CREEP AND SHRINKAGE PERFORMANCE OF KENAF BIO FIBROUS CONCRETE COMPOSITES

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A thesis submitted in fulfilment of the requirement for the award of the degree of Doctor of Philosophy (Civil Engineering)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > DECEMBER 2017

DEDICATION

This thesis is dedicated to my beloved wife Eunice Seyi and children Ezekiel Oreoluwa, Emmanuel Ireoluwa and Elisha Ooreoluwa for their endless love, support, sacrifice, and encouragement.

"Thank you for all the patience and endurance during this PhD voyage."

ACKNOWLEDGEMENT

Praise is to God, the Lord of the world. My profound gratitude is to Al-Mighty God first, by whose will and power this thesis report came into being.

I wish to express my sincere and profound gratitude to my main thesis supervisor, Associate Professor Dr. Jamaludin Mohamad Yatim for his continuing assistance, the encouragement, guidance, critics and understanding throughout the period of my studies. The trust, patience, great insight, modesty and friendly personality demonstrated by him have always been my source of inspiration. I am also very grateful to my thesis co-supervisor, Dr. Yunus Mohd Bin Ishak. He's wonderful personality cannot be quantified.

The author is greatly indebted to Faculty of Civil Engineering (FKA) for the support and facilities provided to carry out the experimental work. Same goes to the academic and non-academic staff of the faculty for their support, assistance and friendly treatment that facilitated the work. I remain immensely grateful for the financial support by the management of Federal University of Technology, Minna, Nigeria, through Federal Government of Nigeria Tertiary Education Trust Fund (TETFUND). Same goes to UTM International Doctoral Fellowship (IDF).

The patience demonstrated by my lovely wife Eunice Seyi and children Ezekiel Oreoluwa, Emmanuel Ireoluwa and Elisha Ooreoluwa during this study is gratefully acknowledged. Same goes to my lovely parent Mr Simeon O. Ogunbode and Mrs Victoria Ogunbode. I appreciate the love bestowed on me and my family by my siblings, Femi, Solomon and Seyi Ogunbode. Dr. and Mrs Olukowi P., Assoc. Prof. Dr. and Mrs Akanmu W.P., Mr and Mrs Adeyi Sam., Mr Olaiju O.A., and Mr and Mrs Sunday Adewale. I will eternally be grateful to you, words cannot appreciate you enough. Assoc. Prof. Dr. Jimoh R.A. and my fellow colleagues in the Department of Building FUT Minna, are appreciated. Finally, the cooperation enjoyed by my research colleagues is highly appreciated.

ABSTRACT

Fibrous Concrete Composite (FCC) is a high performance concrete that possesses an improved tensile strength and ductility with restraint to shrinkage and creep under sustained load compared to Plain Concrete (PC). As a result of global quest for sustainable, renewable and green materials to achieve a bio based economy and low carbon foot print environment, the use of fibre to produce fibrous concrete composite has continuously received significant research attention. While several researches have been conducted on metallic and synthetic fibrous concretes, they exhibit several unavoidable drawbacks and bio fibrous concrete has been proved to be a better alternative. This research investigates the creep and shrinkage performance of concrete reinforced with Kenaf bio fibre. After material characterization, concrete reinforced with fibre optimum volume fraction of 0.5% and length of 50 mm was used for the study. The fresh and hardened properties of the concrete were studied under short term quasi static loading. Thereafter, the compressive creep test, uniaxial tensile creep test and flexural creep test at 25% and 35% stress levels at creep loading ages of 7 and 28-day hydration period were conducted. The long term deformation behaviour of the Kenaf Bio Fibrous Concrete Composite (KBFCC) was observed and monitored. Results show that the compressive creep strains of KBFCC is 60.88% greater than the PC, but the deformation behaviour of the specimens shows 33.78% improvement in ductility. Also, uniaxial tensile creep response of fibrous concrete deforms at the rate of 0.00283 mm/day and 0.00702 mm/day at 25% and 35% stress level respectively, but the deformation rate becomes insignificant after 90 days due to the presence of fibre. In addition, the flexural creep test reveals that 0.064 mm/day and 0.073 mm/day deformation rate at 25% stress level of the KBFCC becomes less significant after 40 days of loading. The outcome of the morphology image analysis on the concrete composite shows that Kenaf fibres act as bridges across the cracks, which enhances the load-transfer capacity of the matrix, thus influencing the long term performance of KBFCC. Accordingly, statistical analysis shows that the CEB-FIP creep model is the best fit model for predicting compressive and tensile creep of KBFCC, while EC2 creep and shrinkage models are for predicting flexural creep and shrinkage strain of KBFCC, respectively. A creep and shrinkage prediction model is proposed based on the experimental data for better prediction of KBFCC. Conclusively, KBFCC exhibits appreciable shrinkage, tensile and flexural strength under static short term and long term sustained loads compared to PC.

ABSTRAK

Komposit konkrit bergentian (FCC) merupakan konkrit yang berkualiti tinggi yang mempunyai kekuatan tegangan dan kekangan kemuluran yang diperbaharui kepada pengecutan dan rayapan di bawah beban sekata berbanding dengan konkrit biasa (PC). Hasil daripada usaha global untuk bahan lestari, diperbaharui dan hijau bagi mencapai ekonomi berasaskan bio dan alam sekitar berkarbon rendah, maka penggunaan gentian bagi menghasilkan komposit konkrit bergentian terus mendapat perhatian yang ketara dalam bidang penyelidikan. Walaupun beberapa kajian telah dijalankan terhadap konkrit berserat metalik dan sintetik, kajian itu menunjukkan beberapa kekurangan yang tidak dapat dielakkan dan konkrit bergentian bio telah terbukti sebagai pilihan alternatif yang lebih baik. Kajian ini mengkaji prestasi rayapan dan pengecutan konkrit bertetulang dengan gentian bio Kenaf. Setelah pencirian bahan, konkrit bertetulang dengan gentian pecahan isipadu optimum sebanyak 0.5% dan panjang 50 mm digunakan untuk kajian ini. Ciri-ciri konkrit yang baharu dan keras telah dikaji di bawah beban statik kuasi jangka pendek. Seterusnya, ujian rayapan mampatan, ujian rayapan tegangan tidak berpaksi dan ujian rayapan lenturan pada 25% dan 35% tahap tekanan pada umur pengambilan rayapan 7 dan 28 hari tempoh penghidratan telah dijalankan. Tingkah laku Komposit Konkrit Bergentian Kenaf Bio (KBFCC) kepada perubahan bentuk dalam tempoh jangka panjang telah dikenal pasti dan dipantau. Keputusan ujian telah menunjukkan bahawa perubahan rayapan mampatan KBFCC adalah 60.88% lebih besar daripada konkrit biasa, tetapi perubahan bentuk tingkah laku terhadap spesimen menunjukkan 33.78% peningkatan dalam kemuluran. Selain itu, tindak balas serapan tegangan tidak berpaksi terhadap konkrit bergentian masing-masing berubah bentuk pada kadar 0.00283 mm/hari dan 0.00702 mm/hari pada tahap tekanan 25% dan 35%, tetapi kadar perubahan bentuk menjadi tidak berubah selepas 90 hari dengan kehadiran gentian. Di samping itu, ujian rintangan lenturan menunjukkan kadar perubahan bentuk pada 0.064 mm/hari dan 0.073 mm/hari dengan tahap tekanan 25% daripada KBFCC menjadi tidak ketara selepas 40 hari pembebanan. Hasil analisis imej morfologi pada komposit konkrit menunjukkan bahawa gentian Kenaf bertindak sebagai agen pengikat yang merentasi retak, yang meningkatkan kapasiti pemindahan beban matriks, justeru mempengaruhi prestasi KBFCC dalam jangka masa yang panjang. Dengan demikian, analisis statistik menunjukkan bahawa model rayapan CEB-FIP merupakan model terbaik untuk menganggarkan mampatan dan rayapan tegangan KBFCC, manakala masing-masing model rayapan dan pengecutan EC2 pula menganggarkan lenturan rayapan dan tegangan pengecutan KBFCC. Model anggaran rayapan dan pengecutan dicadangkan berdasarkan data eksperimen untuk ramalan KBFCC yang lebih baik. Secara kesimpulannya, KBFCC mempamerkan pengecutan, kekuatan tegangan dan lenturan yang ketara di bawah beban jangka pendek dan jangka panjang yang dapat menahan beban statik berbanding PC.

