and MRS for monitoring traffic data.

#### **2.1.4. Automatic Vehicle Identification**

Automatic Vehicle Identification (AVI) systems are probe vehicle-based systems that usually rely on tags which reflect encoded radio signals transmitted from roadside antennas or readers. The reflected signals are modified by the tag identification code so that the tag's information can be read by the system. This type of system has also been adapted to collect traffic monitoring data. In a monitoring application, roadside antennas are installed along major highways where the DOT wants to collect information on travel times or speeds. The unique tag identification numbers are logged each time a vehicle passes by an antenna.

The travel time of the vehicle can then be explicitly calculated by examining when a vehicle passes known antenna locations on the highway. This provides true point-to-point travel times for all vehicles with tags. An example for this method is the system of toll transponders and vehicles' smart tags.

There are several issues related to the deployment of AVI-based monitoring systems:

1. The ability of AVI-based systems to provide useful data is directly linked to the number of potential probes on the road.

2. A sufficient number of probe vehicles must travel a route for the travel time and speed estimate to have statistical validity.
3. These systems require the installation of significant roadside infrastructure in the form of AVI tag readers and communications in order to gather data.
4. These systems have significant cost.

Capital costs for a single AVI site on a six-lane highway range from \$18000 to \$38000, with annual operating costs of \$4000 to \$6000 per site [14, 15]. If sites are to be spaced every 2 to 3 kilometers, this can be a significant cost. The advantage these systems have is that they can produce quality speed and travel time estimates.

#### **2.1.5. Automatic Vehicle Location**

Automatic Vehicle Location (AVL) refers to a suite of probe vehicle-based technologies that track the location of vehicles traveling through the roadway network. The more commonly-used method in AVL relies on global positioning system (GPS) data. GPS data is collected continuously by the vehicle and then periodically transmitted back to a central control facility over a radio backbone, cellular service, or satellite communications network. The location data generated by AVL systems could be mined to generate traffic data. Examples for such a system are transit companies which track location of buses on their routes [16] and combined traffic information providers such as INRIX [17] which fuse their data from several sources including AVL systems (usually from equipped trucks, buses, taxis, etc.).

The advantages of using this system are as follows:

1. Few pieces of fixed infrastructure are required.

2. Vehicles could be monitored anywhere on the network.
3. AVL technologies can provide estimates of speed and travel times on roads where no point sensors (*i.e.*, ILDs, MRS, cameras) are available.

This system has several limitations:

1. AVL has problem with the sample size limitations where only a small subset of the vehicle population is outfitted with AVL equipment.
2. The estimated travel time and speed includes the stop and load delay time of equipped vehicles during their trip (see Section 1.1.2).
3. The data source is also not widely available since participant companies may not share their data with others due to concerns about losing competitive advantages in the marketplace.
4. Usually the routes (*i.e.*, AVL drivers' path) are predictable and the generated data using AVL are not enough for monitoring the entire region.

#### **2.1.6. Wireless Location Technology**

Wireless Location Technology (WLT) involves using wireless devices to track the vehicle (generally the mobile passenger) movements or to transfer information from the vehicles for monitoring purposes. WLT systems are mostly based on the presence of cellular phones in vehicles for monitoring traffic. An example for such system is anonymous tracking of cellular phones [3, 18, 19]. WLT based traffic monitoring has the potential to expand both the number of vehicles being monitored and the size of the roadway network where data could be obtained.

The advantages of WLT using cellular phones are as follows:

1. A majority of car owners own cell phones.
2. Any road with cellular service can theoretically be monitored without installing any infrastructure on the road.
3. Aggregated data are not particular to the fixed points on the road and can provide information about several sampled locations on the road.

The major barriers to widespread use of WLT-based monitoring are as follows:

1. The spatial accuracy of the location estimates used by WLT systems is not as precise as GPS data. (*e.g.*, existing systems cannot distinguish between different phones in the same vehicle or even determine differences in travel speeds between adjacent lanes of traffic.)
2. Producing precise estimates of speed and travel time is highly affected by the precision of location estimates.
3. WLT has issues with the continuous consumption of bandwidth on the wireless link which can also cause congestion in the wireless network, information drops, or unwanted handoffs.
4. Questions about who owns the data and what rights a DOT has to distribute the data still remain since the data is generated by a third party vendor that sells the data as a service to a DOT.
5. Privacy of users in these systems is questionable.

## **2.2. USE OF CELLULAR NETWORKS AND SMARTPHONES**

An alternative to using dedicated sensing infrastructure is to leverage an existing communication systems such as the cellular phone network. The mobile Internet and



Web 2.0 are the underlying technologies and paradigms that have enabled the development of traffic estimation systems based on GPS-enabled phones as well as numerous other cellular device-based traffic monitoring systems. WLT systems are based on the presence of cellular phones in vehicles for monitoring traffic. But, these systems do not have the same location precision as GPS and cannot distinguish between different phones in the same vehicle. In addition, since these systems report data based on at a particular *time*, it is difficult to collect data at a certain *location*. In this dissertation, when we mention WLT systems we mean systems that rely on cellular network based positioning and handsets that do not use GPS. Although, handsets may be equipped with GPS device too.

There are several projects developing the use of wireless technology for traffic monitoring, such as PATH's Group-Enabled Mobility and Safety (GEMS) project [20]. GEMS is based on AVL and WLT technologies with use of Internet queries for delivering data to handheld devices. In the Mobile Millennium project [19], cell phones are the main part of the architecture. The project's concept of virtual lines is similar to our proposed virtual strips, which we propose and describe in Chapter 3. Using the mobile Internet, user-generated content (in this case, traffic data measured by the smart-phone) is sent to a central system, which provides information back to the cell phone owner for personal use. This Web 2.0 user-generated content-based location based service is commonly referred to as "participatory sensing", which refers to the ad-hoc process of voluntarily providing sensing data to a system. GPS-equipped vehicles transmit data upon traversing *virtual geographical sensors*, thus offering a promising

alternative to estimating traffic statistics using fixed point-sensors. This sensing technique leverages market-driven telecommunication infrastructure, thus limiting cost for society and users. Moreover, these virtual sensors, by definition, are not embedded in the physical infrastructure, and their location can be optimized dynamically (adaptively) as traffic conditions change. The location of the sensors can be dictated by the central system to optimize the value of each traffic sensor measurement sent to the system.

The Nericell project [21] is another example of using cellular networks and smart-phone for monitoring of road and traffic conditions. Nericell system uses mobile smart-phones equipped with an array of sensors (GPS, accelerometer, microphone) and communication radios. This project mainly focuses on the sensing components which are installed or setup on smart-phones, and specifically focuses on how these sensors and radios are used to detect bumps and potholes, braking, and honking, and to localize the phone in an energy-efficient manner. While GPS provides higher accurate location estimates compared to cellular technologies, it has several limitations such as some phones do not have GPS at all and the GPS sensor does not work in “urban canyons” (tall buildings and tunnels) or when the phone is inside a pocket. Also the GPS on many phones is power-hungry and drains the battery quickly. The VTrack system [22] addresses these key challenges (*e.g.*, energy consumption and sensor unreliability). In VTrack, energy consumption can be reduced using inaccurate position sensors (WiFi rather than GPS). VTrack uses a hidden Markov model (HMM)-based map matching scheme, with a way to interpolate sparse data to identify the most probable road segments

driven by the user and to attribute travel times to those segments, to obtain accurate travel time estimates from these inaccurate positions.

Besides the value of the data for traffic estimation, ensuring location privacy of the users is an important consideration for the deployment of mobile traffic sensing methods. A GPS-enabled smartphone is capable of recording and transmitting its GPS location every few seconds. While this level of detail can be useful for traffic estimation [23], it can be privacy invasive without the proper safeguards, since the device is ultimately carried by a single user.

We describe the advantages of using virtual strips and spatial sampling in Chapters 3. The same idea can be used via WLT systems. For example, *Virtual Trip Lines* (VTLs), similar to our contributed virtual strips, provide a mobile sensing framework that preserves the privacy of users [24]. VTLs are geographical markers embedded in the map, that trigger probabilistic measurement updates. Each VTL consists of two GPS coordinates which make a virtual line drawn across a roadway of interest. Instead of time-based periodic sampling, VTLs trigger disclosure of speed and location updates by spatial sampling, creating updates at predefined geographic locations on roadways of interest. Additionally, the travel time between pairs of VTLs can be extracted and this type of travel time data will be considered the primary data source used in this approach. This sensing paradigm of virtual trip lines has emerged as a viable solution for real-time traffic monitoring based on large-scale traveler participation. However, standard VTL deployment is static and does not take advantage of the mobility of probes compared to our proposed DTMon where virtual strips are defined dynamically.

### 2.3. USE OF VEHICULAR NETWORKS AND SENSOR NETWORKS

VANETs are networks in which each node is a vehicle. Such systems aim to provide communications between individual vehicles and between vehicles and nearby fixed equipment, or roadside units. The goal of VANETs, and more broadly vehicular networks, is to improve traffic safety by providing timely information to drivers and concerned authorities. The development of VANETs has received much attention from the automotive industry and government agencies, including the US Department of Transportation (DOT) which has launched the Connected Vehicle initiative [25, 26].

There have been a few attempts at using VANETs to monitor traffic. TrafficView [27] is a scalable traffic monitoring system for inter-vehicle communication considering road conditions, but it does not consider low penetration rates or low traffic density. CarTel [28] is distributed mobile sensor computing system that uses cell phones and cars as nodes in a dynamic sensor network. CarTel provides software to collect, process, and visualize data from sensors located on mobile devices to a central portal. Kitani *et al.* [29] have proposed traffic information sharing using public buses traveling regular routes. This VANET-based technique is only useful in urban areas with good public transportation systems and only monitors those areas traveled by the transit system.

As we show in Chapter 3, one of our main contributions is the first introduction of location-aware sensing strategies via VANETs (using DTMon and virtual strips) that are adaptive to the traffic conditions, thereby enabling the sensing infrastructure (*i.e.*, TOs) to take full advantage of the mobility of probes. TOs programmed by TMCs can also define the points of interest dynamically in addition to that in DTMon, TOs are also able to

redefine, modify, add, or remove these virtual strips dynamically based on the monitored traffic condition and real-time monitoring needs. We show the performance of DTMon in various traffic conditions in Chapter 3. The main difference between the WLT systems and our proposed DTMon is the base technology used in our framework. The technology used in DTMon relies on vehicular ad-hoc networks than cellular networks. There has been some work done in Connected Vehicle Research for traffic monitoring under low penetration rates [25, 26]. But, there has been no work done on monitoring traffic dynamically using VANETs for various traffic conditions and market penetration rates. Therefore we believe our research, the proposed framework, and experiments performed using DTMon will be a pioneer in real-time traffic monitoring with benefits of dynamically defined spatial criteria. We also believe that our work on the programmability of TOs can easily be adopted for use with different technologies and infrastructures such as cellular networks. Nevertheless, we aim to find methods that can augment the in-use monitoring technologies. Vehicular networks, cellular networks, or sensors networks or their combinations seem to be future of probe vehicle-based technologies [30].

## **2.4. NEED FOR AN INTEGRATED VANET SIMULATOR**

As we propose DTMon, we need to evaluate its performance under various traffic conditions and market penetration rates [31, 32, 33], and for that we need tools to evaluate vehicular ad-hoc networks with realistic mobility [34].

In order to provide applications that can fulfill this vision, approaches must be thoroughly evaluated. There are a limited number of testbeds with instrumented vehicles

and roadside units. As this is prohibitively expensive for most academic researchers, the majority of evaluation studies have been performed via simulation. VANET simulations have typically been segregated into traffic simulations and network simulations. Traffic simulators, such as CORSIM [35], SUMO [36], VISSIM [37], and VanetMobiSim [38] have been used to generate realistic mobility traces of vehicle traffic. These traces would then be fed into well-known network simulators such as ns-2 [39], QualNet [40], OPNET [41], or GloMoSim [42] to measure network performance. VANET tools such as TraNS [43] and MOVE [44] have been used to facilitate this interaction between traffic and network simulators. More recently, researchers have developed integrated simulators such as ASH [45] and Gorgorin et al. [46] that allow feedback between the applications using the network and the traffic model. This is important because the goal of most VANET applications is to provide drivers with information that may change their driving behavior or allow them to make more informed decisions (*e.g.*, start braking now, or take the next exit to avoid a traffic jam). Interested readers can find detailed comparisons of various VANET simulators in Hassan [47] and Yan et al. [48].

The problem with integrated simulators is that often either the mobility model is overly simplified or the network model is overly simplified. In order to study important networking properties of VANETs, a high quality network simulator is essential. As we will show in Appendix A, we have chosen to balance these two concerns by taking the latest version of the highly-regarded network simulator, ns-3 [49], and adding a well-known traffic mobility model in order to provide an integrated simulator for VANET research.

ns-3 is a discrete-event network simulator written in C++, targeted primarily for research and educational use and intended as a replacement for the popular ns-2 simulator. ns-3 promises to be a more efficient and more accurate simulator than its predecessor (especially for wireless protocols). In addition, during the first quarter of 2010, ns-3 averaged almost 7000 downloads per month [49]. For this reason, we were interested in using ns-3 to perform our VANET simulations. ns-3 provides various mobility models, but none are appropriate to simulate the mobility of vehicles. The mobility of a node in the mobility models included in ns-3 depends only on the node itself. In realistic vehicular mobility, the mobility of the node must depend on the surrounding nodes and the conditions on the road. Furthermore, this node dependency becomes essential when messages in the network can affect the mobility of the nodes on the roads. For example, the receipt of a safety message may result in a speed reduction. Fiore and Harri [50] and Fiore [51] investigated the effects of node mobility on network characteristics. They found that realistic mobility, especially at intersections, has a great impact on networking connectivity metrics and that car-following models, such as the Intelligent Driver Model (IDM) [52], provide realistic movement. In addition, they found that multi-lane scenarios are important when considering network-level clustering. We describe our integrated VANET simulator [34] using the implemented IDM and the MOBIL lane change model [53] in Appendix A.

## 2.5. SUMMARY

We presented related work on traffic monitoring technologies and systems in this chapter. We started with the most commonly used technologies (*e.g.*, ILDs, radars, cameras) and

showed that these fixed-point sensors and detectors are not sufficient for real-time monitoring of traffic. Then, we showed that there have been several works on the use of cellular networks and smart-phones to augment these monitoring systems and technologies. We presented vehicular networks as a solution which can be used to augment the introduced monitoring systems. We described the related work on probe vehicle-based systems either with use of cellular networks or VANETs. And more importantly, we presented related work for our dynamic traffic monitoring system (DTMon) using VANETs. In addition to describing benefits of using each of these technologies and specifically DTMon, we described that there was a need to develop an integrated VANET simulator. We concluded the section with related work on VANET simulators.



## CHAPTER 3

### DTMON: DYNAMIC TRAFFIC MONITORING

We describe the main components of the proposed framework and DTMon in this chapter. The main components of DTMon are the roadside units, *task organizers*, and the equipment inside each vehicle. In addition, we introduce the idea of a *virtual strip*. The communication among vehicles and roadside units in DTMon happens via standard channels using Dedicated Short-Range Communications (DSRC) [6, 7, 8] which is the currently proposed standard for vehicular communications due to its low latency, making it suitable for safety applications. Using DTMon and its components, we introduce methods to monitor traffic data (*i.e.*, flow rate, density, speed, travel time) in urban and rural roads in this chapter. With the assumption that some percentage of vehicles are equipped (the market penetration rate of the system) with a Global Positioning System (GPS) device for positioning, a detailed digital road map for route guidance, and a transceiver for communication (DSRC device), we explain and analyze the impact of market penetration rate and traffic density on the performance of DTMon. We conclude this chapter with strategies for the deployment of TOs.

#### 3.1. COMPONENTS AND METHODOLOGY

We describe the main components of DTMon in this section. We explain our approach and how DTMon can be used to monitor in rural (highway) and urban (arterials and intersections) areas.

### **3.1.1. Task Organizer**

A TO is responsible for communicating with vehicles to inform them about upcoming traffic conditions, assign measurement tasks, collect traffic data and organize the received measurements. The TO may be the property of the local Department of Transportation (DOT) and should be able to directly communicate with the local Traffic Management Center (TMC). The goal of the TO is to provide accurate measurement information to the TMCs and to disseminate timely messages or traffic reports from the TMCs to equipped vehicles.

We assume that there is at least one TO deployed along the road. Each TO is equipped with a DSRC transceiver and communicates with passing equipped vehicles. Deploying multiple task organizers along the road and applying the union of information gathered by them is an option for complex roads and for higher performance. The equipped vehicles and the task organizer use a common piece of application software for communication. Through neighbor discovery (described in Section 3.1.3), we assume that equipped vehicles and the task organizer are aware of the position of equipped vehicles within the standard DSRC communication range (300 meters).

### **3.1.2. Vehicles**

Equipped vehicles contain the following equipment:

- GPS device
- DSRC transceiver
- Vehicle ID (maps to the vehicle network address or the vehicle identification number)

- Detailed digital map
- Processor/memory which interacts with the GPS device and DSRC transceiver
- Common piece of application software for communication and for processing the tasks and managing the events

Equipped vehicles are able to record the following information:

- Speed
- Spatial location represented in the form of (longitude, latitude, altitude)
- Direction
- Timestamp
- Travel time (*i.e.*, travel time of a segment defined by two virtual strips)
- Route (*i.e.*, road, street, highway) number
- Lane number, if available. We assume that vehicles are not directly aware of the lane they are in due to GPS inaccuracies. But, we assume that digital road maps may contain information about the number of lanes on each roadway.

Each transmitted message from a vehicle contains a header that includes all of the above information (shown in **Figure 6**). Vehicles may receive tasks from a TO and forward the tasks to other vehicles. They can also produce new messages and forward them back to the TO. Vehicles may store and carry the messages to the next available TO. Vehicles can talk to the TO directly or indirectly (via messages forwarded to the TO by other vehicles).

ID	Timestamp	(Longitude, Latitude, Altitude)	Speed	Route	Lane	Direction	...
----	-----------	---------------------------------	-------	-------	------	-----------	-----

**Figure 6. A sample header of a message or a report.**

Vehicles have the ability to store events that should be fired at a specific *time*, *speed*, or *location*. As vehicles will be running other VANET applications, we anticipate that much of the information needed to report traffic measurements can be piggybacked on the packets from these other applications or be gathered from data produced by these other applications, as is done by Robinson *et al.*'s Message Dispatcher [54] and in CASCADE by Ibrahim [55]. We assume vehicles and TOs are able to communicate securely using the latest security techniques introduced for VANETs [56, 57, 58, 59, 60, 61, 62].

### 3.1.3. Message Delivery

DTMon can use well-known neighbor discovery, routing, and message forwarding techniques (such as [63, 64, 65, 66, 67]) or opportunistic message dissemination methods (such as OPERA[68] and SODA[69]) to pass tasks and information through the VANET and to the TO. At the moment TOs are deployed, the topology of the roads and the paths that messages may travel are known, therefore geographical routing can be appropriate for our system. Our focus in this thesis is on how to monitor traffic measurement data rather than introducing a new forwarding or routing algorithm, although we investigate how to improve message delivery and the amount of received information. Both vehicles and the TO perform neighbor discovery (**Pseudocode 1**) periodically, for instance each second. In neighbor discovery, each equipped vehicle broadcasts a report containing its

current position, speed, direction, route, ID, and timestamp (see **Figure 6**). This type of neighbor discovery may be part of a larger geographical routing algorithm. This exchange of information can be piggybacked on messages sent by safety applications that are broadcasted many times per second so that little overhead is added. Once a vehicle receives a report from its neighbor, it checks its *neighbor list*. If the neighbor is unknown (*i.e.*, does not exist in the list), it is added. If the neighbor appears in the list, its information is updated. If no reports from a particular neighbor are received for two seconds, it is removed from the neighbor list.

#### **Neighbor Discovery**

$N$  - neighbor list

$R\_ID$  - report that contains current position, speed, direction, route, ID, and timestamp

Broadcast report,  $R\_ID$

Receive  $R\_ID$  reports from neighbors

Update  $N$

- add previously unknown neighbors to  $N$
- update information on known neighbors
- remove expired neighbors from  $N$

#### **Pseudocode 1. Neighbor discovery.**

Messages and measurement tasks can be created by a TO or by a vehicle. They can be stored and carried or forwarded in any direction along a road to reach to the desired location, segment, vehicle, or TO. Vehicles and TOs update their neighbor list of reachable vehicles, *i.e.*, those within the DSRC range of  $R_0$  meters, periodically as described in this section.

In addition to sending messages to neighbors, messages can also be forwarded to a vehicle or TO outside of the sender's range. To use the fewest possible hops when a message is forwarded, it is best that the first hop be the neighbor farthest from the sender in the appropriate direction.

**ForwardMessage ( $V, D, M$ )**

$V$  - current vehicle

$D$  - destination

$M$  - message or task

$N$  - neighbor list

$R_0$  - communication range

$R$  - filtered range

$d(x, y)$  - distance from  $x$  to  $y$

if ( $d(V, D) \leq R_0$ )

    send  $M$  directly to  $D$

else

$L \in N \mid d(V, L) \leq R$

    if ( $L = \text{NULL}$ )

        exit, message cannot be forwarded at this time

    else

$V' \in L \mid d_{\max}(V, V')$

        send  $M$  directly to  $V'$  (with final destination  $D$ )

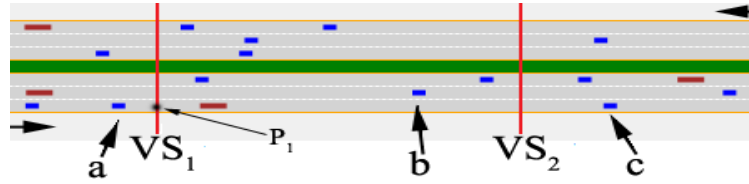
**Pseudocode 2. Forwarding a message to a destination (Multi-Hop).**

We show the basic outline of the forwarding process in **Pseudocode 2**. If the destination vehicle  $D$  is in range of the sender  $V$ , the message is sent directly to  $D$ . Otherwise, the neighbor list is scanned to find the most appropriate next hop. Choosing the appropriate target vehicle for the next hop depends not only on its distance from the sender and signal strength, but also on the speed at which it is traveling.

A situation could arise where a vehicle selects a target vehicle near the boundary of its range, but by the time the message is constructed, the vehicle has moved outside of the sender's communication range. To avoid this situation, vehicles will only select target vehicles from a filtered list. This filtered list is the set of neighbors that are within  $R$  meters of the sender, where  $R < R_0$ . The difference  $(R_0 - R)$  is such that the target vehicle will remain within communication range of the sender until the next update, based on the maximum relative speed between the two vehicles. The received signal strength can be also applied as a factor to select the appropriate vehicle from the filtered list. For example, assume the speed limit is  $33 \text{ m/s}$ , then the maximum relative speed between two vehicles in the worst-case (traveling in opposite directions) would be  $66 \text{ m/s}$ . So, relative to the sender, the target vehicle can move at most 66 meters in one second, or 66 meters between neighbor discovery events. If  $R_0$  is the DSRC range and the frequency of neighbor discovery is 1 second, then  $R$  should be set to at least  $(R_0 - 66)$  meters.

#### **3.1.4. Virtual Strips**

A *virtual strip* (VS) is an imaginary line that crosses a road. Virtual strips, or *strips*, can be defined geometrically as the intersection of a plane with the road (another plane). In practice, the spatial location is represented in the form of longitude, latitude, and altitude which can be used to express the points on every lane of the road that virtual strips intersect. For example, a virtual strip over a road with three lanes can be represented in a simpler way with only three points instead of a line, one point for each lane.



**Figure 7. Illustration of virtual strips and segments.**

A vehicle can reside on only one side of a strip. If vehicle is moving towards a strip, it is *before* the strip, and once it passes the strip it is considered to be *after* the strip. Two virtual strips can be used to create a *virtual segment*, or *segment*. Vehicles can be *inside* the segment or *outside* the segment.

We use **Figure 7** as an illustration of strips and segments. The virtual strips are labeled  $VS_1$  and  $VS_2$  and are boundaries for the virtual segment  $VS_1VS_2$ . Vehicle *a* is before  $VS_1$  and outside the segment  $VS_1VS_2$ , while vehicle *b* is after  $VS_1$ , before  $VS_2$ , and inside the segment. Vehicle *c* is after  $VS_2$  and outside the segment.  $P_1$  is the point representing the location of  $VS_1$  at the right most lane on the road in practice. For a roadway with six lanes like the one illustrated in **Figure 7**, six points would be used to represent  $VS_1$ . These points can be easily mapped and represented on a vehicle's digital map.

### 3.1.5. Approach (Rural Areas/Highways)

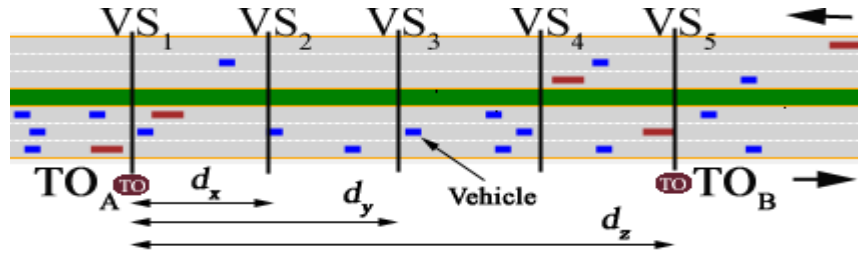
We first consider how DTMon can be used in rural areas or on highways.

