

Research Article

Optimal Facility Location Model Based on Genetic Simulated Annealing Algorithm for Siting Urban Refueling Stations

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This paper analyzes the impact factors and principles of siting urban refueling stations and proposes a three-stage method. The main objective of the method is to minimize refueling vehicles' detour time. The first stage aims at identifying the most frequently traveled road segments for siting refueling stations. The second stage focuses on adding additional refueling stations to serve vehicles whose demands are not directly satisfied by the refueling stations identified in the first stage. The last stage further adjusts and optimizes the refueling station plan generated by the first two stages. A genetic simulated annealing algorithm is proposed to solve the optimization problem in the second stage and the results are compared to those from the genetic algorithm. A case study is also conducted to demonstrate the effectiveness of the proposed method and algorithm. The results indicate the proposed method can provide practical and effective solutions that help planners and government agencies make informed refueling station location decisions.

1. Introduction

With the rapid economic development in China, automobile ownership has increased drastically in the past decade. Due to this, there is an urgent need for upgrading transportation infrastructure to accommodate the newly generated traffic. Refueling stations, either gasoline stations or electric vehicle charging stations, are a very important type of service facility and are essential for the appropriate operations of urban transportation systems. The locations of refueling stations can directly affect the efficacy of urban road traffic system. Inappropriate location designs can substantially increase vehicle travel distances for finding refueling stations and cause unnecessary congestion and emissions. Therefore, investigating the optimal refueling station locations has important practical significance for developing countries such as China. This research is also important for some developed countries that are investing heavily in alternative-fueled vehicles (e.g., electric vehicles), where needs for optimally locating electric vehicle charging stations exist.

Some existing refueling station location models are based on qualitative analysis. These methods can generally be categorized into two groups: maximum coverage and maximum profit methods [1]. The maximum coverage methods are mainly used for high population density and congested areas. Their objective is to maximize the coverage of refueling stations and reduce the unnecessary travel distance caused by vehicles searching for refueling stations. The maximum profit methods are designed for low population density areas. Their primary goal is to ensure that refueling stations at selected locations can have adequate demand to maintain certain level of profit [2]. Both methods cannot provide quantitative results to demonstrate the economic and traffic impacts of different refueling station location plans.

Several related quantitative researches have also been conducted. Current et al. [3] investigated a network design problem. They formulated it as a Maximum Covering/Shortest Path (MCSP) model with two objectives. The first objective was to find the shortest path between any pair of start and end points, and the second objective was to maximize

the number of demand nodes covered by the shortest path. In their research, if a node is on the shortest path or within certain distance of any nodes on the shortest path, then it is considered as being covered. Bapna et al. [4] extended the MCSP formulation to a Maximum Covering/Shortest Spanning Subgraph Problem (MC3SP) that considers multiple pairs of start and end points. The focus of their research was to optimally locate unleaded gasoline stations to facilitate trips among large and populated cities. Building upon the MCSP and MC3SP, Y.-W. Wang and C.-R. Wang [5] formulated a flow-based set covering location model. Similar to [3, 4], they assumed the refueling demand of a city to be concentrated on the center of the city. In other words, a city was considered as a single demand point. All these models [3-5] tried to cover as many nodes or population along the shortest path as possible while keeping the total cost low. However, a more important issue in refueling station location modeling is to cover more OD (origin and destination) flows with refueling demand. Also different from these studies, the focus of our research is to investigate refueling station location problems within urban areas rather than for intercity trips.

Goodchild and Noronha [6] proposed a combined model to site a finite number of refueling stations to maximize the refueling market share of a firm. As pointed out by Kuby and Lim [7], a major problem of this combined model is that it used link flow data instead of OD flow data as the input. Therefore, OD flows may be double-counted as they pass through several road links. Hodgson [8] conducted one of the pioneer studies on siting refueling stations and developed a Flow Capturing Location Model (FCLM). The goal of the FCLM was to capture more OD flows (instead of more link flows as in [6]) on the shortest paths between OD pairs. This can avoid double-counting OD flows captured by multiple refueling stations along their shortest paths. A number of improvements have been done based on the FCLM. Hodgson and Rosing [9] extended the FCLM and proposed a hybrid flow capturing plus p-median model with two conflicting objectives, which were maximizing the coverage of OD flows and minimizing demand-weighted travel distance. Later on, Kuby and Lim [7] proposed a Flow-Refueling Location Model (FRLM) based on the FCLM. A major difference between FCLM and FRLM is that a facility combination concept was proposed. OD flows were covered by a combination of facilities instead of individual facilities in the FRLM model. Recently, Lim and Kuby proposed a number of heuristic algorithms for solving the developed FRLM [10, 11]. These studies [7–10] all considered a single shortest path connecting each OD pair. Their common objective was to optimally locate a prespecified amount of refueling stations such that the total number of shortest paths being covered could be maximized. In the real world, traffic flow between an OD pair rarely takes a single shortest path. Travelers typically follow the user equilibrium (UE) principle and try to minimize their individual travel times. This usually results in many different routes between an OD pair. Unfortunately, this diverse route choice behavior is not considered in these refueling station location studies [7-10].

