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Consequences of Changing Climate and Land Use to 100-Year Flooding

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Consequences of Changing Climate and Land Use to 100-Year Flooding

5th Annual Lamprey River Symposium

January 6, 2012

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Cameron Wake & Fay Rubin, Earth, Oceans and Space, UNH

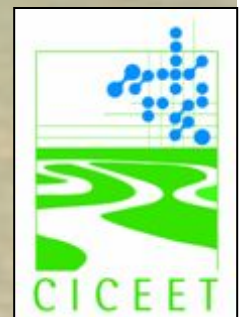
Steve Miller, Great Bay National Estuarine Research Reserve

Michael Simpson, Antioch University New England

Lisa Townson and **Julia Peterson**, UNH Cooperative Extension

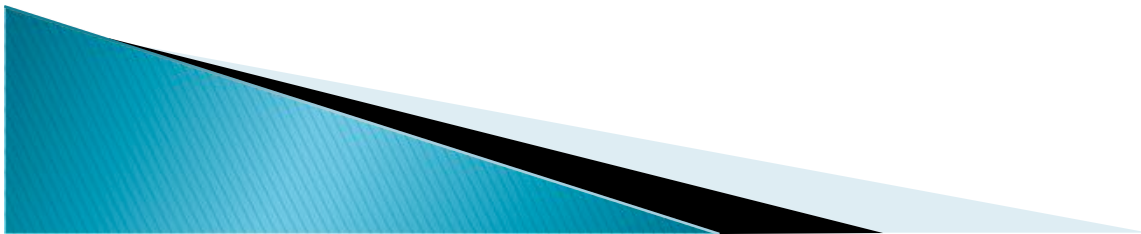
Cliff Sinnott, Rockingham Planning Commission

Funding by NOAA Cooperative Institute for
Coastal and Estuarine Environmental Technology

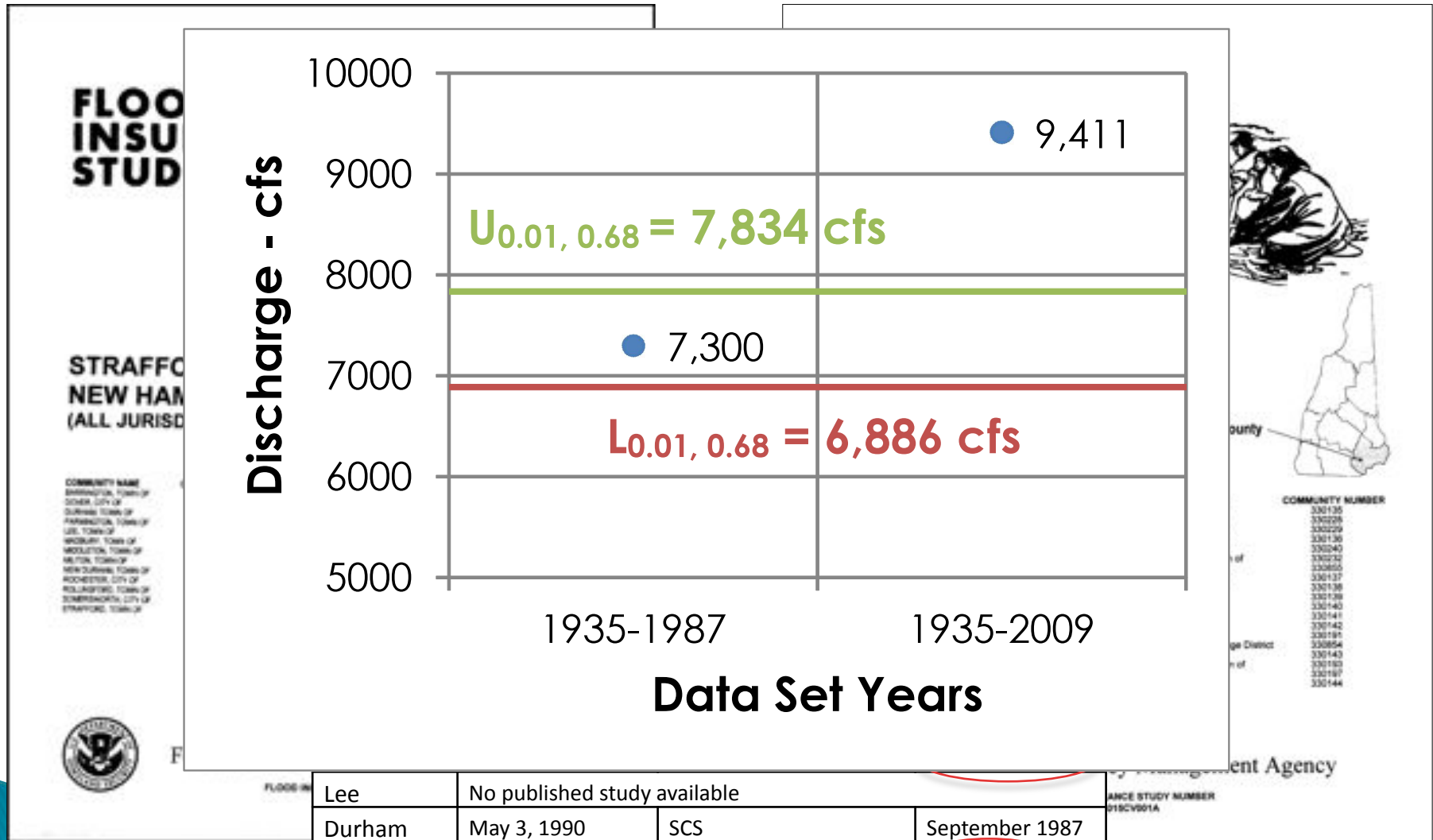


Project Objectives

- ▶ Update hydrologic and hydraulic model for the watershed and river respectively based on current climate and land use
- ▶ Update base flood elevations and flood risk areas
- ▶ Assess flood risk associated with combined future development and climate change
- ▶ Engage municipal and regional planners in project and discuss land use management strategies to mitigate runoff volumes



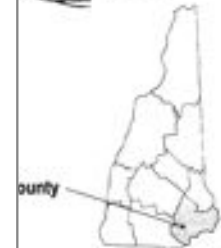
Lamprey River Studies



FLOOD INSURANCE STUDY

STRAFFORD COUNTY NEW HAMPSHIRE (ALL JURISDICTIONS)

COMMUNITY NAME
 BARRINGTON, TOWN OF
 BOW, CITY OF
 DURHAM, TOWN OF
 FARRINGTON, TOWN OF
 LISI, TOWN OF
 MERRIMACK, TOWN OF
 MILTON, TOWN OF
 NEW DURHAM, TOWN OF
 ROCHESTER, CITY OF
 ROLLINGFORD, TOWN OF
 STRAFFORD, CITY OF
 STRAFFORD, TOWN OF

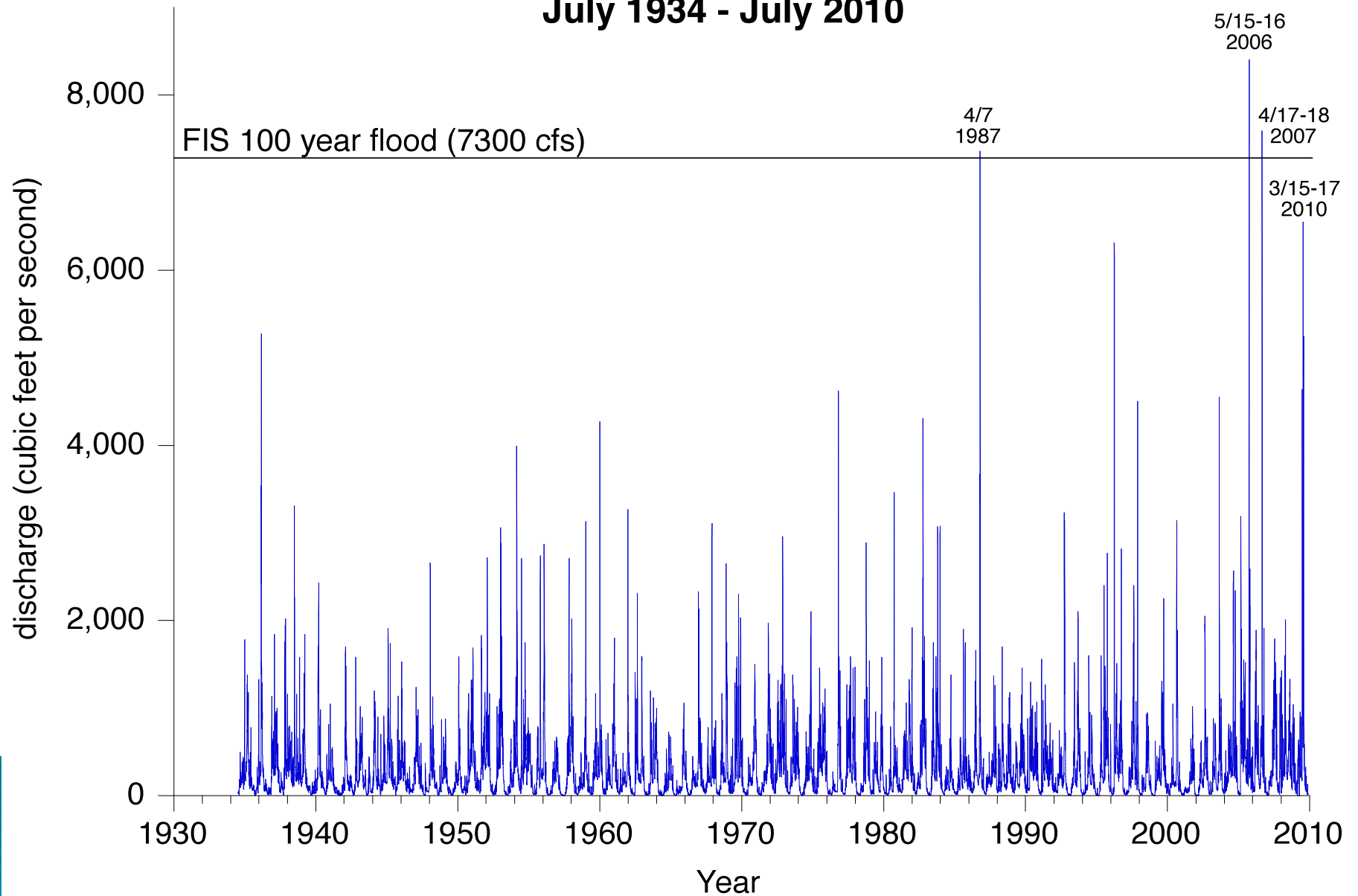


COMMUNITY NUMBER
 330135
 330226
 330229
 330136
 330240
 330232
 330850
 330137
 330138
 330139
 330140
 330141
 330142
 330191
 330854
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 330197
 330144

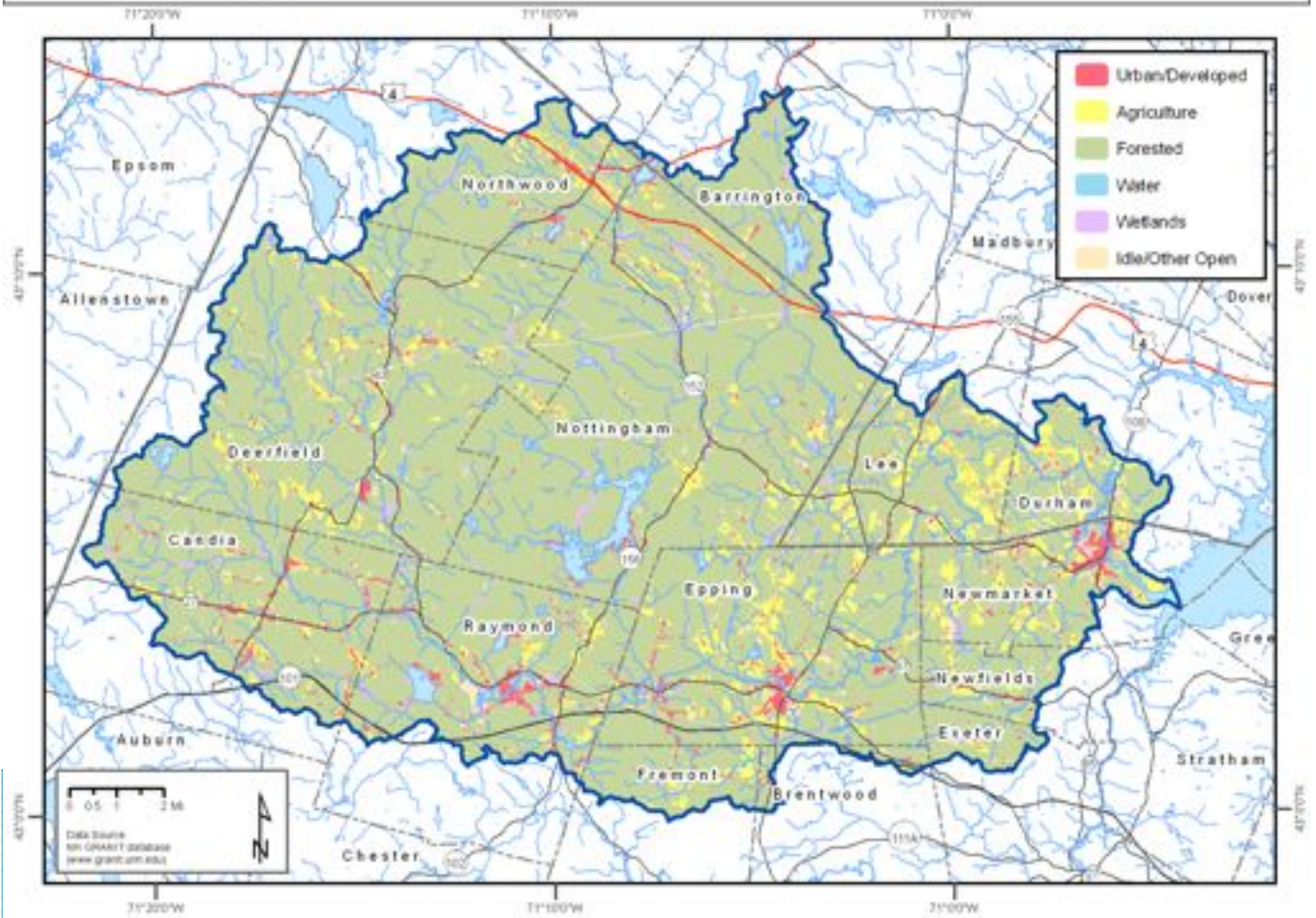
Lee	No published study available		
Durham	May 3, 1990	SCS	September 1987
	August 23, 2001	USGS	April 1998
Newmarket	May 2, 1991	USGS	August 1989