TABLE OF CONTENTS

CHAPTE	R TITLE	PAGE
	DECLARATION	ii
	DEDICATION	iii
	ACKNOWLEDGEMENT	iv
	ABSTRACT	V
	ABSTRAK	vi
	TABLE OF CONTENTS	vii
	LIST OF TABLES	xviii
	LIST OF FIGURES	xxii
	LIST OF ABBREVIATIONS	xxxi
	LIST OF SYMBOLS	xxxiii
	LIST OF APPENDICES	xxxiv
1	INTRODUCTION	1
	1.1 General Appraisal	1
	1.2 Background of the Problem	3
	1.3 Statement of the Problem	5
	1.4 Aim and Objectives	7
	1.5 Scope of the Study	7
	1.6 Thesis Organization	9
2	LITERATURE REVIEW	11
	2.1 Introduction	11
	2.2 Fibres	11
	2.3 Concept of Fibrous Concrete Composite (FCC)	12
	2.3.1 Composite	12
	2.3.2 Bio-Composites	14

	2.3.3 Fibrous Concrete Composite (FCC)	14
2.4	Bio Fibres	18
	2.4.1 Properties of Bio Fibres	22
	2.4.2 Bio Fibre Moisture Content	23
	2.4.2.1 Kinetics of Water Absorption	24
	2.4.2.2 Mechanism of Water Transport	25
	2.4.3 Bio Fibre Chemical Surface Treatment	26
	2.4.4 Bio Fibre Alkaline Surface Treatment	27
2.5	Effects of Fibre Volume Ratio and Length of Fibre	29
2.6	Properties of Bio Fibrous Concrete Composites	30
2.7	Characteristics of Kenaf Fibre	35
	2.7.1 Economy of Kenaf Fibres	38
	2.7.2 Physical and Mechanical of Kenaf Fibre Properties	39
	2.7.3 Interface Properties between Kenaf Fibre and Matrix	41
	2.7.4 Properties of Kenaf Fibrous Composites	41
2.8	Kenaf Bio Fibrous Concrete Composites (KBFCC) and its	
	Mixture Properties	42
	2.8.1 Compression Properties of KBFCC	43
	2.8.2 Tension Properties of KBFCC	45
	2.8.3 Flexural Properties of KBFCC	47
2.9	Elastic Modulus of Concrete	48
	2.9.1 Elastic Modulus Prediction by ACI-318	49
	2.9.2 Elastic Modulus Prediction by BS 8110	50
	2.9.3 Elastic Modulus Prediction by CEB-FIP 1990	50
	2.9.4 Elastic Modulus Prediction by AS 3600	51
	2.9.5 Elastic Modulus Prediction by EC 2	52
2.10	OConcept of Long Term Behaviour of Fibrous Concrete	
	Composite	53
	2.10.1 Characteristics of Long Term Deformations	53
	2.10.2 Long-Term Deformations Behaviour and	
	Mechanisms	54
	2.10.3 The Mechanism Initiating Creep in Concrete	56
	2.10.4 Creep behaviour of Concrete under Sustained Load	56
	2.10.5 Shrinkage Performance of Concrete	57

2.10.6 Importance of Studying Shrinkage of Concrete	58
2.10.7 Previous Studies on Creep and Shrinkage	
deformation of Fibrous Concrete Composites	58
2.11 Criteria for Derivation and Analysis of Creep and Shrinkag	ge
Model	60
2.11.1 Creep Model Derivation	61
2.11.2 Derivation of Creep Coefficient	63
2.11.2.1 Hyperbolic Expression	63
2.11.2.2 Power Expression	64
2.11.2.3 Logarithmic Expression	65
2.11.2.4 Exponential Expression	66
2.11.2.5 Double Power Law	66
2.11.2.6 Triple Power Law	67
2.11.3 Derivation of Shrinkage Strain	68
2.11.4 Findings on Creep and Shrinkage deformation	
behaviour of Fibrous Concrete Composites	68
2.12 Summary of Research Gap	77
RESEARCH METHODOLOGY	79
3.1 Introduction	79
3.2 Experimental Framework	80
3.2.1 Compressive Creep Testing Details	83
3.2.2 Uniaxial Tensile Creep Testing Details	84
3.2.3 Flexural Creep Testing Details	85
3.3 Concrete Materials	87
3.3.1 Kenaf Fibre	88
3.3.2 Ordinary Portland Cement	89
3.3.3 Crushed Coarse Aggregate	90
3.3.4 Fine Aggregate	90
3.3.5 Water	90
3.3.6 Sodium Hydroxide (NaOH)	91
3.3.7 Superplasticizer	92
3.4 Experimental Work for Kenaf Fibre Characterisation	92
3.4.1 Pre-treatment of Kenaf Fibre	93

3

	3.4.2 Chemical Treatment of Kenaf Fibre	94
	3.4.3 Water Sorption	95
	3.4.3.1 Kinetics of Water Absorption	96
	3.4.4 Tensile Test on Kenaf Fibre	97
	3.4.5 Surface Morphology of Kenaf Fibre	98
3.5	Experimental Work on Kenaf Fibre Optimum Length and	
	Volume Fraction for the Production of KBFCC	99
	3.5.1 Kenaf Fibre Inclusion in Concrete Mix	99
	3.5.2 Concrete Mix Design and Optimization	100
	3.5.3 KBFCC Mixing Procedure	102
	3.5.4 Details of Formwork and Hardened Concrete	
	Specimen	104
	3.5.5 Assemblage of Moulds for Uniaxial Tensile	
	Investigation	106
3.6	Test Setup for Fresh and Hardened Properties of KBFCC	107
	3.6.1 Fresh Concrete Properties Test	107
	3.6.1.1 Slump Test (ASTM C143/C143M, 2015)	108
	3.6.1.2 Vebe Test (BS EN, 12350-3: 2009)	108
	3.6.1.3 Compacting Factor Test (BS 1881-103: 1993)	109
	3.6.2 Hardened Concrete Properties Test of KBFCC	110
	3.6.2.1 Initial Surface Absorption (BS 1881: Part 208,	
	1996)	112
	3.6.2.2 Porosity and Water Absorption Test (ASTM	
	C642/C642M, 2013)	113
	3.6.2.3 Ultrasonic Pulse Velocity (UPV) Test (ASTM	
	C597/C597M, 2009)	115
	3.6.2.4 Compressive Strength Test (ASTM	
	C39/C39M, 2012 and BS EN 12390-3 (2002)	116
	3.6.2.5 Indirect Tensile Strength Test (ASTM	
	C496/C496M, 2011)	118
	3.6.2.6 Flexural Strength Test (ASTM C78/C78M,	
	2010)	119
	3.6.2.7 Uniaxial Tensile Strength Test (Method by	
	Babafemi and Boshoff, 2015)	120

3.6.2.8 Modulus of Elasticity Test (ASTM	
C469/C469M, 2014)	122
3.6.2.9 Drying Shrinkage Test (ASTM C157/C157M,	
2008)	123
3.7 Preliminary Short Term Compressive Creep Test on	
Different Specimen Sizes	125
3.8 Experiment Test Controlled Room	126
3.9 Long-Term Performance Test on the PC and KBFCC	127
3.9.1 Compressive Creep Test on Specimens	127
3.9.1.1 Compressive Creep Samples Preparation	127
3.9.1.2 Compressive Creep Test Apparatus	128
3.9.1.3 Compressive Creep Test Procedures	130
3.9.1.4 Compressive Creep Calculations	131
3.9.2 Uniaxial Tensile Creep Test on Specimens	132
3.9.2.1 Uniaxial Tensile Creep Samples Preparation	133
3.9.2.2 Uniaxial Tensile Creep Test Apparatus	133
3.9.2.3 Uniaxial Tensile Creep Test Procedures	134
3.9.2.4 Uniaxial Tensile Creep Calculations	136
3.9.3 Flexural Creep Test on Specimens	137
3.9.3.1 Flexural Creep Samples Preparation	137
3.9.3.2 Flexural Creep Test Apparatus	138
3.9.3.3 Flexural Creep Test Procedures	141
3.10 Theoretical Development and Evaluation of Prediction	
Models of Concrete Creep and Shrinkage	142
3.10.1 Creep Model Code	144
3.10.1.1 CEB-FIP Model Code for Creep	144
3.10.1.2 ACI 209 Model for Creep	146
3.10.1.3 Eurocode 2 (EC 2) Model for Creep	148
3.10.1.4 AS 3600 Model Code for Creep	148
3.10.2 Shrinkage Model Code	149
3.10.2.1 CEB-FIP Model Code for Shrinkage	149
3.10.2.2 ACI 209 Model Code for Shrinkage	151
3.10.2.3 Eurocode 2 (EC 2) Model for Shrinkage	152
3.10.2.4 AS 3600 Model for Shrinkage	153