In the following subsection, a comprehensive analysis of factors affecting refueling station location planning is presented. Based upon this analysis, a three-stage method is proposed to optimally locate refueling stations. Instead of focusing on the regional level and considering each city as a single demand point [3–5], this study centers on urban areas and the entire city is divided into small traffic analysis zones (TAZs). In addition, the model introduced in this study explicitly considers the diverse route choice behavior between origins and destinations and can better reflect real world traffic scenarios.

2. Principles for Siting Urban Refueling Stations

2.1. Principles of Location Design. Siting urban refueling stations should consider a comprehensive set of factors. Government agencies are mostly concerned about congestion, safety, environment, and land use factors, while refueling station operators are more concerned about demand and profit. Besides, fuel price difference at various refueling stations is also an important factor to consider. However, in China gasoline prices are decided by the government and there is almost no difference in terms of price among different gas stations. Therefore, the price competition among different refueling stations/companies is not considered in this study. In general, siting refueling stations should (a) minimize their impacts on urban traffic, (b) be on main roads to ensure profitability, (c) keep an appropriate distance between refueling stations, (d) be far from residential and other sensitive areas, and (e) be based on urban land use planning.

Principle (a) is from the perspective of minimizing the influence of refueling stations on urban road traffic. For example, a vehicle with refueling need should not have to detour too far for refueling. Intuitively, this requires refueling stations to be located on or near the paths of most vehicles, so that they do not need to make a detour or just need to make a very short detour.

Principle (b) considers the economic benefit of refueling stations. The number of refueling vehicles that may potentially be served by a refueling station should be taken into consideration during the location design. Ideally, this number should be within a reasonable range to ensure that a refueling station's profit is above certain level and the refueling demand does not exceed its capacity.

Principle (c) takes the service ranges of refueling stations into consideration. It is to make sure that refueling stations are not too concentrated in certain areas. Keeping an appropriate distance between refueling stations sometimes is necessary due to reasons such as safety.

Principle (d) is mainly about the safety and environmental concerns. Based on this principle, siting refueling stations should consider the safety of refueling station facilities and equipment, refueling vehicles, travelers, and nearby residents. Also, refueling stations should be far away from some environmentally and socially sensitive areas.

Last but certainly not least, principle (e) optimizes refueling station locations from the urban planning point of view. Specifically, urban refueling station locations should take into account land use development and future land use planning. Land use directly affects trip productions and attractions. Consequently, it has major impacts on the distribution of future refueling demand and refueling station locations.

From the standpoint of model development for optimally siting refueling stations, principle (a) can be considered in the objective function, and principles (b) through (e) can be either included as constraints or used to determine candidate locations for future refueling stations.

2.2. Three-Stage Model. Based on the analysis in the previous section, if principle (a) is adopted as the objective, there will be two cases in the refueling station location planning. The first case is that refueling stations are located on the paths of some refueling vehicles. Hence, these vehicles do not need to detour for refueling. The other case is that refueling stations are not the paths of the remaining refueling vehicles and these vehicles have to detour for refueling. In light of these two cases, siting refueling stations can be implemented in three stages. In the first stage, candidate refueling stations on road segments traveled by most refueling vehicles are selected. In the second stage, the focus is on refueling vehicles that are not covered in the first stage. The objective is to minimize the total detour distance of these vehicles by properly selecting additional refueling stations. The third stage is based on the results of the first and second stages. It considers the redistribution of the paths for refueling vehicles due to traffic congestion and refueling station capacities. The redistribution result is used to fine-tune the location plan generated in the first and second stages.

Specifically, the first stage starts with assigning all vehicles (including refueling vehicles) to the road network. Based on the traffic assignment results, the paths of refueling vehicles are identified. From these paths, road segments that cover the highest refueling vehicle OD demand are selected for constructing refueling stations. The refueling ODs covered/served by these road segments are then removed from the original refueling vehicle OD. The regular vehicle OD and the unsatisfied refueling vehicle OD are assigned again to the road network to find additional candidate locations.