Assessing Flood Risk - Lamprey River Watershed

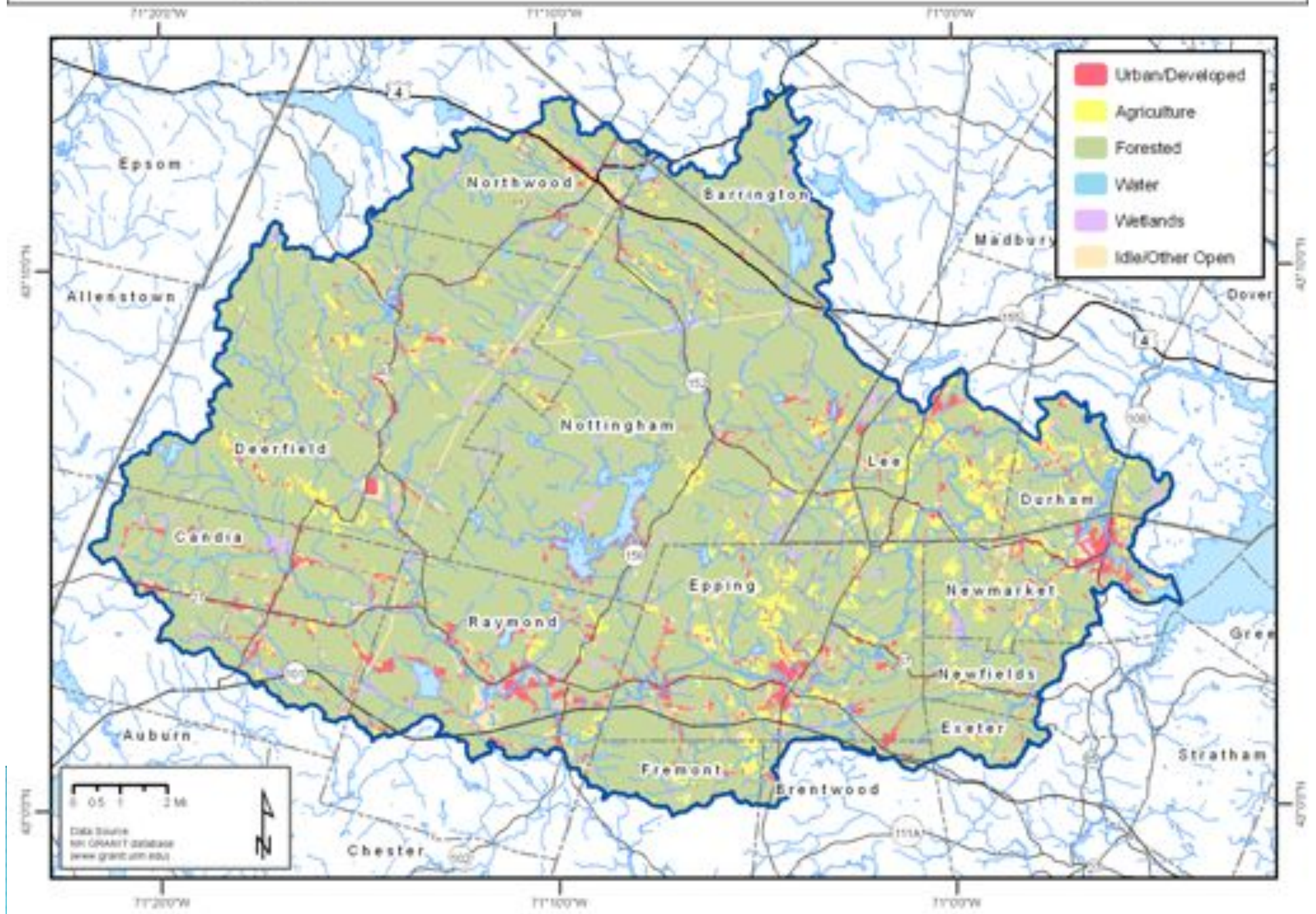
Daily Discharge for Lamprey River near Newmarket, NH July 1934 - July 2010



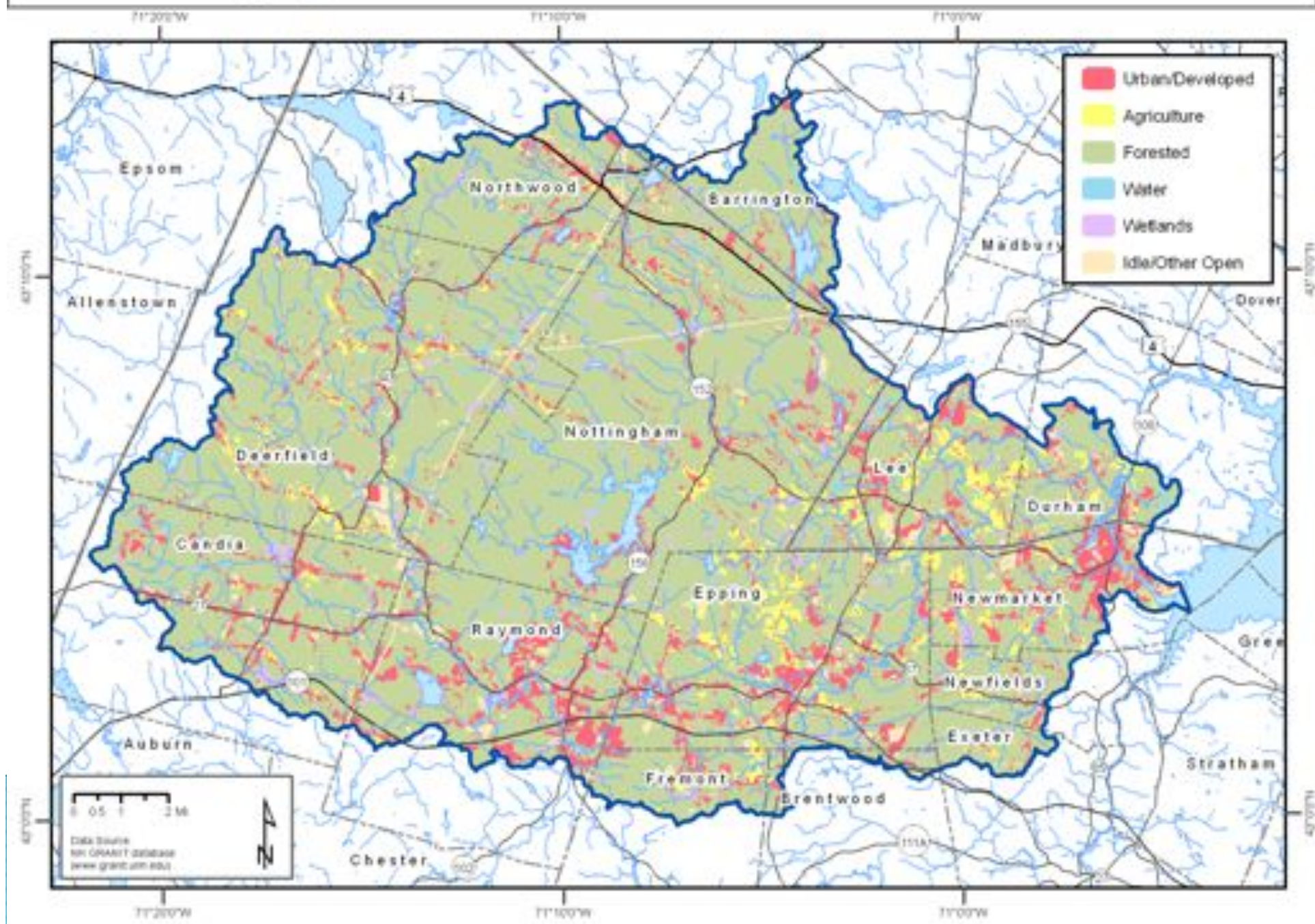
Lamprey River Watershed Generalized Land Use - 1962



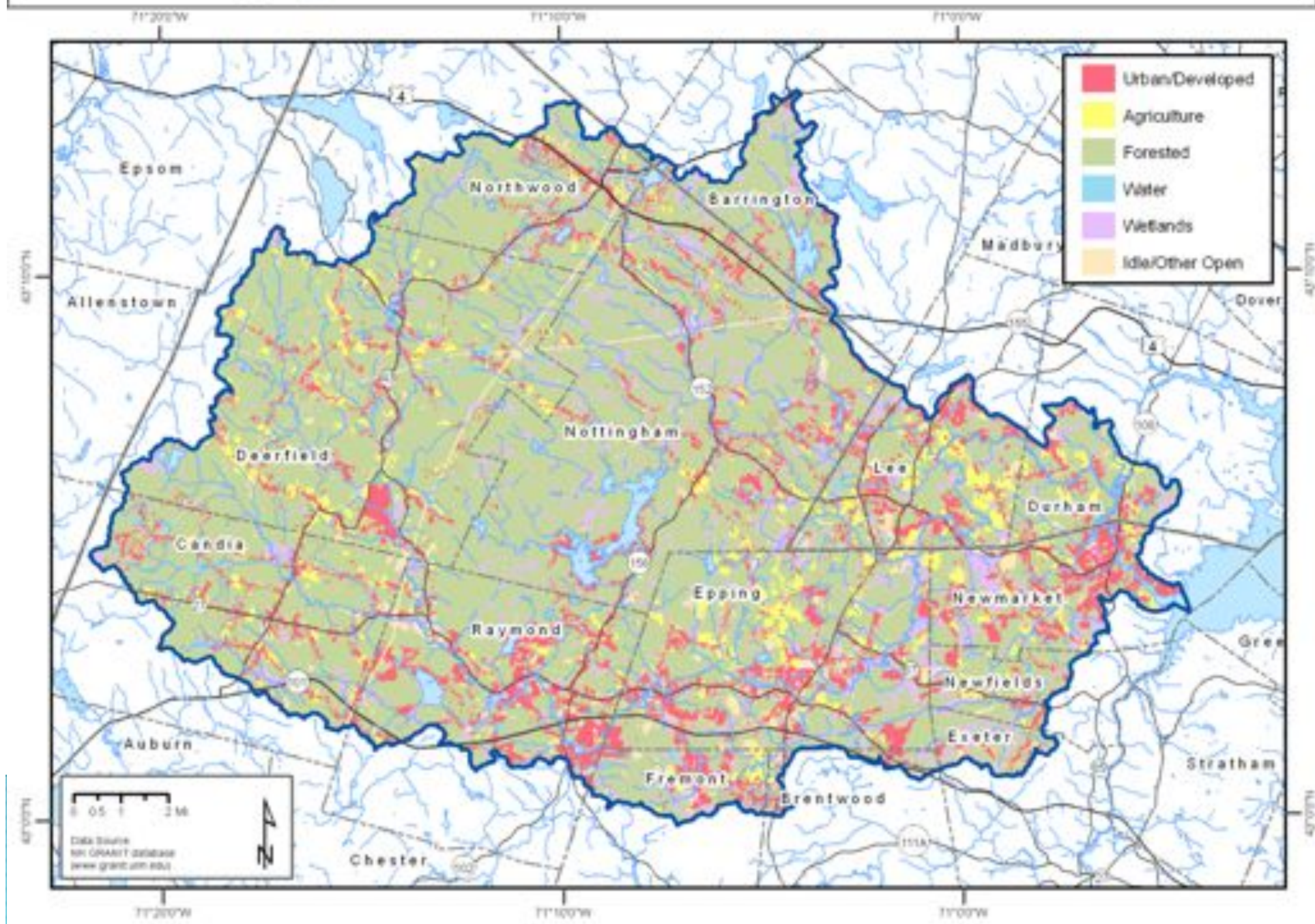
Lamprey River Watershed Generalized Land Use - 1974