	3.10.3 Evaluation of the Creep and Shrinkage Prediction	
	Models	154
	3.10.4 Comparison of Existing Creep Prediction Models to	
	Experimental Creep Test Results	155
	3.11 Summary on the Research Materials and Methods	158
4	EXPERIMENTAL CHARACTERIZATION OF	
	MATERIALS, FRESH AND HARDENED STATE	
	PROPERTIES OF KBFCC	159
	4.1 Introduction	159
	4.2 Characteristics of Kenaf Fibre	159
	4.2.1 Physical Properties of Kenaf Fibre	160
	4.2.1.1 Kenaf Fibre Surface Modification, Appearance	
	and Morphology	160
	4.2.1.2 Effect of Fibre Treatment on Kenaf Fibre	
	Mechanism of Sorption (Water Uptake)	162
	4.2.1.3 Mechanism of Water Transport	164
	4.2.1.4 Kenaf Fibre Diameter	165
	4.2.1.5 Kenaf Fibre Density	167
	4.2.2 Tensile Properties of Kenaf fibre	168
	4.2.2.1 Kenaf Fibre in Untreated State	168
	4.2.2.2 Kenaf fibre in treated state	170
	4.3 Characteristics of Coarse Aggregate	171
	4.3.1 Physical Properties of Coarse Aggregate	171
	4.3.2 Grading of coarse aggregate	173
	4.4 Characteristic of Fine Aggregate	174
	4.4.1 Physical Characteristic of Fine Aggregate	174
	4.4.2 Grading of Fine Aggregate	175
	4.5 Results of Fresh State Properties of PC and KBFCC	176
	4.5.1 Workability	177
	4.5.1.1 Effect of Kenaf Fibre on the Slump	
	of Concrete	177
	4.5.1.2 Effect of Kenaf Fibre on the Vebe of Concrete	179

	4.5.1.3 Effect of Kenaf Fibre on the Compacting	
	Factor of Concrete	180
	4.5.1.4 Correlation between Vebe Time and Slump of	
	PC and KBFCC	181
	4.5.1.5 Correlation between Compacting Factor and	
	Slump of PC and KBFCC	182
	4.5.1.6 Correlation between Compacting Factor and	
	Vebe Time of PC and KBFCC	183
	4.5.2 Importance of the Correlation between Variables	
	Associated with Fresh Properties of PC and KBFCC	184
	4.5.3 Fresh Density	185
4.6	Results of Hardened State Properties of PC and KBFCC	186
	4.6.1 Hardened State Density of PC and KBFCC	186
	4.6.2 Ultrasonic Pulse Velocity of PC and KBFCC	187
	4.6.3 Correlation between Ultrasonic Pulse Velocity (UPV)	
	and Compressive Strength (CPS) of Concrete	188
	4.6.4 Porosity and Water Permeability (Water Absorption)	
	of PC and KBFCC	190
	4.6.5 Initial Surface Absorption of PC and KBFCC	191
	4.6.6 Influence of Kenaf Fibre Reinforcement on the	
	Strength and Deformation Characteristics of PC and	
	KBFCC under Compression, Tension and Flexure.	195
	4.6.6.1 Compressive Strength of PC and KBFCC	195
	4.6.6.2 Indirect Splitting Tensile Strength of PC and	
	KBFCC	197
	4.6.6.3 Uniaxial tensile (Direct Tensile) Strength of	
	PC and KBFCC	199
	4.6.6.4 Flexural Strength of PC and KBFCC	204
	4.6.6.5 Elastic Modulus of PC and KBFCC	206
	4.6.6.6 Relationship between Experimental and	
	Predictive Model of Elastic Modulus of PC and	
	KBFCC	209
	4.6.6.7 Poisson's Ratio of PC and KBFCC	212
	4.6.7 Shrinkage of PC and KBFCC	212

	4.7	Preliminary Compressive Creep Experimental Results	216
		4.7.1 Effect of Specimen Size on Compressive Creep Test	216
	4.8	Summary of Materials Characterization, Fresh and Hardened	
		State Properties of KBFCC	220
5	EX	PERIMENTAL TESTING ON THE INTERFACE	
	MC	ORPHOLOGIES OF KBFCC	222
	5.1	Introduction	222
	5.2	Microstructural Analysis	222
	5.3	Scanning Electron Microscopy	223
	5.4	Interfaces in KBFCC	226
	5.5	Summary of the KBFCC Interface Morphologies Study	228
6	CR	EEP AND SHRINKAGE PERFORMANCE OF KBFCC	229
	6.1	Introduction	229
	6.2	Compressive Creep of PC and KBFCC	229
		6.2.1 Analysis of Compressive Creep Test Results of PC	
		and KBFCC	230
		6.2.1.1 Effects of Kenaf Fibre Inclusion on	
		Compressive Creep Performance of Concrete	230
		6.2.1.2 Loading Condition (Stress Level) Effects on	
		Compressive Creep Performance of PC and	
		KBFCC	232
		6.2.1.3 Loading Age Condition Effects on	
		Compressive Creep Performance of PC and	
		KBFCC	233
		6.2.1.4 Relationship between Compressive Strength	
		and Compressive Creep Strain	234
		6.2.2 Compressive Creep Coefficient	237
		6.2.2.1 Loading Conditions (Stress Level) Effects on	
		Compressive Creep Coefficient	237
		6.2.2.2 Loading Age (Curing Conditions) Effects on	
		Compressive Creep Coefficient	238

	6.2.2.3 Fibre Inclusion Effects on Compressive Creep	
	Coefficient	239
	6.2.2.4 Compressive Strength Effects at loading Age	
	on Compressive Creep Coefficient	241
	6.2.2.5 Trend Analysis on Compressive Creep	
	Coefficient and Standard code Models	243
	6.2.2.6 Statistical Comparison for Compressive Creep	
	Coefficient and Standard Code Models	245
6.3	Uniaxial Tensile Creep of PC and KBFCC	248
	6.3.1 Analysis of Uniaxial Tensile Creep Test Results of	
	PC and KBFCC	248
	6.3.1.1 Uniaxial Tensile Creep Results of Pre-Cracked	
	PC and KBFCC specimens	249
	6.3.1.2 Uniaxial Tensile Creep Coefficient of Pre-	
	cracked PC and KBFCC Specimens	252
	6.3.1.3 Trend Analysis on Tensile Creep Coefficient	
	and Standard Code Models	254
6.4	Flexural Creep of PC and KBFCC	258
	6.4.1 Analysis of Flexural Creep Test Results of PC and	
	KBFCC	258
	6.4.1.1 Load-Deflection of Flexural Test of PC and	
	KBFCC Specimen for Creep	259
	6.4.1.2 Load-Deflection Behaviour of PC and KBFCC	
	Beam Specimen for Creep	260
	6.4.1.3 Effect of Kenaf Fibre on Toughness of KBFCC	
	Beam Specimen for Flexural Creep	264
	6.4.2 Flexural Creep Results of Pre-cracked PC and	xvi
	KBFCC Specimens	267
	6.4.2.1 Effects of Loading Condition (Stress Level) on	
	Flexural Creep Performance of PC and	
	KBFCC	268
	6.4.2.2 Effects of Loading Age Condition (moist	
	curing) on Flexural Creep Performance of PC	
	and KBFCC	270