At the end of the first stage, the demands of some refueling ODs may still be unmet. Therefore, additional refueling stations need to be constructed and some refueling vehicles have to detour. When selecting locations for these additional refueling stations, it is natural to consider the objective of minimizing the total detour distance.

In the first and second stages, refueling vehicles are assigned to the road network in an incremental manner and refueling station locations are added gradually. In the third stage, all vehicles (including refueling vehicles) are assigned to the network simultaneously given the locations selected in the first and second stages. This most likely will result in a traffic flow pattern (e.g., choice of stations for refueling vehicles) that is different from those in the previous two stages. For instance, some previously overloaded refueling stations (based on the result of the second stage) may end up with not having enough demand. For those without enough demand in the second stage, they may not have adequate capacity to satisfy the refueling demand. For the lack of demand scenario, the corresponding refueling stations can be simply removed/unselected. For the lack of capacity scenario, additional refueling stations need to be added. To add new refueling stations, the first step is to find refueling vehicle ODs being assigned to the overloaded refueling stations. Next, road segments on these OD pairs' paths and the refueling traffic volume on each of these segments are identified. Based on the principles discussed in the previous section, these segments' refueling volumes are then sorted in a descending order and used to select new refueling stations. During this adjustment process, at least one new station that helps with diverting refueling demand should be selected. The new location plan may need to be fed into the second stage to be further improved. This three-stage model is summarized in Figure 1.

3. Optimization Model for Siting Urban Refueling Stations

3.1. Variables and Refueling Demand

3.1.1. Definitions of Variables and Parameters. Given a road network G(N, A), N is the set of all nodes in G(N, A), and A is the set of all road sections in G(N, A), a_m $(m = 1, 2, ..., n) \in A$. Suppose all major and minor arterial segments, c_m $(m = 1, 2, ..., n) \in C$, are considered as candidate locations for refueling stations. Before the models are introduced, the following variables and parameters used in the models are defined below.

- (1) x_{a_i} is the traffic flow of road segment a_i ;
- (2) t_{a_i} is the travel time of road segment a_i ;
- (3) ζ is a binary variable. 1 stands for constructing a station and 0 for not;
- (4) *M* is the minimum refueling demand for constructing a refueling station;
- (5) *S* is the service area of the road network;
- (6) p (p = 1, 2, ..., n) is the set of candidate locations for constructing refueling stations;
- (7) b_p is the cost of constructing a refueling station at candidate location p, and B_{max} stands for the maximum average cost;
- (8) $t_{pp'}$ is the travel time between refueling stations p and p' and should be in the range of $[t_{d\min}, t_{d\max}]$;
- (9) ρ stands for the density of refueling stations and its range should be [ρ_{min}, ρ_{max}];
- (10) Q_{ij} is the travel demand between origin *i* and destination *j* and q_{ij} is the corresponding refueling demand;
- (11) f_k^{ij} is the traffic volume on path k between origin i and destination j.

3.1.2. Vehicle Refueling Demand. Except for the vehicle refueling demand q_{ij} , most of the variables and parameters defined in the previous section either are given or can be readily calculated. The vehicle refueling demand however is closely



FIGURE 1: Flowchart for the three stages.

related to the vehicle travel demand (Q_{ij}) and should be determined carefully.

For different OD pairs, even the same vehicle's average refueling frequencies can be different. This is mainly due to two factors. First, different OD pairs have different average travel distances, which naturally result in different fuel consumptions. Second, vehicles from different OD pairs may experience different levels of congestion, which will also affect their fuel consumptions. On a particular day, travel demand of an OD pair is separated into demand with refueling needs and demand without refueling needs. The demand between origin i and destination j with refueling needs is estimated using (1). This study assumes that travelers do not have a special preference for certain refueling stations. This is a reasonable assumption for planning purpose, since in China gasoline prices are the same at different stations and it is hard to tell which refueling station(s) will be more attractive in the future. Consider

$$q_{ij} = D_{ij}\left(u_{ij}\right) = Q_{ij} \cdot \frac{A\left(v\right) \cdot u_{ij}}{C_{\text{gas}}}, \quad \forall i, j.$$
(1)

In (1), D_{ij} is the demand function between *i* and *j*. This monotonic function depends on the travel distance between *i* and *j* and has an upper limit; u_{ij} is the average travel distance between *i* and *j* in kilometers; A(v) is the average vehicle fuel consumption measured in liter/100 km. It is a function of the actual driving speed v in km/hr, which can be estimated using the method in [11]; and C_{gas} is the average fuel tank size measured in liters.