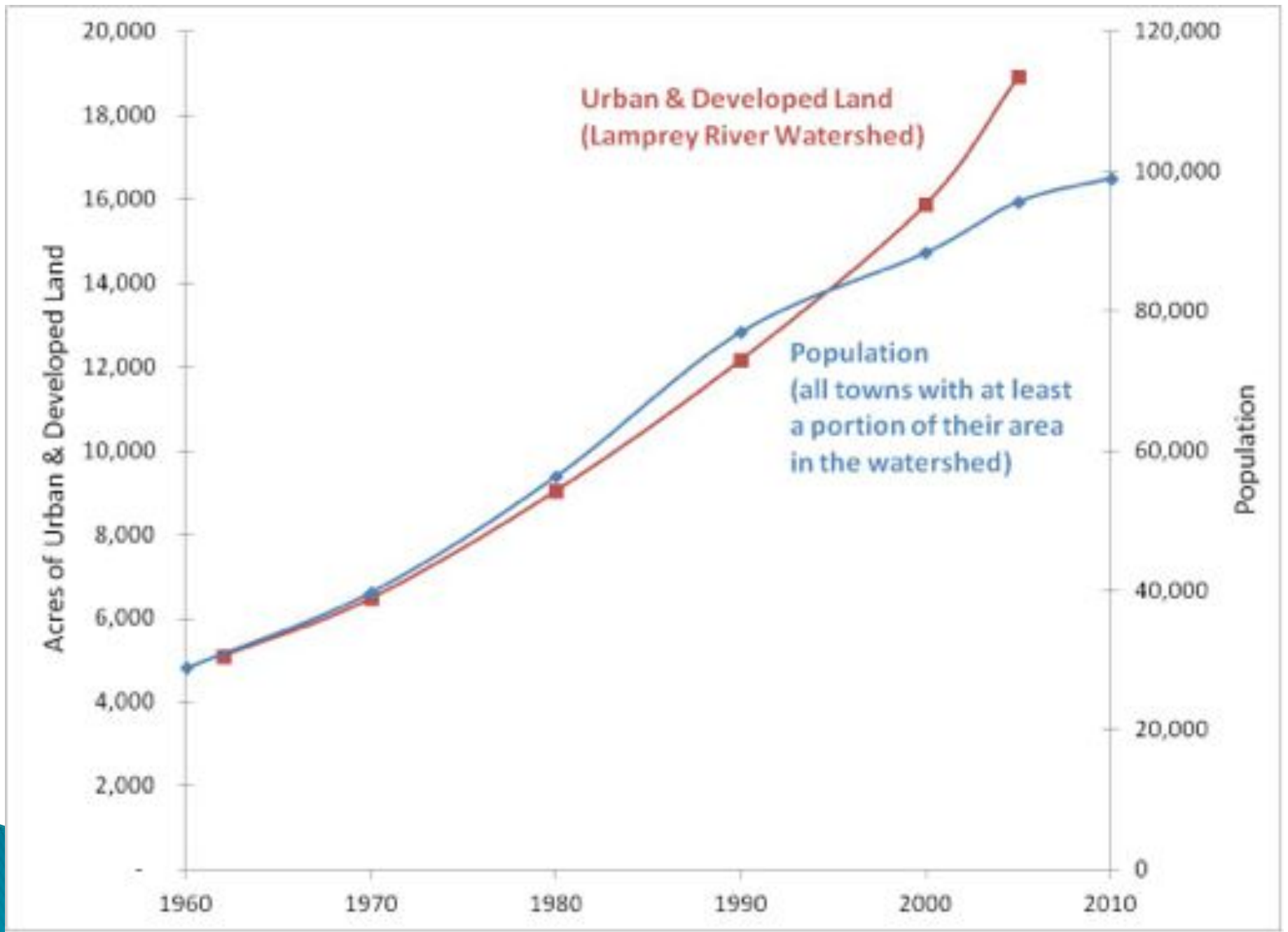


Lamprey River Watershed Generalized Land Use - 1998

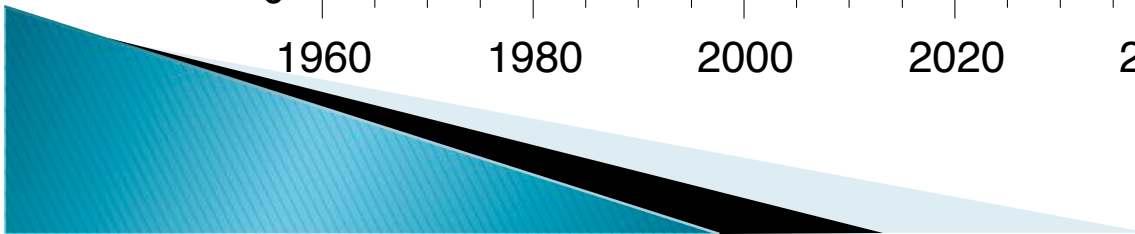
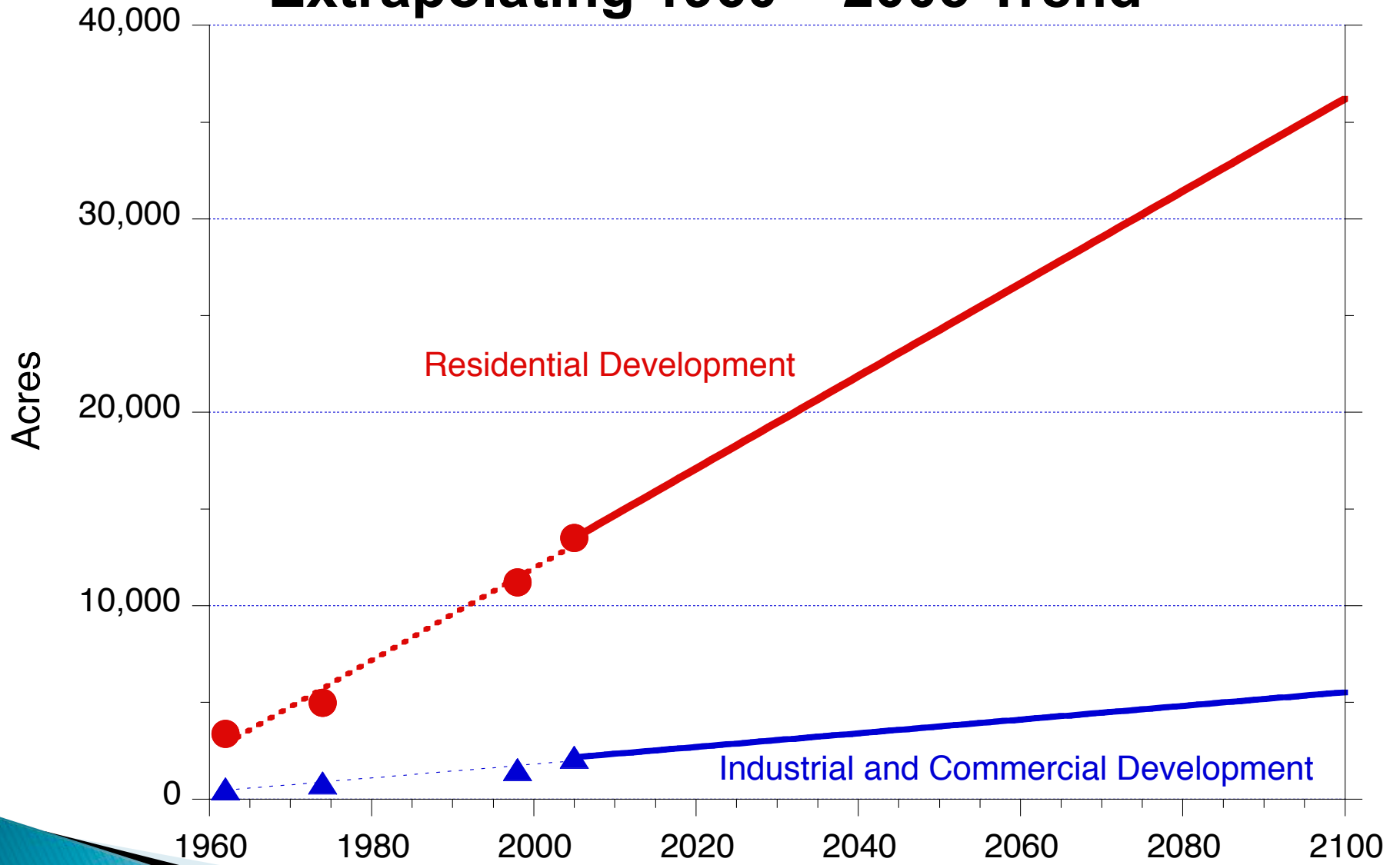


Lamprey River Watershed Generalized Land Use - 2005

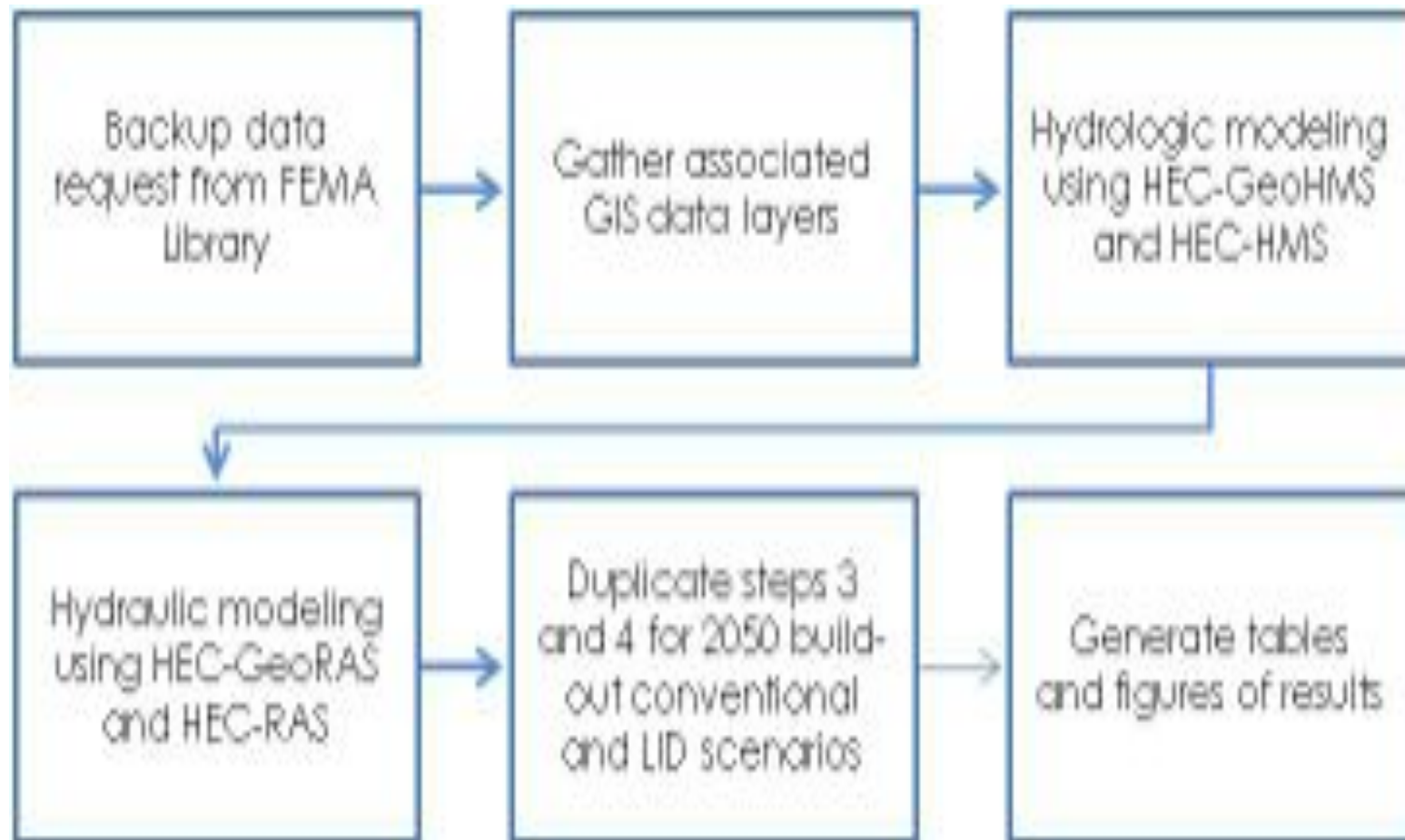




Lamprey River Watershed Buildout Extrapolating 1960 – 2005 Trend



Project overview



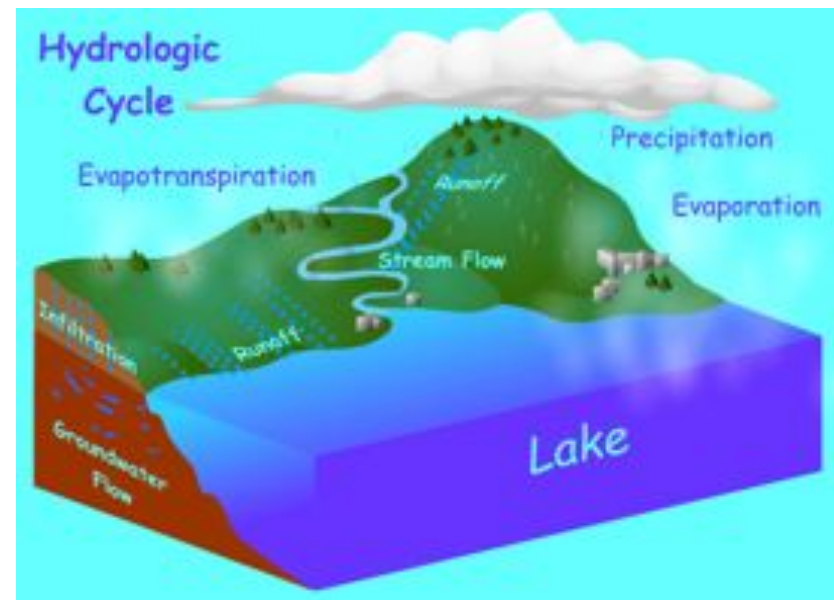
Hydrologic Methodology

▶ FIS:

- Annual peak flow frequency analysis
 - peak annual stream flow
 - standard deviation
 - weighted coefficient of skewness.

