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Cost-Effective Treatment of Existing Guardrail Systems

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COST-EFFECTIVE TREATMENT OF EXISTING GUARDRAIL SYSTEMS

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16. Abstract (Limit: 200 words) <p>A cost-effective means for upgrading existing guardrail systems with deviations from current practice (i.e., low-rail heights, antiquated end treatments, and improper installation) does not exist. As a result these systems remain on U.S. highways. Guardrail systems with deviations from current practice may not perform as intended, thus potentially resulting in fatalities and serious injuries from impacts with these safety devices. It is not plausible to eliminate fatalities and serious injuries from all guardrail system impacts. However, these risks could be reduced with the proper design, testing, installation, and maintenance of guardrail systems.</p> <p>This report offers recommendations for upgrading W-beam guardrail systems based on a benefit-to-cost analysis using the Roadside Safety Analysis Program (RSAP). This analysis was developed to simulate the most frequent and possible scenarios of existing W-beam guardrail systems with deviations from current practice. Before the analysis could be run, the field conditions and common deviations from current practice were reviewed and documented during a field investigation.</p> <p>This field investigation was conducted on rural arterial highways in the state of Kansas to determine the nature of existing guardrail systems with deviations from current practice. The most prominent barrier was the strong-post, W-beam guardrail. Deviations of the existing W-beam were low top-rail mounting-height, antiquated end treatments (i.e. turned-down and blunt-end terminals), rail damage, damaged and missing posts and blockouts, and insufficient length of need. The W-beam guardrail with low rail heights and turned-down and blunt-end terminals were the focus of the RSAP analysis.</p> <p>The varying guardrail system heights were modeled in RSAP by changing the level of containment of the W-beam guardrail system, and the antiquated end treatments were predefined features. The roadway and roadside features including obstacles (culverts and slopes) were modeled after those found in the field investigation. Finally, cost-effective safety treatments were recommended for existing W-beam guardrail system with low rail height and turned-down or blunt-end terminals which shielded culverts and slopes.</p>			
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1 INTRODUCTION

1.1 Background

The primary function of a barrier system is to prevent errant vehicles from impacting a roadside obstacle or encroaching into an area of concern. Barrier systems are intended to shield an obstacle (based on judgment), yet many fatalities and serious injuries have resulted from vehicles impacting these safety devices. In fact, barrier system impacts resulted in approximately 1,000 fatalities and 28,000 injuries in the U.S. in 2010 [1]. Many severe and fatal crashes may be caused by outdated barrier installations that did not satisfy the prior and/or current safety performance criteria, including those established in the *Manual for Assessing Safety Hardware* (MASH) [2] or the *National Cooperative Highway Research Program* (NCHRP) Report No. 350 [3], which systems are still acceptable to the Federal Highway Administration (FHWA) [4]. Existing barrier installations can be found to deviate from current practice in many ways, such as non-typical barrier types, antiquated end treatments, low rail heights, improper installations, variable post spacing, and inadequate lengths of need. Maintenance practices, exposure to the elements, and that older barrier systems were not designed for the current vehicle fleet can also degrade barrier performance. Also well intentioned, but with little understanding of how barriers work, field modifications can degrade barrier performance. It is not plausible to eliminate fatalities and serious injuries from all types of barrier system impacts; but these numbers could be reduced with the proper design, testing, installation, and maintenance of current barrier system technologies.

In the early 1960s, roadside safety was not given the consideration deemed necessary to develop “forgiving roadside safety devices” [5]. Guardrails were used to keep motorists from running off of the road or into roadside obstacles, such as culverts and critical slopes. Little

attention was given to the crash severity of the barrier system itself. This process led to several potential inadequacies in terms of barrier configurations, such as blunt-end guardrail terminals, concrete barrier posts, low rail mounting heights, and other deviations from current practice. Due to limited funds, many of these systems that do not meet current guidelines still exist along highways and roadways today. These deviations from current practices may present major safety concerns to government agencies as well as the motoring public, which need to be evaluated and addressed.