3.2. First-Stage Model Based on FCLM. The first-stage model is shown in (2) below and illustrated in Figure 2. It is built upon the Flow-Capturing Location-Allocation Model (FCLM) proposed by Hodgson [8] and the idea for the first stage discussed previously. One has

Maximum
$$\sum_{a_i \in A} q_{a_i} \zeta_{a_i}$$
s.t.
$$q_{a_i} \ge M \quad (a_i \in P)$$

$$\zeta_{a_i} = \begin{cases} 0 & a_i \notin P \\ 1 & a_i \in P. \end{cases}$$
(2)

3.3. Location Models for the Second Stage. After completing the first stage, the refueling demands of some vehicles are still unmet. When adding additional refueling stations (also called public refueling stations in this study) to address this issue, our objective is to minimize the total detour distance for those ODs with unmet refueling demands. Specifically, a bilevel location model is proposed that reflects the concerns of both government agencies and travelers. In the upper-level model, the total detour time is used as the objective function. Constraints considered include travel time between refueling stations, average construction cost, and refueling station density. The lower-level model assigns the unmet refueling ODs to the network using Stochastic User Equilibrium (SUE).



FIGURE 2: First-stage location model based on FCLM.

This assignment is based on the resultant road network of the first stage. This bilevel second-stage model is also shown in Figure 3. The third stage does not have a specific model. It is simply to fine-tune the result of the second-stage model.

Step 1 (upper-level location model). Consider

min
$$z(x) = \sum_{i} \sum_{j} q_{ij} \left(t'_{ij} - \overline{t}_{ij} \right)$$
 (3)

s.t.
$$t_{d\min} < t_{pp'} < t_{d\max} \quad p, p' \in P$$
 (4)

$$\frac{\sum_{p \in P} \zeta_p b_p}{\sum_{p \in P} \zeta_p} \le B_{\max}$$
(5)

$$\rho_{\min} \le \frac{\sum_{p \in P} \zeta_p}{S} \le \rho_{\max}.$$
(6)

In (3), t'_{ij} is the average travel time of refueling vehicles for OD pair (i, j), \bar{t}_{ij} stands for the average travel time of vehicles that do not need refueling, and the remaining variables have all been defined previously.



FIGURE 3: Location models of the second stage.

Step 2 (lower-level location model). Consider

$$\min \quad Z(x,q) = \sum_{a_i \in A} \int_0^{x_{a_i}} t_{a_i}(\omega) \, d\omega$$
$$- \sum_{ip} \int_0^{q_{ip}} D_{ip}^{-1}(\omega) \, d(\omega)$$
$$- \sum_{pj} \int_0^{q_{pj}} D_{pj}^{-1}(\omega) \, d(\omega)$$
$$- \sum_{ij} \int_0^{q'_{ij}} D_{ij}^{-1}(\omega) \, d(\omega)$$
s.t.
$$\sum_a f_k^{ij} = q'_{ij}, \qquad \sum_a f_k^{ip} = q_{ip}, \qquad \sum_a f_k^{pj} = q_{pj},$$
$$\forall i, j, p$$

$$f_{k}^{ij}, f_{k}^{ip}, f_{k}^{pj} \ge 0, \quad \forall k, i, j, p$$

$$q_{ij}', q_{ip}, q_{pj} \ge 0, \quad \forall i, j, p,$$

$$p \in P,$$
(7)

$$t_{ip^{\text{opt}}} + t_{p^{\text{opt}}j} < t_{iP'} + t_{P's}, \quad p^{\text{opt}} \in P,$$

$$P' = \left\{ p \mid p = P - p^{\text{opt}} \right\}.$$
(8)

In constraint (8), p^{opt} represents the refueling station closest to an OD pair among all candidate refueling stations *P* and *P'* is the set of refueling stations excluding p^{opt} .

4. Algorithms

4.1. Overview. This study relies on two core models: the firststage model based on the FCLM and the second-stage model for adding and fine-tuning refueling stations. The first-stage model is relatively simple. Thus, this paper will briefly discuss the algorithm for the first-stage model and will focus on developing the algorithm for solving the second-stage model.

The traffic assignment in the first-stage model is based on SUE assignment. From the result of each assignment, a road segment with the highest refueling traffic volume is selected. This segment is selected for constructing a refueling station if its refueling volume is greater than a prespecified value. The corresponding OD will be excluded from further considerations. This process repeats until no qualified segments can be found for constructing refueling stations.