▶ Current Project

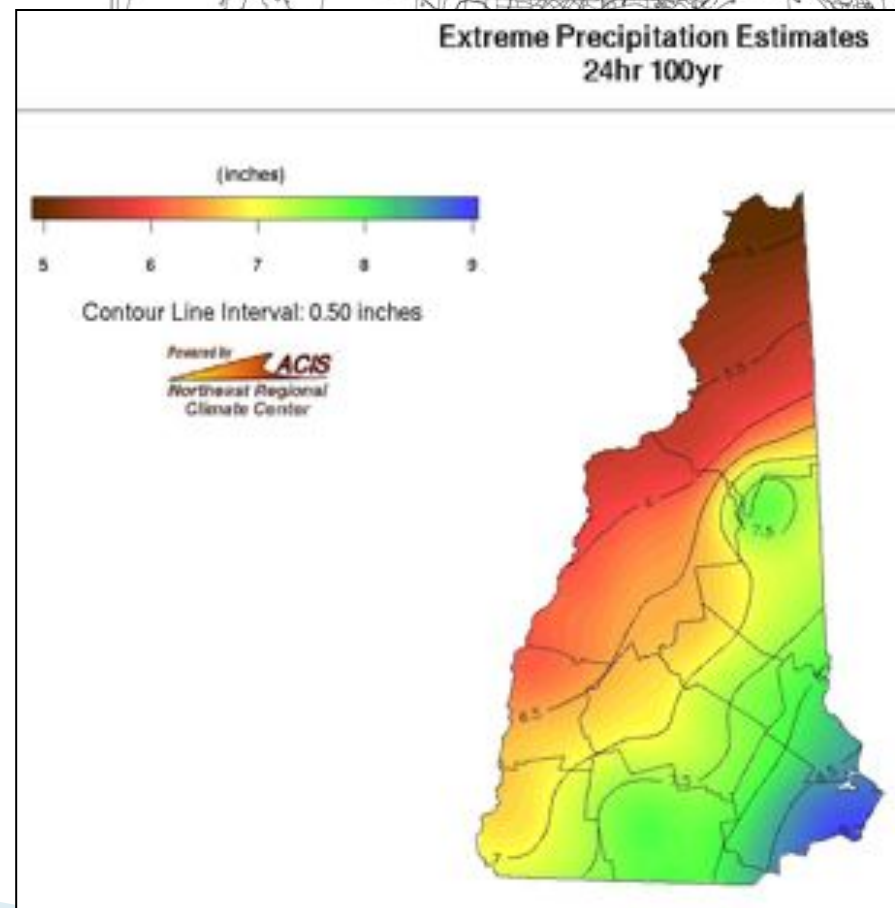
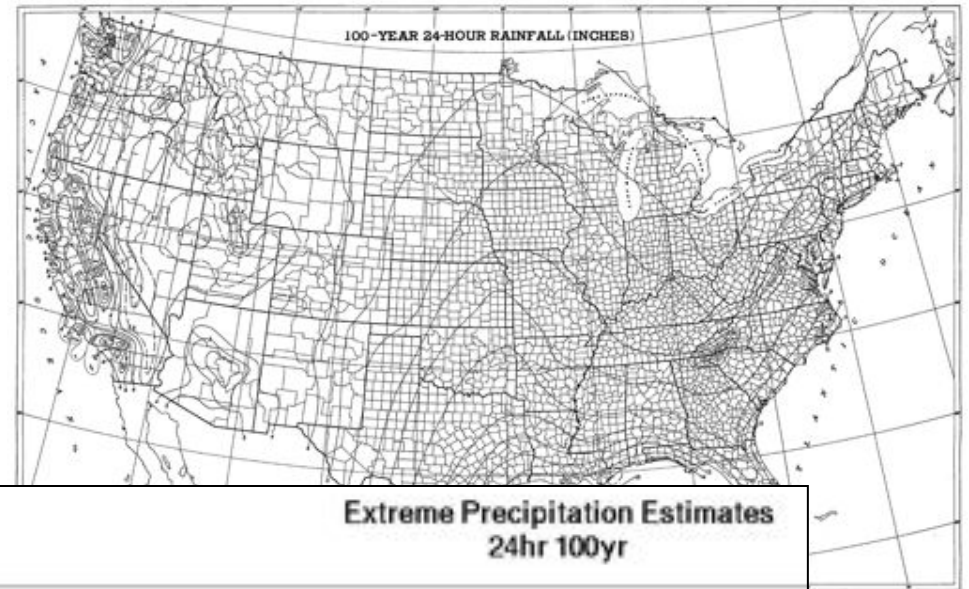
- Rainfall–Runoff
 - Watershed area
 - Time of concentration
 - Runoff curve number (CN)



Precipitation

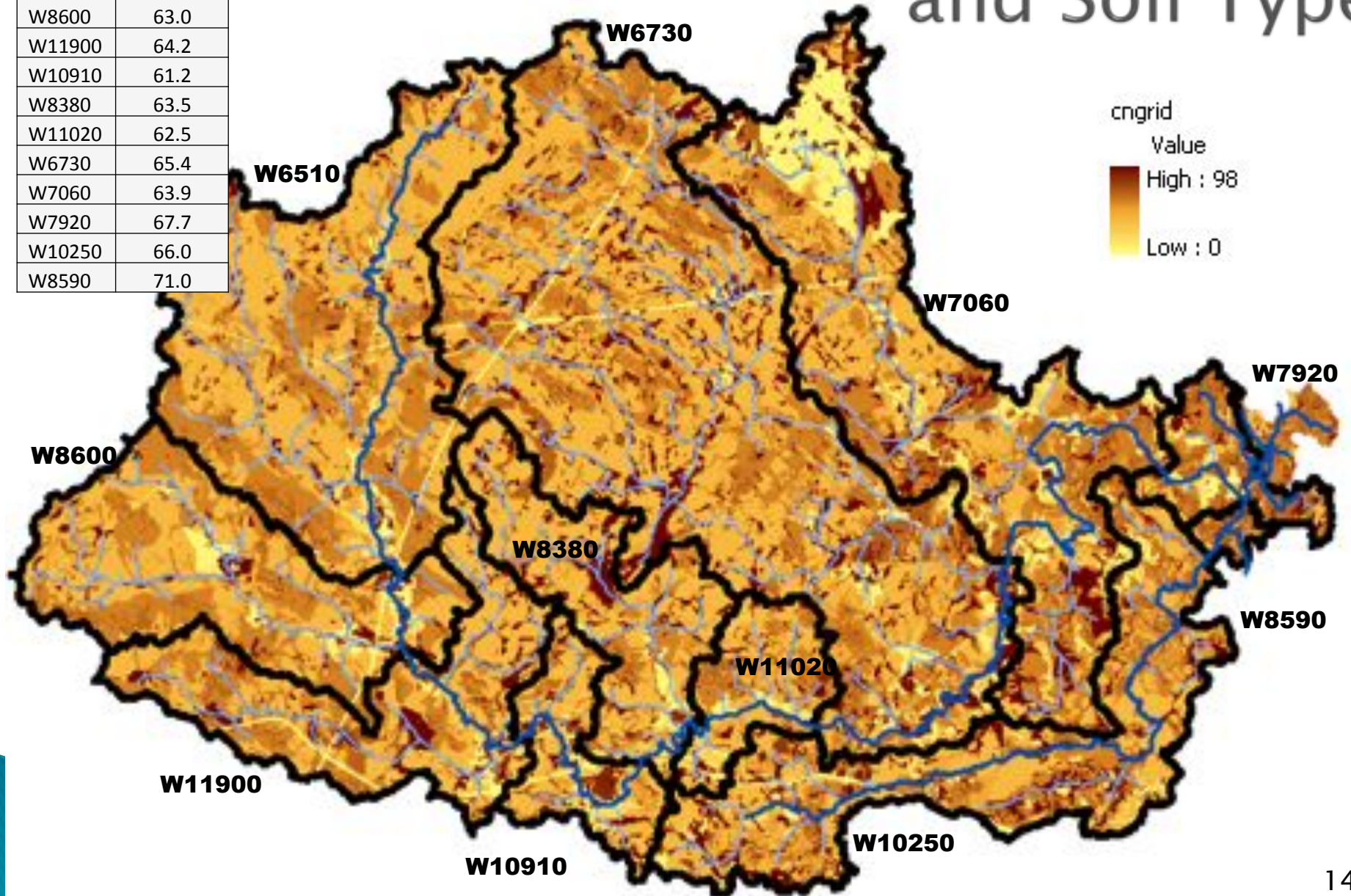
- Rainfall

- Depth–duration–frequency
 - TP-40 Rainfall Frequency Atlas of the United States is being used for effective conditions
 - Northeast Regional Climate Center (NRCC) Atlas for Extreme Precipitation. FEMA guidelines for re-delineation indicate the need to use current depth duration frequency data.

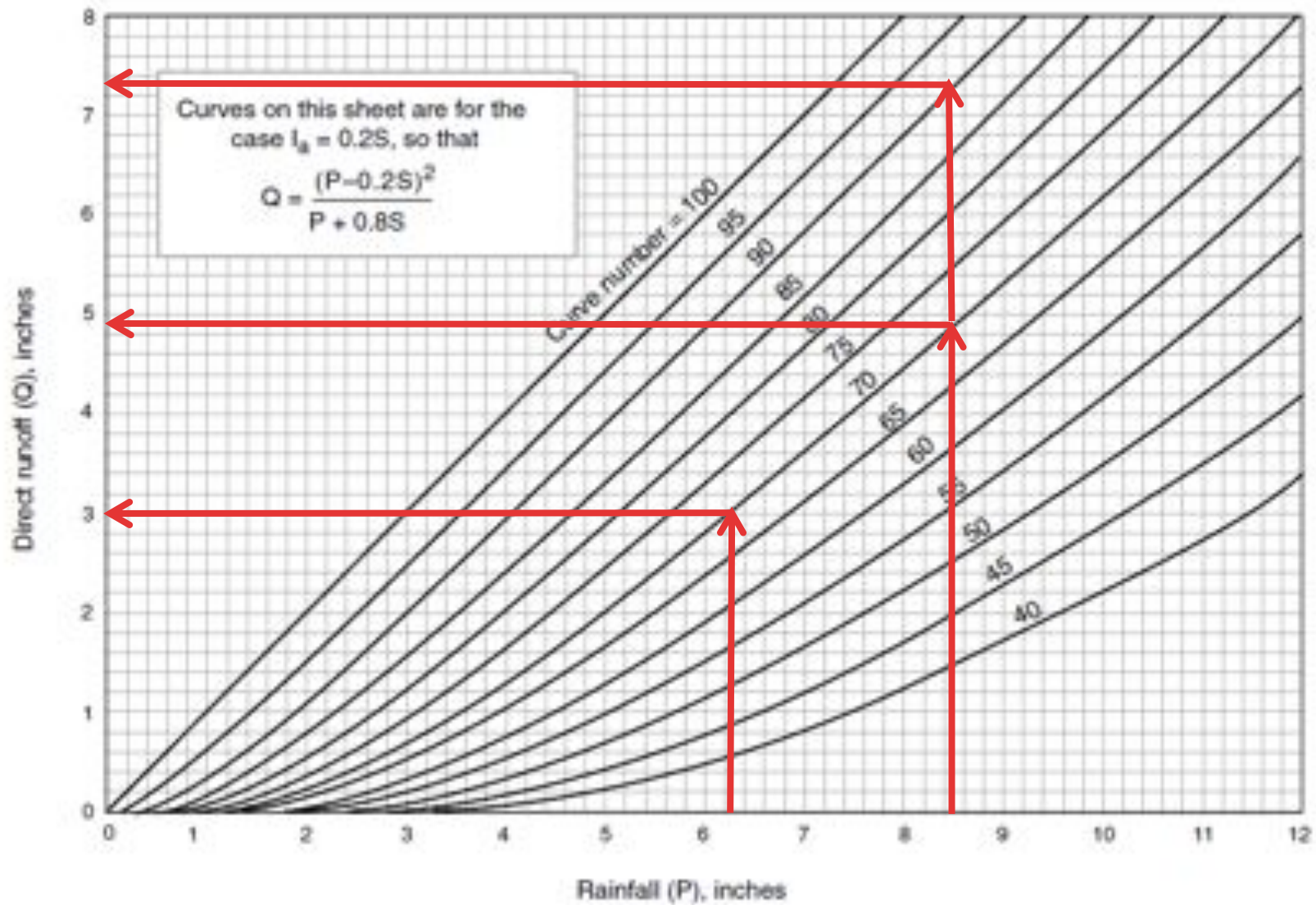


Curve Number based on Land Use and Soil Type

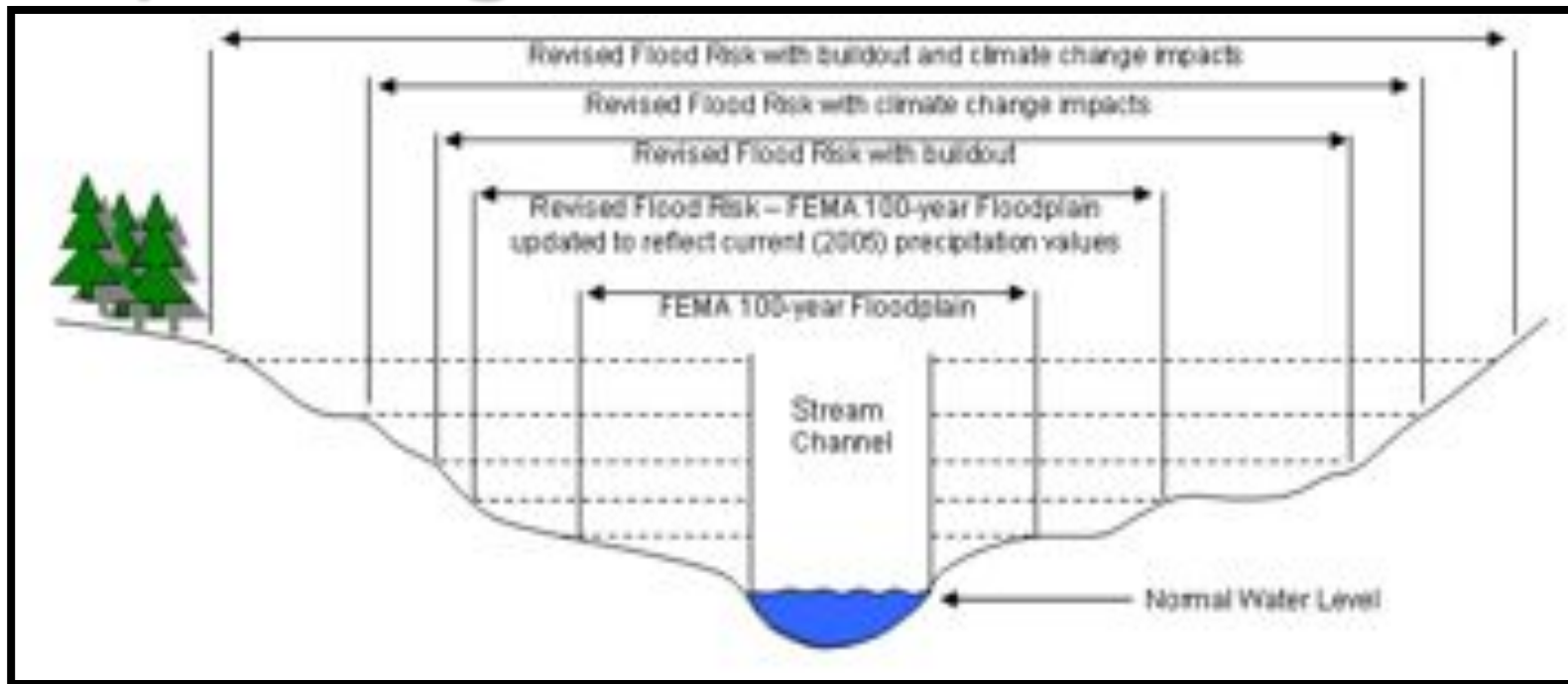
Subbasin	CN
W6510	62.5
W8600	63.0
W11900	64.2
W10910	61.2
W8380	63.5
W11020	62.5
W6730	65.4
W7060	63.9
W7920	67.7
W10250	66.0
W8590	71.0



