

1           **Recycled waste foundry sand as a sustainable subgrade fill and pipe-**  
2           **bedding construction material: engineering and environmental evaluation**

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## ABSTRACT

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Waste foundry sand (WFS) is the primary by-product of foundries. Due to metals present in WFS and negative public perception, this material is commonly discarded to landfill as a waste material. WFS can however be potentially reused as a construction material in civil engineering infrastructure projects. In order to use WFS in a sustainable manner, the engineering properties of this material needs to be properly evaluated and assessed against local requirements. In this research, geotechnical and environmental tests were undertaken to evaluate the properties and viability of WFS for usage in civil engineering construction projects. In addition, control tests were undertaken on recycled glass (RG), a well-accepted waste material that has been successfully implemented in civil engineering applications, for benchmarking purposes. Geotechnical test results, including determination of maximum dry density (MDD) and optimum moisture content (OMC), California bearing ratio (CBR) and permeability, indicate that WFS can satisfactorily be used as fill material in embankments and in pipe-bedding applications. Comparisons of the environmental test results such as chemical composition and leachate analysis, with the requirements of local authorities indicated no particular hazards in the implementation of this material in applications such as road embankment fills and pipe-bedding. The carbon footprint savings through any potential reuse of WFS/RG was furthermore quantified.

**Keywords:** Foundry sand; Environmental; Embankment; Subgrade; Pipe-bedding.

## 68 **Abbreviations**

69 ASLP Australian standard leaching procedure

70 CBR California bearing ratio

71 Cc Coefficient of curvature

72 Cu Coefficient of uniformity

73  $D_{\max}$  maximum particle size

74 Gs Specific gravity

75 MDD Maximum dry density

76 OMC Optimum moisture content

77 RG Recycled glass

78 WFS Waste foundry sand

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## 80 **1 Introduction**

81 Casting and molding of ferrous and non-ferrous materials is undertaken at foundries  
82 (Salokhe and Desai, 2011). This requires specific sized high quality silica sand in order to  
83 manufacture molds used for pouring and casting molten metal. Combined application of  
84 binders and the silica sand provides a precise shape to molds (Lin et al., 2012). Typical  
85 binders used for this action include natural binders (such as bentonite clay) and chemical  
86 binders which are used for high temperature operations (Siddique and Singh, 2011). Once  
87 the desired shape is precisely generated in the mold, the molten metal is poured in. Repeated  
88 utilization of high quality silica sand for casting and molding in foundries results in the  
89 production of waste foundry sand (WFS) (Lin et al., 2012). In fact, the sand used to create  
90 the required shape in the mold is repeatedly used for the casting process until it is

91 thoroughly contaminated, at which point the WFS is discarded, often to landfills  
92 (FHWA, 2004 and Saloke and Desai, 2011).

93 Waste sands are widely used in geotechnical applications and are divided into several major  
94 categories: foundry sands, raw slags, heavy ashes and metal fractions. Among these, WFS is  
95 commonly used due to its availability, mineral-rich properties and overall similarities in  
96 properties to natural and recycled sands (Saloke and Desai, 2011). Typically, WFS can be  
97 categorized into green sand and chemically bonded sand, depending on the type of  
98 binder used in casting (Siddique and Singh, 2011). Depending on the color, WFS can be  
99 distinguished on the basis of binders. Green sand colors black or grey whereas  
100 chemically bonded sand colors medium tan or off white (Siddique and Singh, 2011). As  
101 dumping this by-product is often costly, it has recently been used in applications such as hot  
102 mix asphalt fillers, cement manufacture (FHWA, 2004), embankments (Mast and Fox, 1998;  
103 Partridge et al., 1998) and road subbases (Guney et al., 2006; Goodhue et al., 2001).