#### 3.1.5.1. Traffic Volume, Flow Rate, Speed, and Classification

We can use the same basic procedure to monitor several important traffic metrics: traffic volume, flow rate, time mean speed (TMS), and vehicle classification. **Figure 8**



illustrates a situation where one TO ( $TO_A$ ) is located next to the road and has defined the virtual strip  $VS_1$ . The virtual strips  $VS_1$ ,  $VS_2$ ,  $VS_3$ , and  $VS_4$  can be the desired strips, or points of interest, for monitoring traffic data. Vehicles will report as they pass these strips (e.g., the marked vehicle passing strip  $VS_3$  in **Figure 8**).



**Figure 8.** Five dynamically defined virtual strips and two TOs.

In **Pseudocode 3**, we outline our algorithm to monitor the traffic volume at a desired virtual strip. The source TO ( $TO_{src}$ ) broadcasts a task containing its location, a list of potential destination TOs ( $TO_{des}$ ), the data to collect, and a list of the virtual strips where the data should be collected. Each vehicle passing  $TO_{src}$  receives the task. When a vehicle passes each VS in the list, the task is triggered, and the vehicle sends a message to the closest  $TO_{des}$ . This message contains the original task,  $TO_{src}$ ,  $TO_{des}$ , the VS, the vehicle's current speed, position, and vehicle classification (the type of the vehicle). Note that when there is only one TO deployed on the road,  $list<TO> = TO_{src}$ , therefore  $TO_{des} = TO_{src}$ . Once several messages have been received,  $TO_{des}$  can calculate the flow rate, volume, TMS, vehicle classification, and VMT metrics for a VS over a specific time interval.

**Monitor**[*flow rate, classification, speed, ...*]  
*V* - current vehicle ID  
*P* - current vehicle position  
 $TO_{src}$  - source task organizer  
 $TO_{des}$  - destination task organizer  
 $\langle list \rangle(TO)$  - list of possible destination TOs  
 $\langle list \rangle(VS)$  - list of desired virtual strips  
 $K$  - task (data to collect,  $TO_{src}$ ,  $\langle list \rangle(TO)$ ,  $\langle list \rangle(VS)$ )  
 $E$  - vehicle's event list  
 $M$  - message

Receive task  $K$  from  $TO_{src}$   
 if ( $K \in E$ ) ignore, exit  
 else add  $K$  to  $E$   
 while (exists unmarked  $VS$  in  $list\langle VS \rangle$ )  
   update GPS position  $P$   
   if ( $P < VS$ ) continue  
   if ( $P > VS$  and  $VS$  is not marked)  
     Mark  $VS$  as passed  
      $TO_{des}$  = closest TO to  $VS$  (from  $list\langle TO \rangle$ )  
      $M = K, TO_{des}, VS, speed, classification, timestamp, V,$   
      $P$   
     Forward  $M$  to  $TO_{des}$    \*Note: See Section 4.1 delivery methods  
 Remove  $K$  from  $E$

### Pseudocode 3. Monitoring traffic volume.

Type: *Volume-Speed*  
 Delivery Method: *Store-and-Carry (SAC)*  
 Source TO:  $TO_A(x_a, y_a, z_a)$   
 Target TO:  $TO_B(x_b, y_b, z_b)$   
 Target Strips:  $VS_1, VS_2, VS_3, \dots$

**Figure 9. A sample volume-speed task. This task from  $TO_A$  requires vehicles not to forward the report but to store and carry them to  $TO_B$ . The virtual strips of interest (dynamic points) are listed as target strips.**

**Figure 9** shows a sample format of the volume task. The task description can include multiple virtual strips, the type of message delivery method, and the location of the TOs where message must be delivered.

***Target Strip Outside Range of TO:*** First, we consider the scenario where the target strip is outside the communication range of the nearest TO. Through neighbor discovery, the TO knows about the vehicles that are within DSRC range and before the TO's nearest virtual strip. The TO sends a task to these vehicles, requesting them to forward volume information back to the TO (or to the other TOs given in the task) once they pass the target virtual strip. These vehicles store the task as an event that should be raised when the vehicle passes the location of the target strip. Each time the TO receives a completed volume message, it will increment the number of vehicles that have passed the target strip. Thus it can compute the traffic volume and flow rate at the target strip for any duration.

***Target Strip Within Range of TO:*** If the target strip is within communications range of the TO, no forwarding will be required. In this case, the TO may be able to collect volume, speed, and classification data for each lane of the road. The TO can use its own location as a reference and the location of each vehicle that passes the target strip as a target location. Therefore, it can estimate the lane in which the vehicle is traveling. There might be some GPS inaccuracy when a vehicle reports its location, but the TO can approximate this inaccuracy by assuming a particular width for each lane and comparing the vehicle's reported location to the location of vehicles traveling in the adjacent lanes as a second reference. It is the TO's responsibility to collect the coordinates of vehicles

during some period to estimate the boundaries of the left-most and right-most lanes. The received signal strength indicator (RSSI) may also be used to estimate the lane number, as it has been shown that RSSI can be used by a receiver to estimate the location of the transmitter [70].

***Low Density or Low Penetration Rate Considerations:*** In situations where there are few equipped vehicles (either due to low penetration rate or low density traffic), message forwarding may fail, and the TO may count a much lower volume and density than the ground truth. Road profiles and the past history of the roadway may also be a great help in determining the difference between low penetration rates and low traffic density. There are several papers that have investigated the impact of low density traffic (resulting in a sparse network) in VANETs [63, 65, 71, 72, 73, 74]. We suggest measuring traffic volume at some point inside the communication range of a TO to avoid the need for forwarding and to mitigate the effects of low traffic density.

In Chapter 4, we will describe methods to improve the message delivery. These methods include using the traffic in opposite direction, using dynamic transmission range, and requiring vehicles to store and carry the messages (*i.e.*, whenever they cannot be forwarded) to the next available TO.

### **3.1.5.2. Traffic Density**

Traffic density refers to the number of vehicles on a section of road. Current methods that monitor this type of data, like aerial photography, are not cost effective. Most often, *detector occupancy* [3, 10], the percentage of time that the detector (such as an ILD) is active due to the presence of a vehicle, is used as a surrogate measure for traffic density.

**CalcDensity**

$TO$  - task organizer  
 $C$  - vehicle count inside target segment  
 $VS_1$  - target strip [segment beginning]  
 $VS_2$  - target strip [segment end]  
 $K$  - task (contains  $TO, VS_1, VS_2, C = 1$ )  
 $E$  - event list  
 $N$  - neighbor list  
 $V$  - current vehicle  
 $P$  - current vehicle position  
 $d(x, y)$  - distance from  $x$  to  $y$   
 $R$  - communication range

Receive density task  $K$  from  $TO$   
 if ( $K \in E$ ) ignore, exit  
 else add  $K$  to  $E$   
 while ( $P < VS_1$ )  
     update GPS position  $P$   
 if ( $P > VS_1$ )  
     if ( $P < VS_2$ ) Collect ( $TO, VS_1, VS_2, C$ )  
 else  
      $D = d(P, VS_1)$   
      $R' = \min(R, D)$   
      $N' \in N \mid d(V, N') \leq R'$   
      $V' \in N' \mid d_{max}(V, V')$   
     Forward  $K$  to  $V'$

Collect:

$D = d(P, VS_2)$   
 $N' \in N \mid d(V, N') \leq R$  and  $N' < VS_2$   
 $C += \# N'$   
 if ( $D > R$ )  
      $V' \in N' \mid d_{max}(V, V')$   
     Forward modified  $K$  to  $V'$

**Pseudocode 4. Monitoring traffic density.**

We show how VANETs can be used to collect instantaneous, accurate traffic density of a desired segment on the road. In the following explanation, we assume a limited

deployment with a single TO, although multiple TOs can be applied as described in Sections 3.1.5.1 and 3.2.3.2. The existence of additional TOs would necessarily reduce the amount of forwarding required as the resulting density measurement could be passed to an upcoming TO rather than being passed backwards to the originating TO.

The basic algorithm is outlined in **Pseudocode 4**. The TO periodically selects a vehicle that is in range and sends it a “Density” task containing the location of the TO, the boundaries of the segment of interest ( $VS_1$  and  $VS_2$ ), and a count initialized to 1. If the vehicle is inside the segment of interest, the task is triggered. The vehicle counts the number of its neighbors that are both inside its communication range  $R$  and before  $VS_2$ . Then, the vehicle updates the count in the task message.

If the distance to  $VS_2$  is greater than  $R$ , the vehicle will forward the task (with the updated count) to its farthest neighbor inside  $R$  and before  $VS_2$ . Otherwise, the updated task message will be forwarded back to the TO. If the vehicle receiving the task from the TO is outside the segment of interest, it will initiate message forwarding to a vehicle either at the edge of its communication range or to a vehicle just inside the segment of interest.

Vehicles in charge of counting the number of vehicles can also aggregate the average speed of the vehicles they count. In this case, the same message that contains density information can contain the average speed of vehicles counted inside the segment. This average speed may be another useful metric that the TO can collect. The TO could request this kind of task periodically to monitor the current travel situation on the roadway. We have to mention that this method can only calculate the density of equipped

vehicles and that the quality of computed density is highly dependent on the market penetration rate. Computing density based on flow rate and speed [31, 32, 33] is faulty since a TO's estimated flow rate has significant error as compared to the actual flow rate. We explain this fact in Section 3.2 and Chapter 4.

In our description, we have assumed a 100% penetration rate of equipped vehicles as our method can only count equipped vehicles. With a lower penetration rate, the TO can estimate density if it knows the approximate penetration rate ahead of time. It is possible that messages cannot be forwarded to measure density in an entire segment due to low density traffic or an obstacle blocking the roadway. In such situations, if a delegate vehicle cannot forward the message farther, it will forward to the TO the density up to the last strip in which it was able to collect data. In the worst case, the TO may not receive any message for a requested density task. If this occurs repeatedly over some time interval, the TO should check the traffic volume to infer either low density traffic or severe congestion inside the segment.

### **3.1.5.3. Travel Time and SMS**

We propose a method to collect travel times, which is very similar to the method used to monitor volume-speed data. In general, if vehicles include a timestamp when they report the volume message, the travel time is the difference between the timestamps gathered from two different strips for each vehicle. Thus, the TO only needs to keep a record of vehicles that have passed two particular strips. When it is collecting volume for those strips, the TO can also compute the travel time for an individual vehicle, or the mean travel time for all reporting vehicles over a certain period of time. An alternate method

for calculating the travel time is to let each vehicle compute its travel time after it passes the two strips of the segment and send the result back to the TO included in reports.

Another metric of interest to traffic engineers is the space mean speed (SMS). The SMS is based on the average speeds of vehicles over an extended segment. To compute SMS, the TO needs the travel time of each vehicle that passes through a segment. Then, the TO can calculate the SMS as the length of the segment divided by the average travel time.

### **3.1.6. Approach (Urban Areas/Arterials and Intersections)**

An urban area can be modeled with a combination of streets and intersections. TOs can be located beside streets or they can be located at the corner, center of intersections, or on top of the roads. We suggest that TOs be located at intersections since one TO will be enough to monitor traffic data on joint streets as well as in the intersection. Like ILDs, TOs can be deployed only on those intersections and streets that are considered to be significant for the TMC.

#### **3.1.6.1. Streets**

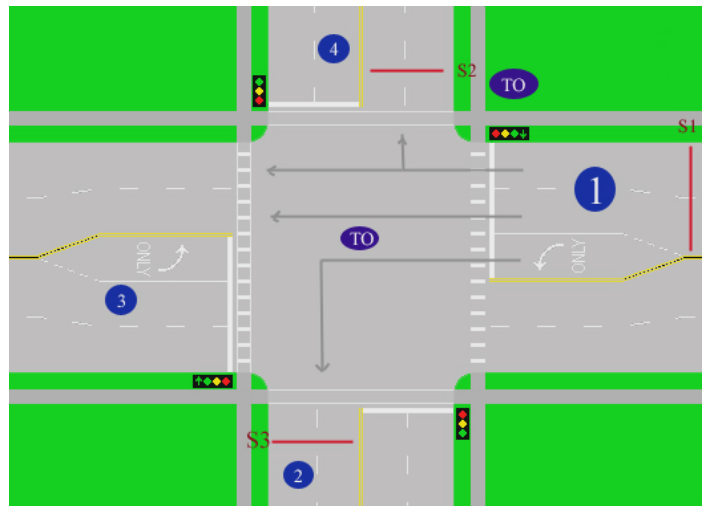
The method for collecting traffic data in streets is similar to method we described in Section 3.1.5. Each street can be assumed to be a virtual segment. Therefore, TOs can collect traffic volume, density, speed, classification, and travel time for any strip and segment of the streets. The TMC or centralized TOs can combine the travel times of connected streets and related intersections to estimate the travel time of desired paths in the area. Each TO is also capable of broadcasting this type of information to vehicles on the road.



### 3.1.6.2. Intersections

Count information is the key data to be collected at intersections as it can be used to calculate traffic flow and volume. We consider two possibilities to model an intersection: *stop sign model* and *traffic light model* [38, 75, 76]. One TO is located either at the center of the intersection or on one side in the corner. The TO broadcasts tasks frequently and monitors individual vehicle reports. Vehicles approaching the intersection may turn right, turn left, or move straight. Therefore they can potentially pass through one strip from three possible strips (except u-turns). For example in **Figure 10**, the content of a task for vehicles (westbound) at the road numbered “1” (practically route number and direction) is as follows:

- this task is for vehicles on “1” before virtual strip  $s_1$
- report the code “1” back to TO whenever you pass virtual strip  $s_1$ ,  $s_2$ , or  $s_3$
- ignore duplicate tasks



**Figure 10.** An illustration of TO and virtual strips in an intersection.

Vehicles will receive tasks (stored as events). They will raise an event whenever they pass the specified strips. The message sent to the TO by a vehicle will contain its speed, classification (the type of the vehicle), route number, timestamp, and the *code number*. The code number, embedded in the task and received by the vehicle, will help the TO collect data for the various turning options. For example, consider a vehicle that receives the task with code “1” and then passes  $s_3$ . The TO can determine that this vehicle turned left at the intersection. In the same way, the TO can monitor the traffic volume for vehicles that turned right, turned left, or continued straight at each intersection. There is no need for vehicles to route or forward messages and tasks to/from the TO since the TO and strips are in the vehicles’ communication range.

### **3.2. IMPACT OF TRAFFIC DENSITY AND MARKET PENETRATION RATE**

DTMon is a probe vehicle-based system. *Market penetration rate*, the number of equipped vehicles (usually as percentage) satisfying VANET standards within a specific population of vehicles, plays a significant role in amount of received and collected information; as a result, the quality of collected data can be highly dependent on the market penetration rate. It is possible to roughly estimate the density of equipped vehicles in an area by knowing the traffic density (function of flow rate and speed) and the market penetration in that area. In addition to market penetration rate, the traffic density of equipped vehicles in the area is a factor which can determine the amount of success in information delivery among vehicles and toward the location of TOs in that area.

Any increase in market penetration rate is gradual, requiring the increase of the production rate in automobile factory lines to produce vehicles equipped for VANETs.

On the other hand, the traffic density of equipped vehicles does not vary only by the market penetration rate but also can vary based on the conditions on the road in various scenarios such as rural area, urban area, traffic with congestion, high speed free flow traffic, low speed free flow traffic, low capacity roadways, affluent neighborhood, etc.

Two broad categories of congestion can be defined: *recurring* and *nonrecurring*. *Recurring congestion* occurs regularly at the same locations on the roadway system at about the same time every day. *Nonrecurring congestion* occurs when a crash or other unusual event (such as a construction zone or severe weather) causes a reduction in the traffic-carrying capacity. These two types of congestion result in an increase in traffic density which can directly and indirectly impact the amount of success in information delivery, latency (delay in delivery), travel time and vehicular delay. We will define *message reception rate* and *information reception rate* to evaluate the performance of TOs in receiving information from virtual strips using VANETs and for different message delivery methods.

In this section, we analyze the effect of the market penetration rate and the traffic density on our system and DTMon. We explain various scenarios and strategies for deploying TOs. We also describe the advantages of TOs and dynamically defined virtual strips in augmenting the current in-use systems.

### **3.2.1. Message Reception Rate and Information Reception Rate**

The *Message reception rate* (MRR) for a particular virtual strip is the percentage of messages generated by equipped vehicles passing the strip that were received by the TO.

The MRR shows what ratio of generated messages has been received to TOs. The upper limit for message reception rate is 1.0 (or 100%).

The *Information reception rate* (IRR) for a particular virtual strip is the percentage of messages received by the TO out of all possible messages generated by vehicles passing the strip (*i.e.*, as if all vehicles were equipped). The IRR indicates how well count information can be collected by the TOs under scenarios with different market *penetration rates* (PR).

The information reception rate can be estimated by knowing the message reception rate and the market penetration rate, as shown in Equation 6.

$$\text{Information Reception Rate} = \text{Penetration Rate} \times \text{Message Reception Rate} \quad (6)$$

For example, in 50% (0.5) market penetration rate, the information reception rate for a strip with 59% (0.59) message reception rate will be estimated as  $0.5 * 0.59 \approx 0.29$  or 29%. The upper limit of the IRR is equal to the PR. Thus, if the PR is 100%, then  $\text{IRR} = \text{MRR}$ .

### 3.2.2. Effect of Market Penetration Rate and Traffic Density

Equations 3, 4, and 5 in Chapter 1 (Section 1.1.6) show the relationship that exists among density and spacing as well as among density, flow rate, and speed. In practice, estimating traffic speed and flow rate is a simpler task. Also, most of the research that models traffic for vehicular networks applies spacing among vehicles (equipped vehicles) to analyze the behavior of the communication and the network [72, 73, 74].

We consider several traffic characteristics in our analysis, including inter-vehicle spacing, density, flow rate, and mean speed. Inter-vehicle spacing is the distance between vehicles. Density is the number of vehicles occupying a certain area, usually represented

in vehicles/*km*. Flow rate is the number of vehicles passing a certain point over a certain amount of time, usually represented in vehicles/*h*. In free-flow traffic, the mean speed typically follows a normal distribution [1, 2]. The relationship between inter-vehicle spacing, flow rate, and mean speed is expressed in Equations 7 and 8:

$$\beta \approx \frac{S}{v} \quad (7)$$

$$\frac{S}{v_{max}} \leq \beta \leq \frac{S}{v_{min}} \quad (8)$$

where

$\beta$  = inter-vehicle spacing (density is  $1/\beta$ )

$v$  = flow rate,  $v_{min} \leq v \leq v_{max}$

$S$  = average speed (mean) of vehicles with normal distribution (TMS can be used as an appropriate estimate for  $S$ ).

The flow rate in low and medium density traffic can have three different types of distributions: Poisson, exponential, or uniform [2]. According to the derived equations for inter-vehicle spacing with an exponential distribution [72, 73, 74], the probability that messages are forwarded successfully is very low for distances farther than the transmission range of the TO, taking into account the market penetration rate.

The flow rate in very low density traffic has a Poisson distribution [2]. The probability that the spacing of equipped vehicles in an interval is less than the communication range is zero, taking into account the PR and traffic speed. Therefore, the chance that a message is able to be forwarded even a short distance will be zero. The conclusion is that in very low density traffic, the IRR will be very small and TOs will not

receive any messages from outside their communication range. Therefore, methods other than message forwarding must be considered to improve the IRR in such traffic conditions.

For the remainder of this section, we consider medium to high density traffic, where the flow rate  $v$  varies in a limited range and the inter-vehicle spacing  $\beta$  will have a uniform distribution. Adding in the market penetration rate  $p$ , we can calculate the inter-vehicle spacing of equipped vehicles  $E_p$  as shown in Equation 9:

$$E_p \approx \frac{S}{pv} \quad (9)$$

$$\lambda_{min} \leq E_p \leq \lambda_{max} \quad (10)$$

$$\lambda_{min} = \frac{S}{pv_{max}} \text{ and } \lambda_{max} = \frac{S}{pv_{min}} \quad (11)$$

where

$p$  = market penetration rate

$E_p$  = inter-vehicle spacing of equipped vehicles ( $E_{l,0} = \beta$  and  $E_{0,0} = \infty$ )

$E_p$ , inter-vehicle spacing of equipped vehicle with market penetration rate  $p$ , has an uniform distribution according to Equations 9, 10, and 11. Equation 12 shows the probability density function (pdf), and Equation 13 shows the cumulative distribution function (cdf) of  $E_p$  respectively.

$$f_{E_p}(x) = \frac{1}{\lambda_{max} - \lambda_{min}} \quad (12)$$

where

$$\lambda_{min} \leq x \leq \lambda_{max} \text{ otherwise } f_{E_p}(x) = 0.$$

$$\begin{aligned}
F_{E_p}(x) &= \frac{x - \lambda_{min}}{\lambda_{max} - \beta_{min}}, & \lambda_{min} \leq x \leq \lambda_{max} \\
F_{E_p}(x) &= 0, & x \leq \lambda_{min} \\
F_{E_p}(x) &= 1, & x \geq \lambda_{max}
\end{aligned} \tag{13}$$

Equation 14 shows the mean, expected values, for inter-vehicle spacing  $E_p$ .

$$E[E_p] = \frac{\lambda_{max} + \lambda_{min}}{2} \tag{14}$$

In this analysis, we assume that a connection between two equipped vehicles can only take place if the spacing between them does not exceed the maximum DSRC transmission range  $R_0$  and propagation loss is negligible. For a vehicle to forward a message, it must be able to find at least one equipped vehicle in its neighbor list within  $R_0$ . Equation 15 shows the probability of a connection between two vehicles,  $P_{connected}$ .

$$P_{connected} = P\{E_p \leq R_0\} = F_{E_p}(R_0) \tag{15}$$

$$P_{disconnected} = P\{E_p > R_0\} = 1 - F_{E_p}(R_0) \tag{16}$$

where

$P_{connected}$  = probability that vehicles are connected.

$P_{disconnected}$  = probability that two vehicles are disconnected or the message cannot be forwarded farther.

A message can be forwarded through a group of connected vehicles (*i.e.*, a cluster of equipped vehicles). The inter-vehicle spacing  $C$  in a cluster of connected vehicles will have the following pdf:

$$f_C(x) = P\{E_p | E_p < R_0\} = \frac{x - \lambda_{min}}{R_0 - \lambda_{min}} \tag{17}$$

Therefore the average distance between two connected vehicles in a cluster will be

$$E[C] = \frac{R_0 + \lambda_{min}}{2} \quad (18)$$

Let  $C_v$  be the number of equipped vehicles in the cluster, then the probability mass function for a cluster with size  $n$  will be

$$f_{C_v}(n) = P_{disconnected}(P_{connected})^{n-1} \quad (19)$$

Let  $E_{pi}$  be the inter-vehicle spacing between vehicle  $i$  and  $i+1$ , then

$$f_{C_v}(1) = P\{E_{p1} > R_0\} = P_{disconnected}$$

$$f_{C_v}(2) = P\{E_{p1} \leq R_0 \wedge E_{p2} > R_0\} = P_{connected} P_{disconnected}$$

$$f_{C_v}(3) = P\{E_{p1} \leq R_0 \wedge E_{p2} \leq R_0 \wedge E_{p3} > R_0\} = (P_{connected})^2 P_{disconnected}$$

...

$$f_{C_v}(n) = P_{disconnected}(P_{connected})^{n-1}$$

Therefore, the expected number of vehicles in the cluster is:

$$E[C_v] = \frac{1}{P_{disconnected}} \quad (20)$$

$E[C_v]$  is as same as the expected number of hops that a message traverses to be forwarded in a segment (from a VS to another VS or to a TO) within the traffic flow. As a result, the average cluster length  $L$  will be

$$E[L] = (E[C_v] - 1)E[C] \quad (21)$$

In a segment of size  $d$  (e.g., the distance between a VS and a TO), the probability of having a successful message reception by a TO can be estimated by knowing the average number of hops required and the probability of connectivity at each hop as shown in Equation 22:

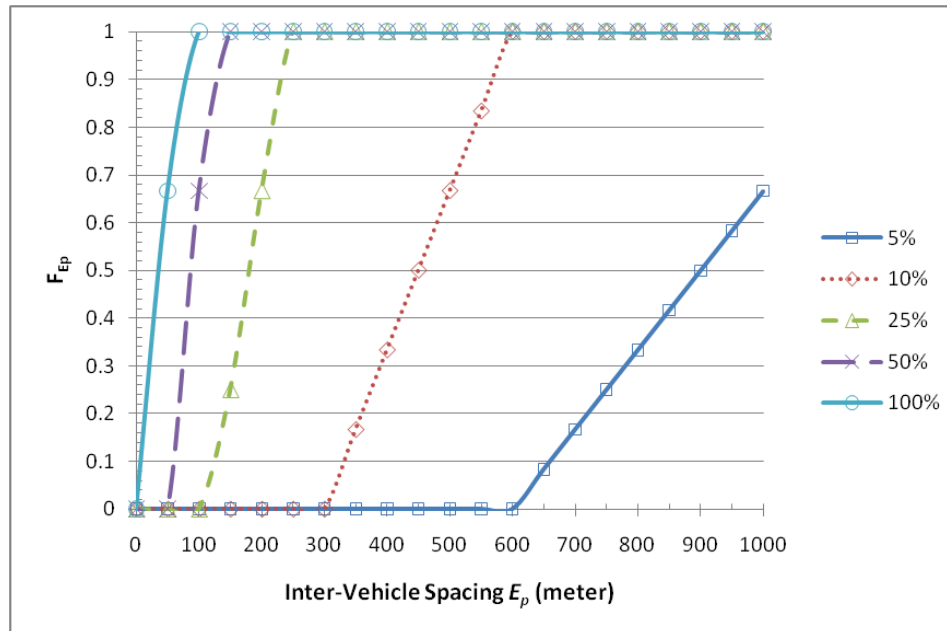
$$f_{reception}(d) \approx (P_{connected})^{n-1}, \quad n = \left\lfloor \frac{d}{E[C]} \right\rfloor \quad (22)$$



Equation 22 implies that it takes on average  $n-1$  hops (last hop directly talks to the TO) to forward a message to the TO. The probability of connectivity at each hop is determined by Equations 13 and 15, which implicitly consider the traffic density (also density of traffic in the opposite direction, if any), traffic speed, market penetration rate, and DSRC communication range  $R_0$ .