Rainfall - Runoff Equation

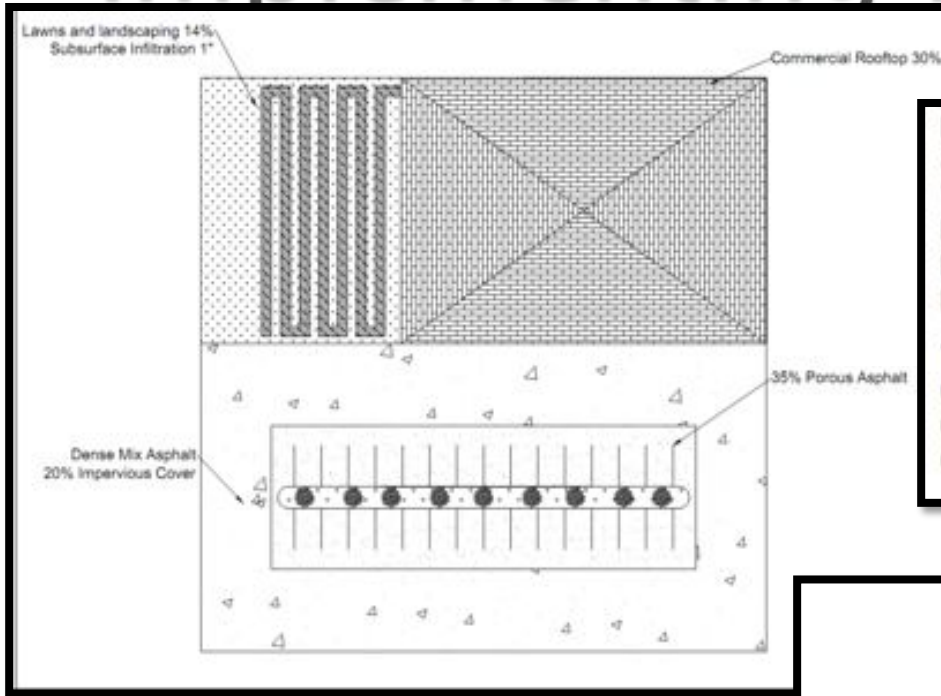


Hydrologic Model Scenarios



Project Models			
Land Use Conditions	Rainfall Rates and Global Change Model Scenario		
	Rainfall Atlas		Climate Period 2035-2069
	TP-40	NRCC	RCM
	6.3 in/24 hr	8.5 in/24 hr	8.5 in/24 hr
2005 Current	X	X	
2050 Build-out			X
2050 LID/Build-out			X

Implementing LID



Commercial Zoning

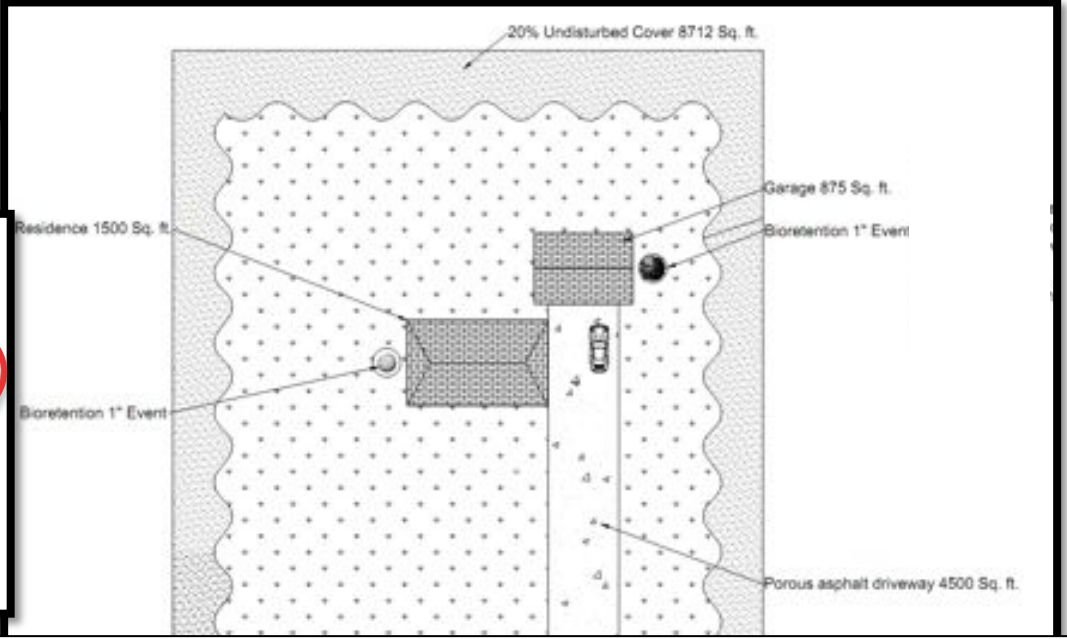
Notes:
 85% IC 0% EIC
 WQV = 1"
 Rooftop Infiltration
 Porous asphalt for parking only
 Bioretention for drive lanes of standard asphalt

Type	A	B	C	D
CN Predevelopment	38	55	70	77
CN Conventional	84	89	92	94
CN LID	64	73	80	83

One Acre Residential

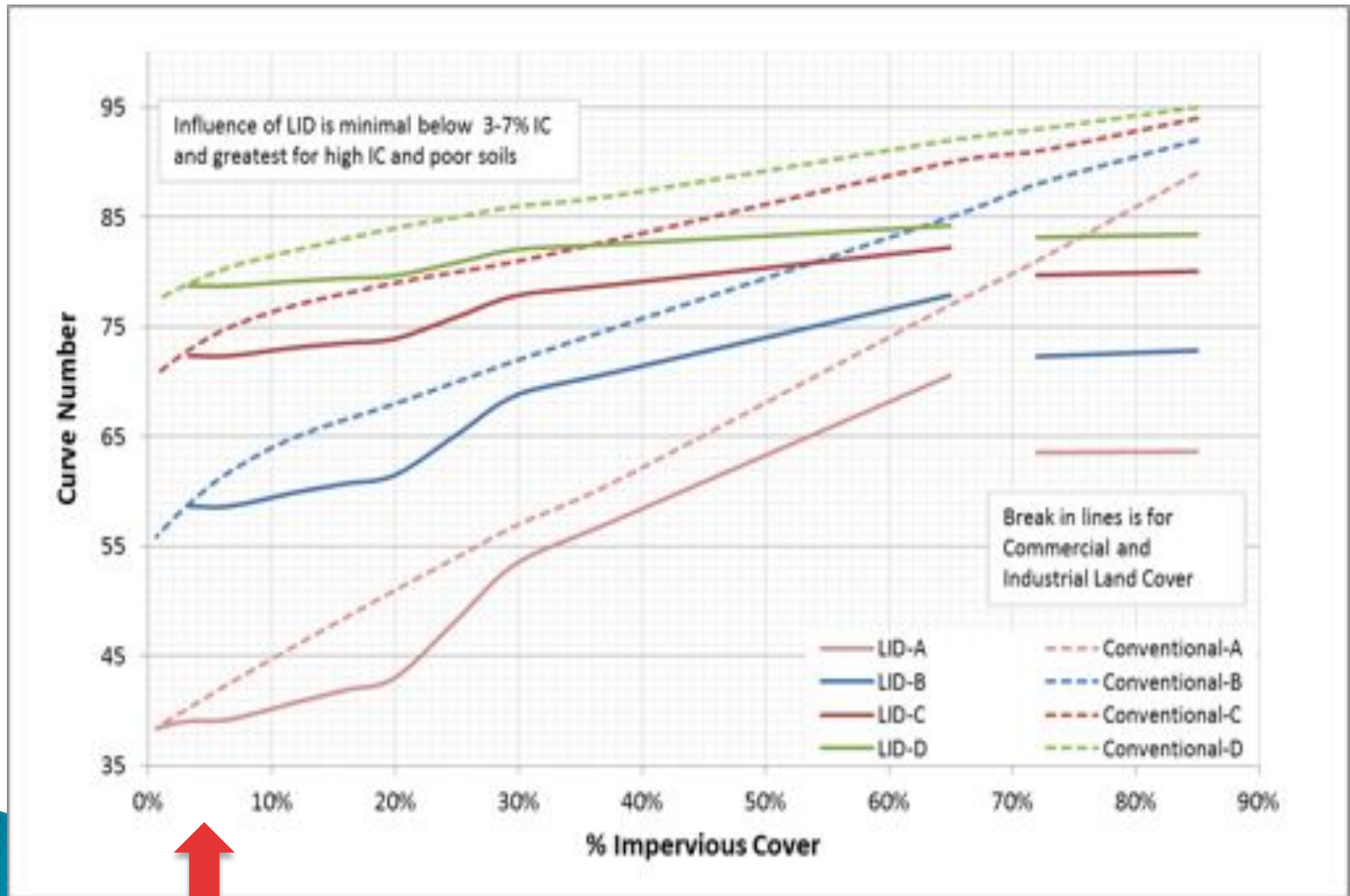
Notes:
 19% IC 0% EIC
 WQV = 1"
 Rooftop and garage bioretention
 Associated impervious surface bioretention
 Porous asphalt driveway

Type	A	B	C	D
CN Predevelopment	38	55	70	77
CN Conventional	51	68	79	84
CN LID	43	62	74	80



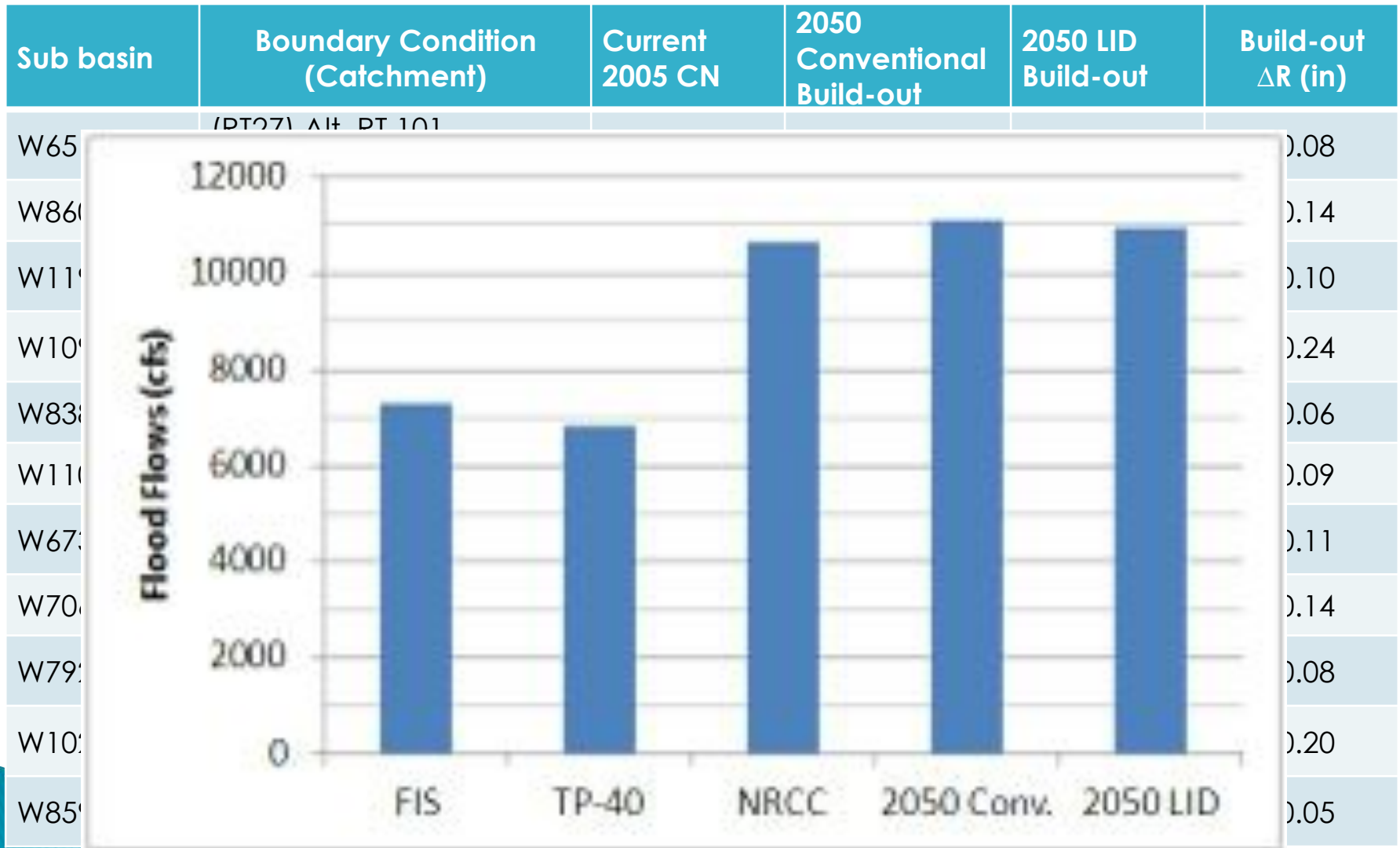
Runoff Curve Number Reduction Method from : 1. McCuen, R. and M. D. E. (1983). Changes in Runoff Curve Number Method; 2. Maryland Department of Environment (2008). Maryland Stormwater Design Manual, Supplement No. 1.