104 Countries such as the USA, India, China, Australia and Taiwan generate millions of tons of  
105 waste WFS, which poses an enormous environmental challenge (Lin et al., 2012). The  
106 sustainable usage of WFS provides an economical and environmentally friendly solution as  
107 compared to the high costs of disposing to landfills and for quarrying virgin materials (Siddique  
108 and Singh, 2011). Partridge et al. (1999) and Guney et al. (2006) have reported that WFS  
109 material is safe to be used in some engineering applications. WFS is hydrophilic by nature and  
110 absorbs high amounts of water. Also, due to existence of phenols, this material may be  
111 corrosive (Siddique and Singh, 2011). Suitability of application of WFS in regards to  
112 environmental issues can be evaluated through leachate analysis. In the landfills for instance,  
113 precipitation and percolation of the water through deposited material generates leachate  
114 (Siddique et al., 2010). In the majority of past research, WFS was either stabilized using  
115 cementitious material (cement, lime, etc.), or used as a substitute to the sand portion of a blend,  
116 such as concrete mixture or hot mix asphalt.

117 **Table 1** presents a summary of results of a few research works, as well as, typical properties  
118 presented in FHWA (2004). In this table, values of optimum moisture content (OMC),  
119 maximum dry density (MDD), and California bearing ratio (CBR) corresponding to specimens  
120 compacted using standard compaction effort are presented. In two of the selected research  
121 works, WFS was used solely without being mixed with other materials. In the others, however,  
122 it was blended with bentonite (Abichou et al., 2000), mixed with cement (Naik et al., 2001), or  
123 used together with geosynthetics (Guney et al., 2006). Generally, just a few research works  
124 were encountered in the literature review in which WFS was used as an individual material,  
125 instead of being mixed with other materials in a blend. In recent years, recycled materials have  
126 been evaluated and deemed acceptable in various civil engineering infrastructure applications  
127 (Arulrajah et al., 2014a). Recycled glass (RG) in particular, has made significant inroads in  
128 recent years and has been deemed suitable for applications such as embankment fills (Wartman  
129 et al. 2004), pavement subbases (Arulrajah et al., 2014b), cement treated pavement base  
130 (Arulrajah et al., 2015a), footpath bases (Arulrajah et al., 2013), as well as light-weight fill  
131 applications (Arulrajah et al., 2015b). The environmental properties of RG have also been  
132 established as being compliant with required regulatory requirements (Imteaz et al., 2012). RG  
133 is furthermore sold commercially in Australia and is marketed as a recycled sand product. RG  
134 is therefore considered an ideal material for benchmarking the performance of WFS as an  
135 engineering fill and pipe-bedding material. Conducting a series of studies on WFS, as with RG,  
136 provides the engineers and designers with adequate knowledge on properties of this material  
137 and paves the way to extensive reuse of this waste material in civil engineering projects. In this  
138 regards, comparing the properties of WFS with an approved recycled material (RG) gives a  
139 clearer appreciation of its suitability in similar applications.

140 Even though the majority of the WFS evaluated in the literature meet the environmental  
141 requirements, applying a leachate analysis protocol is recommended for each new source of

142 WFS that is intended to be used (FHWA, 2004). Furthermore, the majority of the recent  
143 research works only focus on the properties of the blends in which WFS is used as a component,  
144 rather than properties of WFS by itself. Application of WFS without mixing with other  
145 materials, if the requirements are met, can save costs and effort needed for the mix design and  
146 blending and mixture preparation. At the same time, it meets the aim of reusing WFS rather  
147 than dumping it in landfills.

148 In this research, the environmental and engineering properties of WFS, obtained from a  
149 recycling facility in Melbourne, Australia, were evaluated and the suitability of this material as  
150 a subgrade fill and pipe-bedding material was reported. Key gaps in recent research on WFS,  
151 such as comparisons of its properties with another widely accepted alternative recycled sand  
152 product, being RG as an engineering fill and pipe-bedding material were a primary focus of  
153 this research. The properties of WFS as benchmarked with RG will answer key remaining  
154 questions on the engineering and environmental performance of WFS as compared to other  
155 accepted recycled materials in applications such as engineering fill and pipe-bedding, and  
156 positive outcomes will lead to wider acceptance of WFS as a construction material. The carbon  
157 footprint savings through any potential reuse of WFS/RG was furthermore quantified.