	6.4.2.3 Trend Analysis on Flexural Creep Coefficient	
	and Standard Code Models	271
	6.4.2.4 Statistical Comparison for Flexural Creep	
	Coefficient and Standard Code Models	272
6.5	Proposed Prediction Model for PC and KBFCC Creep	
	Coefficient	275
	6.5.1 Evaluation Methods on the Goodness of Fit	275
6.6	Shrinkage of PC and KBFCC	279
	6.6.1 Analysis of Shrinkage Test Results of PC and	
	KBFCC	280
	6.6.1.1 Effects of Kenaf Fibre Inclusion on Shrinkage	
	Performance of Concrete	280
	6.6.1.2 Curing Condition (Moist Curing) Effects on	
	Shrinkage Performance of PC and KBFCC	283
	6.6.1.3 Trend Analysis on Shrinkage Strain and	
	Standard Code Models	284
	6.6.1.4 Statistical Comparison for Shrinkage Strain	
	and Standard Code Models	285
6.7	Proposed PC and KBFCC shrinkage strain Prediction model	288
6.8	Summary of Results on Creep and Shrinkage Analysis	289
CO	NCLUSION AND RECOMMENDATION	293
7.1	Introduction	293
7.2	The Physical and Mechanical Properties of Concrete	
	Containing Kenaf Fibre at Varying Fibre Lengths and	
	Volume Fractions.	293
7.3	The Effect of Kenaf Fibre on the Shrinkage and Creep	
	Properties of KBFCC.	294
7.4	Microstructure Characteristics of KBFCC	295
7.5	Analytical Model for Estimation of Compressive, Tensile	
	and Flexural Creep Behaviour of KBFCC	296
7.6	Significance of the Research	297
7.7	Recommendation for Future Studies	298

REFERENCES

Appendices A-G

300

324-336

LIST OF TABLES

TABLE NO.	TITLE	PAGE
2.1	List of some common bio fibres, origin and species [83]	20
2.2	Mechanical properties of some bio fibres compared to E	
	glass and carbon fibre [85,87,88]	20
2.3	Cost and density of available fibres in the market [89]	21
2.4	Various types of bio fibres chemical conformation [89]	21
2.5	Basic properties of various bio fibres [89]	22
2.6	Advantages and disadvantages of bio fibre [90]	23
2.7	Physical and chemical properties of treated and untreated	
	agave fibre[129]	28
2.8	Mechanical properties of several types of bio fibres	
	[136]	31
2.9	Typical properties of bio fibres [136]	31
2.10	Issues affecting properties of bio fibrous concrete	
	composites [144]	33
2.11	Influence of fibre length and volume fraction on strength	
	factors of jute fibrous cement composites [136,157]	34
2.12	Appraisal of the properties of elephant-grass fibre	
	reinforced roofing Sheets with those reinforced with	
	asbestos fibres [150,153]	34
2.13	Kenaf fibres properties reported by some researchers.	39
2.14	Kenaf fibres and E-glass fibres properties [191]	40
2.15	Published Research articles related to Kenaf fibre	
	between 2007 and 2015	42
2.16	KBFCC mixture proportion [35]	43
2.17	28 day splitting tensile strength of KBFCC cylinders	
	[35]	46

2.18	Test setups for uniaxial direct tensile (notched	
	prisms/cylinder specimens) [54]	47
2.19	Typical range for the static elastic modulus at 28 days	
	for normal weight concrete (Table 7.2, BS 8110 [234])	50
2.20	Effect of type of aggregate on elastic modulus (Table	
	2.1.5, CEB-FIP [235])	51
2.21	Summarized creep deformation rate values of fibrous	
	concrete	72
2.22	Published research articles on creep of fibrous concrete	
	in previous years.	74
3.1	Creep test parameters on PC and KBFCC specimens	87
3.2	Physical, mechanical and chemical characteristics of	
	Kenaf fibres [278].	88
3.3	A typical ordinary portland cement chemical	
	composition	89
3.4	Typical properties of Rheobuild 1100 superplasticiser	92
3.5	PC mix design	101
3.6	Details of Mixture	102
3.7	Summary of specimen type and amount used in the	
	experiment	105
3.8	Initial surface absorption rating	113
3.9	Classification of the quality of concrete on the basis of	
	pulse velocity	116
3.10	Details of short-term creep test	126
3.11	Required percentage creep loads for pre-cracked creep	
	specimens	135
3.12	Summary of the factors considered in predicting creep	
	and shrinkage by different standard model codes	155
4.1	Values of M_{∞} for the treated and untreated Kenaf fibres	
	in distilled water at 25°C temperatures	164
4.2	Values of <i>n</i> , <i>k</i> , diffusion coefficient (<i>D</i>), sorption	
	coefficient (S) and permeability (P) for the treated and	
	untreated Kenaf fibres in distilled water at 25°C	
	temperatures	165

4.3	Statistical parameter of Kenaf fibre diameter	166
4.4	The result of Kenaf fibre density.	167
4.5	Un-treated and treated Kenaf fibre tensile properties in	
	virgin and swollen state	169
4.6	Physical properties of coarse aggregate	172
4.7	Physical properties of fine aggregate	175
4.8	Summary of relationship between respective fresh	
	properties of the KBFCC from linear regression.	185
4.9	Summary of relationship between respective fresh	
	properties of the KBFCC from multiple regression.	185
4.10	Summary of relationship between compressive strength	
	(CPS) and UPV for each fibre length (l_f) in KBFCC	189
4.11	Total porosity and water absorption of PC and KBFCC	
	at varying fibre content and length	191
4.12	Load carrying capacity with increase in crack mouth	
	opening of KBFCC specimen	203
4.13	Experimental elastic modulus results	208
4.14	Comparison of elastic modulus between standard codes	
	and results from experiment on PC and KBFCC	210
4.15	28 days Plain concrete mechanical and physical	
	properties	216
4.16	Relationship of compressive creep coefficient between	
	100 mm diameter x 200 mm and 150 mm diameter x 300	
	mm specimen	220
6.1	Regression analysis on relationship amongst	
	compressive strength and creep strain using Equation 5.1	236
6.2	Regression analysis on relationship of compressive	
	strength to creep coefficient using Equation 6.2	241
6.3	Mean residual (R_e) of compressive creep coefficient for	
	specimens loaded at 7 and 28 days	246
6.4	Ranking of the overall compressive creep coefficient of	
	the prediction models	247
6.5	Total uniaxial creep CMOD after 180 days	252
6.6	Uniaxial tensile creep consideration	253

6.7	Mean residual (R_e) of tensile creep coefficient for	
	specimens loaded at 28 days	256
6.8	Ranking of the overall tensile creep coefficient of the	
	prediction models	257
6.9	Loads required for fibre activation and flexural creep	
	loading.	259
6.10	Load-deflection $(L-d)$ relationship of mid span for PC 7	
	days beam at flexural test.	262
6.11	Load-deflection (L-d) relationship of mid span for PC 28	
	days beam at flexural test.	262
6.12	Load-deflection (L-d) relationship of mid span for	
	KBFCC 7 days beam at flexural test.	262
6.13	Load-deflection (L-d) relationship of mid span for	
	KBFCC 28 days beam at flexural test.	262
6.14	Area covered under the load deflection curve of PC and	
	KBFCC specimen	265
6.15	Mean residual (R_e) of flexural creep coefficient for	
	specimens loaded at 28 days	273
6.16	Ranking of the overall flexural creep coefficient of the	
	prediction models	274
6.17	Regression analysis values from curve fitting expression	
	of the proposed creep coefficient model	279
6.18	Test result on shrinkage strains of the PC and KBFCC	
	mixtures evaluated at various curing ages	281
6.19	Mean residual (R_e) of shrinkage strain for specimens	
	loaded at 7 and 28 days	286
6.20	Ranking of the overall shrinkage strain of the prediction	
	models	287
6.21	Regression analysis values from curve fitting expression	
	of the experimental data of shrinkage strain	289