Implementing LID

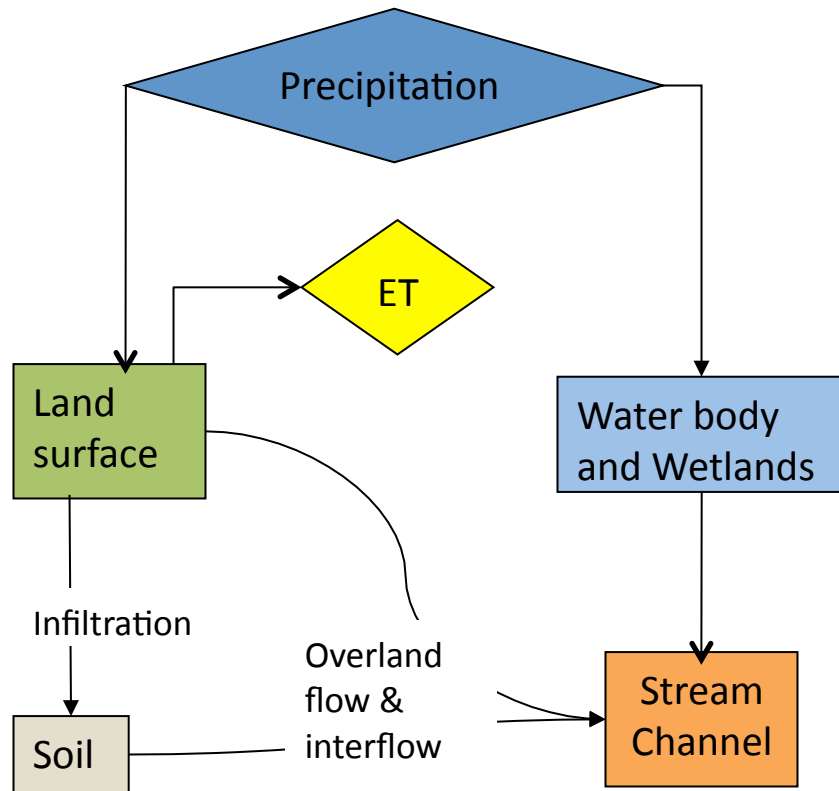


Watershed scale CN values and runoff differences

SCS 'Q' = HMS 'R'



Watershed scale CN



Typical representation
of watershed runoff

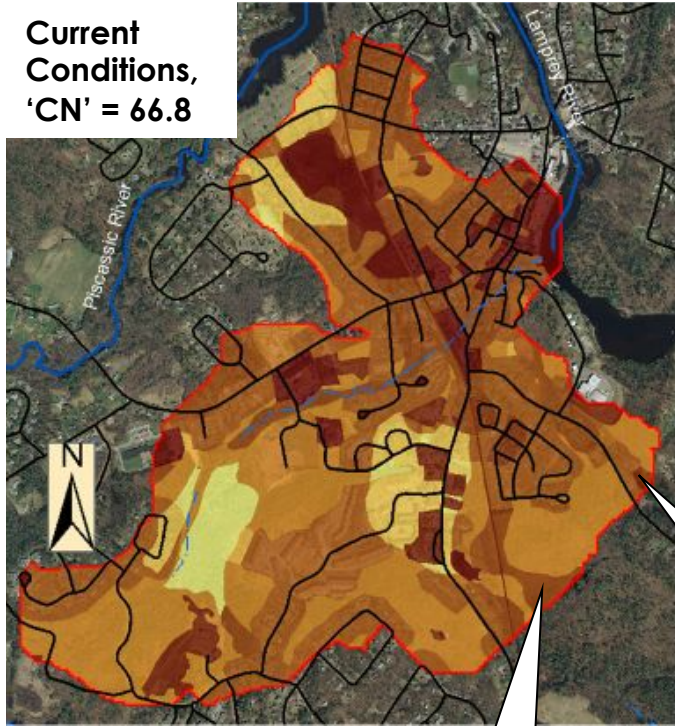
No change in conditions:
11.5% (25 sq.mi.) is water/
wetlands

Future build-out
45% (95 sq. mi.) still forested and
open space

Watershed communities require at
least two (2) acre lot size for
single residential use
(12% impervious cover/lot)

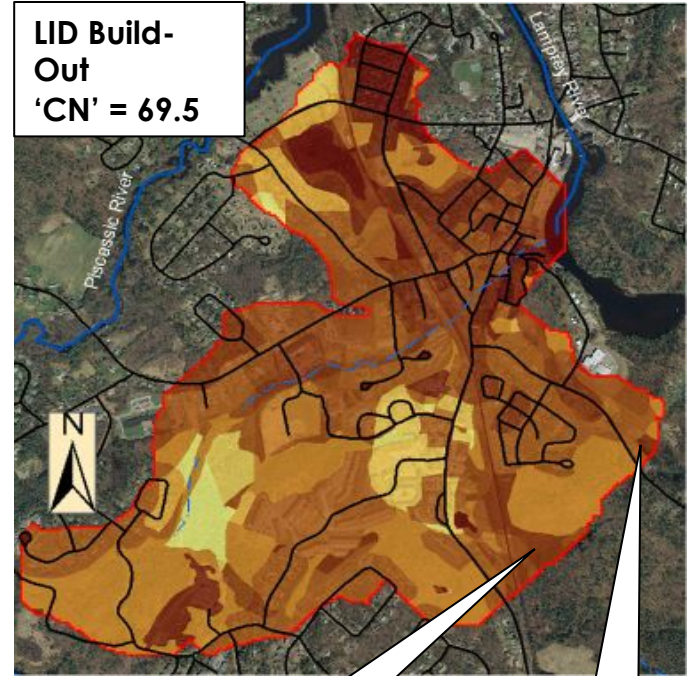


Current Conditions, 'CN' = 66.8



Moonlight Brook, Newmarket

LID Build-Out 'CN' = 69.5



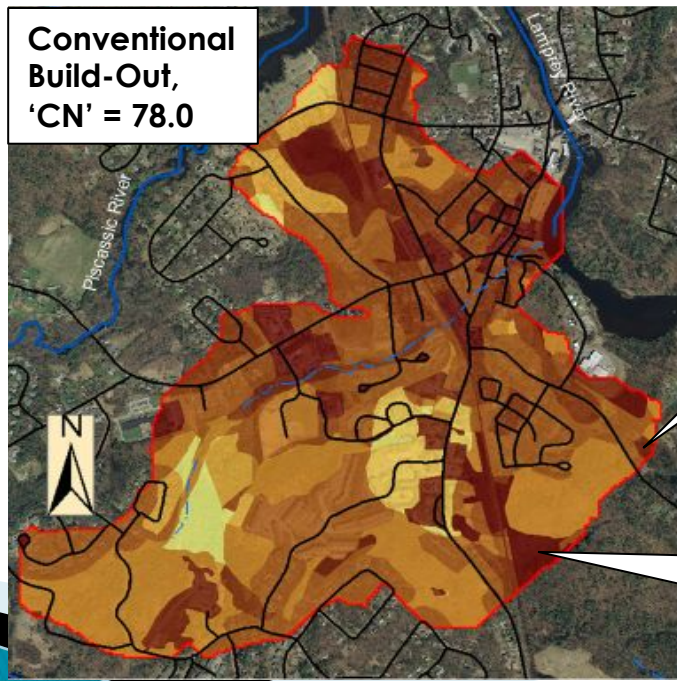
Forested Type D soil
CN = 77

Comm./Ind. Type D soil
CN = 80

Residential Type D soil
CN = 80

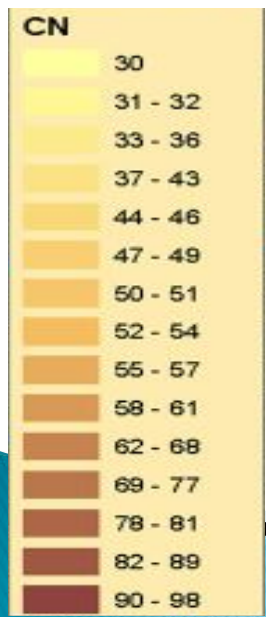
Forested Type C soil
CN = 70

Conventional Build-Out, 'CN' = 78.0



Residential Type D soil
CN = 84

Comm./Ind. Type D soil
CN = 94



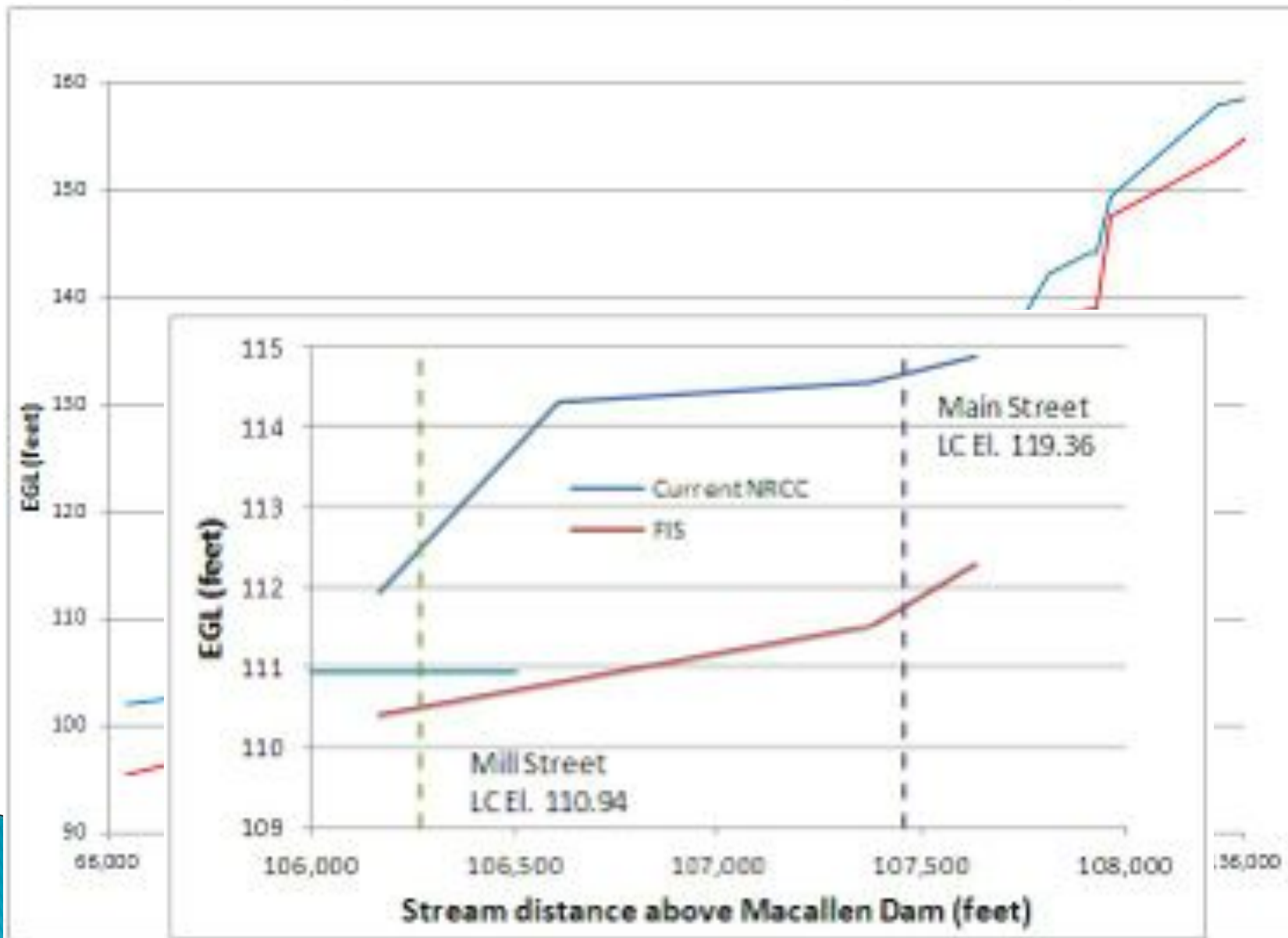
Urban scale composite CNs, runoff, and discharge

SCS 'Q' = HMS 'R'
 SCS 'q' = HMS 'Q'

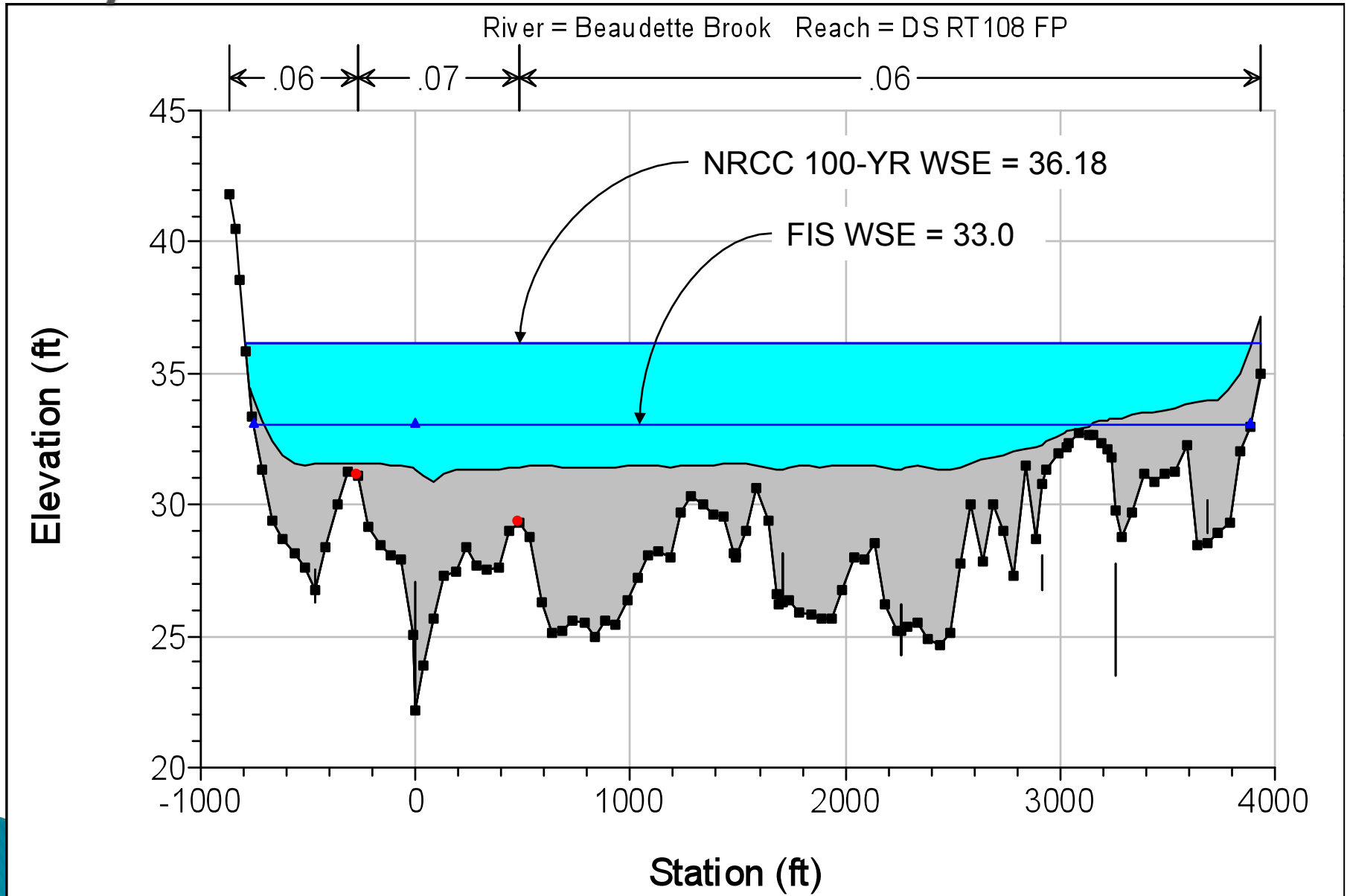
Subwatershed	Moonlight Brook, Newmarket			Intermittent Stream, Epping			Intermittent Stream, Raymond		
Area (sq. mi.)	0.88			1.2			0.86		
Subwatershed values	CN	R (in)	Q (cfs)	CN	R (in)	Q (cfs)	CN	R (in)	Q (cfs)
Current 2005	66.8	4.5	655	70	4.9	1,031	65.8	4.4	508
2050 conventional build-out	78	5.9	852	81.7	6.3	1,320	79.0	6.0	696
2050 LID build-out	69.5	4.8	704	69.4	4.8	1,016	66.6	4.5	520