**Figures 11, 12, and 13** illustrate the cumulative density function  $F_{E_p}$  in traffic with an average speed  $S$  of 110 km/h (30 m/s, or 65 mph) and for three ranges of flow rate (in veh/h) regardless of number of lanes on the road. The flow rate ranges are as follows:

1. *Medium*,  $1800 \leq v \leq 3600$
2. *Medium-High*,  $3600 \leq v \leq 5400$
3. *High*,  $5400 \leq v \leq 7200$



**Figure 11. cdf of  $E_p$  in different PRs for medium flow rate.**

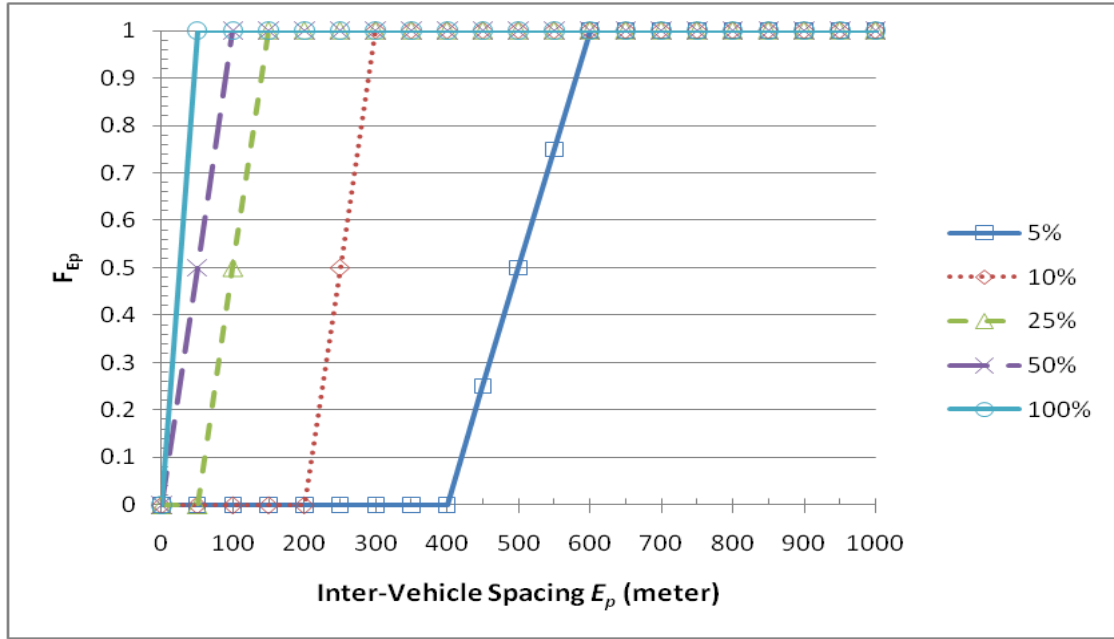


Figure 12. cdf of  $E_p$  in different PRs for medium-high flow rate.

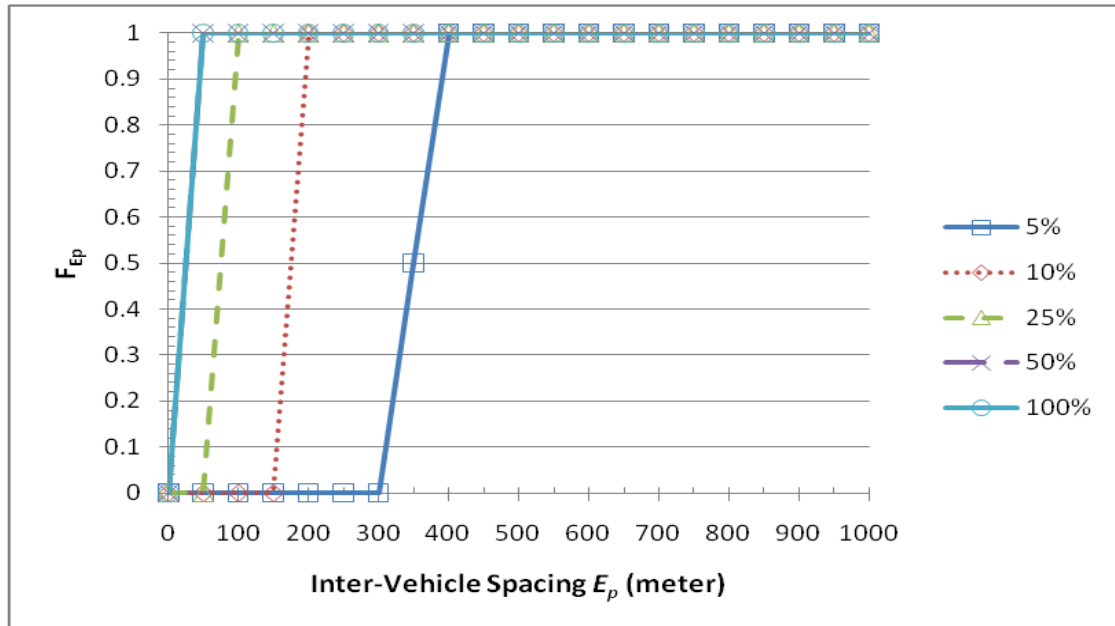


Figure 13. cdf of  $E_p$  in different PRs for high flow rate.

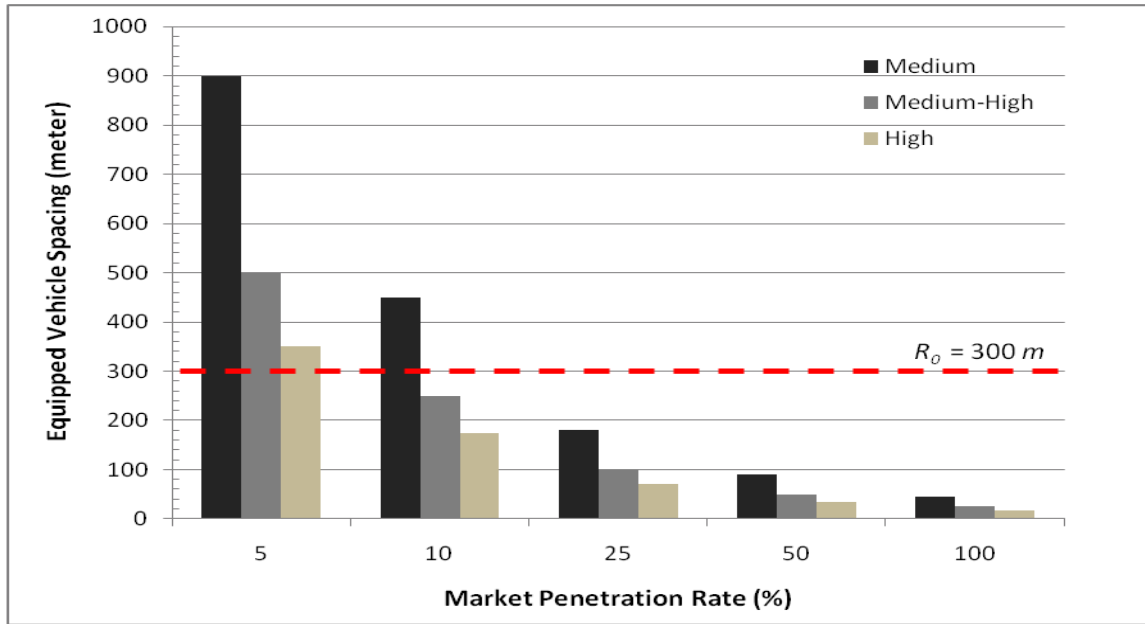


Figure 14.  $E[C]$  for different market penetration rates and traffic flow ranges.

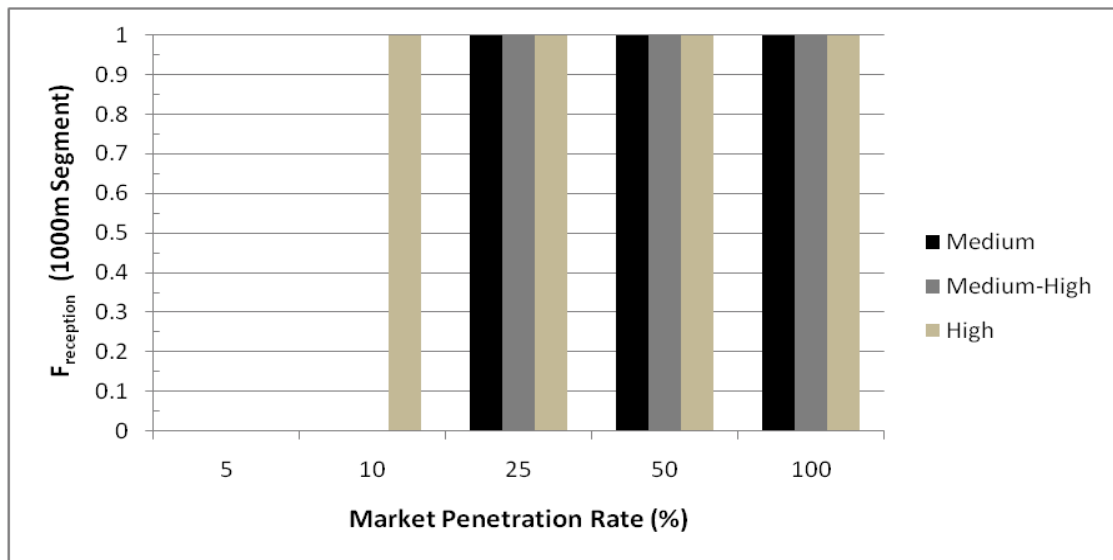


Figure 15. The probability of success in forwarding through a 1000 meter segment with different market penetration rates and traffic flow ranges.

Theoretically, the maximum equipped inter-vehicle spacing  $\lambda_{max}$  will be less than  $R_0$  (300 m) and  $P_{connected}$  will be 1.0 (100%) when the penetration rate  $p$  is above 0.1 (10%) for flow rates  $v_{min} \leq v \leq v_{max}$ .

**Figure 14** shows that the expected inter-vehicle spacing is above  $R_0$  for  $p$  less than or equal to 10% even with a high traffic flow rate. Therefore, the probability that a message is successfully forwarded a distance of 1000 m or longer is zero. These results emphasize the fact that using TOs and message forwarding may only work in a highly equipped system. The farther the distance between the VS and the TO, the higher the traffic density must be and the more vehicles that must be equipped to achieve a high IRR. Therefore, with low PR, either VS should be placed near TOs or methods that avoid forwarding should be used to produce a high MRR. Our analysis does not consider fading effects, but this would only make the conclusions stronger. In many situations, it is not feasible to use a forwarding-only message delivery scheme and other methods should be examined.

### 3.2.3. Deployment Of TOs

The technologies described in Chapter 2 (Sections 2.1 and 2.2) can only detect vehicles at fixed points on the roadway, therefore they are usually deployed according to the topology of freeways, streets, and intersections considering the needs of traffic engineers and the importance of required traffic data. The type of sensor and existing limitations and barriers for each, which impacts the quality of collected data, may play a decisive role. For example, nonintrusive sensors are installed in a way that their view, and hence their data collection ability, is not occluded by other vehicles that are present within the viewing area of the sensor. In rural areas, availability of a power source is a factor in

determining the location of the sensor (if not powered by solar cells). The method of communication with TMCs is also a factor. For example, microwave radar sensors are usually placed in locations with satisfactory cellular signal reception which enables remote communication with the TMCs using a cellular phone mechanism. Video detectors are preferred to be connected to TMCs with fiber optic cables which provides higher bandwidth required for transferring larger data such as image frames.

TOs are nonintrusive units that can communicate with equipped vehicles, although it is possible to integrate and combine these units with the other technologies such as cameras or microwave radar sensors to improve their performance. Overall, TOs can be deployed over/beside any desired point of interest on the road as long as they are supplied with power. TOs have these advantages over previously discussed technologies in terms of deployment:

- TOs are nonintrusive.
- Weather conditions, occlusion, mounting height and location, vehicle mixture, road configuration, etc. do not affect the strategy of their deployments
- TOs are not limited to only fixed points on the road.
- One TO is able to collect data of from a wide area outside its communication range.
- Dynamic definition of virtual strips forms dynamic points and areas of interest in enabled.
- One TO is sufficient for each intersection.

- One TO is sufficient to for several branches on the road (*e.g.*, a TO at highway can detect vehicles from the ramps too).
- TOs can be assembled harmlessly next to existing sensors and detectors
- TOs can be moveable (mobile).
- Centralized multiple TOs (sometimes mixed with other sensors) provide significant flexibility on TOs deployment for wider area coverage
- A change in a point of interest does not require a change in location of TO, but may only require for redefinition of the location of virtual strips.

#### **3.2.3.1. Single TO and Multiple Points of Interest**

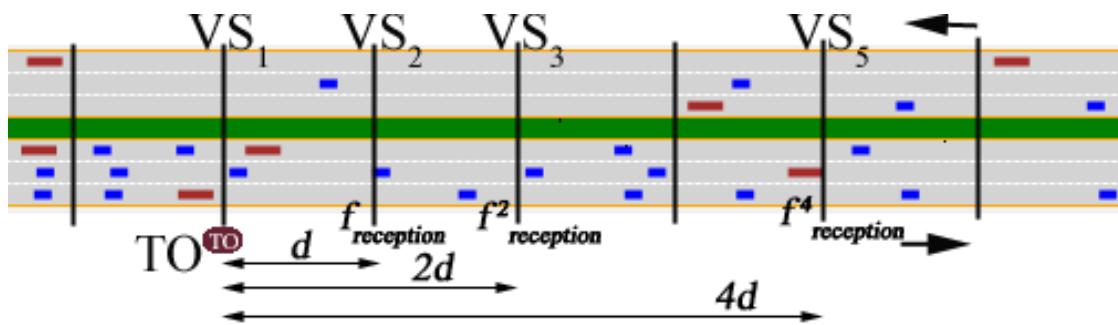
The equations 6 and 22 in Section 3.2.2 imply that DTMon's best performance in monitoring traffic data cannot exceed the information reception rate, which is directly affected by the market penetration rate. This performance is in its extreme when virtual strips are defined nearby the TO where no message forwarding is required and vehicles directly communicate with the TO. Equation 22 in Section 3.2.2 shows that a TO's performance in collecting information from farther strips relies on the spacing of equipped vehicles which must remain below the DSRC communication range (300 m) for a successful delivery. This is true even when the equipped vehicles in opposite direction participate. We explain the methods of message delivery that may improve these situations in Chapter 4.

Doubling the distance of a virtual strip from a TO, will decrease the message reception rate exponentially. Nevertheless, traffic data such as TMS, SMS, and travel time can be estimated with just a few received samples. Also, a TO can dynamically

change the location of virtual strips (included in tasks) to monitor various points of interest on the road. The redefinition of virtual strips can also happen based on a TO's observation of received data. This requires a decision-making program (*e.g.*, incident detection algorithm, real-time and up-to-date monitoring mechanism, etc.). This program can be run on TMC computers which control the TOs or can be directly installed and run on TOs.

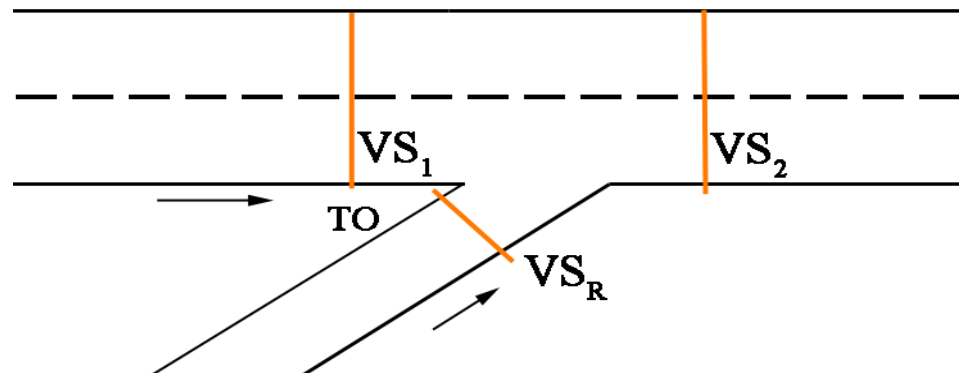
**Figure 16** illustrates a single TO and the message reception rate related to its distance from the virtual strips. The message reception rate for  $VS_1$  is considered to be 1.0 (100%). The average probability of success in receiving a message from distance  $d$  is  $f_{reception}$ . If the traffic condition remains same, the probability of success decreases exponentially and will be  $(f_{reception})^2$  when the distance doubles.

**Figure 17** shows that a single TO is sufficient to cover a major road and its incoming and outgoing branches (*e.g.*, entrance or exit ramps, an intersection, etc.).



**Figure 16. Relationship between distance  $d$  and probability of message reception**

$f_{reception}$



**Figure 17. An ability of a single TO in multi-branch roadways.**

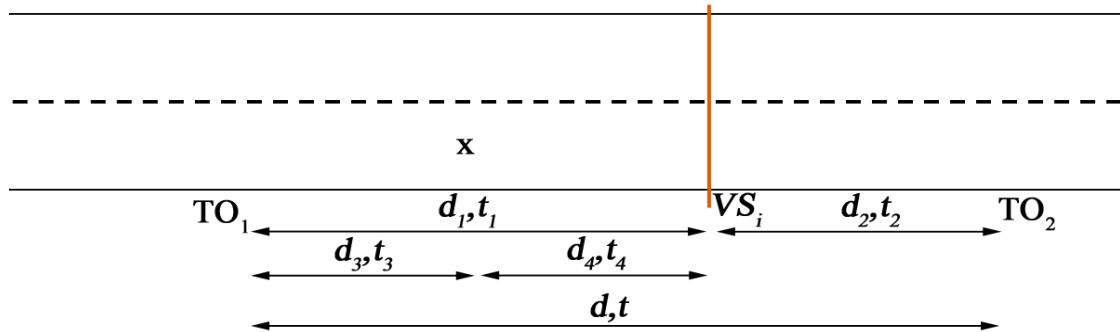
A single TO is able to collect traffic data from locations like  $VS_I$  and  $VS_R$  (e.g., a ramp). The TO is also able to collect traffic data from locations beyond the merging point, like  $VS_2$ . The traffic data collected by the TO can be used to determine differences in flow rate, TMS, SMS, and sometimes congestion. The TO can define additional virtual strips for a more complex roadway. The TO will not define multiple VS in a short distance as it may generate excess network overhead and the traffic data collected from very small segments is not typically useful.

### 3.2.3.2. Multiple TOs

The coverage of a single TO is limited. Farther locations (defined virtual strips) may not be accessible by a single TO, and message delivery can encounter disconnectivity in the network. The need for multiple TOs in an area is inevitable. Tasks may include multiple destinations (location of TOs). This adds the ability for a message to be stored and dropped off at the next available TO during the travel, therefore few messages may be lost due to unsuccessful forwarding. Also, having a TO in front adds the possibility of forward forwarding (messages can travel forward not backward). In addition to covering



a wider area, the use of multiple TOs can improve the message delivery mechanism. Latency may be reduced in some occasions. Furthermore, generated reports and warnings by TOs are spread faster and more reliably in the region. In addition, vehicles involved in transferring the traffic data can share their experience of past points on the road (virtual strips). Centralized TOs, assumed to be all connected to TMCs or each other, can provide the union of information about the region. TOs can also be mobile and are not necessarily statically placed. TOs can be moved by traffic managers to different parts of the roadway system as needed. The TOs should remain stationary long enough to collect at least an aggregation period's worth of messages from assigned tasks. As long as it can maintain a network connection to the TMC, in practice, the TO could be placed in a special vehicle and driven to needed locations.



**Figure 18. Two TOs, desired virtual strip, and actual travel times.**

**Figure 18** shows two TOs, their distance from each other and from the desired strip  $VS_i$  in addition to the travel time for each segment. If only one TO,  $TO_1$ , is used to collect

data from vehicles passing  $VS_i$  located in distance  $d_I$ , then messages can only be forwarded back to  $TO_I$ .

*Message delay* is the delay from when a message was generated to when the TO received it. In fact, *latency* in traffic data is the encountered message delay. The TO can calculate the average delay from when a message was generated to when the TO received it using the message timestamp in addition to the time spent to aggregate the traffic data. The metrics can be computed simultaneously by the TO or in the TMC, therefore the latency is close to the message delay. We will show in Chapter 4 that average delay in receiving the messages is a small number of milliseconds [31, 32, 33]. In a bi-directional roadway, messages can also be stored and carried to  $TO_I$  by vehicles in the opposite direction [31, 32, 33]. The average delay (latency) will be half the average travel time in segment  $TO_I VS_i$  in the opposite direction.

If a message cannot be forwarded farther back from some point (marked x in **Figure 18**), it will encounter unsuccessful message delivery. By adding an additional TO,  $TO_2$ , on the way, this message can be forwarded ahead toward  $TO_2$  or stored and carried to  $TO_2$ . The latency of a message carried from x to  $TO_2$  is  $(t_2+t_4)$  which is the average travel time from x from  $TO_2$ . There is no need to forward the messages back to  $TO_I$  if  $VS_i$  is nearby  $TO_2$ . A message can be forwarded to  $TO_2$  with little delay (almost milliseconds) or be stored and carried to  $TO_2$  with average delay  $t_2$ , which is the travel time from  $VS_i$  to  $TO_2$ .

In a case where the union of  $TO_1$  and  $TO_2$  is used and messages can be forwarded in any direction or stored and carried to available TOs with a mechanism to avoiding duplicate messages, the average delay (latency) is as follows:

$$\overline{delay} = \frac{(n_f \bar{t}_f + n_c \bar{t}_c)}{n} \quad (23)$$

$$\overline{delay} = w_f \bar{t}_f + w_c \bar{t}_c \quad (24)$$

where

$n_f$  = total number of distinct received forwarded messages received by forwarding

$n_c$  = total number of distinct received carried messages

$n$  = total number of distinct received messages

$t_f$  = forwarding delay  $\approx 0.0$

$t_c$  = carrying delay  $\approx$  average travel time

$w_f = n_f/n$

$w_c = n_c/n$

The average delay of a message generated in segment  $TO_1TO_2$  is  $(t/2)$  which is the average travel time from any point inside segment  $TO_1TO_2$ . The average delay for messages related to  $VS_i$  will be  $w_f t_1 + w_c t_2$ .

Note that, the average travel time at each interval highly depends on the actual traffic conditions on the road. For example, nonrecurring congestion happening between  $VS_i$  and  $TO_2$  causes an increase in  $t_2$ . Therefore all messages behind the congestion that are stored to be carried will be delivered with latency higher than expected. In addition, if the message reception rate is high (*i.e.*, density of equipped vehicles is high) most of messages are forwarded, and  $w_c$  will be small and latency tends toward zero. Otherwise,

most messages are destined to be dropped or be carried, therefore  $w_f$  will be small and the latency tends toward the average travel time.

### 3.3. SUMMARY

In this chapter, we explained the main components of DTMon such as *task organizers*, *equipped vehicles*, and *virtual strips*. We described methods in DTMon to monitor traffic data such flow rate, volume, density, speed, and travel time in rural roads (*e.g.*, highways) and in urban roads (*e.g.*, streets and intersections). We analyzed and showed the impact of market penetration rate and traffic density on message reception and message delay in DTMon. We demonstrated that there is a need for use of various methods of message delivery to achieve higher information reception rates. We explained the deployment strategies of TOs, such as the use of multiple TOs.

## CHAPTER 4

### EVALUATION

The theoretical analysis in Chapter 3 (Section 3.2) showed that the reception rate in receiving data for virtual strips farther from TOs is low when the inter-vehicle spacing of equipped vehicles is high. This can be caused by a low traffic flow rate, a low penetration rate, or both with various traffic speeds. Nevertheless, in this chapter we evaluate DTMon and show that the quality of estimated metrics such as time mean speed (TMS), space mean speed (SMS), and travel time will not be affected even with low information reception rates in free flow (*i.e.* non-congested) traffic. Furthermore, we show that it is possible to improve the reception rate from farther virtual strips in such situations. We evaluate DTMon when it uses traffic in the opposite direction, dynamic transmission range, or a store-and-carry technique, in which the messages will be delivered to the next possible TO on the way.