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## 159 **2 Materials and Methods**

160 The WFS and RG used in this research were provided from a recycling construction and  
161 demolition facility in Melbourne, Australia. The WFS was black in color, due to the presence  
162 of contaminants, during operational works. The RG was a mixed colored glass, which is too  
163 fine a material to be color sorted back into bottle-making, and thus enters the waste stream

164 (Arulrajah et al., 2014b). **Figure 1(a)** shows a photo of WFS while **Figure 1(b)** shows a photo  
165 of RG.

166 The particle size distribution of WFS was obtained using ASTM D6913-04 (2009). In addition  
167 to the sieves recommended in the standard, 2.36 mm, 1.7 mm and 1.18 mm sieves were used  
168 so that a more precise PSD was achieved. Also, 250 g samples were used so that overloading  
169 limits for each sieve according to ASTM D6913-04 (2009) was met. Specific gravity ( $G_s$ ) of  
170 the material was obtained using ASTM D854-14 (2014). In this regard, 100 g of dry material  
171 was used and method B (Procedure for oven-dry samples) was applied using a 500 mL  
172 pycnometer. Deairing was done using a vacuum pump and a shaking table for agitating the  
173 slurry while it was under vacuum for two hours.

174 Standard compaction procedure, according to ASTM D698-15 (2015), was carried out to  
175 determine the moisture content-dry density relationship of the materials. A 101.6 mm diameter  
176 by 116.43 mm high mold was used and the specimens with 5 different moisture contents,  
177 ranging between 7 to 14%, were prepared. Each specimen was compacted in 3 layers, under  
178 standard compaction effort of 25 blows.

179 California bearing ratio (CBR) tests were conducted in accordance with ASTM D1883-14  
180 (2014). A 152 mm diameter by 177.1 mm high mold was used, and WFS and RG were wetted  
181 to their corresponding optimum moisture content (OMC) and were compacted in 3 layers using  
182 standard compaction effort. In order to investigate the swelling potential of the material  
183 (existence of clay), a dial gauge was used while the CBR specimens were submerged in water  
184 for 96 hr. The CBR values at 2.54 mm and 5.08 mm penetration were then obtained using  
185 stress-penetration curves, with the higher CBR value being reported. In this regard, correction

186 for concavity of the stress-penetration curves done following ASTM D1883-14 (2014)  
187 procedure.

188 Hydraulic conductivity of the materials was obtained using constant head permeability test  
189 according to (ASTM-D2434, 2006) which is applicable for granular materials. Samples were  
190 compacted in a 152 mm diameter mold in 3 layers using standard compaction effort. The head  
191 difference was 1.14 meter of water column. Permeability of a recycled/reused material is a  
192 useful measure for evaluation of its potentials for leaching.

193 An X-ray fluorescence test was conducted to determine the chemical composition of the WFS  
194 and RG. The hazard category of WFS was determined based on the Environmental Protection  
195 Authority (EPA, 1999 and 2010) Victoria and Australian standard leaching procedure (ASLP)  
196 (AS, 1997), which is a bottle leaching procedure. The allowable maximum particle size for this  
197 procedure is 2.4 mm, which is greater than  $D_{max}$  of materials used in this research, hence, no  
198 sieving was required. The environmental properties of the WFS were tested for different types  
199 of heavy metals by following the Australian standards protocol (AS, 1997) for the preparation  
200 of leachate, using neutral water ( $pH = 7$ ) as leaching fluid. Leachate was produced by  
201 contacting the WFS and RG with the leaching fluid. This was done by placing the material in  
202 the bottle of the apparatus and adding the leaching fluid. The bottle was then sealed and  
203 mounted into an agitator to be shaken for 18 hours. The mix was then filtered using a glass  
204 fiber filter and the filtered liquid was used for leachate analysis. If the ASLP leachate  
205 concentrations are less than the specified limits, or if it can be demonstrated to be of natural  
206 origin, the WFS can be categorized as suitable for fill materials.