The upper-level of the second-stage model is a nonlinear program and the lower-level is a SUE problem. Heuristic algorithms are very popular for solving nonlinear programming problems. For instance, genetic algorithm (GA) is often used for finding global optimal solutions, and simulated annealing (SA) has good local search capability. This paper proposes an integrated genetic simulated annealing (GSA) algorithm to solve the upper-level model, generating feasible solutions for the lower-level model. The lower-level model basically performs SUE traffic assignments and provides the upper-level model with $\{x_{a_i}, t_{a_i}\}$. In this study, both GA and GSA algorithms are considered and their results are compared.

4.2. Genetic Simulated Annealing (GSA) Algorithm. GA was motivated by Darwin's theory of natural selection, while SA was inspired by the process of heating and cooling of materials in a controlled setting. By integrating these two algorithms, we hope to further improve GA by enhancing its local search with SA.

The proposed algorithm consists of an inner loop and an outer loop, which are based on GA and SA, respectively. The inner loop generates binary vectors (e.g., chromosomes) representing various refueling station location solutions and evaluates their fitness. The algorithm then switches to the outer loop and applies SA to further improve the fitness of these chromosomes locally. Upon finishing the local search, the algorithm returns to the inner loop and applies crossover and mutation operators to avoid being stuck at local minimum solutions. The crossover and mutation operators, initial temperature of annealing, and the annealing rate are all important factors that may affect the algorithm's performance and should be carefully considered. The following steps describe the proposed algorithm in more detail.

Step 1. Since this is essentially a facility location problem, a binary vector of length n is used to represent each refueling station location solution. Each vector element (or each gene) represents a candidate location. 1 means the corresponding location is selected to construct a refueling station, and 0 means it is not selected. Pseudorandom number generator (PRNG) is used to generate initial refueling station location solution set Pop(k), k = 0. The solution set size is Popsize, meaning it includes Popsize solutions. Also, the initial temperature is set to T_0 ; final temperature is T_e ; cooling rate is TR; and the maximum number of iterations for the outer loop is set to Oitermax.

Step 2. This step calculates the fitness values for individual solutions/chromosomes. Based on the results, the fittest chromosome *i* and its fitness value F(i) are determined. This chromosome is the one that has the best objective function value (i.e., fitness value) among all chromosomes in the current population. Therefore, we set $S_{\text{best}} = i$ and $F_{\min} = F(i)$. Also, $f_i(T_k)$ is used to denote the target value for chromosome *i* under temperature T_k , where $i \in \text{Pop}(k)$ is a refueling station location solution in current solution set.

Step 3. The entire algorithm will stop upon reaching the final temperature T_e and it will output the best chromosome S_{best} and the optimal fitness value F_{\min} . Otherwise it will randomly select a state $j \in N(i)$ from the neighborhood of each chromosome $i \in \text{Pop}(k)$ in the current solution set. The new state j will be either accepted or rejected according to the acceptance probability $p_{ij} = \min\{1, \exp[-((f_j(T_k) - f_i(T_k))/T_k)]\}$. $f_i(T_k)$ and $f_j(T_k)$ are the target values of states i and j, respectively. This step takes Popsize iterations and generates a new solution set Newpop1 for refueling stations.

Step 4. This step calculates the fitness values $f_i(T_k) = \exp\{-((f_i(T_k) - f_{\min})/T_k)\}$ of all solutions in Newpop1, where f_{\min} is the minimum fitness value among all solutions. At the end, Popsize solutions/chromosomes from Newpop1 are selected with probabilities inversely proportional to their fitness values to form a new solution set Newpop2.

Step 5. Crossover and mutation are applied to Newpop2, which generate crosspop and mutpop, respectively. The cross-over and mutation processes are described below.

(i) Crossover Operator. For crossover, two parent chromosomes (representing two refueling station location solutions) A_1 and A_2 are selected to form a new chromosome. Each gene of the new chromosome is selected from the corresponding gene in either A_1 or A_2 based on probabilities inversely proportional to their fitness values. Suppose the fitness values of A_1 and A_2 are f_1 and f_2 , respectively; the crossover process is carried out as follows.

(a) Calculate the probabilities for genes to be selected from A₁ and A₂

For
$$A_1: P_1 = \frac{f_1}{f_1 + f_2}$$
,
For $A_2: P_2 = \frac{f_2}{f_1 + f_2}$.
(9)

(b) Generate a sequence α_i (i = 1, 2, ..., k) with each element being a random number within [0, 1]. Compare each element in this sequence with P_1 or P_2 to determine whether a gene from A_1 or A_2 should be selected. For example, if $P_1 < P_2$, for $\alpha_i < P_1$, the *i*th gene from A_1 will be selected for the *i*th gene in the new solution; otherwise, the *i*th gene from A_2 will be selected.