In addition, we evaluate the performance of DTMon to measure travel times and speeds in transient flow traffic caused by non-recurring congestion. We show DTMon's ability to gather high-quality real-time traffic data such as travel time and speed. These metrics can be used to detect transitions in traffic flow (*e.g.*, caused by congestion) especially where accurate flow rate information is not available. We evaluate the accuracy and latency of DTMon in providing traffic measurements using different methods of message delivery. We show the advantages of using dynamically-defined measurement points for monitoring transient flow traffic. We compare DTMon with

currently in-use probe-based systems (*e.g.*, AVL) and fixed-point sensors and detectors (*e.g.*, ILD).

#### 4.1. MESSAGE DELIVERY METHODS

We will compare the performance of the following message delivery methods in improving message and information reception rates as well as the quality of reported traffic data:

- **Regular Forwarding (RF):** A vehicle passing a virtual strip will forward the message to the closest possible TO from the list of TOs defined in the task (see Chapter 3 Section 3.5).
- **Dynamic Transmission Range (DTR):** A vehicle will use RF initially with the standard DSRC range of 300 *m*. If the message cannot be forwarded (*i.e.*, there is no vehicle within 300 *m*), then the vehicle will increase its transmission range to 600 *m*. If the vehicle is still not able to find a neighbor, it will increase its transmission range to 1000 *m*. Note that IEEE 802.11p [6, 7, 8] allows for transmission power settings that can result in a range of 1000 *m* in certain instances.
- **Store-and-Carry (SAC):** A vehicle will store the message and physically carry it to the next TO.
- **RF+SAC:** A vehicle will forward the message to the closest TO using RF and will also store and carry the message to the next TO in order to ensure reception by a TO. Duplicate reports are detected by the central server using the message generation time and location.

- **DTR+SAC:** A vehicle will forward the message to the closet TO using DTR and also store and carry the same message to the next TO.

In bi-directional roadways, vehicles traveling in the opposite direction can participate in forwarding or carrying messages, which may further improve the performance of these methods.

## 4.2. FREE FLOW TRAFFIC

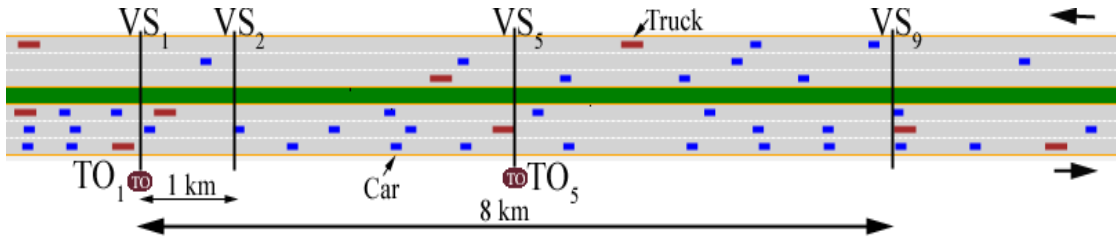
We will evaluate and compare the message delivery methods described in Section 4.1, considering message and information reception rates, message delay, and quality of traffic data in free flow traffic.

### 4.2.1. Methodology

We perform several experiments using VANET modules that we developed [34] for the *ns-3* simulator [49]. Our VANET simulator and these modules are described in Appendix A. The goal is to compare the message reception rates and message delays of the various delivery methods described Section 4.1 under various market penetration rates.

We focus on the highway scenario to highlight situations with potentially poor connectivity. We used a six-lane bi-directional highway with two TOs and four VS as shown in **Figure 19**. The subscripts on the TO and VS labels indicate their distance in *km* from the highway entrance. Vehicles enter the highway with a medium flow rate (average of 1800 *veh/h*) and a desired speed between  $30 \pm 5$  *m/s* ( $110 \pm 18$  *km/h*). Recall, we have defined the possible ranges of the flow rates in our analysis in Chapter 3 (Section 3.2.2). We performed 10 30-minute simulation runs for each message delivery method and present the average over all runs.

In each experiment,  $TO_1$  tasked passing vehicles with reporting when they passed  $VS_2$ ,  $VS_5$ , and  $VS_9$ . Depending on the message delivery method, these reports were delivered back to  $TO_1$  or were delivered to  $TO_5$ . Our simulations use the log-distance signal fading model.



**Figure 19. A six-lane bi-directional highway with two TOs and four VS.**

#### 4.2.2. Evaluation

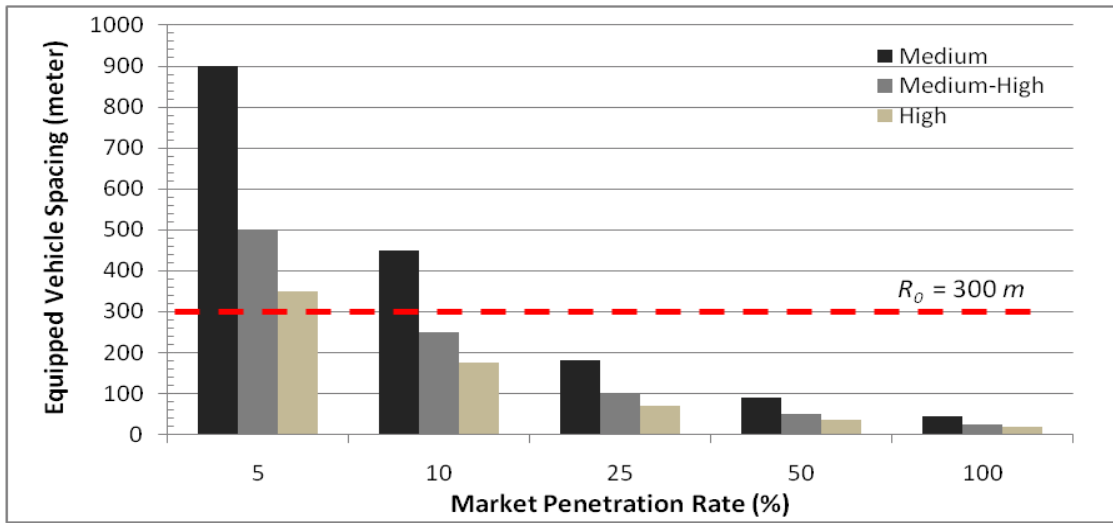
Using SAC will increase the reception rate (Section 4.2.2.1), but comes with the tradeoff of increasing the message delay (Section 4.2.2.2). We note that a 100% reception rate does not mean that all of the possible information is collected as only equipped vehicles can report to the TO. We consider the question of traffic data quality and the information reception rate in Section 4.2.2.3.

##### 4.2.2.1. Message Reception Rate

**Figure 20** is a repeat of **Figure 14** from Chapter 3 (Section 3.2.2). It shows the expected inter-vehicle spacing given different penetration rates and traffic flow rates. The results shown in **Figure 20** indicate that with a medium flow rate and a 300 m communication range, market penetration rates (PR) of 10% and below will result in total disconnectivity

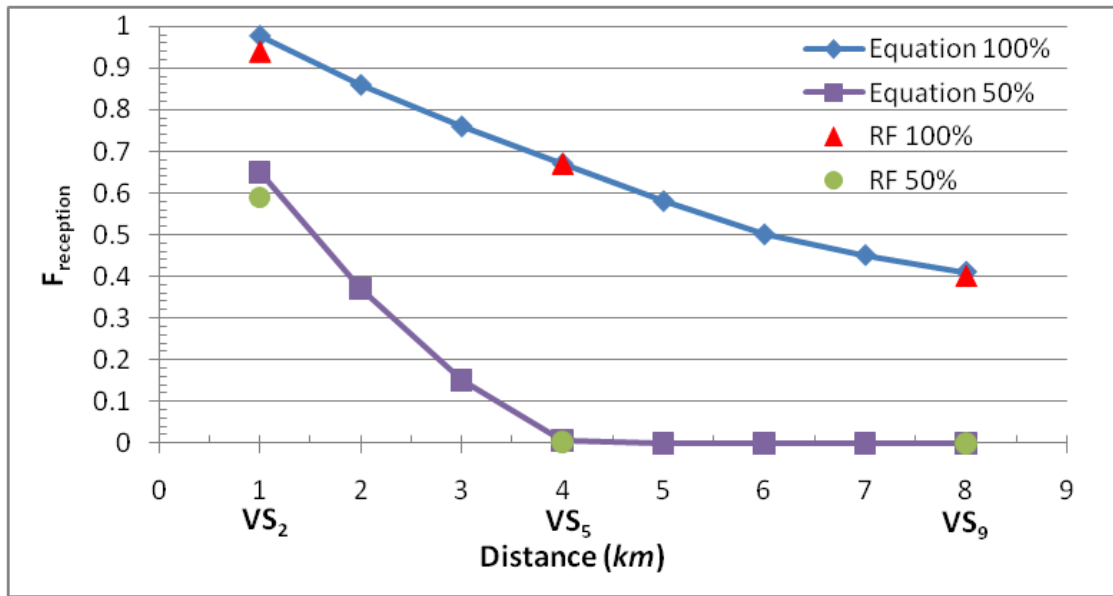


as the average equipped inter-vehicle spacing is higher than the communication range. Thus, with a PR of 5-10%, no messages will be received by  $TO_1$  from  $VS_2$  or farther strips when regular forwarding (RF) is used. With a PR of 5%, the situation is the same for dynamic transmission range (DTR). Our simulations use the log-distance signal fading model, so even though **Figure 20** indicates that the average inter-vehicle spacing is less than the maximum range of 1000 m, the signal is not actually able to propagate that far. Since there is total disconnectivity, the SAC methods, including RF+SAC and DTR+SAC, will have equal performance, as all messages will be carried to  $TO_5$ . This will increase the message reception rate (MRR) from 0% to 100% and the information reception rate (IRR) will be equal to the PR.



**Figure 20.** Expected inter-vehicle spacing for different market penetration rates with medium (1800 veh/h) and higher flow rates (3600 and 5400 veh/h) (Reprint of Figure 14).

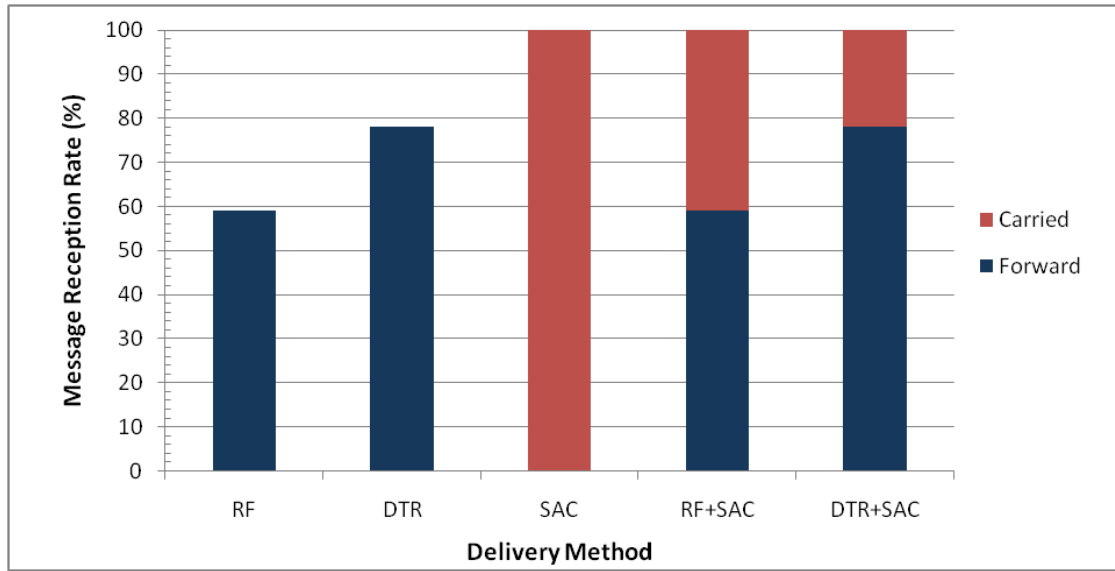
**Figure 21** shows  $F_{reception}$  for  $TO_1$  of messages sent by vehicles passing  $VS_1$ ,  $VS_2$ , and  $VS_4$  calculated by Equation 22 in Chapter 3 (Section 3.2.2) and the probability of reception by  $TO_1$  obtained by simulation using RF with PRs of 50% and 100%. The simulation produced an average speed of 27.45 m/s, so that was the speed used for  $S$  in Equation 9 (Chapter 3 Section 3.2.2). **Figure 21** shows that the analysis is confirmed by the simulation results. The MRR drops when the VSs are farther from  $TO_1$ . Even with 100% PR,  $TO_1$  will miss 10% of the messages from  $VS_2$  (1 km away) and about 30% of messages from  $VS_5$  (4 km away).



**Figure 21.**  $F_{reception}$  based on the distance from the VS to  $TO_1$ .

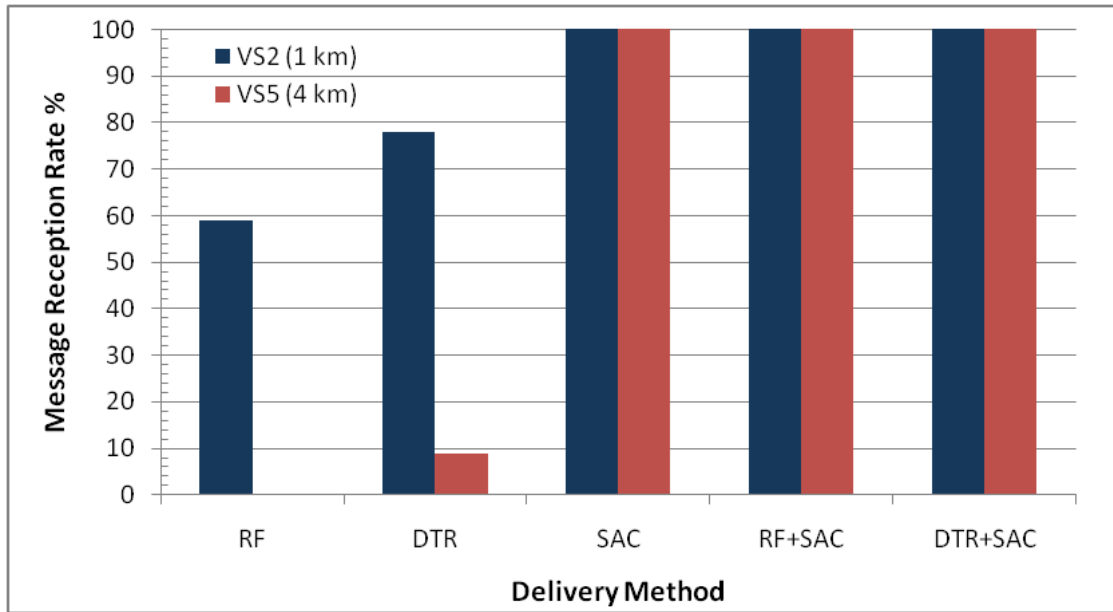
**Figure 22** shows the MRR for messages from  $VS_2$  using different delivery methods with 50% PR. The figure also indicates what portion of the MRR is due to forwarding to  $TO_1$  or carrying to  $TO_5$ . RF results in over 50% MRR, and DTR improves that to over

75%. But, the addition of SAC (either alone or in combination with RF or DTR) results in 100% MRR. Any messages that cannot be forwarded back to  $TO_1$  are able to be carried and delivered successfully to  $TO_5$ .



**Figure 22. MRR from  $VS_2$  with 50% PR.**

**Figure 23** shows how the MRR is affected by the distance of the report origination from  $TO_1$  with 50% PR. Note that because of carrying, the distance does not affect the delivery results when SAC is used. For both RF and DTR, the MRR drops dramatically when the distance from  $TO_1$  is increased from 1 km ( $VS_2$ ) to 4 km ( $VS_5$ ). This is due to periods of disconnectivity in the traffic when the message cannot be forwarded back to  $TO_1$ . Thus, if the VS is far from the originating TO, SAC methods should be used to achieve high MRR.



**Figure 23. MRR from VS<sub>2</sub> (1 km away) and VS<sub>5</sub> (4 km away) with 50% PR.**

**Table 1. MRR from VS<sub>2</sub> using traffic in opposite direction.**

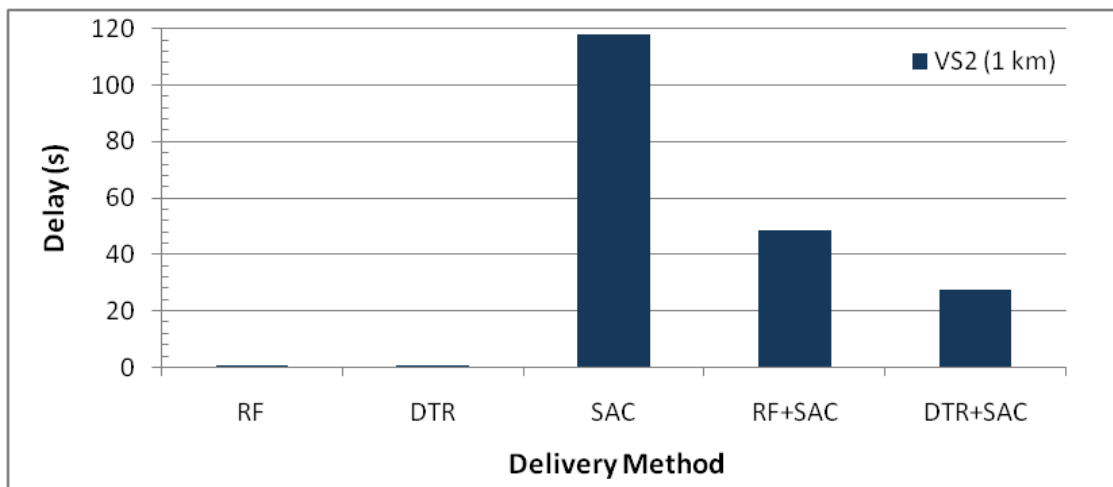
Penetration Rate	RF, without opp dir.	RF, with opp dir.	DTR, without opp dir.	DTR, with opp dir.
5%	0%	0%	1.1%	2.4%
50%	59%	72%	78%	96.7%

Using vehicles traveling in the opposite direction on bi-directional roadways can decrease the equipped inter-vehicle spacing for message delivery and therefore improve the MRR. **Table 1** shows the MRR when traffic in the opposite direction (with the same medium flow rate) is also used for forwarding. With a 50% PR, using opposite direction traffic with either RF or DTR improves the MRR from VS<sub>2</sub> by 20-25%. For DTR, this results in a total MRR of close to 100%. Unfortunately, since the opposite direction

traffic has the same flow rate as the forward direction, there is still much disconnectivity with a 5% PR and thus, little or no improvement in the MRR.

#### 4.2.2.2. Message Delay

Message delay is the time from a message being generated by a vehicle until the message is received at a TO (either  $TO_1$  or  $TO_5$ ). **Figures 22 and 23** showed that SAC greatly improves the MRR, but there is a tradeoff in increased delay, which may impact its usefulness in obtaining real-time traffic statistics.



**Figure 24.** Average message delay from  $VS_2$  with different delivery methods.

**Figure 24** shows the average message delay from  $VS_2$  to a TO (either  $TO_1$  or  $TO_5$ ) for the different delivery methods. When delivery is successful, RF and DTR have very low delay, on the order of milliseconds. SAC has the highest delay because of the travel time that vehicles encounter while driving towards  $TO_5$ . Using SAC combined with message forwarding will reduce this delay. For example, where the average delay for

SAC alone is 118 s, the average delay for RF+SAC is about 50 s. This is because 59% of the messages with RF+SAC are able to be forwarded to  $TO_1$  using RF, which has a delay of only 7 ms. The remaining 41% were carried to  $TO_5$  with an average delay of 118 s, which is the average travel time for a 3 km segment. The distance between the TOs and the distance from the VS to the closest TO has a direct impact on message delay using SAC. Using traffic in the opposite direction may reduce the overall message delay by allowing more forwarding to take place. In **Figure 24** we only show messages originating at  $VS_2$  because  $VS_5$  is co-located with  $TO_5$  and thus, would have no delay regardless of the forwarding method. Even if there were only one TO (*e.g.*,  $TO_1$ ), a successful delivery using RF or DTR would have a delay on the order of milliseconds.

#### 4.2.2.3. Quality of Traffic Data

Traffic management centers (TMCs) are interested in gathering travel times and traffic speeds over certain sections of the highway. There are two types of speed that TMCs consider, *time mean speed* (TMS) and *space mean speed* (SMS). TMS is the average speed of vehicles passing a *point* on a roadway, and SMS is the average speed of vehicles based on the average travel time of vehicles traversing a *segment* of roadway. DTMon can only count equipped vehicles, therefore traffic flow rate and density estimates are precise only with a high PR. But, DTMon can provide high quality estimates of travel time and traffic speed with just a few received messages, thus it can be used even in low PR situations.

**Table 2** shows the results of a t-test ( $\alpha = 0.05$ ) between actual traffic data (*i.e.*, ground truth from the simulation) and the traffic data collected by the TOs (using RF+SAC) from VS<sub>2</sub> with 5% PR. The estimates we consider here are TMS in *m/s*, travel time (TT) in *s*, and SMS in *m/s*. The results show that DTMon can be used to estimate TT and SMS with 95% confidence for PRs as low as 5%. Note that with the 100% MRR provided by RF+SAC and 5% PR, the information reception rate (IRR) is only 5%. So, these results show that TT and SMS can be accurately estimated with as little as 5% of the traffic reporting. It does not matter whether this is due to low PR or low MRR. We note that there is a significant difference between the actual TMS and the estimated TMS at 5% PR. This is because there are so few samples (*i.e.*, equipped vehicles) at the particular point used for computing the TMS. With an increase in the PR to 50%, the TMS is more accurate.

**Table 2. T-Test 5% PR ( $\alpha = 0.05$ ).**

	Actual		RF+SAC		t-Stat	p-Value	Sig.?
	Mean	Var	Mean	Var			
TMS	27.26	0.13	26.65	1.21	-9.7002	0.0006	Yes
TT	39.88	0.19	39.38	12.97	0.3165	0.7673	No
SMS	25.07	0.40	25.39	2.0	-0.3117	0.8076	No

**Table 3** shows the results of a t-test between RF+SAC and actual data with 50% PR. Here, there is no significant difference between the actual data and that collected by the TOs.

**Table 3. T-test 50% PR ( $\alpha = 0.05$ ).**

	Actual		RF+SAC		t-Stat	p-Value	Sig.?
	<i>Mean</i>	<i>Var</i>	<i>Mean</i>	<i>Var</i>			
TMS	26.34	0.23	27.08	0.67	1.0689	0.2005	No
TT	40.62	0.28	38.84	0.05	0.4025	0.5127	No
SMS	24.62	0.03	25.74	0.01	2.2064	0.3911	No

**Table 4. T-test 50% PR ( $\alpha = 0.05$ ).**

	RF		RF+SAC		t-Stat	p-Value	Sig.?
	<i>Mean</i>	<i>Var</i>	<i>Mean</i>	<i>Var</i>			
TMS	27.45	0.41	27.08	0.67	2.9473	0.2082	No
TT	39.17	1.35	38.84	0.05	0.5075	0.7010	No
SMS	25.52	2.01	25.74	0.01	0.0521	0.9668	No

Since the forwarding method can affect the number of samples received by the TOs, we look at how well the data obtained using RF compared to RF+SAC with 50% PR. We show the results of the t-test in **Table 4**. There is no significant difference between the data collected with RF+SAC and with RF. So with a higher PR, TMCs could use forwarding-only techniques to lower the message delay, while still receiving high quality estimates.

#### 4.3. TRAFFIC WITH TRANSIENT FLOW

Our goal in this section is to evaluate the performance of the DTMon to measure travel times and speeds in transient flow traffic caused by non-recurring congestion. Because the coverage area of a single TO is limited and transient traffic conditions can cause periods of network disconnectivity, there is a need for multiple TOs in areas that are



prone to experiencing non-recurring congestion events. When multiple TOs are used, message delay and latency may affect the quality of data that can be accurately gathered. In addition, it is important to carefully assign measurement points in transient traffic as the conditions can change quickly. We show how DTMon can allow for the dynamic creation of new measurement points (VS) and the movement of existing VS.

#### **4.3.1. Congestion**

There are two types of traffic congestion. *Recurring congestion* occurs regularly at the same locations on the roadway system at about the same time every day. *Non-recurring congestion* occurs when a crash or other unusual event (such as a construction zone, merging traffic, or severe weather) causes a reduction in the traffic-carrying capacity. Detecting recurring congestion is not a critical monitoring process and requires a long-term overview (*i.e.*, weekly, monthly, or annual) of a large archive of collected traffic data, estimated metrics, and structural features of the roads (*e.g.*, capacity) in addition to extra data-mining to detect the recurrence of similar patterns, all of which can be done offline. Detecting and monitoring non-recurring congestion, on the other hand, is an important function of TMCs. Extended periods of non-recurring congestion may require immediate action, including notifying emergency personnel, highway patrols, or even changing traffic light timings on surface streets to handle the extra capacity from diverted highway vehicles.