LIST OF FIGURES

FIGURE NO.	TITLE	PAGE
2.1	Classification of fibres	12
2.2	Tensile reaction categorization of cementitious	
	composites [60]	15
2.3	Bio fibre classifications [82]	18
2.4	Structure of bio fibre [83]	19
2.5	Chemical structure of cellulose [81]	19
2.6	Lignin structure [81]	19
2.7	Tensile strength of alkaline treated of Kenaf fibres [129]	29
2.8	Effect of fibre content on the bending strength of Basalt	
	Fibre [133]	30
2.9	(a) A photograph of Kenaf plant (b) Photograph of the	
	cross-section of Kenaf plant stem (c) Photograph of a	
	Kenaf fibre	36
2.10	SEM images of untreated and treated surface of Kenaf	
	fibre (a) Untreated Kenaf fibre (b) Treated Kenaf fibre	
	[199]	40
2.11	SEM images of the 5% alkali solution at 3hr immersion	
	time alkaline treatment on Kenaf fibre (a) Untreated	
	Kenaf fibre (b) Treated Kenaf fibre [199]	40
2.12	Influence of fibre volume fraction on compressive	
	strength of KBFCC at 28 days [35]	44
2.13	Effect of w/c ratio on 28 day strength of plain concrete.	
	Adapted from [224]	44
2.14	Stress-strain curve for PC and KBFCC cylinders at 28	
	days [35].	45

2.15	Mid-span load-deflection behaviour of (a) control and	
	(b) 1.2% KBFCC prisms at 28 days [35].	48
2.16	Hydrated cement paste and water type in it [14]	53
2.17	(a) Total strain of concrete with time (b) general form of	
	ε-t curve for materials subjected to creep [239].	54
2.18	Strain-time relationship of concrete under sustained	
	loading [248].	57
2.19	Creep isochrones [33]	62
2.20	Creep compliance function [33]	63
2.21	Uni-axial tensile creep performance of pre-cracked	
	samples [267]	71
2.22	Summary of research gap	78
3.1	Experimental framework	82
3.2	Curled long Kenaf fibre	88
3.3	Ordinary portland cement	89
3.4	Distilled Water Machine	91
3.5	Sodium Hydroxide (NaOH)	92
3.6	(a) Kenaf fibre grouped after cutting (b) Untreated Kenaf	
	Fibre (c) Treated Kenaf Fibre	93
3.7	Treatment process: (a) pre-treatment (b) immersion in	
	NaOH (c) distilled water for washing fibre (d) drying of	
	treated fibre	95
3.8	Fibre tensile properties testing machine	98
3.9	SEM testing machine: (a) auto fine coater (b) SEM	
	analysis machine set up	99
3.10	Chopped Kenaf fibre	100
3.11	Revolving pan mixer for concrete	103
3.12	Mixing procedure for KBFCC	104
3.13	Steel mounds; (a) Cubes (100 x 100 x 100 mm), (b)	
	Prisms (100 x 100 x 500 mm), and (c) Cylinder (100 mm	
	x 200 mm).	105
3.14	Fabricated wooden mounds: (a) Prisms for shrinkage	
	(100 x 100 x 285 mm) (b) Prisms for uniaxial tensile	

	$(100 \times 100 \times 500 \text{ mm})$ and (a) Driama for	
	creep test (100 x 100 x 500 mm) and (c) Prisms for	106
	flexural creep test (75 x 75 x 600 mm).	106
3.15	Steel hooks and moulds with steel hooks protruding	
	from the ends	107
3.16	Details of schematic representation of wooden block	107
3.17	An operational procedures for slump test: (a) Abrams	
	slump apparatus, (b and c) Measurement of slump	108
3.18	Operational procedures for Vebe test	109
3.19	Operational procedures for compacting factor test	110
3.20	Specimens for the respective hardened properties test	111
3.21	Initial surface absorption tests	112
3.22	Setup for the determination of total porosity and water	
	absorption.	115
3.23	An operational procedure for ultrasonic pulse velocity	
	(UPV) test	116
3.24	An operational procedure for compressive strength test	118
3.25	An operational procedure for splitting tensile strength	
	test	119
3.26	An operational procedure for flexural strength test	120
3.27	An operational procedure for uniaxial tensile strength	
	test	121
3.28	An operational procedure for modulus of elasticity test	123
3.29	An operational sequence for Shrinkage test.	125
3.30	Controlled room measuring and monitoring device	127
3.31	Schematic representation of creep test set-up for	
	standard size specimens	129
3.32	An operational procedure for compression creep test	130
3.33	A schematic representation of the uniaxial tensile creep	
	rig	134
3.34	An operational procedure for Uniaxial tensile creep test	134
3.35	Creep rigs illustrating specimens under sustained loads	_
	and Demec stud (point) position over a gauge length of	
	150 mm.	136
		150

3.36	75 mm x 75 mm x 600 mm test specimens for flexural	
	creep testing	138
3.37	An operational procedure of flexural creep test	139
3.38	Schematic representation of third point flexural creep	
	loading	141
4.1	The physical appearance of Kenaf fibre before and after	
	alkali treatment	160
4.2	Scanning electron micrographs of Kenaf fibre surface (a)	
	untreated (500x), (b) Treated by mercerisation (500x)	161
4.3	SEM micrograph of (a) Untreated, (b) 5% NaOH at 3hrs,	
	(c) 7%NaOH at 3hrs, (d) 5%NaOH at 24hrs and (e)	
	7%NaOH at 24hrs.	162
4.4	Water sorption curves for distilled water/treated and	
	untreated Kenaf fibre.	163
4.5	The diameter of a typical fibre bundle.	166
4.6	Average diameter of the untreated and treated Kenaf	
	fibre.	167
4.7	Stress-strain manner of un-treated Kenaf fibre in virgin,	
	swollen and desorbed phase.	169
4.8	Morphology image of Kenaf fibre tensile fracture	170
4.9	Stress-strain performance of treated Kenaf fibre in the	
	normal (unswollen) and swollen phase	170
4.10	Grading curve of the coarse aggregates in consonance to	
	ASTM C 33 limits	174
4.11	Grading of the fine aggregate in consonance to ASTM	
	C33 limits	176
4.12	Slump of PC and KBFCC	178
4.13	Effect of fibre volume fraction (v_f) and length (l_f) on	
	slump of concrete	179
4.14	Effect of fibre volume fraction (v_f) and length (l_f) on	
	Vebe of concrete	180
4.15	Effect of fibre volume fraction (v_f) and length (l_f) on	
	compacting factor of concrete	181
4.16	Correlation between Vebe time and Slump of concrete	182

4.17	Correlation between Compacting factor and Slump of	
	concrete	183
4.18	Correlation between Compacting factor and Vebe time	
	of concrete	184
4.19	Fresh density of PC and KBFCC	186
4.20	Effect of fibre content and length on density of hardened	
	concrete	187
4.21	Effect of Fibre content and length on UPV of PC and	
	KBFCC	188
4.22	Relationships between UPV and Compressive strength	
	(CPS) of PC and KBFCC	189
4.23	Effect of Kenaf fibre on initial surface absorption of	
	concrete at 7 days	193
4.24	Effect of Kenaf fibre on initial surface absorption of	
	concrete at 28 days	194
4.25	Effect of Kenaf fibre on compressive strength of	
	concrete at 7 days hydration period	196
4.26	Effect of Kenaf fibre on compressive strength of	
	concrete at 28 days hydration period	197
4.27	Failure mode of (a) PC cube specimen (b) KBFCC cube	
	specimen (c) KBFCC failure surface under	
	compressive load.	197
4.28	Effect of Kenaf fibre on Indirect splitting tensile strength	
	of concrete at 7 days hydration period.	198
4.29	Effect of Kenaf fibre on Indirect splitting tensile strength	
	of concrete at 28 days hydration period.	199
4.30	Failure mode of cylindrical specimen in indirect splitting	
	tensile (a) PC specimen (b) KBFCC specimen.	199
4.31	Ave. ultimate and post-peak (fibre activation) uni-axial	
	tensile strength for PC and KBFCC specimen at 28 days	201
4.32	Average Stress – displacement curve of (a) PC specimen	
	(b) KBFCC specimen under uni-axial tensile load at 28	
	days.	202