(*ii*) Mutation Operator. Assume a parent solution selected for mutation is $X = x_1 x_2 x_3 \cdots x_n$. x_k $(k = 1, 2, 3 \dots, n)$ is the mutation point and it is defined on the domain of $[A_k, B_k]$. A new solution is generated based on mutation as follows:

$$x_{k'} = \begin{cases} x_k + f(t, B_k - x_k) \operatorname{rand}(0, 1) = 0\\ x_k + f(t, x_k - A_k) \operatorname{rand}(0, 1) = 1 \\ f(t, y) = y \left(1 - r^{\beta(1 - (t/T))}\right), \end{cases}$$
(10)

where *r* is a random real number uniformly distributed between 0 and 1, *t* is the current number of iterations, *T* sets the maximum number of iterations, and β is a prespecified parameter and can be adjusted based on *t*. It is set to a large value at the beginning and reduces gradually as *t* increases. rand(0, 1) randomly selects 0 or 1 as the output. Finally, the mutated chromosomes and their parent chromosomes *X* are both included in the solution set.

The mutation operator is applied to chromosomes with the highest and lowest fitness values. Applying it to the fittest chromosome is for accelerating the speed for local search, while applying it to the least-fit chromosome is to increase the diversity of the population and to avoid premature convergences.

Step 6. Set Pop(k + 1) = mutpop and find chromosome *i* that minimizes $f_i(T_k)$. If $f_i(T_k) \le f_{\min}$, set $S_{\text{best}} = i$ and $F_{\min} = f_i(T_k)$. Finally, set $T_k = T_0 \exp(-C \cdot k^{1/N})$ and k = k + 1 and go to Step 3.

5. Case Study

5.1. Description of Case Study Problem

5.1.1. Road Network and Parameters. To validate the proposed model and algorithm, a case study was conducted using the urban road network in Figures 4 and 5. This network



FIGURE 4: Road network with node and segment numbers.

consisted of 128 nodes and 234 segments. In Figure 5, green lines represent major arterials, yellow lines are for minor arterials, and blue lines denote secondary roads. The numbers next to each segment are its capacity and free-flow travel time. The entire study area was about 90 km². In order to mitigate traffic congestion in downtown, no refueling stations were allowed within the red circle in Figure 5. Also, it was assumed that no refueling stations can be built on secondary roads.

Additionally, the following assumptions were made: the maximum average construction cost for a refueling station is 3 million Chinese yuan; the density of refueling stations is between 0.1 and 0.3 stations/km²; the minimum distance between two refueling stations is 4 km; and each station should serve at least 300 vehicles in order to be profitable and no more than 500 vehicles due to capacity constraint.

5.1.2. Vehicle Refueling Demand. It was assumed that each node is the centroid of a Traffic Analysis Zone (TAZ). The travel demand between two TAZs was estimated by $f_{ij} = P_i P_j / d_{ij}^{1.5}$, where P_i and P_j stand for the trip production of nodes *i* and the trip attraction of node *j*, respectively. Both P_i and P_j were assumed to be 5000 trips/day for all nodes. d_{ij} is the distance between two nodes.

Considering peak-hour factor and mode split, the peakhour travel demand between two TAZs was estimated to be 38,537 pcu/h. By applying (1), the corresponding refueling demand was estimated to be 6,775 pcu/h. Due to page limit, detailed steps for the above calculations are not included in this paper. Also, based on the refueling station density and capacity constraints, it was assumed that between 14 and 23 refueling stations should be constructed.

5.2. Modeling Results

5.2.1. Results of the First-Stage Model. TransCAD 5.0 was used to solve the first-stage model. As shown in Table 1, the first-stage model finished after 8 iterations. During this stage, the capacity of refueling stations was not considered. In other words, we assumed that a refueling station can satisfy all passing vehicles' refueling demand.

Table 1 shows that the refueling demand of Segment 194 is 278 pcu/h, which is less than 300 pcu/h and does not meet the minimum demand requirement for constructing a refueling station. Therefore, the first-stage model stopped after the 8th iteration. The result in Table 1 suggests that there is no obvious correlation between segment flow and segment refueling demand. The result of the first-stage model is also shown in Figure 6.

The stations selected during the first stage can directly satisfy a refueling demand of 3,430 pcu/h, which is about half of the total demand, meaning that approximately 50% of the refueling vehicles do not need to detour. In the following section, the unmet refueling demand will be discussed.