### 207 3 Results and Discussion

208 The geotechnical and environmental properties of WFS were compared with those of RG, a  
209 well-accepted recycled waste material for benchmarking purposes. **Figure 2** presents the  
210 particle size distribution of WFS and RG and also reports on other properties including  
211 maximum particle size ( $D_{max}$ ), mean particle size ( $D_{50}$ ), coefficient of uniformity ( $C_u$ ) and  
212 coefficient of curvature ( $C_c$ ). The particle size distribution curves indicate that the WFS  
213 contains about 2% fines, has a  $D_{max}$  of 2.36 mm, and has a  $C_c$  lower than 6. Therefore, it is  
214 classified as poorly graded sand while RG is well graded sand. Atterberg limit tests are not  
215 applicable for these materials, due to very low percentage of fine particles. In the majority of  
216 the research works mentioned in the introduction section, WFS was poorly graded.

217 **Figure 3** presents the compaction curve of WFS, as well as the OMC and MDD of WFS and  
218 RG. The compaction curve shows that compared to RG, WFS has lower MDD, even though  
219 WFS has greater specific gravity value. This is attributed to the fact that the RG blend was  
220 well-graded, whereas WFS blend is poorly-graded. Also, greater OMC of WFS suggests that  
221 water absorption of this material is higher than that of RG. The MDD of WFS falls in the range  
222 of typical foundry sand (without fine particles) available in the literature (**Table 1**). However,  
223 the optimum moisture content of WFS in this research is greater than the upper range of typical  
224 WFS with no clay/silt presented in FHWA (2004). This might be due to presence of about 2%  
225 clay in the WFS used in this research. Also, OMC as high as 15.5 was reported in Partridge et  
226 al. (1999) which is well above that of WFS of this research.

227 No significant reading was observed on the dial gauges after 96 hours of submerging the CBR  
228 specimens in water, suggesting that these materials were non-swelling and contained negligible  
229 or low percentage of clay. CBR was then conducted on the specimens. **Figure 4** presents the  
230 stress-penetration curves for WFS and RG. CBR values for WFS were greater than the typically

231 specified within the range of 2% to 5%. This is the local road authority specification  
232 requirements for a structural fill material in road embankments. Therefore, WFS meets the  
233 requirements to be used in road applications, to RG. Evidently, RG achieves greater CBR  
234 values than WFS, which can be attributed to its larger particle size, as well as a well-graded  
235 particle size distribution. The CBR value of the WFS is close to the lower limit of the typical  
236 WFS presented in (FHWA, 2004). However, the minimum CBR value reported in the literature  
237 was 4.3 and belongs to Kleven et al. (2000).

238 Hydraulic conductivity of the WFS was  $5.20 \times 10^{-8}$  m/s, which is highly lower than that of RG  
239 ( $9.79 \times 10^{-6}$ ). Permeability of the WFS used in this research is a bit greater than the lower limit  
240 presented in **Table 1** for typical WFS without fine particles, but falls between the range  
241 presented by Abichou et al. (2000). Generally, permeability of WFS tends to be lower than  
242 typical sand and is not therefore considered as a freely draining material (Partridge et al., 1999).  
243 This makes it suitable for construction materials where low permeability is required, such as  
244 landfill covers, liners, and even earth dam cores (Deng and Tikalsky, 2008).

245 A summary of the geotechnical properties of WFS is presented in **Table 2** and compared with  
246 those of RG. Generally, RG presents better properties, including higher MDD and CBR value;  
247 however, WFS also presents acceptable properties for embankment fill applications. From an  
248 engineering material perspective, the properties of the WFS coupled with its satisfactory  
249 engineering and environmental results indicate that the material is ideal for usage as a fill  
250 material in embankments or retaining, walls as well as a pipe-bedding material. The properties  
251 of the WFS used in this research are to a great extent similar to those used in previous research  
252 with satisfactory results (**Table 1**).