4.33	Effect of Kenaf fibre on flexural strength of concrete at 7	
	days hydration period	205
4.34	Effect of Kenaf fibre on flexural strength of concrete at	
	28 days hydration period	206
4.35	Failure mode of prism in flexure (a) PC specimen (b)	
	KBFCC specimen (c) Kenaf fibre bridging the cracked	
	planes.	206
4.36	Failure Pattern of PC and KBFCC at varying fibre	
	content and length for MOE testing	209
4.37	Relationship between elastic modulus and fibre volume	
	fraction at varying fibre length: (a) 25 mm fibre length,	
	(b) 50 mm fibre length and (c) 75 mm fibre length	211
4.38	Influence of fibre addition and age of exposure on the	
	free drying Shrinkage of concrete: (a) 25 mm Fibre	
	length, (b) 50 mm Fibre length and (c) 75 mm Fibre	
	length	215
4.39	Strain deformation of compressive creep of 150 mm	
	diameter x 300 mm and 100 mm diameter x 200 mm	
	cylinder specimen sizes	218
4.40	Compressive creep coefficient of 150 mm diameter x	
	300 mm and 100 mm diameter x 200 mm cylinder	
	control specimen sizes	219
4.41	Strain deformation of shrinkage of 150 mm diameter x	
	300 mm and 100 mm diameter x 200 mm cylinder	
	control specimen sizes	219
4.42	Relationship of compressive creep coefficient between	
	150 mm diameter x 300 mm and 100 mm diameter x 200	
	mm cylinder with various codes and experimental	
	outcome	220
5.1	SEM of 28 days tensile fracture of PC and KBFCC	
	specimen	224
5.2	SEM of (a) fibre matrix interface and (b) bridging action	
	of fibres of KBFCC specimen	225

5.3	SEM of strong bond between Kenaf fibre and concrete	
	of KBFCC specimen	226
5.4	SEM of (a) nest-like clusters (b) fibre groove and	
	cementitious sticks on the interface of KBFCC specimen	227
5.5	SEM of different faces of the KBFCC specimen	228
6.1	Kenaf fibre effects on compressive creep performance of	
	concrete at (a) 7 days loading age (b) 28 days loading	
	age	231
6.2	Stress level effects on compressive creep of PC and	
	KBFCC moist-cured for 7 and 28 days	233
6.3	Effects of loading age (curing condition) on compressive	
	creep of PC and KBFCC loaded at 25% and 35% of	
	compressive strength	234
6.4	Relationship between compressive strength and creep	
	strain of concrete	236
6.5	Relationship of compressive strength to instantaneous	
	strain measured in creep test.	237
6.6	Stress level effects on compressive creep coefficient of	
	concrete moist-cured for 7 and 28 days	238
6.7	Loading age condition effects on creep coefficient of	
	concrete	239
6.8	Effects of fibre inclusion on creep coefficient of PC and	
	KBFCC at 7 day loading	240
6.9	Effects of fibre inclusion on creep coefficient of PC and	
	KBFCC at 28 day loading	240
6.10	Relationship amongst compressive strength and	
	corresponding creep coefficient at 371 days	242
6.11	(a)-(d) Comparison of compressive creep coefficient	
	between experiment results and existing standard codes	
	for concrete tested at 7 days	244
6.12	(a)-(d) Comparison of compressive creep coefficient	
	between experiment results and existing standard codes	
	for concrete tested at 28 days	245

6.13	(a-e) Uni-axial tensile creep crack opening of PC and	
	KBFCC at 25 and 35% load level.	251
6.14	Uni-axial tensile creep coefficients of cracked PC and	
	KBFCC at 25% and 35% load level.	254
6.15	(a-d) Comparison of tensile creep coefficient between	
	experiments results and existing standard codes for	
	concrete tested at 28 days	255
6.16	Load-deflection (L-d) diagram of the PC and KBFCC	
	first set specimen at 7 and 28 days curing age.	259
6.17	Load-deflection $(L-d)$ relationship of PC 7 days beam at	
	flexural test.	263
6.18	Load-deflection (L-d) relationship of PC 28 days beam	
	at flexural test.	263
6.19	Load-deflection (L-d) relationship of KBFCC 7 days	
	beam at flexural test.	263
6.20	Load-deflection (L-d) relationship of KBFCC 28 days	
	beam at flexural test.	264
6.21	Area covered under the load-deflection (<i>L</i> - <i>d</i>)	
	relationship curve 7 days cured PC beam	265
6.22	Area covered under the load-deflection (<i>L</i> - <i>d</i>)	
	relationship curve 28 days cured PC beam	266
6.23	Area covered under the load-deflection (L-d)	
	relationship curve 7 days cured KBFCC beam	266
6.24	Area covered under the load-deflection (<i>L</i> - <i>d</i>)	
	relationship curve 28 days cured KBFCC beam	266
6.25	Flexural creep deflections of PC beam specimen at 25%	
	and 35% load level.	268
6.26	Flexural creep deflections of KBFCC beam specimen at	
	25% and 35% load level.	268
6.27	Stress level effects at 259 days flexural creep value of	
	PC and KBFCC moist-cured for 7 and 28 days	270
6.28	Effects of loading age (curing condition) on flexural	
	creep of PC and KBFCC loaded at 25% and 35% of	
	ARS at 259 days.	271

6.29	(a-d) Comparison of flexural creep coefficient between	
	experiments results and existing standard codes for	
	concrete tested at 28 days.	272
6.30	Compression creep prediction using regression curve	
	fitting	277
6.31	Tensile creep prediction using regression curve fitting	278
6.32	Flexural creep prediction using regression curve fitting	278
6.33	Kenaf fibre effects on shrinkage performance of concrete	
	at (a) 7 and 28 day moist curing.	282
6.34	Effects of curing condition on shrinkage of PC and	
	KBFCC mixes at 378 days	283
6.35	Comparison of shrinkage strain between experiment	
	results	285
6.36	Shrinkage strain regression using curve fitting	289

LIST OF ABBREVIATIONS

ASTM	-	American Society for Testing and Materials
ACI	-	American Concrete Institute
ARS	-	Average Residual Strength
AS	-	Australian Standard
BFCC	-	Bio Fibrous Concrete Composite
BS	-	British Standard
CEB-FIP	-	Fédération internationale du béton – International Federation for Structural Concrete
CPF	-	Compacting Factor
CPS	-	Compressive Strength
CMOD	-	Crack Mouth Opening Displacement
EC 2	-	Eurocode 2
FCC	-	Fibrous Concrete Composite
FS	-	Flexural Strength
ISA	-	Initial surface absorption
KBFCC	-	Kenaf Bio Fibrous Concrete Composite
KF	-	Kenaf Fibre
KFRC	-	Kenaf Fibre Reinforced Concrete
L-d	-	Load-deflection
MOE	-	Modulus of Elasticity
NaOH	-	Sodium Hydroxide
PC	-	Plain Concrete
RH	-	Relative Humidity
SLC	-	Slump of Concrete
SSD	-	Saturated Surface Dry
STS	-	Indirect Splitting Tensile Strength
UPV	-	Ultrasonic Pulse Velocity

VBT-Vebe TimeDOF-Degree of freedom

LIST OF SYMBOLS

$k_1, k_2, k_3 =$	-	Coefficients
$arepsilon_{E}\left(t ight)$	-	Elastic strain at time t (micron)
$\varepsilon_{cr}\left(t ight)$	-	Creep strain at time t (micron)
$\varepsilon_{s}\left(t ight)$	-	Shrinkage strain at time t (micron)
${\Phi}$	-	Creep Coefficient
Ecr	-	Creep Strain
f_{cm}	-	Mean concrete strength
t	-	Time
to	-	Initial time at the beginning of loading or drying
σ	-	Stress
Ε	-	Elastic modulus
ρ	-	Density of concrete
f'c	-	Characteristic cylinder strength
COV_m	-	Coefficient of variation for model prediction
K_{o}	-	Constant related to the elastic modulus of the aggregate
Φ_t	-	Tensile creep coefficient
M_∞	-	Water absorption at the saturation point
Mt	-	Water absorption at time <i>t</i>
n	-	Mechanism of sorption
E_{t}	-	Elastic modulus at age t
$arPsi_{f}$	-	Flexural creep coefficient
$J_{(t,to)}$	-	Creep function

LIST OF APPENDICES

APPENDIX

TITLE

PAGE

A	Mix Design	279
В	Fibre Volume Calculation	280
C	Fibre Water Content Calculation	281
D	Measured and Calculated Results from Creep Tests	282
E	The Prediction of Creep and Shrinkage by AS 3600	286
F	The Prediction of Creep and Shrinkage by EC 2	287
G	List of Publications	334

CHAPTER 1

INTRODUCTION

1.1 General Appraisal

The usefulness of concrete in various building and civil engineering applications is incontestable. Over the years, it has so far been positively used in hydraulic structures, shotcrete, offshore structures, slabs on grade, structures in seismic regions, thin and thick repairs, architectural panels, crash barriers, precast products, footings, global transportation infrastructure systems such as network of roads, bridges, railways, airports, canals and many other applications. The reason for its widespread acceptability for use in various infrastructure productions is not farfetched from the benefit of providing the lowest quotient among cost and strength as equated to other available materials [1–3]. Despite these applaudable qualities of concrete, two unattractive properties: low ductility and breakability possessed by concrete still makes it prone to collapse which occurs just after the creation of deformation and initial crack [1]. This adversely limits the performance of concrete over long-term when exposed to sustained loads like creep and shrinkage [4]. Hence, concrete has a poor resistance to cracking. Steel reinforcement came up as a response to cracking in concrete due to stronger tensile strength it possesses over concrete. In their study, Clarke et al., [5] explained that reinforcement steel bar is saddled to carry the tensile stresses imposed on the concrete and curtail possible cracking of the concrete or cause the concrete to remain largely in compression under load by prestressing it.