#### **4.3.2. Latency**

In DTMon, tasks can include the locations of multiple TOs. This adds the ability for messages to be stored and be dropped off at the next available TO during the travel (using

RF+SAC). This will reduce the number of messages lost due to unsuccessful forwarding. In addition to covering a wider area, the use of multiple TOs can improve the message delivery mechanism. The delay between a vehicle passing a VS and delivering the report to a TO may be reduced in some instances. Furthermore, generated reports and traffic warnings by TOs can be spread faster and more reliably in the region.

As described in Sections 3.2.3.2 and 4.2.2.2, *message delay* is the delay from when a message was generated to when it was received by a TO. Since vehicles timestamp all reports, the receiving TO can calculate the message delay for each message. When messages are delivered to multiple TOs in a region, there is additional processing delay associated with sending the message to the central TMC and having that data aggregated with previously received data. We use the term *latency* for the total time from when a message was generated to when it is ready to be used in analysis. In a single-TO system, the latency is the same as the message delay. TMCs often aggregate data and use specific averaging intervals (*e.g.*, 1 *min*, 5 *min*) when reporting traffic conditions. Latency is important if it rises to higher than the averaging interval. In this case, some late-arriving data may not be included in the estimate.

#### **4.3.3. Dynamically Defined Virtual Strips**

The location of VS can be re-defined and modified by a TO at any time. A TO can divide the roadway into several virtual segments. The size of the segments can vary by relocating the VS. Also, additional VS can be defined inside any desired virtual segment. This allows the TO receive more traffic information by defining additional points of interest on the road without the cost of using additional TOs. Currently, achieving this

would require the installment of numerous sensors and detectors along the road. In AVL and WLT systems, this would require a significant increase in the sampling rate to maintain the desired level of accuracy [77].

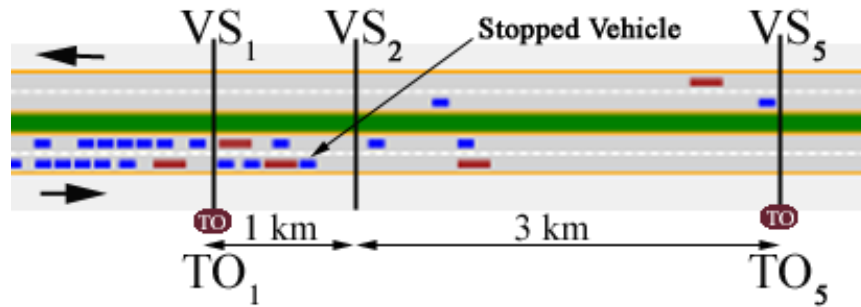
Congestion can produce a transition in traffic flow which cannot be detected by probe-based systems when the PR is not high. We show that the transition in the flow rate can produce a transition in travel time and SMS, which can be detected by DTMon. The travel time algorithms proposed by Sethi *et al.* [78] and Sermons [79] utilize both the travel time and average speed measures of the congested segment and the adjacent segment. DTMon using dynamically defined strips/segments can satisfy this data requirement. Two variables, travel time and speed, are compared to historical averages for each segment to infer if congestion occurs on this segment. It was found by Sethi that traffic measures for the congested segment were most useful for detecting congestion located in the downstream portion of the segment, while traffic measures for the next upstream segment worked well for detecting congestion occurring in the upstream or middle portion of the segment. The segment in the upstream portion that shows no significant change in traffic measures is inferred to be the end-of-the-queue.

Identifying the end-of-the-queue is important for traffic engineers as this is the place where most secondary incidents occur. The primary incident is the cause of the congestion, but secondary incidents tend to occur at the point where the speed transition is the sharpest. Traffic engineers would like to be able to identify these locations so that approaching drivers can be warned in advance, potentially avoiding the secondary incidents. We explain how DTMon can be used to determine the end of the queue in

congested traffic in Section 4.3.5.5. We show that transitions in travel times and speeds detected by DTMon are two major factors in inferring the possibility of the congestion on the roadway.

#### 4.3.4. Methodology

We again use our VANET modules [34] to examine DTMon's ability to collect high-quality traffic data, especially travel times and SMS, in transient flow traffic (*i.e.*, non-recurring congestion).



**Figure 25.** Location of TOs and virtual strips (not to scale).

As shown in **Figure 25**,  $TO_1$  is located 1 km away from the entrance of a bi-directional four-lane highway.  $TO_5$  is located 5 km away from the beginning of the highway as an optional secondary TO for the cases when RF+SAC is used. The vehicles enter the highway with a medium flow rate (average 1800 veh/h) to simulate slightly sparse traffic [31, 33] where the possibility of message reception by TOs is much lower than in dense traffic. Trucks, which keep a larger inter-vehicle spacing and have slower

acceleration and deceleration than cars, comprise 20% of the vehicles. The desired speed of all vehicles is  $65 \pm 5$  *mph* ( $29 \pm 2.2$  *m/s*).

We induce congestion by stopping a vehicle in the first lane, 1.5 *km* away from the entrance, between  $VS_1$  and  $VS_2$ , 5 minutes after the simulation has started. The stopped vehicle is outside the communication range (300 *m*) of  $TO_1$  and is relatively far from  $TO_5$ . The vehicle will remain stopped for 5 minutes, then the vehicle will start moving, allowing traffic flow to gradually return to normal. During the stopped phase, following vehicles will slow and try to change lanes around the stopped vehicle. This causes traffic congestion in both lanes. We have observed through experimentation that 5 minutes is long enough to create a transient flow. Note that congestion could have occurred at any point on the road, and virtual strips can be defined dynamically by TOs. We examine advantage of dynamically defined virtual strips in Section 4.3.5.5. At this point, we named only those VS that surround the starting point of the congestion.

The performance of DTMon in monitoring traffic data is compared with the actual simulation status (ground truth) at  $VS_2$  (2 *km* away from the highway entrance and 0.5 *km* from the stopped vehicle). To compare our system with AVL, we assume that some percentage of the trucks are able to periodically communicate directly to an operating center to report their current status. In this comparison, we assume that AVL is used only by trucks, which we note have slower speeds, acceleration, deceleration than cars. For comparison with fixed point sensors and detectors (*e.g.*, ILDs or microwave radar sensors), we use actual simulation data sampled from  $VS_1$  and  $VS_2$ . We execute 10 runs of the simulation (20 *min* each) for each experiment. We test PRs of 5, 10, 25, 50, and

100%. Travel times, speeds, delays, and message reception rate are measured by TOs. We compare the results for each scenario using t-test with  $\alpha$  0.05 and 95% confidence.

#### 4.3.5. Evaluation

There are several factors that can affect the quality of data gathered by DTMon. The first factor is the percentage of equipped vehicles, or the market penetration rate (PR). Second, we are concerned with the amount of information (*i.e.*, location, speed, time) that is received from vehicles. Third, we must consider how quickly and in what manner the information is received. Methods that can collect more information from vehicles (higher MRR and IRR) with less latency are preferred in up-to-date traffic monitoring.

##### 4.3.5.1. MRR and IRR using RF or RF+SAC

For strips within transmission range of  $TO_1$  (*e.g.*,  $VS_1$ ), the message reception rate (MRR) is 100% and the information reception rate (IRR) equals the market penetration rate (PR). For farther strips where message delivery is required, the MRR using regular forwarding (RF) varies according to the distance from the TO and the PR. When RF+SAC is used, only PR affects the MRR.

**Table 5** shows the MRR for RF, RF using traffic in the opposite direction, and RF+SAC for different penetration rates. With low PR, no messages were able to be forwarded using RF. Even using traffic in the opposite direction does not improve the MRR. In 25% PR,  $TO_1$  receives only 5.71% of messages (only 1.43% of total information) from  $VS_2$ . Only RF+SAC was able to deliver messages with full MRR. In medium PR (*e.g.*, around 50%), RF can deliver almost half of the messages although the IRR still remains below 30%. Using traffic in opposite direction only improved MRR by

7%. In 100% PR,  $TO_1$  misses about 20% of messages when RF alone is used (due to medium traffic flow rate and high inter-vehicle spacing). Using traffic in the opposite direction for delivery improves the MRR by 15%.

**Table 5. MRR for different penetration rates.**

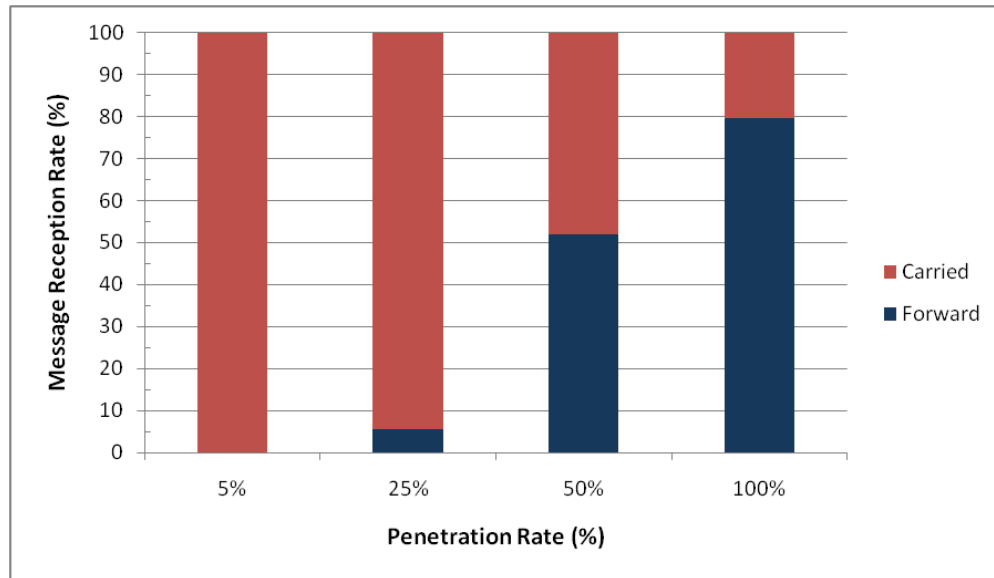
PR	Actual	RF	RF+w/opp	RF+SAC
5%	100	0.00	0.00	100
25%	100	5.71	5.71	100
50%	100	52.03	56.20	100
100%	100	79.50	91.01	100

**Table 6** shows the corresponding IRR. Note that, having low IRR in low penetration rates means that the system cannot estimate traffic flow rate or density. For example, DTMon in 25% PR can only count 1.43% (25% with SAC) of vehicles in the traffic, thus the flow rates estimated by DTMon will be 0.014 (0.25 with SAC) of the actual flow rate on the road. This is a limitation of any system that does not directly count the physical presence of vehicles.

**Table 6. IRR for different penetration rates.**

PR	Actual	RF	RF+w/opp	RF+SAC
5%	100	0.00	0.00	5
25%	100	1.43	1.43	25
50%	100	26.01	28.10	50
100%	100	79.50	91.01	100

**Figure 26** shows the percentage of messages that were forwarded or carried using RF+SAC in different penetration rates. Even in 100% PR, 20% of messages were carried. **Figure 26** also shows that with 100% PR using RF, in which no messages are carried, DTMon will count 20% fewer vehicles than actually exist. This is because inter-vehicle spacing can still be above the DSRC transmission range (300 *m*) in medium traffic flow. This is especially true for sections after a point of congestion but before VS<sub>2</sub> where the traffic flow rate decreases and the gap among vehicles increases.



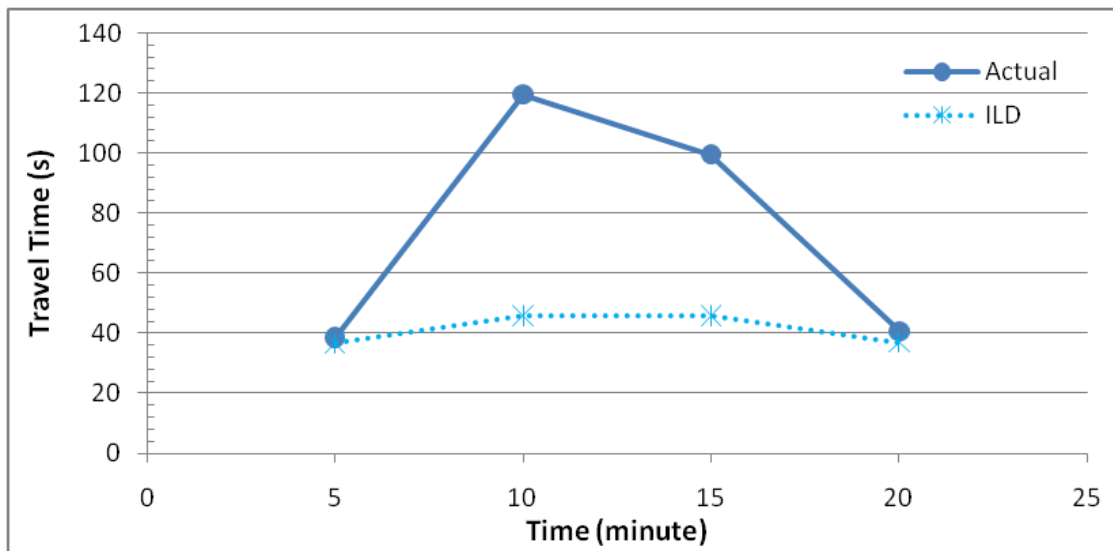
**Figure 26. MRR from VS<sub>2</sub> using RF+SAC in different PRs.**

#### 4.3.5.2. Travel Time and Space Mean Speed

First, we show that ILDs cannot accurately estimate travel times during transient traffic flow. Next, we investigate how well DTMon is able to measure travel times and SMS compared to current technologies.



**Figure 27** shows the travel time in seconds, averaged every 5 minutes, for ILDs along with the actual (ground truth) travel times. Recall that congestion starts from beginning of the second interval ( $t = 5$  minutes), reaches its peak at the end of this interval ( $t = 10$  minutes), and starts to release during the third interval ( $t = 10$ -15 minutes). Since the ILDs estimate the travel time as the segment size divided by the averaged traffic spot speed, they cannot accurately follow the transient nature of the non-recurring congestion event.



**Figure 27. Estimated travel time by ILDs compared to actual (aggregation every 5 minutes).**

Probe-based monitoring can perform better than ILDs for measuring travel times and SMS, so we compare the estimated travel time and SMS during each interval (1 *min* or 5 *min*) among data collected using RF, RF+SAC, and with AVL. RF<sub>p</sub> is RF with a PR of  $p$ .

In AVL where only trucks are equipped,  $AVL_p$  is AVL with  $p$  percentage of trucks equipped. For example,  $AVL_{25}$  means 25% of the trucks in traffic are equipped and use AVL.  $AVL_{25}$  can be compared with  $RF_5$  in DTMon since 5% of the total population of vehicles are equipped and 20% of all vehicles are trucks.

**Tables 7 and 8** show the results of a t-test with  $\alpha = 0.05$  (95% confidence) for travel time (in *seconds*) in four 5-minute intervals, as well as the average travel time and SMS (in *m/s*) for the entire 20 minutes with 5% PR. The *t-Stat* column is the result of the t-test, showing if the mean of the samples is larger or smaller than the mean of the actual data. The *p-Value* column is the probability that the sample and the actual data come from different distributions. If *p-Value* is less than  $\alpha$  (0.05), then the sample population and the actual population are deemed to have a significant statistical difference.

**Table 7. T-test  $AVL_{25}$  and Actual.**

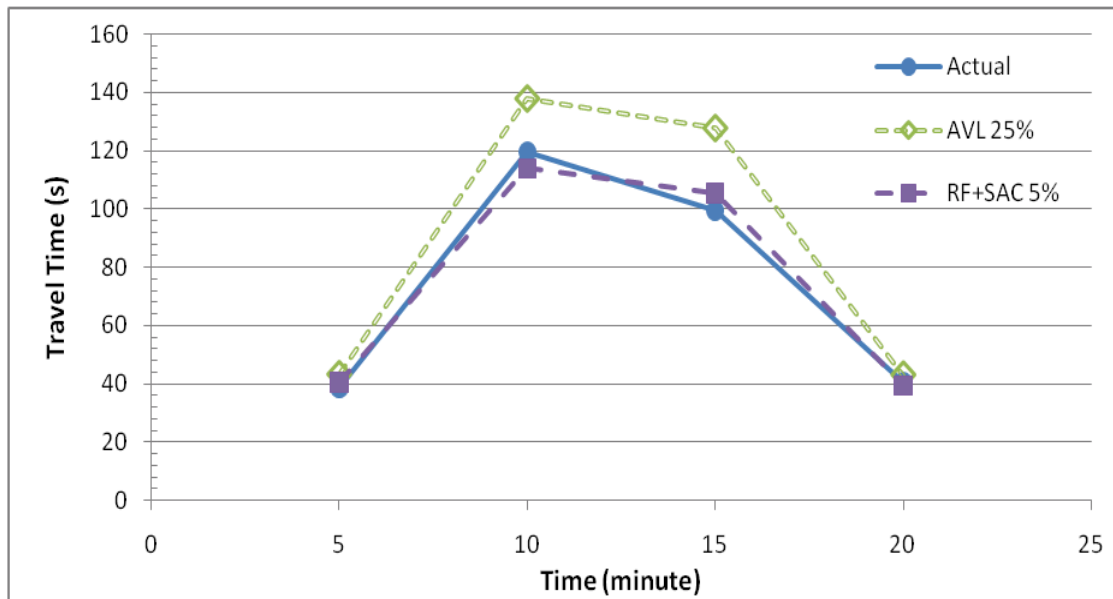
Time (min)	Actual		$AVL_{25}$		t-Stat	p-Value	Sig.?
	Mean	Var	Mean	Var			
0-5	38.55	0.40	43.12	0.03	-1.5326	0.0393	Yes
5-10	119.51	0.46	138.01	0.02	-7.0277	0.0055	Yes
10-15	99.59	0.32	127.86	0.60	-1.8161	0.0018	Yes
15-20	40.62	0.28	42.97	1.10	-2.1121	0.0400	Yes
0-20	74.57	1456.39	87.99	1163.09	-0.8172	0.0360	Yes
SMS	13.41	-	11.36	-	-	-	Yes

Travel time and SMS estimated by  $AVL_{25}$  systems have significant differences compared to the actual data as shown in **Table 7**. This is because mainly trucks have lower acceleration and deceleration than cars. **Table 8** shows that even in 5% PR,  $RF_5$ +SAC provides travel times and SMS with no significant difference from the ground

truth. In addition, the eminent change in travel time with a large mean difference occurring in the second and third intervals shows the possibility of congestion or transition in traffic flow.

**Table 8. T-test RF<sub>5</sub>+SAC and Actual.**

Time (min)	Actual		RF <sub>5</sub> +SAC		t-Stat	p-Value	Sig.?
	Mean	Var	Mean	Var			
0-5	38.55	0.40	40.40	5.19	-0.9700	0.4371	No
5-10	119.51	0.46	113.89	666.13	0.6910	0.0559	No
10-15	99.59	0.32	105.39	902.07	-2.8911	0.0223	Yes
15-20	40.62	0.28	39.38	21.68	0.0060	0.9577	No
0-20	74.57	1456.39	69.77	1025.95	0.0773	0.9391	No
SMS	13.41	-	14.33	-	-	-	No



**Figure 28. Average travel time (aggregation every 5 minutes).**

**Figure 28** shows the travel times for actual, AVL<sub>25</sub>, and RF+SAC<sub>5</sub>. Note that RF<sub>5</sub> has 0.0 MRR and therefore no travel time can be estimated. As indicated in **Tables 7** and **8**, RF+SAC<sub>5</sub> can produce travel times much closer to the actual traffic status than AVL<sub>25</sub>. We note that the performance of AVL in estimating travel times and speeds could be closer to that of RF+SAC if other types of vehicles (*e.g.*, sedans) were also equipped with AVL devices. In such cases, AVL (in the best case and avoiding the inaccuracy in sampled data and interpolation by AVL) may perform as accurately as RF+SAC.

**Table 9. T-test AVL<sub>100</sub> and Actual.**

Time (min)	Actual		AVL <sub>100</sub>		t-Stat	p-Value	Sig.?
	Mean	Var	Mean	Var			
0-5	38.55	0.40	44.95	0.01	-15.8148	0.0402	Yes
5-10	119.51	0.46	144.98	0.0007	-54.8502	0.0116	Yes
10-15	99.59	0.32	139.86	0.03	-153.0912	0.0041	Yes
15-20	40.62	0.28	44.79	0.08	-23.6415	0.0269	Yes
0-20	74.57	1456.39	93.64	2722.33	-3.4170	0.0111	Yes
SMS	13.41	-	10.67	-	-	-	Yes

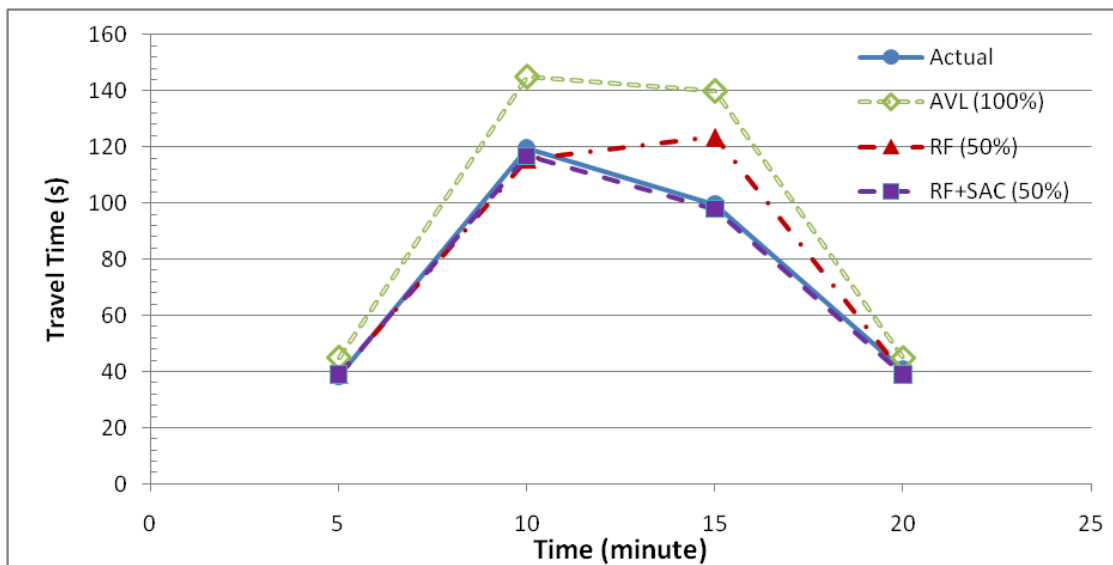
**Table 9** shows that even AVL<sub>100</sub> provides a significantly different estimation of travel time and SMS compared with the actual status of the highway. **Table 10** shows that RF<sub>50</sub> (and by extension, RF+SAC<sub>50</sub>) can provide high quality estimation of travel time and SMS that is not significantly different than the actual traffic conditions. The addition of SAC to RF will only add to the data quality because even more data can be delivered than with RF alone. **Table 11** shows the comparison between RF<sub>50</sub> and RF+SAC<sub>50</sub>. Some of the messages with RF+SAC may have higher message delay, but as long as the delay is less than the aggregation interval, the data can be used in the traffic analysis.