FIGURE 5: Segment capacity and travel time.

TABLE 1: Result	of the	first-stage	model.
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Number of iterations	Segment number	Segment direction	Segment flow (pcu/h)	Refueling demand (pcu/h)
1	125	$112 \rightarrow 121$	4637	788
2	162	$133 \rightarrow 132$	2902	610
3	111	$105 \rightarrow 106$	3966	552
4	103	$101 \rightarrow 102$	2341	421
5	56	$75 \rightarrow 74$	2612	418
6	154	$128 \rightarrow 129$	1983	333
7	98	$98 \rightarrow 107$	1853	308
8	194	$154 \rightarrow 202$	2141	278

5.2.2. Result of the Second-Stage Model

(*i*) *Model Initialization*. As described previously, the secondstage model first generated Popsize = 100 chromosomes and each chromosome consisted of 151 genes. Each gene is a binary variable, representing whether the corresponding segment is selected for constructing a refueling station.

(*ii*) Parameter Setting. To prevent premature convergence, the following parameters were used: initial temperature in the annealing process $T_0 = 1000$, terminating temperature $T_e = 0$, cooling speed TR = 0.95, maximum number of iterations for each temperature was 100, crossover rate was 0.40~0.99,

mutation rate was $0.0001 \sim 0.1$, crossover probability was 0.9, mutation probability was 0.04, and the maximum number of iterations Oitermax = 2000.

The constraints in the second-stage model were considered by introducing a penalty. All solutions not meeting constraints (4)~(6) were punished by a multiplication of 10. The elitist strategy was adopted to select chromosomes to be included in the next generation, meaning chromosomes were selected with probabilities inversely proportional to their fitness values.

(*iii*) *Modeling Result*. VB and GISDK in TransCAD were used to implement the second-stage algorithm. Among them, VB

Number of iterations	Shortest total detour time (min)	Detour times of other solutions (min)						
1	26752	27078	28244	27927	28470		28154	28637
2	26066	27129	28141	28923	28228		27130	26639
3	25045	28991	26479	27130	27910		28192	28087
4	25384	28136	28467	26486	27130		25631	26177
5	25218	25596	25922	26140	26551		26841	25676
6	23806	24469	25921	26685	24534		25170	25191
7	22362	22796	22344	22867	22845		25917	22948
8	22269	23082	23505	23600	22796		22796	25692
:	÷	÷	÷	÷	÷	÷	÷	÷
872	6991	8729	9455	7086	7138		7283	7136
873	6782	8007	8795	6926	6792		8869	6936
874	6688	6688	6688	8360	8024		6688	6688
875	6688	6688	6688	8125	7727		6688	6688
876	6688	6688	6688	6688	6688		6688	6688



• Urban refueling station

FIGURE 6: Locations selected for refueling station during the first stage.

was used for coding the GSA algorithm, and GISDK was used for implementing the SUE assignment algorithm. After 876 cooling iterations, the temperature was 3.06E-17 and an optimal solution was obtained. The average run time was 24 minutes and 46 seconds. The obtained shortest total detour time for refueling vehicles was 6,688 minutes.

Table 2 lists the shortest total detour time and the detour times for different chromosomes at each iteration step. The iterative process is also illustrated in Figure 7, in which the horizontal axis stands for temperature and the vertical axis stands for fitness value. Figure 7 shows that the optimal fitness curve goes up and down at the beginning and tends to stabilize and level out after 876 iterations. This optimal fitness curve is not monotonically decreasing because the simulated annealing module sometimes accepts chromosomes with suboptimal (i.e., worse) fitness values with a small probability.



FIGURE 7: Iterative process of the second-stage model.

TABLE 3: Algorithm performance comparison.

Algorithm	Average number of iterations	Average CPU time
Genetic simulated annealing algorithm	876	24 min 46 s
Genetic algorithm	1226	34 min 39 s

The newly added stations in the second stage are shown in Figure 8. These stations provide service to the remaining 49.37% refueling vehicles.

(*iv*) *Comparison of Algorithms*. In this section, GA and the GSA algorithms are compared. The result is reported in Table 3. As can be seen, on average it takes the GA more iterations (1226) than the GSA algorithm to obtain the same solution. Also, the GA requires longer CPU time. For a larger

 TABLE 2: Result of the second-stage model.