Steel reinforcement has been successfully used in concrete over the years and it is still in use. Conversely, cracking still occurs under load and this creates a pathway for various deleterious species such as chlorides, sulphates, moisture and carbon dioxide [6]. This leads to the corrosion of the reinforcement thus affecting the durability of the concrete structures. Other substitutes apart from steel reinforcement are as well obtainable for the reinforcing of concrete to control cracking. One of such substitutes presently used is fibres [6]. The inclusion of short discontinuous randomly oriented fibres (natural, glass, steel, and synthetic) has remained a practice among others towards contributing to the improvement of the two negative properties of concrete mentioned earlier [7-12]. It has been reported by ACI [13], Mehta and Monteiro [14] that fibre inclusion offers a bridging capability once the initial crack take place afore the full parting of a beam. Also the studies of [7,15] supported this assertion of ability of fibre to provide concrete with post crack strength. The improvement of the mechanical and durability properties of concrete such as; crack opening, stiffness, crack promulgation, tensile strength and deformation characteristics such as creep and shrinkage of concrete amid others. These have given fibrous concrete favourable acceptance in becoming a widely used composite material in construction projects. Structural elements deform all the way through their lifespan (creep and shrinkage), which may possibly lead to serviceability concerns such a deflection, cracking, etc. Whereas fibrous concrete has presented substantial ductility and energy absorption aptitude in the short term, the sustainability of such properties in the long term is still undefined. These sustained loads could be as a result of mechanical stress known as creep or environmental stress from temperature and relative humidity causing the shrinkage of concrete elements. Creep is defined as the plastic deformation under sustain load. Creep strain depends primarily on the duration of sustained loading. It has been widely acknowledged that creep of concrete is greatly influenced by the surrounding ambient, admixtures and load intensity. Also creep has been seen to induce the deflection of structural member with time. Hence the study on creep of concrete is necessary to prevent failure of structure [16]. While shrinkage of concrete is described as time-dependent volume change that occurs due to a number of mechanisms. Shrinkage has been reported to occur due to the movement of water in both fresh and hardened states [17]. The creep and shrinkage of concrete element are categorised as time-dependent deformation properties of concrete.

There exist several types of fibres that is incorporated in concrete, but the most commonly used are the natural (vegetable), steel, glass, asbestos, carbon and polypropylene type of fibres [1,8,12,18,19]. These resource fibres have gains in the matrix proportioning of cement composites. Bio fibres are believed to be more

environmentally pleasant to the users; this is why they are currently getting appreciable consideration for substituting the glass, synthetic and steel fibres [12,20]. Researchers [21–23] in the past years have investigated and compared the benefit and properties of natural, steel and synthetic fibre. They succinctly described natural fibres to possess many benefits than the synthetic and other type of fibres. Such advantages are low density, carbon dioxide requisitioning, low cost, recyclability, issue of sustainability, biodegradability, and competitive specific mechanical properties [24–26]. However, if the compressive and tensile strength of bio fibre concrete is to some degree lesser than the control concrete mix, its deformation behaviour displays more enhancement in ductility [11,18,22,23]. Some studies have been carried out on the properties of concrete with the bio fibres which is usually referred to as fibrous concrete from sugar cane, coconut coir, malva, hemp, ramie bast, jute, pineapple leaf, elephant grass, bamboo, akwata and sisal with encouraging results recorded [12,27–30].

1.2 Background of the Problem

In view of the current global challenges, the construction industry has been focusing on the concept of sustainability, particularly the inclusion of biodegradable fibre in concrete [31,32]. Bio fibres as being adjudged as a means of mitigating the effect of carbon footprint to the environment. This has evoked a lot of response from industries who are seeking more eco-efficient production and sustainable commerce. Bio fibres are a major renewable (CO₂ neutral) resource for bio-based economical developments. Serviceability and durability performance has been given more emphasis in the design and analysis of concrete structures. The ultimate limit state requirement is no longer the only main focus in structural design as durability and service performance are as well important for the safety, aesthetics and economic values of the structure or concrete composite. Creep and shrinkage are a usually a critical property used for evaluating buckling, stresses, cracking, deflection and failure of brittle materials such as concrete for structures under constant loads. Nevertheless, owing to the fact that the effects for under-prediction of creep and shrinkage are timedependent, attention and provisions on these factors are often been ignored [33]. Creep and shrinkage prediction models meant for concrete structure design references are obtainable in AS 3600, CEB-FIP Model Code 1990, B3 Model, ACI-209, Eurocode 2, and BS 8110 Model concrete standard codes. These models are however mostly developed for plain concrete (PC). Therefore, there is need to examine their suitability in the prediction of bio fibrous concrete, and also to development an analytical creep and shrinkage model for the design of bio fibrous concrete such as KBFCC.

The deformation experienced in concrete structure due to tensile low strength and energy absorption low capacity problems can be controlled by replacing it with fibrous concrete. This is a sustainable substitute concrete type where long-term performance and durability are the key considerations. Remarkably, the commonly used fibre types in the production of fibrous concrete such as steel, asbestos, synthetic and glass are usually associated with high cost, corrosion, non-renewable, high specific weight and being harmful to the environment. These factors are not good for our world and the construction industry which is striving towards achieving sustainable environment. Therefore, bio fibre such as Kenaf fibre which is cheaper, environmentally friendly, could be a sustainable choice for the construction industry. To use this fibre in concrete, a detail research on its long-term performance under mechanical load (creep) or environmental load (shrinkage) is required. Recent studies revealed the immense potential and interest generated due to the application of Kenaf fibre in the construction industry, automobile industry, wood-based sector and textile industry. Consequently, the Malaysian government and some other developing nations have pursued vigorously the cultivation and various measures to promote downstream value processing of Kenaf among smallholders and estate owners [34]. Appreciable experimental and theoretical researches have been carried out to understand the mechanical properties of KBFCC [11,23,35]. Most of these studies are limited to the short-term performance of KBFCC under static mechanical loads and environmental loads [11,23,35,36]. Conclusively, it has been observed that the study on concrete composite system made from bio fibrous concrete has been of interest due to its need for the evaluation of stresses, deflection, cracking, bulking and failure of structures made from KBFCC. This will avail material engineers and structural designer's knowledge and data on the material properties and structural behaviour pertaining to serviceability performance.

1.3 Statement of the Problem

The world's population and wealth increase in this century have heightened the rising needs for sustainable materials. These needs have become imperative due to the fact that landfills are filling up, earth climate is changing, and natural resources are diminishing. This has promulgated the recent researchers to finding alternative materials for the replacement of the use of synthetic fibre, steel fibre, and steel bar in concrete because of their unfriendliness to the environment, their ignition of environmental issues, poor recyclability, non-biodegradability, expensive costs and high maintenances and repair cost of damaged structure via corrosion of steel.