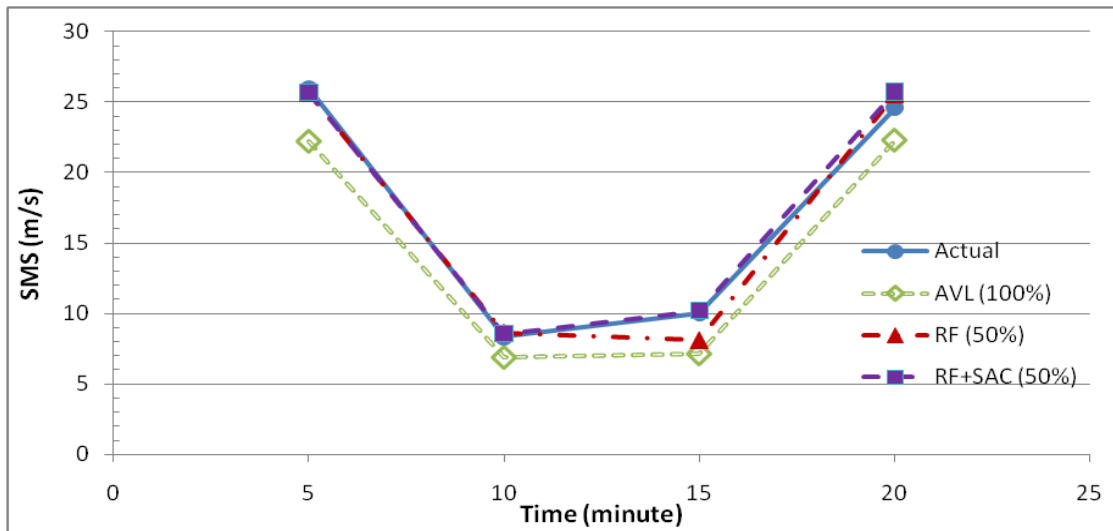
**Table 10. T-test  $RF_{50}$  and Actual.**

Time (min)	Actual		$RF_{50}$		t-Stat	p-Value	Sig.?
	Mean	Var	Mean	Var			
0-5	38.55	0.40	39.00	2.01	-0.3117	0.80761	No
5-10	119.51	0.46	115.78	9.85	2.1514	0.2769	No
10-15	99.59	0.32	123.55	367.38	-1.7165	0.3358	No
15-20	40.62	0.28	39.17	1.35	3.2641	0.1892	No
0-20	74.57	1456.39	79.37	1918.00	-0.9630	0.3676	No
SMS	13.41	-	12.59	-	-	-	No

**Table 11. T-test  $RF_{50}+SAC$  and  $RF_{50}$ .**

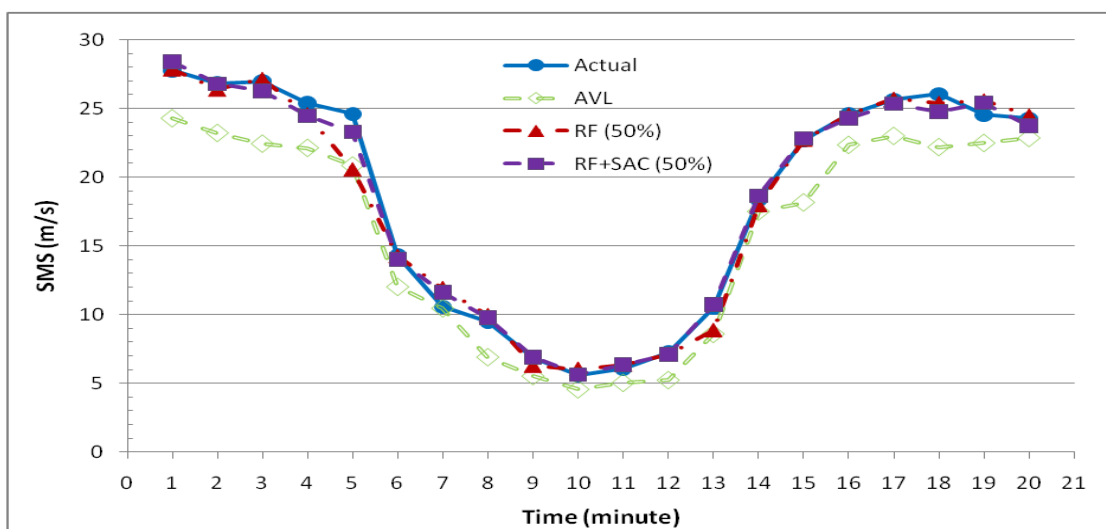
Time (min)	$RF_{50}$		$RF_{50}+SAC$		t-Stat	p-Value	Sig.?
	Mean	Var	Mean	Var			
0-5	39.00	2.01	38.94	0.01	0.0521	0.9668	No
5-10	115.78	9.85	116.72	0.15	-0.4858	0.7120	No
10-15	123.55	367.38	97.92	0.01	1.8805	0.3111	No
15-20	39.17	1.35	38.84	0.05	0.5075	0.7010	No
0-20	79.37	1918.00	73.10	1388.43	1.2612	0.2476	No
SMS	12.59	-	13.67	-	-	-	No

**Figure 29. Travel time in 50% PR (aggregation every 5 minutes).**



**Figure 30. SMS in 50% PR (aggregation every 5 minutes).**

Figures 29 and 30 show the estimated travel time and SMS, respectively, in 50% PR. Because of the 100% MRR that RF+SAC provides, DTMon is able to track the actual traffic status even during the transient flow periods.



**Figure 31. SMS in 50% PR (aggregation every minute).**

**Figure 31** shows the trend in SMS when the aggregation interval is 1 minute. This indicates that transitions in traffic flow rate can affect travel times and speeds. Even at this level of aggregation, RF+SAC can track the actual status closely.

**Table 12** summarizes the comparison of DTMon with conventional sensors and detectors currently in use as well as other probe vehicle-based systems, such as AVL. Note that WLT system performance is very similar to AVL except that WLT has also issues with accurately detecting the location and timing of vehicles at a specific point.

**Table 12. Comparison of DTMon with other technologies.**

Good Estimate?	Sensors and Detectors	AVL (using trucks)	DTMon
Travel Time	Not Available	Overestimate	Yes
SMS	Not Available	Underestimate	Yes

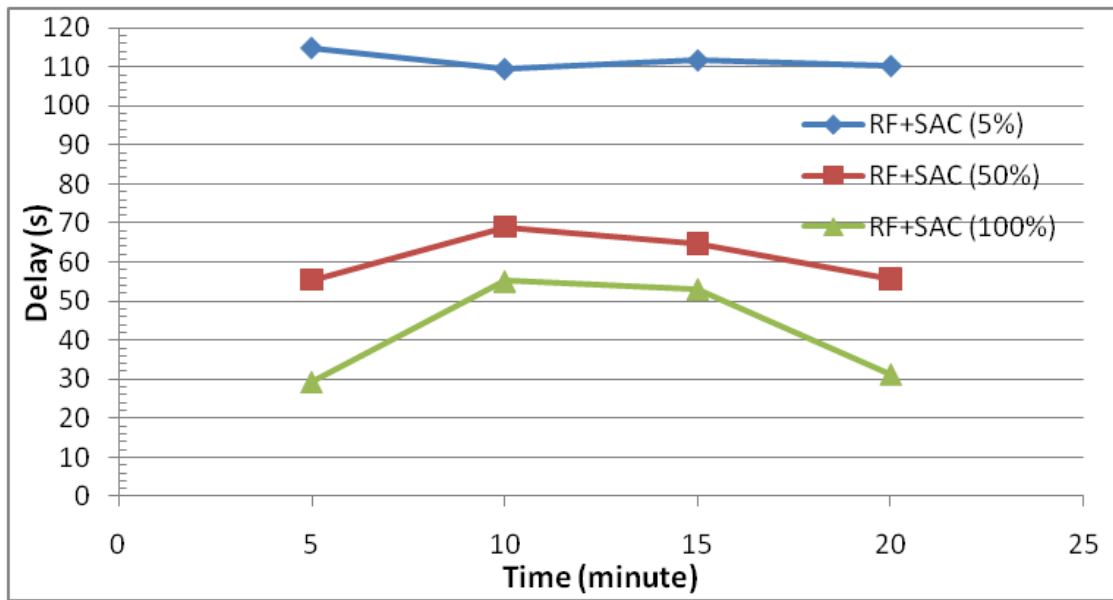
As mentioned earlier, point-based sensors and detectors cannot directly measure travel times. AVL systems are typically installed in commercial trucks (or taxis, etc.), which have different characteristics than the majority of traffic (slower acceleration/deceleration, larger inter-vehicle gaps, resulting in longer travel times).

#### **4.3.5.3. Latency (Message Delay)**

Sensors and detectors sense vehicles instantly. Their collected data are usually transferred to a TMC periodically via cable or wireless communication with some amount of latency. The aggregation usually happens every 5 minutes [14, 16]. In AVL systems, vehicles' location and speed are probed periodically with negligible delay but these locations and speeds must be interpolated for a desired location and specific point on the road. For example, a vehicle with maximum speed 30 *m/s* may be probed anywhere in the range of

150 meters around  $VS_2$  when the sampling period is every 5 seconds (which is a high rate). In the best case, a vehicle's location is probed when it is exactly passing  $VS_2$  at probe time. With a lower sampling rate, this offset increases.

Latency in DTMon varies depending upon several factors. The variation in the delay depends on the method of message delivery. For example, RF adds very small delay (in terms of milliseconds), while store-and-carry adds some travel time to the carried messages. In the previous section, we showed that TOs are capable of collecting and estimating high quality travel times and speeds as well as their trends even in traffic with transient flow rate and with various market penetration rates.



**Figure 32. Message delay.**

**Figure 32** shows the message delay when RF+SAC is used with different PR. Recall from **Table 5**, in 5% PR, the MRR (of messages sent from  $VS_2$  to  $TO_1$ ) is zero when RF



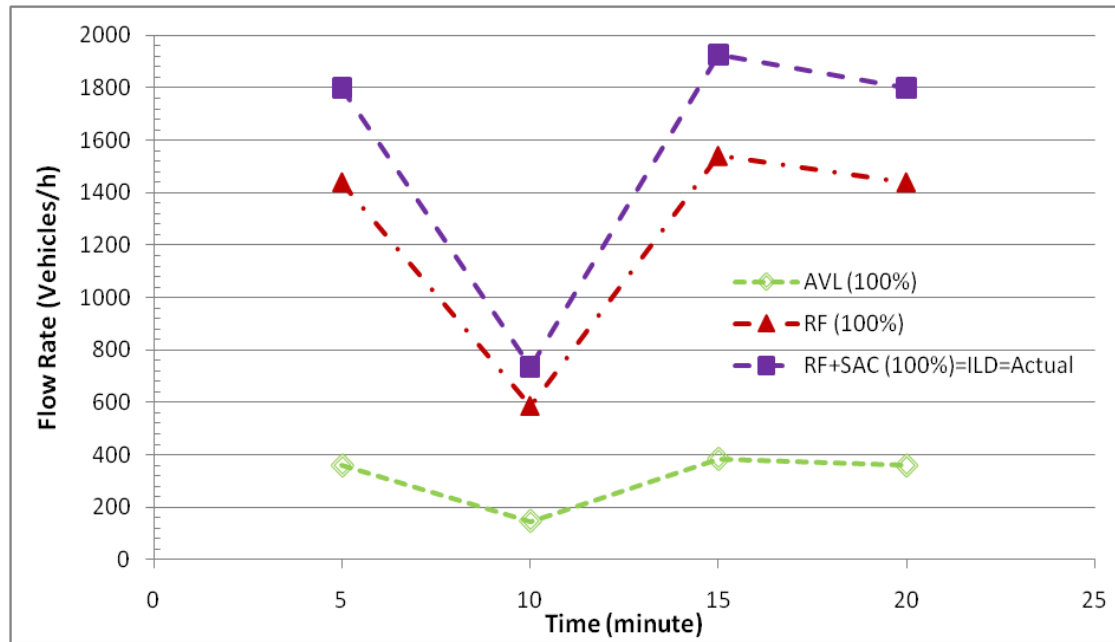
is used and only RF+SAC could deliver the messages. This increases the message delay by the average travel time from  $VS_2$  to  $TO_5$ . In medium PR like 50%, almost half of the messages could be forwarded to  $TO_1$ , with an average message delay of 7 ms. The rest of the messages are carried to  $TO_5$ . Congestion during the second and third interval (between 5-10 minutes) also adds additional delay to the messages that are carried. In high PR like 100%, more messages are forwarded (80%) than carried (20%) and the average message delay becomes lower.

Message delay can be a factor in selecting the method of message delivery. In **Tables 10** and **11** we showed that in 50% PR and higher, RF and RF+SAC can provide high quality estimation of traffic speed and travel time and can also sense transition and trends in traffic flow based on travel time and SMS with 95% confidence. The advantage of using RF alone is that we can use a single TO instead of multiple TOs to cover the area, resulting in a latency below one second. The disadvantage of RF is a lower IRR especially for VS far from a TO. In contrast, RF+SAC results in higher IRR, especially in low PR, but with added latency. This latency can vary according to the traffic conditions and the distance from TOs.

Message delay can also be used to show how fresh the generated reports are. This may be crucial in real-time traffic monitoring. For example, aggregation may happen every minute as shown in **Figure 31**, but during congestion the message delay was 70 seconds for RF+SAC<sub>50</sub> (**Figure 32**), resulting in deviations from the actual traffic status.

#### 4.3.5.4. Count Information

Count-based metrics such as flow rate, volume, and density can only be estimated when the market penetration is high. **Figure 33** shows the computed flow rate by DTMon with 100% PR during each time interval. Only 20% of the traffic are trucks, therefore the estimated flow rate using AVL is inaccurate.  $RF_{100}$  on average has 79.50% IRR and missed 21.5% of the vehicles that passed  $VS_2$ .  $RF_{100}+SAC$  collects all messages and reports the same flow rate as ILDs and the actual traffic status. In 100% PR, RF and  $RF+SAC$  can detect the transition in flow rate as shown in **Figure 33**. Thus, for high PR scenarios, DTMon can be a good replacement system for sensor and detectors. In low or medium PR, DTMon can be used to augment current systems and add important travel time and SMS data.



**Figure 33. Flow rate at  $VS_2$  in 100% PR (aggregation every 5 minutes).**

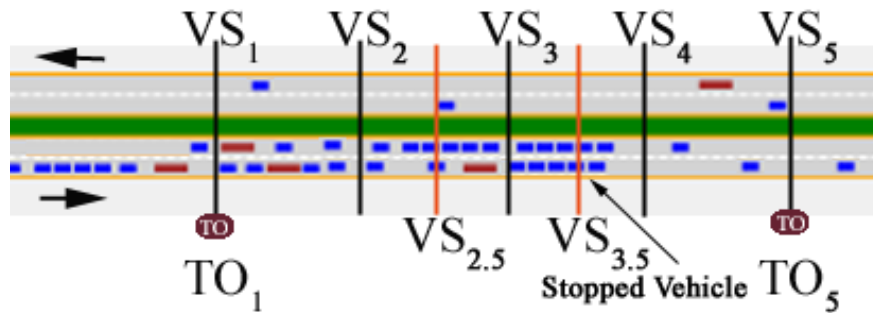
#### 4.3.5.5. End-of-the-queue Virtual Segment

As we explained in Section 4.3.3, identifying the end-of-the-queue is important for traffic engineers as this is the place where most secondary incidents occur. The primary incident is the cause of the congestion, but secondary incidents tend to occur at the point where the speed transition is the sharpest. Traffic engineers would like to be able to identify these locations so that approaching drivers can be warned in advance, potentially avoiding the secondary incidents. We explained the importance of dynamically defined virtual strips and virtual segments for detecting end-of-the-queue in traffic with congestion in the previous sections. We showed that the transition in the traffic flow rate cannot be detected in low PRs, although a transition in the traffic flow rate can produce a transition in travel time and speed which can be detected using DTMon.

We show an example of detecting an end-of-the-queue using DTMon in this section. A vehicle breaks down after 600s of simulation (10 *min*) in the first lane at a location approximately 3.5 *km* from the highway entrance, far from  $TO_1$  which is located 1 *km* away from the highway entrance. The vehicle blocks the first lane of the road in two-lane highway for a long period (30 *min*). This causes kilometers of backup and congestion in the traffic that initially has a flow rate 3600 *veh/h* and speed 29 *m/s*. After a total of 40 minutes have passed, the blocking vehicle starts moving in order to release the congestion for the remaining 20 minutes of the one hour simulation.

During the 30 minutes of blocking, a backup starts to form, and the end of the backup will move. The end will fall into different virtual strips/segments of 1 *km* size  $VS_1$ ,  $VS_2$ ,  $VS_3$ ,  $VS_4$  ( $VS_1VS_2$ ,  $VS_2VS_3$ , and  $VS_3VS_4$  respectively) which were predefined by  $TO_1$ .

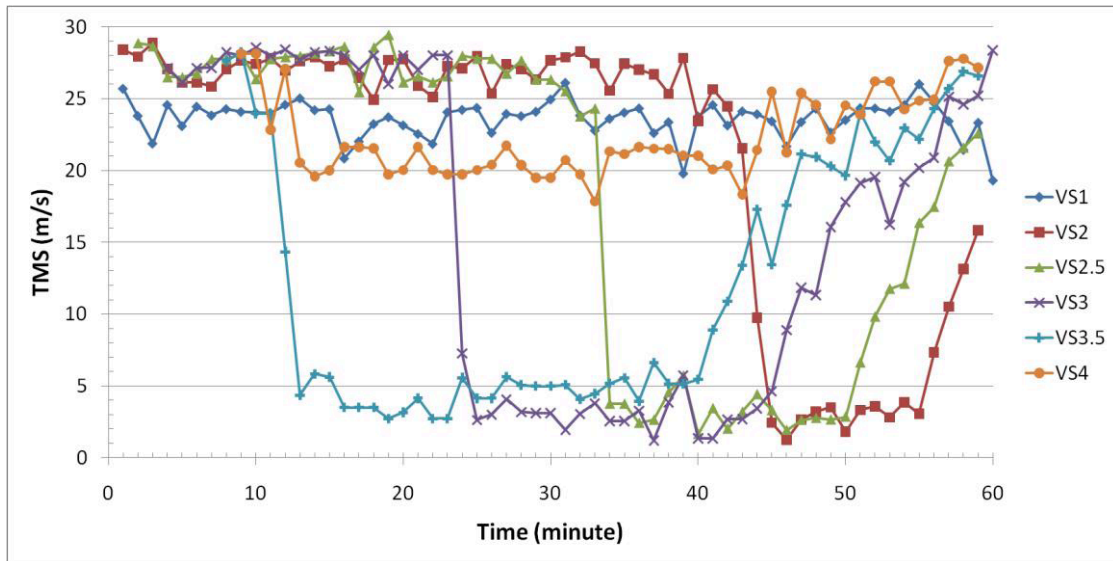
The virtual strips  $VS_{2.5}$  and  $VS_{3.5}$  will be dynamically defined by  $TO_1$  during the congestion, after the vehicle's break-down (and its location) is reported to  $TO_1$  (as shown in **Figure 34**).  $TO_1$  (and  $TO_5$  if necessary for low penetration rates) monitor the traffic speed and travel time for each defined strip and segment. Note that we have shown the impact of penetration rate, distance, and use of store-and-carry in our previous experiments. In this experiment, we plan to show how  $TO_1$  (with average message delay of less than 1 second where RF is used) can infer the end location of the backup in traffic.



**Figure 34. Location of TOs and VS (not to scale).**

**Figure 35** shows the traffic speed at defined virtual strips when averaged every minute. The transition in traffic speed at a specific VS can be used to infer that the backup has passed that VS, since traffic behind that strip has a slower speed. For example at time 24 *min*, the backup has not reached  $VS_3$  because the monitored traffic speed for that strip was still high. But at time 26 *min*, the backup endpoint must have passed  $VS_3$  because the traffic passing  $VS_3$  reported a very low speed. **Figure 35** shows the ability of

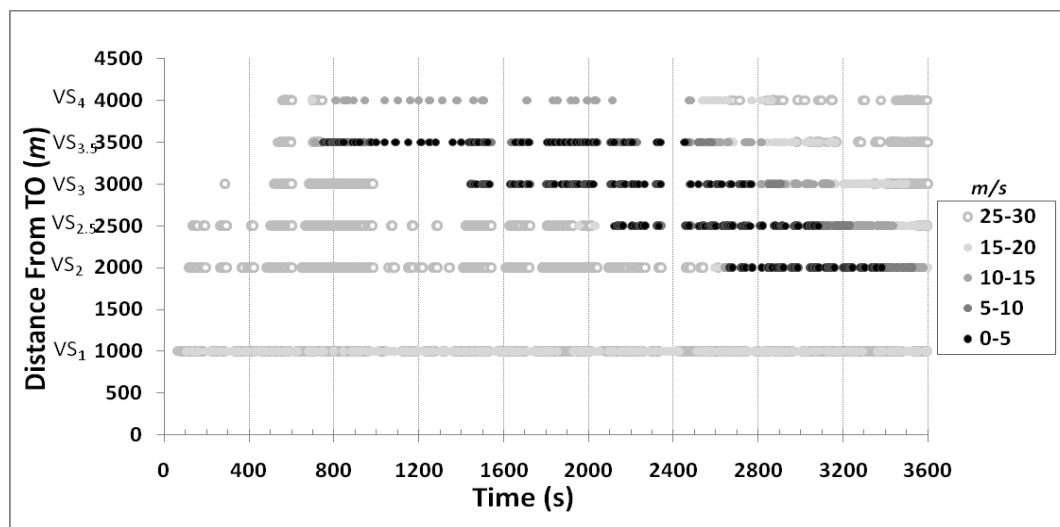
DTMon to detect the transition in traffic speed, which is one important factor to locate the end of the queue.



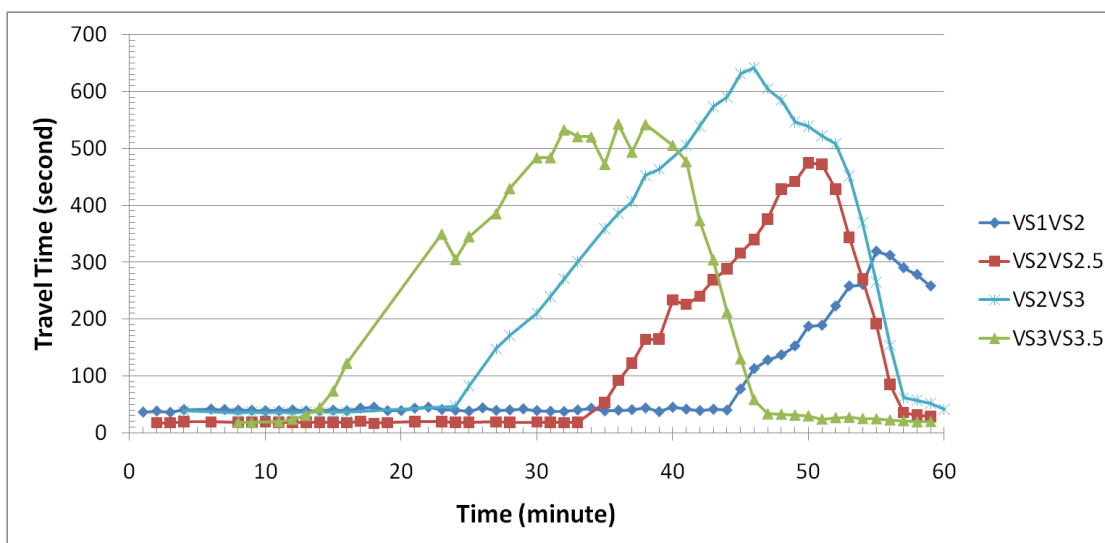
**Figure 35. TMS at virtual strips. Traffic stoppage occurs during 10-40 min.**

In **Figure 36**, we show an alternate view of data in **Figure 35** to highlight the movement of the end of the queue. The darker points indicate lower traffic speeds. If the traffic speed for a VS (*e.g.*, VS<sub>2.5</sub> at 2100s) is dark and the traffic speed for the location ahead is dark too (*e.g.*, VS<sub>3</sub> at 2100s), the TO can infer that congestion has reached the VS. When the points become brighter (*e.g.*, VS<sub>3</sub> at 3000s), it means that the speeds are increasing, therefore the TO can infer that the congestion must have been released at VS<sub>3</sub>. **Figure 36** shows the ability of DTMon to detect the transition in traffic at various points on the road and at any dynamically defined virtual strip. DTMon can compare these

transitions in speed at any two consecutive virtual strips. This is helpful to locate the end of the back up or the release in the back up.



**Figure 36. Time, Space, and Speed.**



**Figure 37. Travel time for defined virtual segments.**

**Figure 37** shows the travel time averaged every minute at the dynamically defined virtual strips. Transitions in travel time in a specific virtual segment can be used to infer whether the backup has reached that virtual segment or not. For example, travel time in segment  $VS_2VS_3$  starts to increase after 25 min, showing that the backup has reached this segment and has started to expand toward the farther strips. The TO can dynamically define  $VS_{2.5}$  in this segment to monitor whether the backup has reached the upper section ( $VS_{2.5}VS_3$ ) or lower section ( $VS_2VS_{2.5}$ ) of the segment  $VS_2VS_3$ . For example, the traffic speed at  $VS_{2.5}$  (see **Figures 35** and **36**) has sharply decreased after 35 min, and the travel time for the segment  $VS_2VS_{2.5}$  has increased, showing that the backup must have reached into  $VS_2VS_{2.5}$ , the lower section of the segment  $VS_2VS_3$ . In contrast, the end-of-the-queue must have been inside the upper section between 25 min to 35 min. Overall, **Figures 35**, **36**, and **37** show the advantage of dynamically defined virtual strips in DTMon, and DTMon's ability to infer the location of the end of the queue during congestion.

#### 4.4. ESTIMATING MARKET PENETRATION RATE

In the previous sections, we have shown that DTMon can be used to estimate high-quality travel times and speeds with relatively low PRs. But, DTMon, as with any probe-based system, can only estimate high-quality flow rates and density with high PRs. As the quality of flow rate and density measurements are proportional to the PR, the level of PR needed for high-quality estimates depends on the acceptable error level. It is possible to augment DTMon with current technologies such as sensors and detectors (*i.e.*, MRS or ILDs) and use the count information data collected by these technologies fused with data

collected by TOs. As a result, data collected by the TOs are augmented by data gathered from sensors and detectors for the same virtual strip (same point of interest on the road). This gives a TO the ability to estimate the PR by observing the total number of equipped and unequipped vehicles which pass virtual strips in the range of the TO. In this case, the total number of vehicles (equipped + unequipped) is collected by sensors and the total number of equipped vehicles is collected by the TO. Once the PR is estimated, we can apply it to density and flow rate measurements to increase the quality of those estimates. We expect that the PR in an area will change very slowly over time, so this estimation could be done once and used to augment DTMon's density and flow rate estimates for several months.

#### **4.5. USING STATISTICAL SAMPLING IN DTMON**

There are statistical methods (*e.g.*, Sample Size with Acceptable Absolute Precision for Finite Populations [80]) that can be used to determine how many samples (reports) are required to be received by a TO to maintain a desired level of accuracy and confidence in the traffic estimates. These methods require the input of acceptable variance, acceptable significance level, and acceptable absolute error and output the required sample size.