$$q_a = (q_{ua})(A)(Q_a) < q_b = (q_{ub})(A)(Q_b)$$

Hydraulic Results - Energy Grade Line through Epping



Hydraulic Results – Route 108



Conclusions

- ▶ Hydrologic and hydraulic models provided updated conditions for the examination of land use changes
 - Meets FEMA guidance for redelineation
 - 75 yr record: 1935 – 2009 $Q_{100} = 9,411$ cfs
 - $L_{0.01,0.68} = 6,886$ cfs $H_{0.01,0.68} = 7,834$ cfs
 - Significant increase in flood flows (FIS to current NRCC)
 - USGS gage: 7,300 cfs to 10,649 cfs
 - Hamil Brook: 1,300 cfs to 4,667 cfs
 - Blake Road, Epping: 4,720 cfs to 6,708 cfs
 - Significant increase in BFE (FIS to current NRCC)
 - Route 108: 33.0 feet to 36.18 feet
 - ❖ NRCC elevation almost two feet higher than April 2007 event
 - 3 feet average along length of Lamprey River profile

Conclusions

- ▶ Land use:
 - 90% increase of impervious cover since 1990 in Newfields, Deerfield, Fremont, Epping, and Brentwood
 - Newmarket and Epping exceed PREPs goal of $< 10\%$ IC
 - Expected land conversion by 2050 $> 100\%$ change
- ▶ Rainfall Rates
 - TP-40 Atlas 6.3 in/24 hr
 - NRCC Atlas 8.5 in/24 hr
 - ❖ Increasing IC and Rainfall rates have considerable effect on runoff

Conclusions

▶ Development Strategies:

- Implement LID to reduce runoff curve number (CN)
 - Largest effect seen at urban scale (Current to Convention to LID)

Watercourse	Current	2050 Conv.	2050 LID
Moonlight Brook, Newmarket	66.8	78	69.5
Intermittent Stream, Enning	70	81.7	69.4
Stream,	65.8	79.0	66.6

Notes:

85% IC 0% EIC

WQV = 1"

Rooftop Infiltration

Porous asphalt for parking only

Bioretention for drive lanes of standard asphalt

Type	A	B	C	D
CN Predevelopment	38	55	70	77
CN Conventional	84	89	92	94
CN LID	64	73	80	83

Conclusions

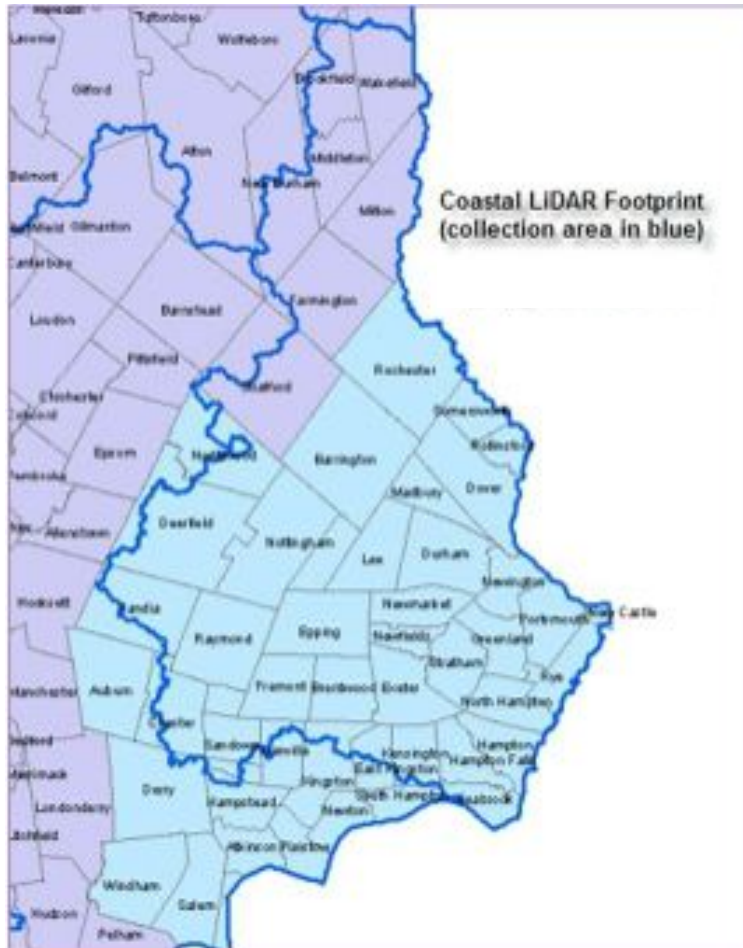
▶ Development Strategies:

- Implement LID to reduce runoff (inches)
 - Largest effect seen at urban scale (Current to Convention to LID)

Watercourse	Current (in.)	2050 Conv. (in.)	2050 LID (in.)
Moonlight Brook, Newmarket	4.5	5.9	4.8
Intermittent Stream, Epping	4.9	6.3	4.8
Intermittent Stream, Raymond	4.4	6.0	4.5

$$q_a = (q_{ua})(A)(Q_a) < q_b = (q_{ub})(A)(Q_b)$$


Future Effort

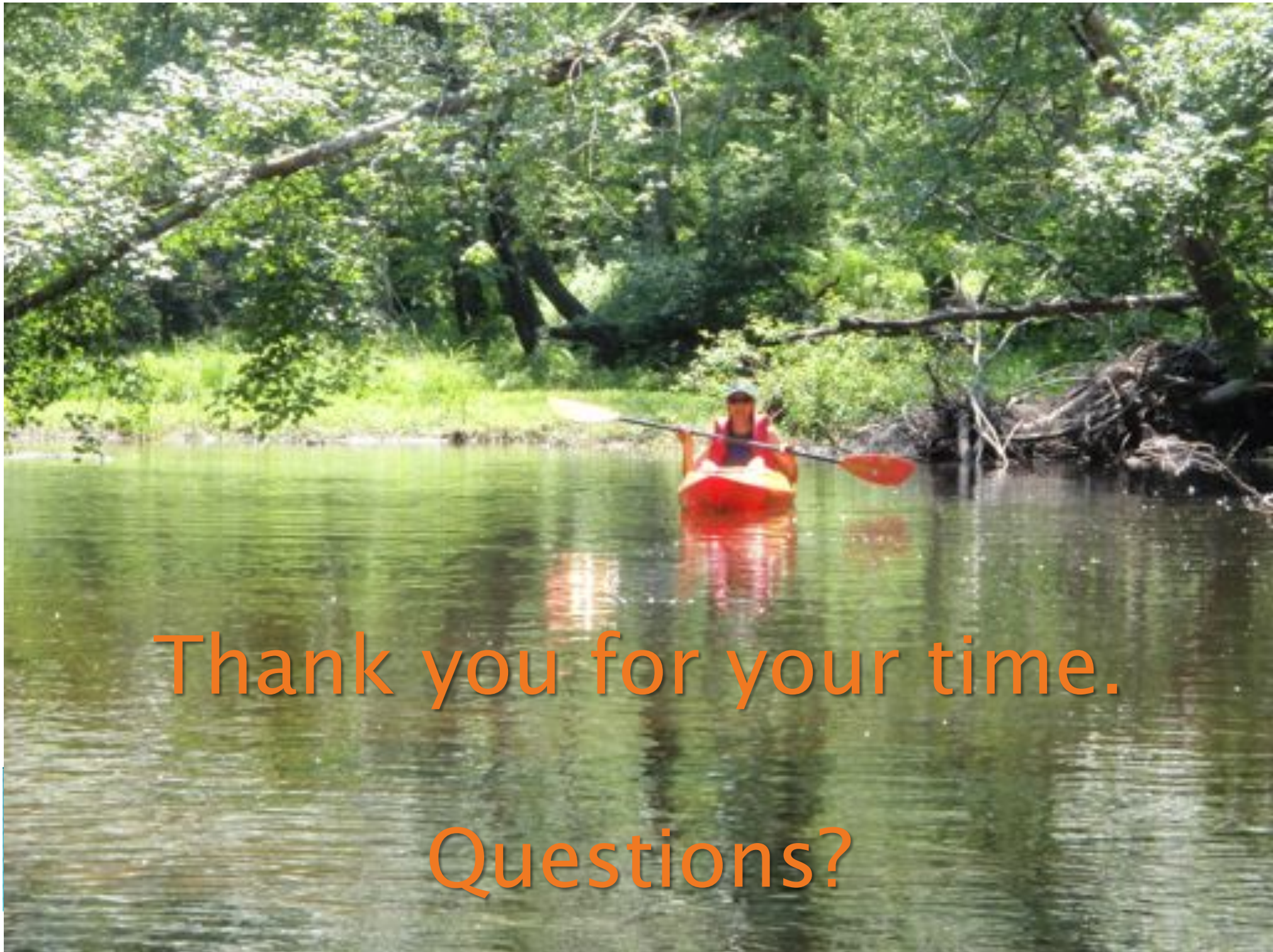


- Establish flood elevation based on datum NAVD88
- Generate cross sections at a spacing to avoid interpolation
- Include Piscassic and other tributaries in Zone A
- Include additional RT152, Lee as other bypass areas
- Calibrate results to additional USGS observations
- Extend study reach into Deerfield
- Widen the cross sections to generate the 500-year floodplain

New Flood Plain Maps and Questions of Legal Authority, Measures and Consequences

In Collaboration with Vermont Law School

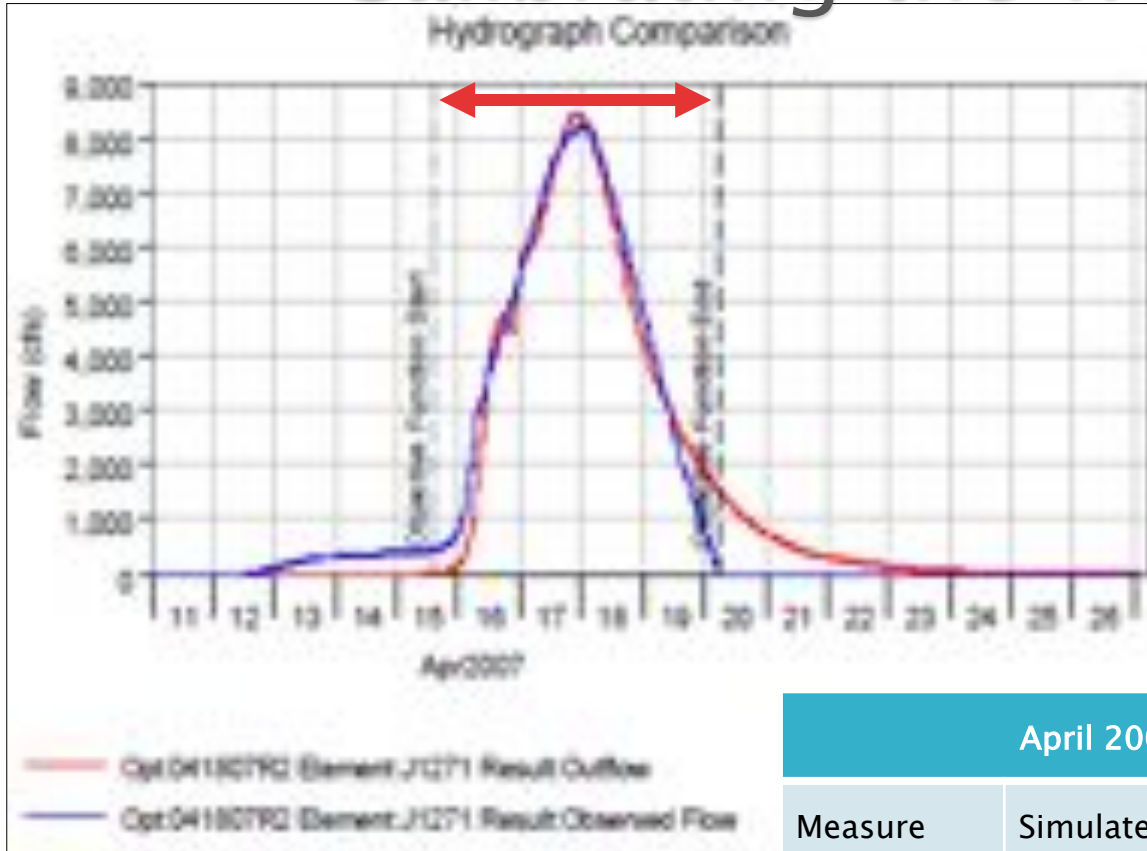
1. What is the potential liability of government if they fail to reduce vulnerability to the risk of flood based on UNH's information?
 2. What legal and policy approaches may communities adopt to reduce flood risks in the expanded flood hazard?
 3. Do New Hampshire communities have the legal authority to design and implement regulatory controls based on projected conditions (e.g., flooding levels)?
 4. What legal standard of scientific and technical reliability must be met in order to support regulatory measures based on current/future environmental conditions?
 5. What is the potential regulatory takings exposure if communities impose regulatory controls that are designed to address anticipated future environmental conditions?
- 



Thank you for your time.