These general environmental issues are climate change, ozone depletion, ecotoxicity, fossil fuel depletion, water extraction, waste disposal, eutrophication (overenrichment of water sources), acid deposition, summer smog (low-level ozone creation) and minerals extraction. These problems initiated the increase in carbon dioxide, CO₂ gas which generates unsafe environment and human health problems. Additionally, the emission produced during the petroleum product production (synthetic fibre) could bring about global warming and cause an increase in greenhouse effect. Bio fibres tend to be more sustainable compared to synthetic fibres. Carbon, Polypropylene and aramid based synthetic fibres have been introduced and applied in areas of construction such as buildings, bridges and pipelines. Glass fibres are produced from silica which is derived from sea sand. Continuous exploitation of sea sand for the production of glass fibres has led to other whole new complications. Sea sand will run out if its use continuously and the production of glass fibre also requires high budgets. Several fibrous concrete that is popularly used is from synthetic (Carbon, Polypropylene), steel and glass sources. Further research has to be done to advance the suitability of bio fibres as reinforcement to replace usage of synthetic (Carbon, Polypropylene), steel and glass fibre to produce more economical, light weight, degradable, environmentally friendly, bio-based economy and structural concrete composites.

Fibrous concrete composites are produced to restrain the propagation of a crack in the concrete and to improve its tensile strength and ductility/deformation properties. Bio fibrous concrete composites (BFCC) is an environmentally sustainable material. Its potential advantages which varied from using bio fibres to decreasing synthetic and steel fibres which are from petroleum and steel source, respectively. Due to the hydrophilic properties of bio fibre, this makes them unsuitable for use in concrete reinforcement and strengthening. An elaborate study to reduce the hydrophilic effect is necessary in order to utilize the positive properties accrued to Kenaf fibres in fibrous concrete production. The alkaline treatment of Kenaf fibre has been reported in the literature to inhibit the possibility of its decay over time. Despite the elaborate research on the short-term deformation behaviour of BFCC, publications directly related to long-term deformation behaviour of BFCC compressive, uniaxial tensile, flexural creep and shrinkage are uncommon. Though the usage of Kenaf fibre in concrete is however of current interest with gradual growing reports on its short-term mechanical demeanour in scholarly articles; there still exists a dearth of knowledge on the physical and mechanical performance of KBFCC. Also, for acceptability and immense application of KBFCC in engineering practice, the concrete science and industry must be provided with proof of systematic scientific study that shows and that analyse the features of BFCC on short and long-term structural performance. High on the list of main structural performance properties is creep and shrinkage of the concrete composites.

Long-term performance of KBFCC under sustained static loads (creep) and shrinkage, as well as analytical models for estimation of creep and shrinkage properties, is yet to be studied. Furthermore, studies on the cement matrix bonding interface of KBFCC is rare to find in the literature. To have a better understanding of the relationship and behaviour of KBFCC under sustained loads, a proper investigation has to be conducted. This calls for an extensive study into the long-term performance of KBFCC under sustained loads as a result of mechanical stress and environmental load commonly referred to as creep and shrinkage, respectively. Similarly, there is need to understand the cement matrix bonding interface of the KBFCC by carrying out the morphological examination of the concrete exposed to the short and long-term loading.

1.4 Aim and Objectives

The aim of this research is to investigate the creep and shrinkage performance of KBFCC. In order to achieve the above aim, the following specific objectives were formulated.

- i. To examine the physical and mechanical properties of concrete containing kenaf fibre at varying fibre lengths and volume fractions.
- To examine the effect of Kenaf fibre on the shrinkage and creep properties of KBFCC in compression, tension and flexure.
- iii. To assess the effect of mechanical loading on the microstructure characteristics of KBFCC.
- iv. To evaluate the prediction model of compressive, tensile, flexural creep and shrinkage of KBFCC and propose a model for estimation of creep and shrinkage behaviour of KBFCC.

1.5 Scope of the Study

This research work is experimental and focused on assessing the long-term performance of KBFCC under shrinkage and compressive, tensile, flexural sustained loads which is within the limit of the set objectives. The scope of the study was divided into four stages:

- i. Kenaf fibre characterisations, material properties testing, short-term mechanical and time-dependent properties testing of KBFCC were carried out. The optimum fibre length and volume fraction was determined from the physical and mechanical properties of KBFC tested. The optimum KBFCC mix of 0.5% fibre volume fraction and 50 mm fibre length was used in the production of specimen used for the shrinkage and creep testing. Also, the design and construction of uniaxial tensile and flexural creep specimen mould, rig, test set-up and procedures were defined.
- ii. ASTM C512 [37] standard was used as a reference in carrying out the compressive creep test. The works of Babafemi [6] and Fladr [38] served as a reference to the experiment conducted on uni-axial tensile creep and flexural

creep, respectively. The shrinkage test was conducted in conformity with ASTM C157 [39].

- KBFCC specimen testing under long-term shrinkage, compressive creep, iii. uniaxial tensile creep and flexural creep loading was conducted on standard size specimens. 100 mm diameter x 200 mm cylinder was used as a modification to the 150 mm diameter x 300 mm diameter given in ASTM C512 [37] for the compressive creep test. 75 mm x 75 mm x 600 mm prism and 100 mm x 100 mm x 500 mm prism were used for uni-axial tensile creep and flexural creep, respectively, as given in the work of Babafemi [6] and Fladr [38]. For the shrinkage test, the prism specimen dimension of 100 mm x 100 mm x 285 mm prism was used in accordance with ASTM C157 [39]. The tensile creep specimens were pre-cracked and the crack mouth opening displacement was determined as the creep deformation characteristic of the bio fibrous concrete. The focal variables considered are the effect of fibre inclusion in concrete (0% and 0.5% KF at 50 mm length), sustained stress loads (25% and 35%) and age at loading (7 and 28 days) on creep. For the shrinkage testing, the tested specimen where made of three different group of 25 mm, 50 mm and 75 mm Kenaf fibre length at 0%, 0.25%, 0.50%, 0.75%, 1.0%, 1.5% and 2.0% fibre volume fraction. The specimens were tested at drying age of (7 and 28 days). The creep and shrinkage tests were all carried out in the controlled room of 23±2°C and RH of 50±4. The optimum fibre volume fraction and length was used in the prediction model analysis.
- iv. The morphology of Kenaf fibre, KBFCC and their cement matrix interface of the tensile fractured surface under short term and long term mechanical load were investigated.
- v. The experimental data was analysed and the relationships among different properties of KBFCC was determined. The creep and shrinkage (timedependent deformation properties) prediction model codes evaluated in this study are from ACI-209, CEB-FIP 1990 Model Code, Eurocode 2 (EC 2), and Australian Standard 3600 (AS 3600). The best prediction model code for KBFCC was identified after evaluation.

1.6 Thesis Organization

The thesis was presented in seven chapters. Chapter 1 presents a general appraisal and a brief description of the background problem. In addition, the chapter also spelt out the aim and objectives, scope and limitation, research hypothesis, the significance of research and the research approach.

Chapter 2 is concerned with the critical review of the relevant and related literature.

Chapter 3 provides the materials and the chronological sequence of the methodology that is employed for successful completion of the research using appropriate standard and modification where necessary in conducting the tests.

Chapter 4 reveals the characterisation of the constituent materials, comprising the physical properties and chemical composition. The treatment of Kenaf fibre and its water sorptivity characteristics and mechanical test are discussed. This chapter also descried the effect of Kenaf fibre geometry (length) and volume fraction on fresh and hardened concrete properties. Parameters studied in this chapter include workability regarding the slump, compacting factor, Vebe of concrete, and fresh density. Also, the relationship between some data is developed to establish a correlation. It also presents the results obtained and discussion made on the evaluation of mechanical and durability properties. Tests falling in this class include; flexural, modulus of elasticity, compressive, water absorption (porosity), tensile strength, and shrinkage. The optimum content and length of the fibre meant to be used in the production of the creep and further shrinkage study was determined and presented.

Chapter 5 deals with the evaluation of the morphologies of the KBFCC. Also, the microstructure characteristics of the fibre matrix interface of KBFCC exposed to sustained loading was examined and discussed in this chapter. The scanning electron micrograph (SEM) results are presented and discussed in this chapter.

Chapter 6 focuses on the creep and shrinkage performance of KBFCC. The evaluation, statistical analysis, determination of best prediction model code for prediction of creep and shrinkage of KBFCC.

Chapter 7 presents the conclusion of this thesis by stating the outcomes and, successes of the study and the contribution of the research to the existing information. Recommendations are made for further research in related areas to improve the

properties of concrete using Kenaf bio fibre for the production of a green and sustainable concrete.

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305

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310

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321

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