In DTMon, the traffic conditions (including the PR) play a major role in the number of messages that a TO successfully receives. Even in 100% PR, the TO may not have a high message reception rate from virtual strips that are out of its range, therefore the TO will task all passing equipped vehicles in order to be sure it can keep the message reception rate high from farther strips. This will help to keep the message reception rate at its maximum in any traffic condition with various penetration rates. Also, the accuracy



of estimated metrics will remain at its maximum. We do not consider the problem of sampling and distributing tasks by TOs to a smaller population of equipped vehicles. The goal of traffic monitoring by the TMC is to collect and archive all possible data for accurate record-keeping and archiving. As the goal of DTMon is to support this data collection, we strive to collect all possible data. This is feasible because there is not much overhead in DTMon, as mentioned in Chapter 3 (Sections 3.1.3 and 3.1.5.3). Reports where sampling could be employed, such as travel times and speeds, can be piggybacked on flow rate and density (volume) reports, where the data from the maximum number of vehicles is required for high-quality estimates.

#### **4.6. SUMMARY**

We analyzed the ability of DTMon to provide high-quality traffic data in free-flow traffic with low to medium market penetration rates. We showed how MRR and IRR were affected by the market penetration rate, traffic speed, traffic flow rate, and distance of the measurement point (VS) from the TO. The IRR directly impacts the quality of certain traffic data. We evaluated different methods of delivering messages to improve the IRR and showed that an improvement in the message reception rate can have the cost of increased message delay. But, regardless of the method of message delivery, we showed that DTMon can collect high-quality travel time and speed data in free-flow traffic where the possibility of receiving messages from just a few vehicles exists.

In general, average speed, travel time, SMS are not affected by missing messages (unless all are lost or failed to be delivered). The amount of difference in calculated averages for speed (TMS and SMS) and travel time are negligible. This shows that with

a lower penetration rate and a higher miss rate, these averages can still be good estimations for actual traffic speed and travel time. On the other hand, the estimate of traffic flow rate, volume, and density are affected by information reception rate. DTMon can only provide good estimate of count information in traffic with high penetration rates.

Overall, store-and-carry can always deliver the data from strips when multiple TOs are deployed, especially for low market penetration rates. This may add cost to system as well as latency in delivering messages. DTR can only improve the system when the market penetration rate is medium or high, and it will perform to equal RF in low market penetration rates or with low density traffic. Nevertheless, TMS, SMS, and travel time can be estimated even with few samples in free flow traffic.

We also showed that DTMon can provide high quality travel time and SMS data in transient flow traffic, such as that caused by non-recurring congestion. Currently-deployed sensors and detectors can only estimate these metrics, and with transient flow traffic, the estimates are often far off from reality. With low market penetration rates, DTMon's measurements may incur some delay caused by vehicles having to carry their data to a nearby TO. But, even in these situations, DTMon can be used to augment current fixed point sensors and detectors. With high market penetration rates, DTMon could replace these sensors and detectors altogether.

In addition to providing up-to-date travel statistics, DTMon can be used to detect transitions in traffic flow using travel time and speed measurements gathered from dynamically defined virtual strips. We showed the advantage of using dynamically defined virtual strips in DTMon and that DTMon can detect end of the queue by

monitoring traffic speed and travel time and their transitions from various dynamic points on the road.

We have shown that DTMon provides better performance than AVL systems in monitoring these metrics. The decision regarding which of DTMon's message delivery methods to use depends on the market penetration rate, message delay, and acceptable latency.

## CHAPTER 5

### CONCLUSION AND FUTURE WORK

In this dissertation we introduced DTMon, a framework for monitoring traffic data. The main components of the DTMon are *task organizers*, *virtual strips*, and *equipped vehicles*. Task organizers (TOs) are roadside units that can communicate with equipped vehicles and with a traffic management center (TMC). These TOs can be programmed by the TMC to task vehicles with performing traffic measurements over various sections of the roadway. Virtual strips (VS) are the measurement points for TOs. VS can be changed dynamically and placed anywhere within several kilometers of the TO. This is a vast improvement over current technology, where specific measurement points must be decided in advance and hardware installed in those locations. DTMon can be used in both rural and urban areas with very little infrastructure. We have shown that in medium to high density traffic, a single TO can be used to collect data over several kilometers of roadway. The installation of an additional TO past the desired monitoring points allows for the use of store-and-carry techniques, which would improve DTMon's performance in low density traffic. This system presents a clear advantage over current point-based monitoring systems in that it can collect highly-desired travel time and speed metrics. In addition, the VS measurement points can be dynamically defined based on changing traffic conditions.

We analyzed the ability of DTMon to provide high-quality traffic data in free-flow traffic with low to medium market penetration rates. We introduced the information

reception rate (IRR) and showed how it was affected by the market penetration rate (PR), traffic speed, traffic flow rate, and distance of the measurement point (VS) from the TO. The IRR directly impacts the quality of certain traffic data. We evaluated different methods of delivering messages to improve the IRR and showed that an improvement in the message reception rate can have the cost of increased message delay. But, regardless of the method of message delivery, we showed that DTMon can collect high-quality travel time and speed data in free-flow traffic where the possibility of receiving messages from just a few vehicles exists.

We showed that DTMon can provide high quality travel time and SMS data in transient flow traffic, such as that caused by non-recurring congestion. Currently-deployed sensors and detectors can only estimate these metrics, and with transient flow traffic, the estimates are often far off from reality. With low market penetration rates, DTMon's measurements may incur some delay caused by vehicles having to carry their data to a nearby TO. But, even in these situations, DTMon can be used to augment current fixed point sensors and detectors. With high market penetration rates, DTMon could replace these sensors and detectors altogether. In addition to providing up-to-date travel statistics, DTMon can be used to detect transitions in traffic flow using travel time and speed measurements gathered from dynamically-defined virtual strips. We showed that DTMon is able to detect end of the queue locations during congestion using dynamically-defined virtual strips and detecting transition in travel times and speeds. We have shown that DTMon provides better performance than Automatic Vehicle Location (AVL) systems in monitoring these metrics. The decision regarding which of DTMon's

message delivery methods relies on the market penetration rate, message delay, and acceptable latency.

In Appendix A, we described the first implementation of a vehicular mobility model integrated with the networking functions in ns-3. The integrated VANET simulator that include both mobility and network models and allowed us to evaluate the performance of DTMon in comparison with the rival technologies and over various traffic conditions (*e.g.*, free flow traffic, traffic with congestion, etc.).

The summary of this dissertation is as follows:

- **DTMon** is a probe vehicle-based system that uses spatial sampling to dynamically monitor traffic data. DTMon has two major components:
  - Task Organizers
  - Virtual strips
- **DTMon** can provide high quality estimates of **travel time** and **speed**.
- **DTMon** can provide high quality estimates of flow rate and density in higher penetration rates.
- **Hybrid** message delivery **improves** information reception rate with the cost of increased latency as an option for low penetration rates.
  - RF and RF+SAC have similar performance in higher penetration rates
  - Using RF+SAC is an improving option in low penetration rates
- **DTMon** can **detect** transitions in traffic flow using estimated travel time and speed.

- **DTMon** can **detect end of queue** situations in congested traffic via dynamically-defined virtual strips and transition detection using travel time and speed.
- **DTMon** can **augment** current technologies and systems in monitoring important traffic data.
- We contributed the **first implementation** of a vehicular mobility model **integrated** with the networking functions in ns-3.
  - Full user control for **studying a wide variety of scenarios** in VANETs using **realistic mobility and network** models
  - **Customization** of almost **all aspects** of the simulation
  - Settings and functions for wireless communication
  - Manually and automatically creation of scenarios
  - Allowing for feedback

## 5.1. CONTRIBUTIONS

The contributions of this dissertation are as follows:

1. *A method for using probe vehicles to perform spatial sampling of traffic conditions* – Probe vehicles can provide real-time measurements of speed and travel time. Using spatial sampling allows for these measurements to be made at specific locations of interest on the roadway. This avoids the need for interpolation and estimation that is required when temporal sampling of probe vehicles is performed.

2. *An analysis of the factors that can impact the quality of monitored traffic data when using vehicular networks* – These factors are market penetration rate, traffic conditions, communication range, distance between communicating entities, methods of message delivery, message reception rate, and message delay. We have performed an analysis of these factors and how each contributes to the quality of traffic data that can be reported. We note that both traffic conditions and market penetration rate affect the distance between communicating entities.
3. *An evaluation of the impact of different methods of message delivery on the quality of traffic data that can be gathered by vehicular networks* – We compared four different message delivery methods (regular forwarding, dynamic transmission range, store-and-carry, and a hybrid approach) by measuring message reception rate and message delay in different traffic conditions and different market penetration rates. We found that a hybrid approach (regular forwarding coupled with store-and-carry and using vehicles traveling in the opposite direction) can significantly improve the performance of DTMon in poorly connected traffic conditions. We also showed that when the market penetration rate is high, the message delay can be reduced significantly.
4. *An evaluation of the effectiveness of DTMon as compared with current technologies such as inductive loop detectors (ILD) and automatic vehicle location (AVL)* – We show that DTMon can be used to report travel times that are not statistically different from the actual travel times. In comparison, we show



that technologies such as ILD cannot measure travel times with this level of accuracy.

5. *A demonstration of the usefulness of DTMon's monitoring approach for monitoring congested traffic conditions* – We have shown the ability of DTMon to allow a TMC to dynamically place additional monitoring points (virtual strips) in locations where congestion is building up. This allows the TMC to detect transitions in traffic flow using speeds and travel times, without having to rely on flow rate information. We have shown that DTMon can be used to detect and track the end-of-the-queue in traffic with congestion.
6. *Highway mobility modules for the ns-3 network simulator* – We have contributed the first highway mobility modules designed to produce realistic vehicle mobility and communications in ns-3. The mobility model has been validated against the well-known vehicular mobility models, and the networking components use ns-3, which has been validated against wireless models. These modules have been released to the ns-3 community and are now being used by other researchers around the world.

## **5.2. FUTURE WORK**

Our future work is to continue studying the design of traffic monitoring systems that could be deployed in different configurations based on roadway topology and traffic conditions. As we have seen for some types of data, the market penetration rate affects the quality of data gathered. We have investigated deploying multiple RSUs (*e.g.*, TOs) in a region and using oncoming traffic to help bridge gaps in connectivity. But, future

work may require an investigation of methods for processing collected data from mobile nodes (*e.g.*, cell phones) or from other sources. This opens various avenues of research, including communication protocols, wireless characteristics of vehicular (or mobile or sensor) networks, security, packet delivery and routing algorithms, standardization, real-time data-mining, and simulation/modeling. In addition to the research on these types of networks, this work can motivate new directions on the design of appropriate operating systems and applications of the required devices. These devices may be static or mobile and may be servers or clients of traffic data. Low cost, high performance data mining techniques (usually bound with data sources, search engines, and maps) must be investigated.

We will further investigate the use of dynamically-defined virtual strips and TOs in DTMon to evaluate the performance of our proposed framework in urban area specifically where TOs are deployed at intersections. Therefore, we have plans to extend our implementation of VANET simulation modules for urban areas (*e.g.*, intersections) and add the ability to read in and use detailed maps instead of a single straight highway. We also have plans to implement and develop the most recent WAVE/DSRC standard in ns-3. This will allow users to simulate up-to-date wireless communication for VANETs based on the standard. We hope that our addition to ns-3 along with our future work will allow researchers to easily perform high-quality VANET simulations.

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## **APPENDIX A**

### **SIMULATION MODULES FOR VANETS AND HIGHWAY**

#### **MOBILITY IN NS-3**

The study of VANETs, and in our case DTMon, requires efficient and accurate simulation tools. As the mobility of vehicles and driver behavior can be affected by network messages, these tools must include a vehicle mobility model integrated with a quality network simulator. We present the first implementation of a well-known vehicle mobility model to ns-3, the next generation of the popular ns-2 networking simulator. Vehicle mobility and network communication are integrated through events. User-created event handlers can send network messages or alter vehicle mobility each time a network message is received and each time vehicle mobility is updated by the model. To aid in creating simulations, we have implemented a straight highway model that manages vehicle mobility, while allowing for various user customizations. We show that the results of our implementation of the mobility model matches that of the model's author and provide an example of using our implementation in ns-3.

The problem with integrated simulators is that often either the mobility model is overly simplified or the network model is overly simplified. In order to study important networking properties of VANETs, a high quality network simulator is essential. We have chosen to balance these two concerns by taking the latest version of the highly-regarded network simulator, ns-3 and adding a well-known traffic mobility model in order to provide an integrated simulator for VANET research. This work [34] has been

published and is being used by various researchers and users. ns-3 is a discrete-event network simulator written in C++, targeted primarily for research and educational use, and intended as a replacement for the popular ns-2 simulator. ns-3 promises to be a more efficient and more accurate simulator than its predecessor (especially for wireless protocols). For this reason, we were interested in using ns-3 to perform our VANET simulations.

ns-3 provides various mobility models, but none are appropriate to simulate the mobility of vehicles. The mobility of a node in the mobility models included in ns-3 depends only on the node itself. In realistic vehicular mobility, the mobility of the node must depend on the surrounding nodes and the conditions on the road. Furthermore, this node dependency becomes essential when messages in the network can affect the mobility of the nodes on the roads. For example, the receipt of a safety message may result in a speed reduction.

We have implemented IDM and the MOBIL lane change model in ns-3 (see Sections A.1.3 and A.1.4). In addition, we have provided a *Highway* class to represent a straight multi-lane, bi-directional roadway. In our simulations, the *Highway* object is the “brain” of the system and efficiently manages the behavior of vehicles and their mobility on the road. Each vehicle is a fully-fledged wireless node in ns-3. In this way, vehicles can move with realistic mobility and communicate with each other to form a VANET. In our network and mobility combined design, a user can simulate VANETs in highways with customized road-side and on-board units. Users can create user-defined actions and event

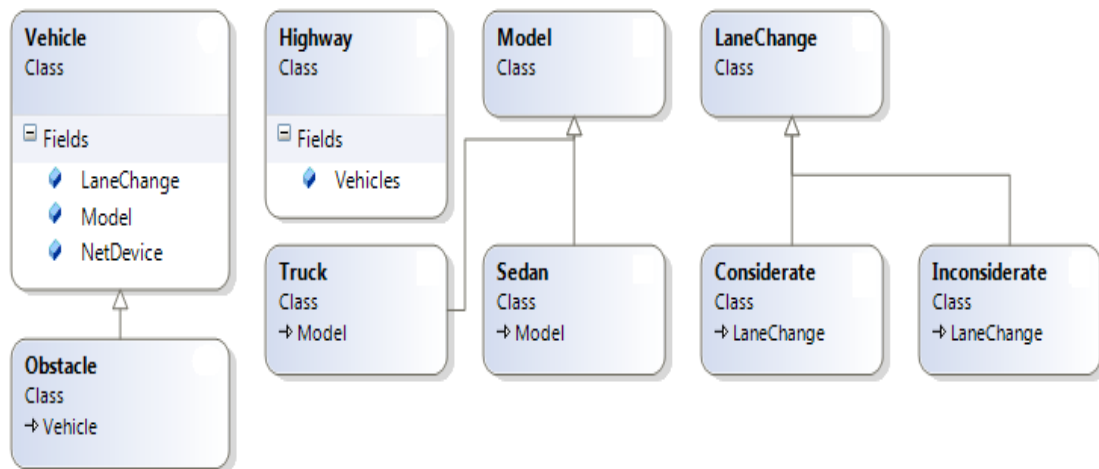


handlers to customize simulation scenarios, allowing them to study vehicular traffic, network traffic, or both.

We explain the main components of our design in Section A.1 and highlight possible user customizations, such as adding helicopters or embedded highway sensors, in Section A.2. In Section A.3, we discuss validation of our IDM/MOBIL implementation in ns-3, and in Section A.4 we discuss an example of our additions to ns-3.

### A.1. ARCHITECTURE

Here we describe the components of our design, which consists of five main classes (Figure 38):



**Figure 38. Class diagram of the main components in our design.**

1. *Vehicle* - a mobile node that contains a wireless communications device
2. *Obstacle* - a *Vehicle* that has no mobility
3. *Model* - the IDM car-following mobility model

4. *LaneChange* - the MOBIL lane change model
5. *Highway* - holds *Vehicle* and *Obstacle* objects and uses a *Vehicle's Model* and *LaneChange* properties to control its mobility

*Highway* uses the first four classes to generate the traffic in a highway. Since vehicular mobility models, and especially car-following models like the one we implement, need to know the position and mobility of other vehicles, the *Highway* object must be used to control the mobility of all vehicles. Users can customize *Highway* (including highway length, uni-directional or bi-directional traffic flow, number of lanes, lane width, and center median width) to create a variety of simulation scenarios.

In the following sections, we will describe each of the classes in order. The source code, examples, and documentation [34, 81] are available for researchers and developers.

#### **A.1.1. Vehicle**

A *Vehicle* is a mobile node that contains a wireless communications device. A *Vehicle* has the following properties:

- *vehicleID*
- *width* - width of the vehicle in meters
- *length* - length of the vehicle in meters
- *lane* - lane number on the highway where the vehicle is located
- *direction* -  $\{-1, 1\}$  (Assume eastbound is 1 and westbound is -1).
- *position* - a vector  $(x, y, z)$ , where  $x$  is the rear position of the vehicle,  $y$  is the center of the vehicle, and  $z$  is the altitude of the vehicle above the highway (all units in meters)

- *velocity* - in  $m/s$
- *acceleration* - in  $m/s^2$
- *model* - mobility model settings, desired velocity is associated with the mobility model
- *lanechange* - lane change model settings

In our design, the *Highway* object is in charge of managing the positions, directions, and the lane numbers of its vehicles. A *Vehicle*'s acceleration and velocity can be set manually or can be calculated based on the IDM mobility model rules. A *Vehicle* is able to change lanes, if necessary and possible, based on the MOBIL lane change model. *Vehicle* objects can either be manually created and inserted onto the *Highway*, or they can be automatically injected into the *Highway*.

Since a *Vehicle* contains a wireless communications device, we can control the vehicle's WiFi capabilities. *Vehicles* are able to communicate (send/receive) through the standard ns-3 WiFi channels. The messages, including sent and received packets, and all related events can be captured by setting the appropriate event handlers to the implemented callbacks, which are designed and considered for these purposes. A *Vehicle* can unicast packets or it can send broadcast messages. The user has full control on how to schedule the sending process and how to handle the receive callback. There are also several callbacks for the purpose of tracing the different layers of the network and the mobility of the vehicle. These help the user create and trace simulation scenarios easily.

### A.1.2. Obstacle

An *Obstacle* is a static node that contains a wireless communications device. It is inherited from the *Vehicle* class and has all of the capabilities of a *Vehicle* except that it cannot be mobile (*i.e.*, velocity = acceleration = model = lanechange = 0). An obstacle can be used as an barrier to close a lane or to temporarily create stoppages that result in congestion on the highway. An obstacle can also be used as a roadside unit or other piece of infrastructure along, but outside of, the highway. If an *Obstacle* is placed on the highway, it must have a direction and lane number. Anything that can be done to a *Vehicle* object can be done to an *Obstacle* object (aside from affecting mobility), so in the rest of this paper we will just use the term *Vehicle*.

### A.1.3. Mobility Model

*Model* is the class that implements the mobility model for a *Vehicle*. We have implemented the Intelligent Driver Model (IDM) in ns-3 based on equations and parameters developed by Treiber [52, 82]. IDM is a car-following model, meaning that each vehicle's acceleration or deceleration depends upon its own velocity, its desired velocity, and the position and velocity of the vehicle immediately in front in the same lane, which Treiber calls the *front vehicle*.

Each vehicle in IDM has a desired velocity, safety time headway (time needed to cover a gap between two vehicles, *e.g.*, the “2 second rule”), acceleration in free-flow traffic, comfortable braking deceleration, and desired minimum distance to the front vehicle. IDM uses these parameters and the current state of the vehicle and front vehicle to compute the new acceleration. Acceleration is, in turn, used to update the velocity and

position of the vehicle. Note that acceleration necessarily decreases towards 0 when the velocity of the vehicle approaches the desired velocity.

The function *CalculateAcceleration* in the *Model* class uses the IDM equations to calculate and return the new acceleration at each time step. The vehicle's new velocity and position are then adjusted based on this new acceleration.

For customizability, each vehicle can have its own set of IDM parameters. Treiber suggests different parameter settings for cars and trucks. For example, trucks have a lower desired velocity and acceleration in free-flow traffic, and longer deceleration gap than cars. "Careful" drivers would have a high safety time headway, and "pushy" drivers would have a low safety time headway, higher desired velocity, higher acceleration, and higher deceleration.

In our design, we also allow each *Vehicle* object to have its own IDM parameters. We have included reasonable default values for cars (the *Sedan* class) and trucks (the *Truck* class). The user can create their own vehicle types with different parameter values for specific experiments. For example, a user may want to create a mix of careful and pushy drivers, or include sports cars, police cars, emergency vehicles, and buses, all of which would have very different mobility characteristics.

#### **A.1.4. Lane Change Model**

*LaneChange* is the class that implements the lane changing model for a *Vehicle*. We have implemented the MOBIL lane change model based on equations and parameters developed by Treiber [53, 83]. Each lane change in this model must satisfy both the safety criterion and the incentive criterion. The safety criterion states that the lane change

must not cause the vehicle that is being changed in front of (the *back vehicle*) to decelerate unsafely (faster than a certain threshold). The incentive criterion is satisfied if the lane-changing vehicle's advantage is greater than the other vehicles' disadvantages. Note that although the incentive criterion is usually much easier to satisfy than the safety criterion, *both* must hold for the lane change to occur. In addition, the IDM rules still apply, meaning that the new front vehicle must be a certain distance ahead in order for the lane change to occur.

To compute the incentive criterion, MOBIL first calculates the lane-changing vehicle's advantage. This is simply the difference between the vehicle's current acceleration and the vehicle's new acceleration after the lane change. The goal is to increase the acceleration, or to reduce the braking deceleration, which are essentially the same things. The disadvantage to both the back vehicle in the current lane and the back vehicle in the new lane are considered. Again, this is done by comparing the acceleration before the lane change with the acceleration after lane change.

To allow for some variability in how aggressive drivers are in deciding when to change lanes, MOBIL weights the other vehicles' disadvantage with a politeness factor,  $p$ . When  $p \geq 1$ , the driver is considerate and puts others' disadvantages equal to or ahead of their own advantage. In reality, most drivers are in the  $0 < p \leq 0.5$  range, where some weight is given to other drivers' disadvantage. If  $p = 0$ , the driver is inconsiderate, discounting the disadvantage to others.

MOBIL also includes a right-lane bias parameter when computing the incentive criterion. This parameter allows for modeling situations in countries where passing a

vehicle on the right is not allowed. The parameter can also be used to allow vehicles to pass from either side or prevent trucks from travelling in the leftmost lanes.

The function *CheckLaneChange* in our *LaneChange* class returns a boolean to indicate if the lane-change can take place or not. *CheckLaneChange* uses the MOBIL equations and suggested parameters along with the statuses of the lane-changing vehicle, the current front vehicle, the new front vehicle, and the new back vehicle. As with our IDM implementation, we have included reasonable default values for each of these parameters. We provide a *Considerate* driver class and an *Inconsiderate* driver class. The user can, of course, create their own driver types with different parameters.

### **A.1.5. Highway**

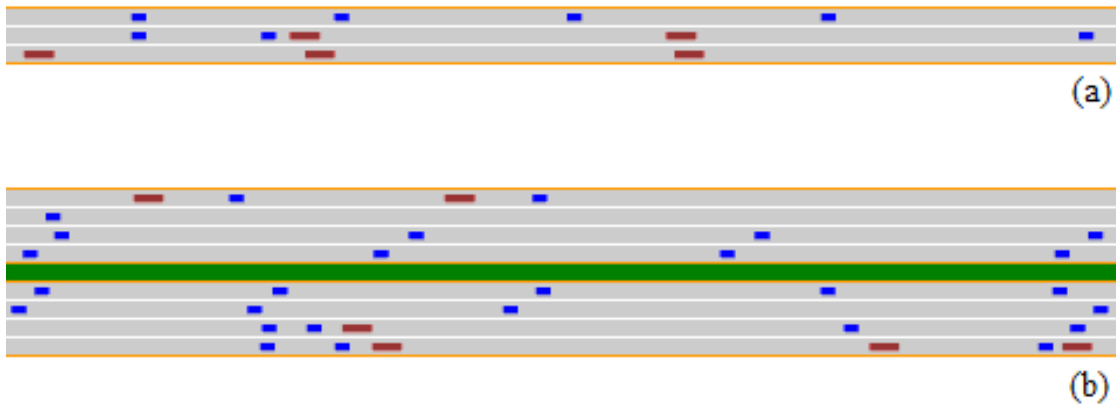
*Highway* is the class that holds *Vehicles* and manages their mobility. We will discuss *Highway*'s physical properties, *Vehicle* management tasks, and how users can control vehicles on the highway in order to customize simulations.