253 **Table 3** presents the chemical composition of the WFS used in this research obtained from X-  
254 ray fluorescence (XRF). Total amount of major components in WFS ( $\text{SiO}_2$ ,  $\text{Al}_2\text{O}_3$ , and  $\text{Fe}_2\text{O}_3$ )

255 is 97.50%. Major components of RG include SiO<sub>2</sub>, CaO, and Al<sub>2</sub>O<sub>3</sub> which constitute 97.69%  
256 of the blend. Evidently, both the materials contain large SiO<sub>2</sub> content due to their origins from  
257 sands. Generally, high amounts of SiO<sub>2</sub> in aggregates result in greater hardness (Siriphun et al.,  
258 2016).

259 A disadvantage in applications with WFS could be the potential of leaching toxic substances  
260 Leachate analysis, especially for WFS, is important since it has been exposed to melt metals in  
261 high temperatures during the casting process. This could introduce toxic metals into WFS  
262 (Guney et al., 2006). The majority of the studies carried out on evaluation of the leachate from  
263 WFS show that concentration of hazardous material was lower than the limits provided by the  
264 authorities. However, a few research works, such as (Coz et al., 2004), among others, have  
265 reported concentration of contaminants in WFS that exceeded the safety limits. This suggests  
266 necessity of conducting leachate analysis on any new source of WFS that is intended to be used  
267 for construction and have potential of leaching. **Table 4** presents the leachate analysis data of  
268 the WFS and RG and compares it to the requirements for fill material, drinking water and  
269 hazardous waste. Based on the U.S. Environmental Protection Agency, a material is considered  
270 as hazardous if any metal is present in concentrations greater than 100 times that of the drinking  
271 water standards (Wartman et al., 2004). A comparison of the leaching results indicates that all  
272 metal contaminants are well within allowable limits for the usage of WFS as a fill material. In  
273 RG, however, only for lead, the leachate concentration gets close to threshold defined by EPA  
274 Victoria for solid inert waste. But considering that the leachate values, reported in **Table 4** for  
275 WFS, are extracted using more aggressive acidic and borate solutions compared to neutral pH  
276 water, it can be expected that in case of using this material in the field and event of storm water  
277 passing through the material, the concentration of heavy metals will be less than what reported  
278 in **Table 4**. This means that the material will not pose any risk to the ground water tables or  
279 water streams beyond what is commonly accepted for fill material and solid inert waste.

280 **Figure 5** presents a schematic and a water flow balance diagram for the usage of WFS fill  
281 material in a typical application as a road embankment fill material. Precipitation due to rainfall  
282 will hit the pavement surface layer, with some of it subsequently evaporating and the balance  
283 becoming run-off that will discharge down the slopes and into the drains provided at the bottom  
284 of the road embankment. Some infiltration will occur into the WFS fill material layer. Leachate  
285 will seep into the ground water table below; hence, the necessity for the environmental testing  
286 analysis undertaken in this research. Based on the above-mentioned leaching and engineering  
287 analyses, the WFS is found to be suitable as a non-structural fill material for road  
288 embankments. As a structural fill material in road embankments, the particle size distribution  
289 of the aggregates meets the requirements of local road authority specifications.

290 Evidently recycled materials will contribute to total energy savings considering the effects of  
291 embodied energy. Embodied energy is the total energy that is associated in bringing a material  
292 to its existing virgin state (Soga et al., 2011). Embodied energy is closely related to the resource  
293 depletion and greenhouse gas emission, as more embodied energy means more greenhouse gas  
294 emissions. Moreover, dumping the high embodied energy material contributes high energy  
295 depletion/waste. Hence, this parameter reflects the energy-efficiency and environmental effect  
296 of a material.

297 Earlier studies revealed that the use of RG as engineering material is able to save total energy  
298 related to the material up to 2 orders of magnitude, as compared to virgin aggregate-cement  
299 (EPA, 2012; Nassar and Soroushian, 2013; Tsai, 2005). WFS is a recycled waste material and  
300 is not intentionally produced for construction. Hence, the embodied energy of WFS is regarded  
301 as zero. In contrast, the embodied energy of conventional Portland cement additive is as high  
302 as 4.6 MJ/kg (Hammond and Jones, 2008). Ignoring the transportation cost (which will be  
303 close/similar to other virgin material), the total energy consumption related to the use of WFS  
304 as construction material in practice (e.g., non-structural fill material) is therefore zero, whereas

305 that of a conventional aggregate-cement material depends on the cement dosage and weight  
306 employed in any construction project. If WFS is used to replace quarry sand resource, then  
307 based on the unit data reported by Racusin and McArleton (2012) per ton the use of WFS will  
308 save embodied energy of 81 MJ; and will reduce carbon emissions of 4.8 kg CO<sub>2</sub> and 5.1 kg  
309 CO<sub>2</sub> e.