Questions?

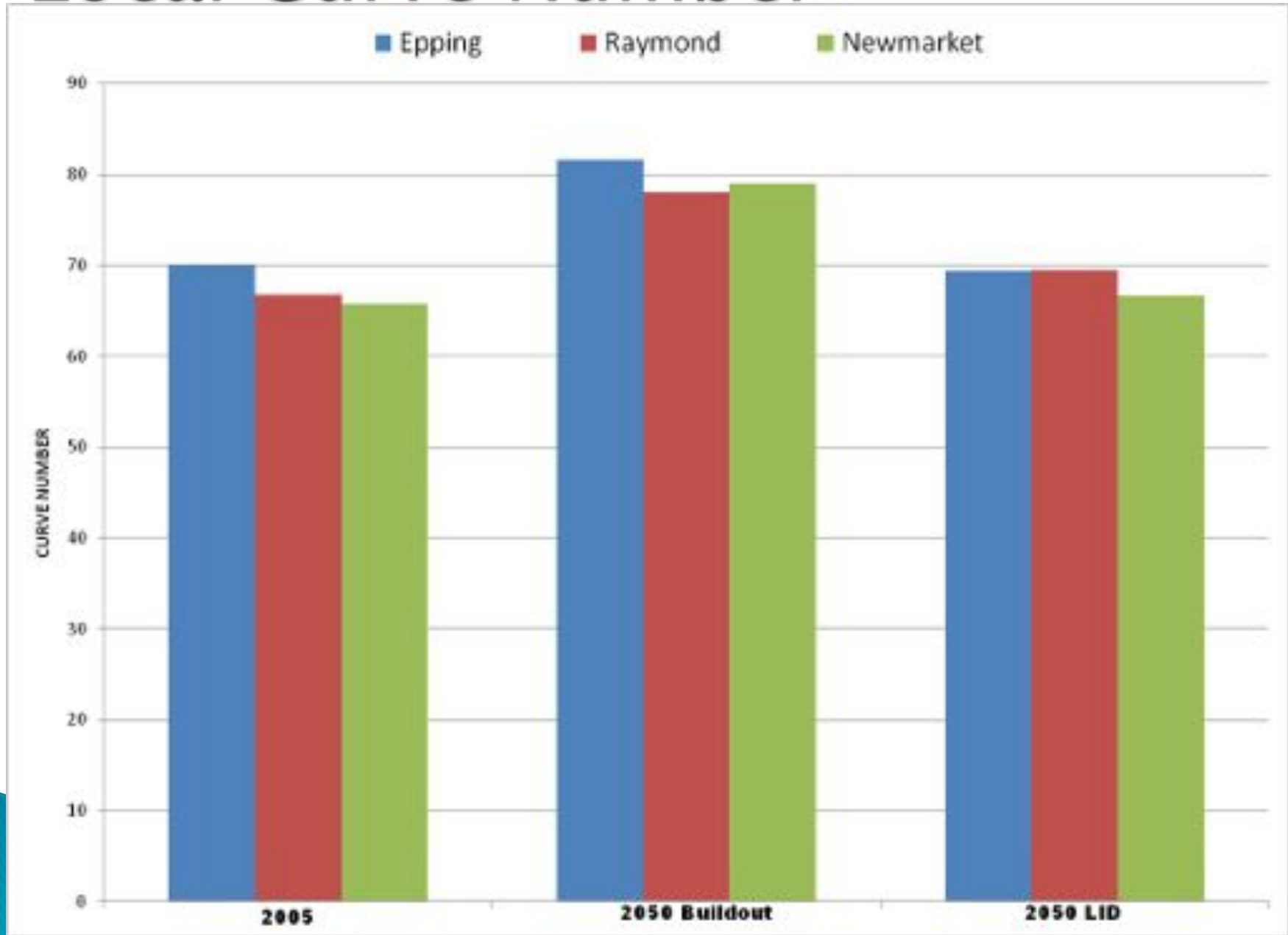
Calibrating the Watershed



April 2007 gage discharge, less baseflow

Measure	Simulated	Observed	Difference	% Difference
Runoff volume (IN)	<u>4.36</u>	<u>4.29</u>	0.07	1.63
Peak flow (CFS)	<u>8332</u>	<u>8223</u>	109	1.32
Time of Peak	17Apr2007, 23:02	18Apr2007, 01:45	2 hr 43 min	0.11
Time of Center of Mass	18Apr2007, 08:16	17Apr2007, 18:59	13 hr 15 min	0.52

Local Curve Number



Implementing LID

SCS graphical method:

$$q_b = (q_{ub})(A)(Q_b)$$

Where: q_b = before development peak discharge (cfs)

q_{ub} = unit peak discharge (csm/in)

A = drainage area (square mile)

Q_b = before development depth of runoff (inches)

$$CN^* = \frac{200}{\left[(P + 2Q + 2) - \sqrt{5PQ + 4Q^2} \right]}$$

Where: CN^* = Revised curve number

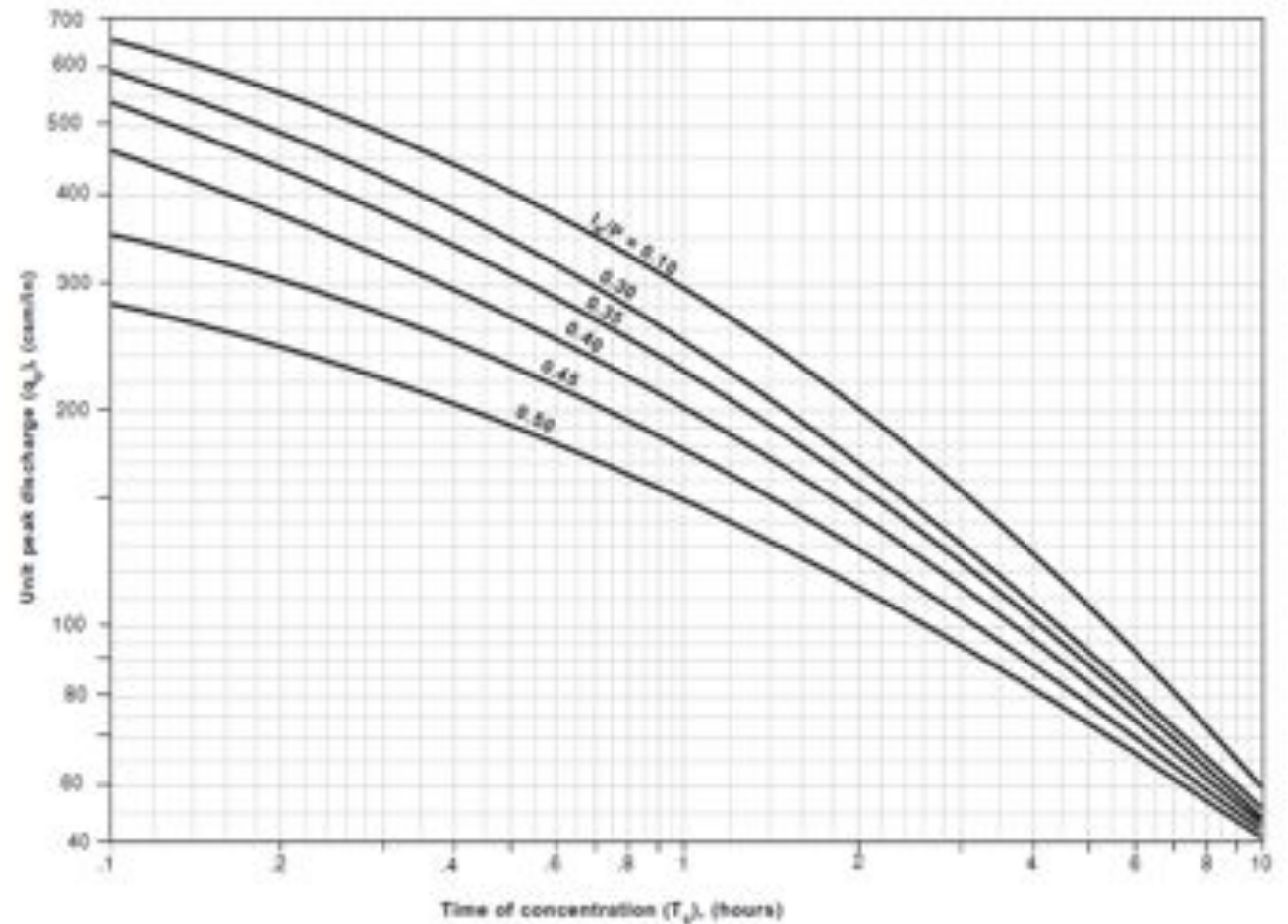
P = Design rainfall depth (inches)

Q = After development runoff depth minus the runoff depth stored by infiltration practices (inches)

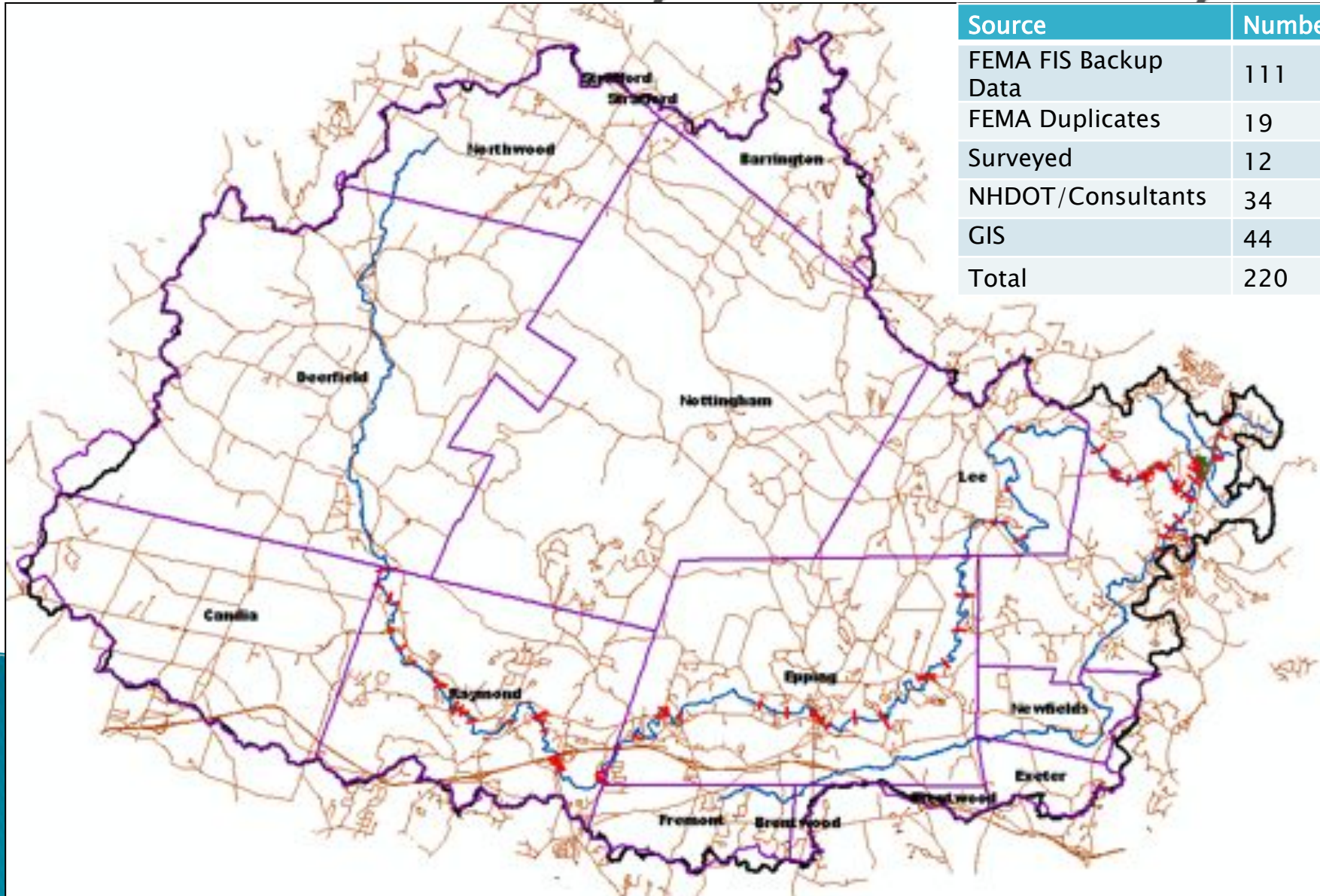


Curve number	I_a (in)	Curve number	I_a (in)
40	3.000	70	0.857
41	2.878	71	0.817
42	2.762	72	0.778
43	2.651	73	0.740
44	2.545	74	0.703
45	2.444	75	0.667
46	2.348	76	0.632
47	2.255	77	0.597
48	2.167	78	0.564
49	2.082	79	0.532
50	2.000	80	0.500
51	1.922	81	0.469
52	1.846	82	0.439
53	1.774	83	0.410
54	1.704	84	0.381
55	1.636	85	0.353
56	1.571	86	0.326
57	1.509	87	0.299
58	1.448	88	0.273
59	1.390	89	0.247
60	1.333	90	0.222
61	1.279	91	0.198
62	1.226	92	0.174
63	1.175	93	0.151
64	1.125	94	0.128
65	1.077	95	0.105
66	1.030	96	0.083
67	0.985	97	0.062
68	0.941	98	0.041
69	0.899		

Exhibit 4.11 Unit peak discharge (q_p) for NRCS (SCS) type III rainfall distribution



HEC-GeoRAS Hydraulic Analysis



Source	Number
FEMA FIS Backup Data	111
FEMA Duplicates	19
Surveyed	12
NHDOT/Consultants	34
GIS	44
Total	220