#### **A.1.5.1. Physical Properties**

*Highway* represents a straight highway topology and has the following physical properties:

- *length* – length of the highway in meters (up to 10,000 *m*)
- *number of lanes* – in each direction [1,5]
- *lane width* – in meters
- *median gap* – width of the median, in meters
- *bidirectional* – true if the highway contains two-way traffic, false if the highway is one-way

**Figure 39** shows two example highway configurations. **Figure 39a** is a unidirectional highway with three lanes, and **Figure 39b** is a bidirectional highway with four lanes in each direction.



**Figure 39.** A small segment of a highway. Cars are represented by small rectangles, and trucks are represented by larger rectangles. (a) unidirectional highway with three lanes, (b) bidirectional highway with four lanes in each direction and a separating median.

#### A.1.5.2. Vehicle Management

There are several *Vehicle* management functions that *Highway* performs. *Highway* can automatically create *Vehicle* objects with certain parameters, automatically insert these created objects into lanes, and move each *Vehicle* according to its mobility and lane change models.

**Automatic Creation and Injection of Vehicles:** When the *AutoInjection* parameter of *Highway* is true, *Vehicle* objects will be automatically created and injected onto the



highway. For this purpose, *Highway* creates default mobility models with parameters set appropriately for the standard car and truck, named *SedanModel* and *TruckModel*, respectively. *Highway* also creates default lane change models with appropriate parameters set for cars and trucks. The ratio of cars to trucks that are created is controlled by the *injectionMix* parameter. Automatically-created *Vehicle* objects are provided with default WiFi Phy/Mac settings appropriate for VANETs.

*Highway* stores each lane as a list structure. When a *Vehicle* object is added to *Highway*, it is inserted in its proper place according to its lane, direction, and  $x$  position. For auto-injection, there is a *minGap* parameter that specifies the minimum distance between two vehicles entering the highway. Newly created *Vehicle* objects are not inserted until the  $x$  position of the last *Vehicle* in the lane is at least *minGap* meters from the start of the highway. Vehicles are inserted with a negative  $x$  position, so that the front of the vehicle starts at the start of the highway ( $x = 0$ ) according to user selected flow rate distribution (*i.e.* uniform, exponential, normal, log-normal, Poisson) and velocity distribution (*i.e.*, uniform, exponential, normal, log-normal). Each lane is checked to see if a *Vehicle* can be added, in round-robin fashion, starting with the rightmost lane (lane = 0) in the eastbound direction (direction = 1) and ending with the leftmost lane in the westbound direction (direction = -1, if using bidirectional traffic). Thus, on a bidirectional highway, vehicles are added to both directions considering the flow rate and velocity in each direction.

**Mobility of Vehicles:** Every *DeltaT* seconds, *Highway* calls its *step* function which updates the position, velocity, and acceleration of each *Vehicle* according to its mobility

model. In this way, vehicles with different mobility characteristics (*e.g.*, trucks, emergency vehicles) can be represented on the same highway. *Vehicles* are updated by lane in round-robin fashion, starting with the *Vehicles* in the rightmost lane in the eastbound direction. After the update, if a *Vehicle*'s  $x$  position is greater than the length of the *Highway*, the *Vehicle* is removed from the lane list. After all *Vehicle* positions have been updated, automatic injection of new *Vehicles* occurs.

The opportunity for each vehicle to change lanes is evaluated every  $10 * \Delta T$  seconds to prevent unrealistic lane-changing patterns (*e.g.*, vehicles changing lanes multiple times in less than 1 second). If a vehicle can safely change lanes (according to the *Vehicle*'s MOBIL parameters), *Highway* removes the *Vehicle* from the current lane and adds it to the target lane at the  $x$  position specified according to IDM/MOBIL. When a lane change is allowed, it occurs before mobility is updated, so a *Vehicle* changing lanes only has its mobility updated one time in  $\Delta T$  seconds.

The best case driver reaction time is 0.7 seconds [84]. Vehicle positions should be updated more often than the driver reaction time, so we choose 0.1 seconds for the default value of  $\Delta T$  as a tradeoff between efficiency and accuracy. Reducing  $\Delta T$  (*i.e.*, having the *step* function called more often) will produce a more detailed translation of the position of the vehicle, but will result in a slower simulation (**Figure 40**). Increasing  $\Delta T$  (*i.e.*, having the *step* function called less often) will cause less accuracy in mobility since each step may result in a larger displacement of the vehicles (**Figure 41**).

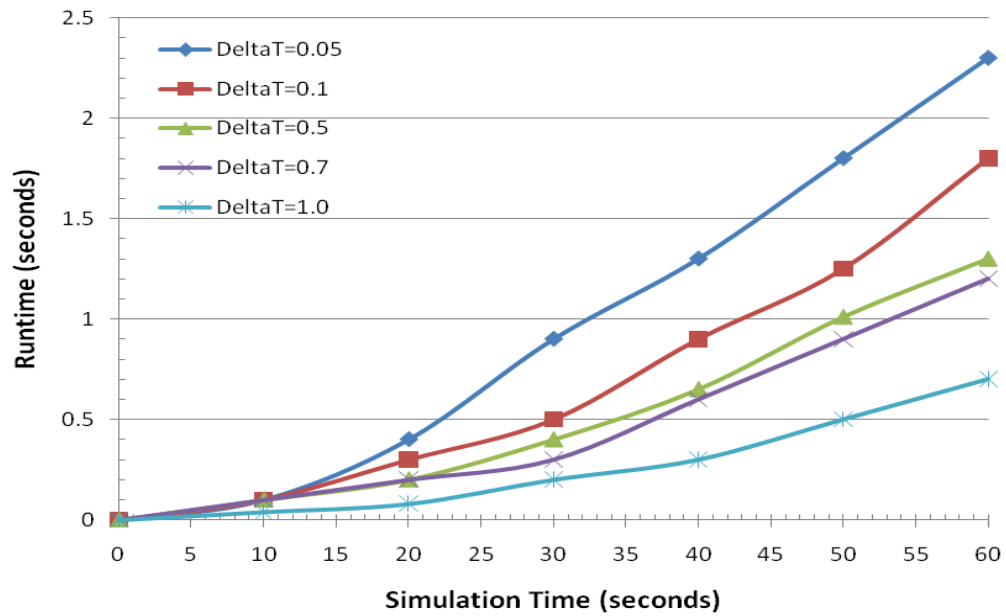


Figure 40. The elapsed real time for 1 minute of dense traffic simulation (average 180 vehicle/km).

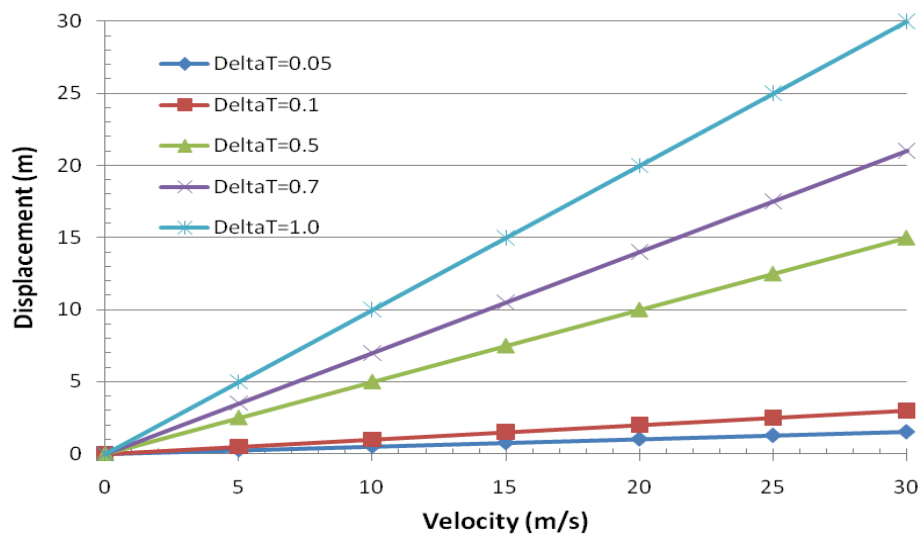


Figure 41. A vehicle's displacement vs. velocity in a single simulation step with different  $\Delta T$  values.

**User Control of Vehicles:** To allow for feedback between the network and the mobility model, there must be a way for the user's application code to interact with individual *Vehicle* objects. *Highway* allows the user to access any *Vehicle* object through its *VehicleID* using *FindVehicle()*. The user can then use this object to change any of the *Vehicle*'s parameters. In addition, *Highway* provides *FindVehiclesInRange()* which returns a list of all *Vehicle* objects within *range* meters of the given *Vehicle*. *FindVehiclesInSegment()* returns a list of all *Vehicle* objects in a particular lane between positions  $x_1$  and  $x_2$ . To access these *Vehicle* objects at particular times, *Highway* triggers several events that can be bound to an event handler created by the user. The events *InitVehicle*, *ControlVehicle*, and *ReceiveData* are discussed below. In addition, there are several other events, such as *DevRxTrace* and *PhyRxErrorTrace*, for the purposes of tracing the communication channel, the PHY/MAC layer, and the behavior of the network devices installed on vehicles.

*InitVehicle* is triggered at *Highway* initialization time. This gives the user the ability to create customized scenarios or modify the initial settings. Although the user can create and position *Vehicle* objects at any time, inside this event handler is the ideal place to create and place initial objects on the highway. If *AutoInjection* is set to true in *Highway*, automatically-created *Vehicles* will move around the previously placed *Vehicles*. The event handler is passed a pointer to the *Highway* and a reference to a *vehicleID* (set to 1 initially). Any manually-created *Vehicles* should use and increment this *vehicleID* so that all objects will have unique IDs. Note that any manually-created *Vehicles* will be controlled by *Highway* according to the *Vehicle*'s mobility model. The event handler

should return *true* if *Vehicles* have been manually added to the *Highway* or default settings have been modified. In this case, *Highway* will sort the lane lists based on the *Vehicle* positions. If no *Vehicles* have been added, there is no reason to sort the lists, so the event handler should return *false*.

For each *Vehicle*, *ControlVehicle* is triggered by the *step* function, which is executed every *DeltaT* seconds. In this way, the user has full control of each *Vehicle* at each time step. For example, a particular *Vehicle* could be made to decelerate or stop in order to create traffic congestion. In addition, this event handler is an ideal place to output the locations of all *Vehicles* in order to produce traffic visualizations. The event handler is passed a pointer to the *Highway*, a pointer to the particular *Vehicle*, and the value of *DeltaT*. If the event handler has changed the *Vehicle*'s position, it will return *true*, so that the *Vehicle*'s acceleration will not be updated by the mobility model. Otherwise, the event handler will return *false* so that *Highway* will adjust the *Vehicle*'s position according to its mobility model.

*ReceiveData* is triggered when any *Vehicle* successfully receives data from the network. The event handler is passed a pointer to the *Vehicle* that received the data, a pointer to the data packet, and the address of the packet's sender.

## A.2. CUSTOMIZATIONS

We provide a basic framework for a straight highway scenario and tools for generating communicating vehicles traveling with a realistic mobility. There are many possible customizations that can be made using this framework. We describe a few customizations that can be made with *Vehicle* objects.

Any *Vehicle* can be associated with a parameterized mobility or lane-change model. This allows the user to create simulations that contain various types of vehicles. For example, a police car is a vehicle that during a chase has a higher desired speed and acceleration than a normal vehicle. In addition, the user could set the networking parameters such that the police car also has a more powerful transceiver than a normal vehicle. In another instance, a helicopter used to transmit advertisements, warnings, or reports could be simulated as a *Vehicle* with a positive  $z$  value (altitude). Since the helicopter does not travel on the highway, it should not be added to or managed by *Highway*. Instead, at every time step (*i.e.*, in the *ControlVehicle* event handler), the helicopter's position should be updated manually.

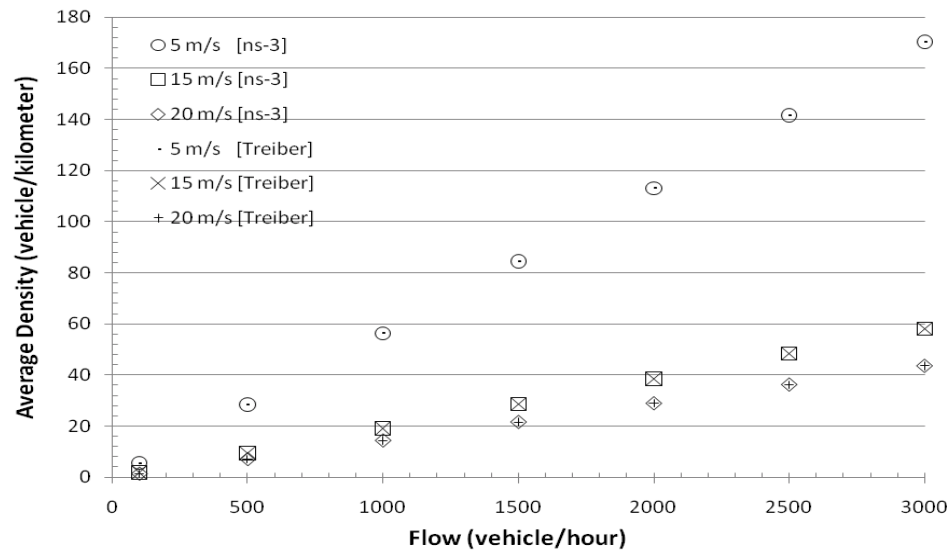
Stationary roadside units, such as digital guides, placed outside the highway can be created using *Obstacles*. As with *Vehicles* that are outside the highway, these devices would should not be added to *Highway*. As another example, a gantry on top of a highway could be represented as an *Obstacle* with a positive  $z$  value. Sensors under the road could be *Obstacles* with negative  $z$  values. These devices may have different communications requirements than standard vehicles, so the user is free to adjust the network parameters as well.

### A.3. VALIDATION

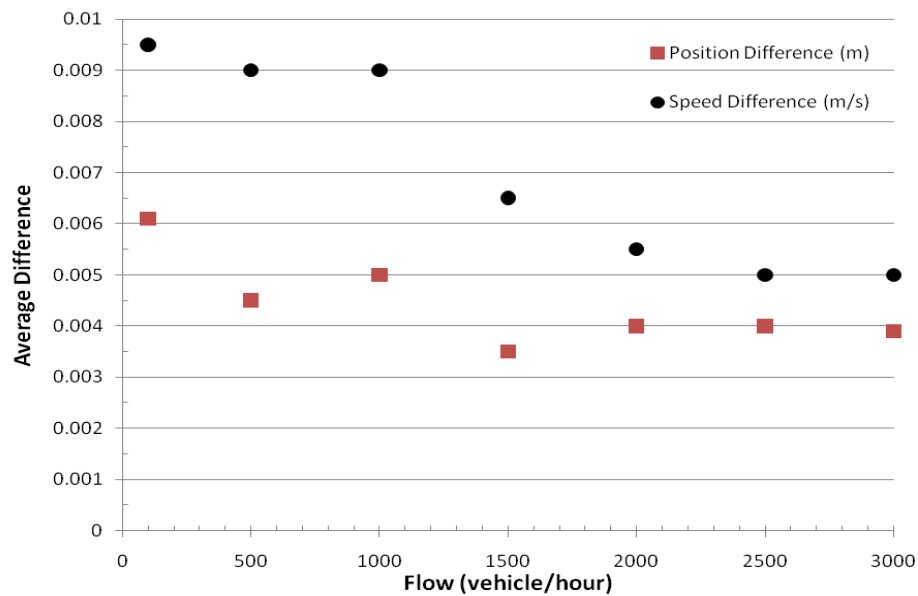
We validate our implementation of IDM/MOBIL in ns-3 against Treiber's own implementation of IDM/MOBIL in a Java applet [82, 83]. The first step is to validate that the functions *Model::CalculateAcceleration()* and *LaneChange::CheckLaneChange()* produce output correctly in comparison with Treiber's formula, model, and code

individually with various input and mobility model settings. The second step is to produce simple traffic in a one lane roadway and compare the vehicle's acceleration, deceleration, velocity, and position at each simulation interval. Finally, we need to show that despite the difference in our design and the logic of *step* function, we are able to create traffic similar to that created by Treiber's applet.

The first two steps have been performed during code implementation and testing. Here we show the results of the third step of validation. We use Treiber's Java applet to produce traffic on a straight two lane roadway for several traffic inflow rates. We record traffic statistics (simulation time, vehicle type, acceleration, velocity, position, and lane) at two points. Point *A* is the roadway entrance, and point *B* is 500 *m* from the entrance. We apply the generated traffic recorded at point *A* in Treiber's applet to our ns-3 simulation and record the traffic statistics at point *B*. This is to mitigate the different injection models used by Treiber's applet and our code. We compare the traffic at point *B* in Treiber's applet with the traffic at point *B* in our ns-3 code during a 5 minute simulation. **Figure 42** shows the average traffic density over the 500 *m* as the traffic inflow rate increases and with different desired speeds. The results between the two applications are almost identical. **Figure 43** shows the average differences in position and speed between the two applications for each vehicle as it passes point *B*. Again, there is very little difference between the two. The position differences are less than 7 *mm*, and the speed differences are less than 1 *cm/s*.



**Figure 42.** Comparison between average density results of our code in ns-3 and Java applet for different traffic inflow and different desired velocity.



**Figure 43.** Average difference in position ( $m$ ) and average speed ( $m/s$ ) between ns-3 version and Treiber Java applet for different traffic densities.

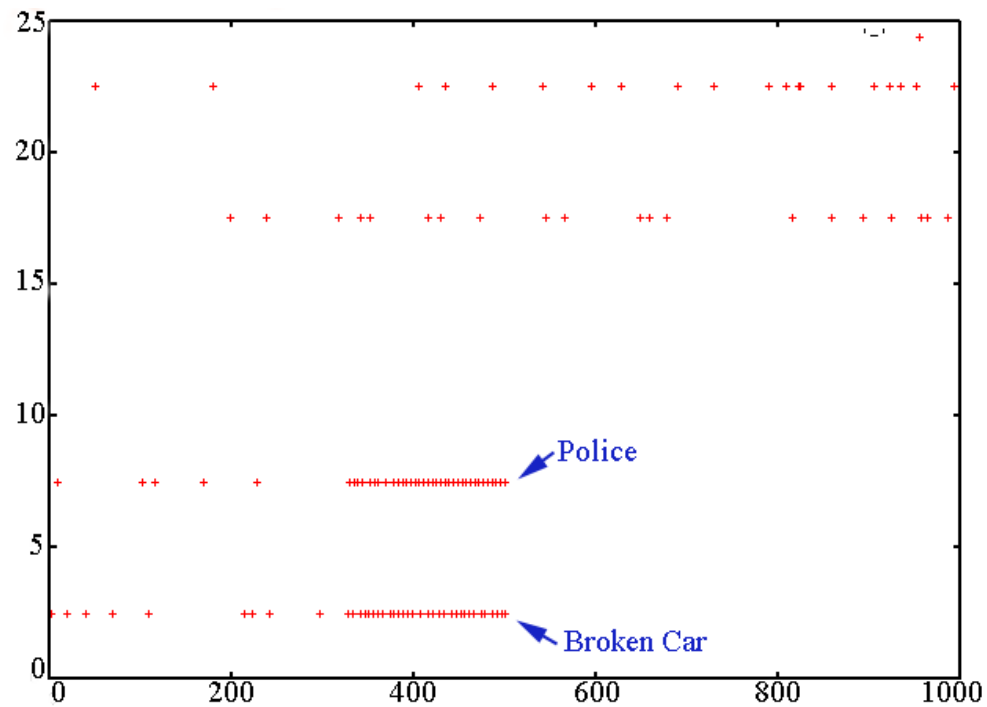


#### A.4. EXAMPLE

We have provided an example to show how to create a customized highway, set parameters, handle events, and control which vehicles send and receive customized messages. This example, also available as the part of our open source project [85], demonstrates how a user can have full control of events to produce the desired scenarios and experiments. The example generates output suitable for plotting vehicle positions using *gnuplot* or other graph-plotting tool.

We have created a *Controller* class to handle events and create special vehicles. The highway is a bidirectional 1 *km* roadway with two lanes in each direction. The lane width and median width are both 5 meters. The sedan-truck mixture is 80%, so 80% of vehicles are sedans and 20% are trucks. Automatically-generated vehicles will enter and be injected to the highway with at least a 10 meter gap. We place a broken car (*Obstacle* object) in the middle of the highway (lane=0, direction=1, x=500) which broadcasts a safety message revealing its location and asking for help every 5 seconds. We also create a police car with a *VehicleID* of 2. The police car is faster than a normal car and has a higher wireless transmission range. It listens for messages and unicasts a reply for each received request. The police car will decelerate when it reaches the broken car and will eventually stop nearby.

The generated output points can be directed to *gnuplot* to be plotted and animated. **Figure 44** shows the *gnuplot* snapshot after 2 minutes and 40 seconds of the simulation. The police car reached the broken car at 500 meters after 20 seconds and stopped in the second lane, causing congestion.



**Figure 44.** A sample plotted highway output for a 1000 *m* roadway with two lanes in each direction. This snapshot is taken at time 2 minutes, 40 seconds. The police car has stopped in the lane next to the broken car at time 20 seconds, causing the congestion behind it.

Below, we show a skeleton of a *Controller* class and *main()* function. Comments that are shown in italics are placeholders for user-defined code.

### Controller.h

```
class Controller : public Object
{
private:
    Ptr<Highway> m_highway;
    // other local variables
public:
    Controller();
    Controller(Ptr<Highway> highway);
    // event handlers
```

```

        bool InitVehicle (Ptr<Highway> highway, int& vehicleID);
        bool ControlVehicle (Ptr<Highway> highway, Ptr<Vehicle>
vehicle, double dt);
        void ReceiveData (Ptr<Vehicle> veh, Ptr<const Packet>
pckt, Address addr);
        // other function declarations
};

```

### Controller.cc

```

Controller::Controller() {}
Controller::Controller(Ptr<Highway> highway) {m_highway =
highway;}

bool Controller::InitVehicle(Ptr<Highway> highway, int&
vehicleID)
{
    // objects to create, settings to change at highway
initialization time
    return true; // let Highway sort vehicles in highway lanes
}

bool Controller::ControlVehicle(Ptr<Highway> hw, Ptr<Vehicle>
veh, double dt)
{
    // actions that should occur each time this vehicle's
mobility is updated
    return false; // let Highway manage the mobility of this
vehicle
}

void Controller::ReceiveData(Ptr<Vehicle> veh, Ptr<const
Packet> pckt, Address addr)
{
    // actions that should occur each time a message is
received by this vehicle
}

```

### main()

```

int main (int argc, char *argv[])
{
    // Create Highway and Controller
    Ptr<Highway> highway = CreateObject<Highway>();
    Ptr<Controller> controller =
CreateObject<Controller>(highway);
}

```

```

    // Set highway parameters

    // Bind highway events to event handlers in controller
    highway-
>SetInitVehicleCallback(MakeCallback(&Controller::InitVehicle,
controller);
    highway-
>SetControlVehicleCallback(MakeCallback(&Controller::ControlVehicle,
controller);
    highway-
>SetReceiveDataCallback(MakeCallback(&Controller::ReceiveData,
controller);

    // Schedule the highway and run the simulation
    Simulator::Schedule(Seconds(0.0), &Start, highway);
    Simulator::Schedule(Seconds(100.0), &Stop, highway);
    Simulator::Stop(Seconds(100.00));
    Simulator::Run();
    Simulator::Destroy();
    return 0;
}

```

## A.5. SUMMARY

In this appendix, we described the first implementation of a vehicular mobility model integrated with the networking functions in ns-3. Integrated VANET simulators that include both mobility and network models are essential, allowing network communications to affect vehicle mobility, which is one of the main goals of future VANET deployments (*e.g.*, network messages may prompt drivers to slow down early or to take an alternate route). Our implementation allows for this feedback by triggering an event each time a network message is received and each time vehicle mobility is updated. User-created event handlers can then send network messages or alter the mobility of the vehicle in response to the triggered event. These features can facilitate more detailed simulations of VANETs.

Realistic vehicle mobility is achieved through the validated implementation of the IDM car-following model and the MOBIL lane-change model. We introduced the *Highway* class, which not only simulates a straight roadway, but also manages the mobility of all vehicles on the highway. Our implementation also allows the user to take advantage of automatically created and inserted vehicles or to manually insert vehicles at any point along the highway. In addition, our implementation allows for the customization of almost all aspects of the simulation so that the research can study a wide variety of scenarios (*e.g.*, DTMon).

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Hadi Arbabi was a top-ranked student in the fields of mathematics and physics at Dr. Hashroudi College, class of 1997. He received his Bachelors Degree in Computer Engineering from Shiraz University in 2001. He was also recognized as the top-ranked student of the class of 2001. In Fall 2005, he joined the masters program in the Department of Computer Science at Old Dominion University. He wrote his thesis in the area of cellular networks under the supervision of Dr. Stephan Olariu. Hadi received his Masters Degree in Computer Science in Summer 2007. He joined the PhD program in the same department in the same year and decided to pursue his PhD under the supervision of Dr. Michele C. Weigle. Hadi had been active in the areas of simulation/modeling, programming languages, wireless networks, sensor networks, vehicular networks, and intelligent transportation systems. With interests in data mining and simulation/modeling, Hadi's PhD focus, including his dissertation, was on vehicular ad-hoc networks. Specifically, his focus was the application of these networks to dynamically monitoring roadway traffic.