#### 310 **4 Recommendations for future research**

311 In the present research, WFS was evaluated in terms of environmental and basic geotechnical  
312 properties. It met the local authority requirements for environmental safety. However, more  
313 advanced geotechnical testing is required to investigate its suitability in a range of other civil  
314 engineering applications. Since it is a type of recycled sand, investigating the shear strength  
315 properties and compressibility of the WFS is recommended. In addition to that, blending this  
316 material with other recycled materials, such as recycled construction and demolition materials  
317 with the aim of using a 100% recycled blend is recommended. A field trial on WFS will  
318 furthermore provide conclusive evidence of actual performance of this material under actual  
319 loading conditions. In regards to environmental assessment, as some contaminants (although  
320 below specified limit) are present in the WFS sample, it is recommended to investigate whether  
321 concentrations of contaminants can be reduced through some soil treatment, i.e. soil washing.

#### 322 **5 Conclusions**

323 A series of geotechnical and environmental tests were conducted on WFS and benchmarked  
324 against RG to evaluate the engineering properties of WFS and to investigate the viability of  
325 using this by-product of foundry industries in road construction. WFS were found to meet the  
326 local road authority requirements as a non-structural fill and pipe bedding material. The particle  
327 size distribution curves indicate that the WFS was poorly graded and comprised essentially of

328 sand sized particles. CBR values for WFS are greater than the typically specified within the  
329 range of 2% to 5%, which is the local road authority specification requirements for a structural  
330 fill material in road embankments. The WFS contained a large SiO<sub>2</sub> content due to its origins  
331 from natural sands. Comparing geotechnical testing results of WFS with RG indicates that the  
332 properties of WFS are lower than that of RG. However, engineering properties of WFS, such  
333 as compaction and CBR values make it acceptable for fill embankment applications.

334 Leachate analysis results were obtained and compared with the requirements of regulatory  
335 authorities. Results indicated no environmental risks for using WFS in road applications, such  
336 as embankment fill and pipe bedding. Evidently the leachate through this material is not  
337 suitable for drinking. Pollutants in the leachate will go through diffusion and dispersion  
338 processes before it reaches the ground water source, as such concentrations of any pollutants  
339 will be significantly reduced. Such transport of pollutants can be precisely calculated using  
340 groundwater flow models, which is out of scope for this research. Moreover, the use of WFS  
341 instead of quarry sand will save embodied energy, as well as reducing carbon footprint.

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Table 1. Summary of the WFS properties presented in the literature

| Research work                                | $G_s$     | $D_{max}$ (mm) | OMC (%)   | MDD (Mg/m <sup>3</sup> ) | CBR (%) | USCS                   | Permeability (m/s)                          | Safe Environmentally | Can be used solely |
|--|-----------|----------------|-----------|--------------------------|---------|------------------------|---|----------------------|--------------------|
| Partridge et al. (1999)                      | 2.53      | -              | 15.5      | 1.43                     | 16.8    | -                      | $1.2 \cdot 10^{-8}$                         | Yes                  | Yes                |
| Kleven et al. (2000)                         | 2.52-2.73 | 4.75           | 9.6-13.8  | 1.69-1.88                | 4.3-40  | SP/SM (majority)       | -   | Not reported         | Yes                |
| Abichou et al. (2000)                        | 2.51-2.62 |                | 10.8-12.3 | 1.65-1.86                |         | SM/SC (majority)       | $9 \cdot 10^{-11}$ -<br>$5.3 \cdot 10^{-7}$ | Not reported         | No                 |
| Naik et al. (2001)                           | 2.79      | 2.36           | -         | -                        | -       | SP                     | -   | Yes                  | No                 |
| Goodhue et al. (2001)                        | 2.52-2.68 | 4.75           | 9.6-15    | 1.72-1.88                | -       | SP-SM/<br>SW-SM/<br>SC | -   | Yes                  | No                 |
| Typical WFS (with clay/silt) (FHWA, 2004)    | 2.5-2.7   | 1.18-4.75      | 8-12      | 1.76-1.84                | 11-30   | SP-SM/<br>SP-SC        | $10^{-9}$ - $10^{-5}$                       | Inconclusive         | Inconclusive       |
| Typical WFS (without clay/silt) (FHWA, 2004) | 2.6-2.8   | 1.18-4.75      | 8-10      | 1.60-1.76                | 11-30   | SP                     | $10^{-8}$ - $10^{-4}$                       | Inconclusive         | Inconclusive       |