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Parameter	Population size	Crossover rate	Mutation rate	Iterations	Calculating time	Bypass time
Optimization scheme	100	0.9	0.04	876	24'46''	6688′
Test scheme 1	90	0.9	0.04	704	20'15''	6847'
Test scheme 2	110	0.9	0.04	991	32'51"	6688′
Test scheme 3	100	0.85	0.04	657	18'34''	7351′
Test scheme 4	100	0.95	0.04	1324	$40^{\prime}42^{\prime\prime}$	6688′
Test scheme 5	100	0.9	0.03	754	21'23"	7023′
Test scheme 6	100	0.9	0.05	1521	51'34''	6688′





Urban refueling station

FIGURE 8: Refueling stations selected during the second stage.

and more complex road network, the advantages of the GSA algorithm could be more significant.

(v) Sensitivity Analysis of Model Parameters. To assess the sensitivity of the GSA algorithm to model parameters, we experimented with various population sizes, crossover rates, and mutation rates. The modeling results are summarized and compared in Table 4. It seems that these parameters do have an important impact on model performance and should be decided carefully based on test runs.

5.2.3. Result of the Third Stage. The stations selected in the previous two stages are shown in Figure 9, since these stations were selected in an incremental manner. If we assign the refueling traffic to the network considering all stations in Figure 9 simultaneously, the refueling vehicles will be redistributed among these stations as shown in Table 5, which shows that three refueling stations now have demand greater than their capacities (i.e., 500 pcu/h) and another three stations are unprofitable (demand less than 300 pcu/h).

The purpose of the third stage was to adjust the solutions generated in the previous two stages according to the method described in Figure 1. The result is shown in Table 6. As we adjusted/relocated refueling stations, the standard deviation



FIGURE 9: Preliminary location plan for refueling stations.

TABLE 5: Refueling traffic volumes of each refueling station based on the location plan generated by the second-stage model.

Station number	Refueling demand
Number 2	605
Number 5	577
Number 6	535
Number 1	476
Number 3	468
Number 4	417
Number 7	378
Number 8	347
Number 14	332
Number 9	329
Number 10	327
Number 17	321
Number 11	318
Number 16	313
Number 12	316
Number 13	277
Number 15	241
Number 18	198

TABLE 6: Result of the third stage.

Number of adjustments	Adjusting station	Adjusting method	STDEV	Total detour time/min
0	—	_	114.7	6688
1	Number 15	$link157 \rightarrow link185$	94.8	7058
2	Number 18	$link165 \rightarrow link168$	71.3	7367
3	Number 13	$Link34 \rightarrow link13$	62.1	7520

TABLE 7: Final refueling traffic volumes at each station.

Station number	Refueling volume
Number 1	481
Number 2	473
Number 3	473
Number 5	450
Number 6	443
Number 4	421
Number 7	382
Number 8	364
Number 9	352
Number 14	342
Number 15	334
Number 10	333
Number 17	326
Number 13	324
Number 11	321
Number 18	320
Number 12	319
Number 16	317

(STDEV) of the numbers of refueling vehicles served by different refueling stations decreased. However, the total detour time increased. After 3 adjustments, the numbers of refueling vehicles served by each refueling station all fell in the range of [300, 500] pcu/h as shown in Table 7. If we continue to relocate refueling stations, the total detour time of refueling vehicles most likely will keep increasing. Therefore, the third stage stopped here, since all selected refueling stations had a reasonable demand and the total detour time was still low. The final solution to this case study is shown in Figure 10.

The refueling stations selected during the first stage were directly on the paths of refueling vehicles, and the average detour time of these refueling vehicles was 0 minutes; the average detour time of detouring refueling vehicles served by stations selected during the second stage was approximately 2 minutes; the average detour time of new detouring refueling vehicles due to the adjustments in the third stage was 2.5 minutes; the average detour time of all detouring refueling vehicles was 2.1 minutes; and the average detouring time of all refueling vehicles was 1.1 minutes.



FIGURE 10: Final solution to the case study.

6. Conclusions

Urban refueling stations are an important component of urban transportation systems. Based on the potential impacts of urban refueling stations, several principles of siting urban refueling stations have been proposed. Built upon these principles and impact factors, this paper proposed a threestage method to optimally site urban refueling stations, developed a genetic simulated annealing algorithm for solving the three-stage model, and conducted a case study to verify the effectiveness of the model and algorithm. The case study results suggest that (i) an optimal urban refueling station plan developed using the three-stage method can significantly shorten the detour time of refueling vehicles. Consequently, this can help reduce the negative impacts of detouring refueling vehicles on urban traffic, which in many cities is already highly congested; and (ii) the three-stage model takes the user equilibrium principle into consideration and can generate reasonable and practical facility location solutions.

Conflict of Interests

The authors declare that there is no conflict of interests regarding the publication of this paper.

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