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Table 2. Engineering properties of WFS and RG

| Engineering Parameter               | WFS                   | RG                    |
|-------------------------------------|-----------------------|-----------------------|
| Specific Gravity ( $G_s$ )          | 2.59                  | 2.48                  |
| Coefficient of Uniformity ( $C_u$ ) | 2.06                  | 7.5                   |
| Coefficient of Curvature ( $C_c$ )  | 0.92                  | 1.5                   |
| Standard Proctor OMC (%)            | 12.5                  | 12.05                 |
| Standard Proctor MDD ( $Mg/m^3$ )   | 1.748                 | 1.777                 |
| CBR (%)                             | 10.9                  | 39                    |
| Permeability (m/s)                  | $5.20 \times 10^{-8}$ | $9.79 \times 10^{-6}$ |

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Table 3. Chemical composition of WFS and RG

| Chemical  | WFS    | RG     |
|---|--------|--------|
| Composition (%)                                   |        |        |
| Silica (SiO <sub>2</sub> )                        | 84.145 | 80.124 |
| Aluminium oxide (Al <sub>2</sub> O <sub>3</sub> ) | 11.817 | 3.980  |
| Ferric oxide (Fe <sub>2</sub> O <sub>3</sub> )    | 1.533  | 0.688  |
| Calcium oxide (CaO)                               | 1.507  | 13.583 |
| Sulfur trioxide (SO <sub>3</sub> )                | 0.453  | 0.436  |
| Potassium oxide (K <sub>2</sub> O)                | 0.287  | 0.561  |
| Titanium dioxide (TiO <sub>2</sub> )              | 0.257  | 0.399  |
| Manganese dioxide (MnO <sub>2</sub> )             | -      | 0.027  |
| Chromia (Cr <sub>2</sub> O <sub>3</sub> )         | -      | 0.071  |
| Zinc oxide (ZnO)                                  | -      | 0.027  |

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Table 4. Leachate analysis data for WFS and RG.

| Contaminant | WFS<br>(mg/L) | RG<br>(mg/L) | Industrial Waste<br>Upper Limit (EPA<br>2009) (mg/L) | Drinking Water<br>Upper Limit(EPA<br>1999) (mg/L) |
|-------------|---------------|--------------|--|---|
| Arsenic     | -             | <0.01        | 0.35   | 0.05  |
| Barium      | 0.133         | 0.1          | 35   | 2   |
| Chromium    | <0.1          | <0.01        | 2.5  | 0.1   |
| Copper      | <0.1          | -            | 100  | 1.3   |
| Lead        | <0.1          | 0.19         | 0.5  | 0.015   |
| Nickel      | <0.1          | -            | 1  | 0.1   |
| Selenium    | <0.05         | <0.01        | 0.5  | 0.05  |
| Vanadium    | <0.1          | -            | -  | -   |
| Zinc        | 1.067         | -            | 150  | -   |
| Mercury     | <0.001        | <0.001       | 0.05   | 0.002   |

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529 Figure 3. Compaction curves for WFS and RG.

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531 Figure 5. Water flow balance chart for WFS as a fill material in road embankments.

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