Ideally, all outdated barrier installations would be upgraded to satisfy current safety and design guidelines. However, available funding is often insufficient to meet this goal. Barrier installation guidelines are based on the assumption that most barrier systems are installed during highway construction projects, therefore benefit from an economic standpoint that limits overall transportation and labor costs of construction crew at the site. For example, when a highway project requires reconfiguration of the roadside, incorporating additional grading to accommodate guardrail terminals is relatively inexpensive. As such, agencies may be encouraged to upgrade existing outdated barrier systems when a roadway undergoes a 3R project (resurfacing, rehabilitation, or restoration of the roadway) or when the barrier system has extensive damage. Barrier systems may also be updated by a public agency going through a systematic improvement program. It is necessary to determine when an existing barrier installation needs cost-effective upgrade even if there is no improvement project planned for the roadway. This type of barrier system upgrade should consider an economic analysis of a barrier system improvement, which includes accident, construction, maintenance, and repair costs for all options being evaluated.

Although it is recommended to have the most current and best available safety hardware on our nation's highways and roadways, existing outdated barrier systems may still provide substantial benefit to the motorist population [6]. These existing barriers still provide some level of vehicle containment, delineation, and are less expensive for highway agencies to maintain compared to replacing them with new barrier systems. However, at some point the accident costs associated with outdated barrier system will exceed the cost to install a new improved barrier system. Therefore, a need exists to develop guidelines for determining when it is cost-effective to allow an existing barrier system to remain in place, when it is necessary to remove the existing barrier system, or when the existing barrier system should be replaced with an updated or upgraded barrier system.

Barrier installation guidelines are configured to provide the safest practical design for errant vehicles. Unfortunately, some barrier components are relatively conservative. For example, barrier length guidelines provided in the American Association of State Highway and Transportation Officials (AASHTO) *Roadside Design Guide* (RDG) [7] are based on vehicle runout distances traveled along the medians of divided highways observed from a 1960's investigation [8]. Another study of encroachments in Canada indicated that encroachment lengths measured in a 1970's investigation greatly overstated the distance that vehicles traveled along the roadside, causing the current guidelines pertaining to barrier length of need to be re-evaluated [9].

Many parameters associated with barrier installation guidelines, including length, can significantly increase the cost of upgrading older installations. However, these parameters may not contribute much to the reduction of injuries and fatalities in ran-off-road crashes. For example, as a barrier system is extended, the additional number of crashes with the protected

obstacle decline, but vehicle accidents into the barrier system and installation cost associated with the additional length increases steadily. Hence, the length of the barrier system reaches a point of diminishing return as it is lengthened.

1.2 Research Objective

The primary research objective of this study was to develop guidelines for determining when it is cost-effective to upgrade existing outdated barrier installations with the use of a benefit-to-cost (B/C) analysis.

1.3 Research Scope

The research objective was achieved by performing several tasks. First, a field investigation was conducted to find barrier systems located on two-way, two-lane highways in several states. This step included detailed descriptions and geometries of barrier systems that do not meet current guidelines along with roadway geometries and roadside conditions. Next, a detailed data review was performed on the information obtained from the field investigation in order to better understand how existing barrier systems deviate from current practice. Then, a sensitivity analysis and engineering judgment were used to determine what types of barrier systems, roadway features, and obstacles were to be evaluated. Subsequently, these parameters were investigated and evaluated within a set of detailed scenarios, which formed the basis of a B/C analysis utilizing the Roadside Safety Analysis Program (RSAP) [10]. Next, the results from the RSAP runs were tabulated to identify when existing barrier systems were satisfactory, needed to be removed, or needed to be upgraded. Finally, guidelines, conclusions, and recommendations were prepared regarding the cost-effective upgrade of existing barrier systems based on the results obtained from the benefit-to-cost analysis.

2 LITERATURE REVIEW

2.1 Federal Policies

Existing guardrail systems may be restored to their initial state by replacing rotted posts and blockouts, adjust posts and posts upward to get its design height, replace damaged rail sections, etc. If a barrier is essentially non-functional (i.e., it cannot reasonably be expected to function satisfactorily under most expected impacts), it should be upgraded to current criteria [7]. Numerous FHWA memorandums and technical advisories have also been issued to assist with guidelines on repairing, replacing, or upgrading existing guardrail systems. One such document states that if safety improvements beyond restoration are made to an existing guardrail system, the entire system should be brought up to current standards [11]. As such, changes and alterations to an existing guardrail system cannot be implemented on a piece-by-piece basis. For example, it arguably may be considered negligent to install a current crashworthy guardrail end terminal on the end of an existing outdated guardrail system. Often, the upgrade of an existing guardrail system can only be accommodated with the removal of the entire system as well as the subsequent installation of a new system that conforms to current design practices and meets impact safety guidelines. Due to the moderate amount of guardrails that do not meet current guidelines along highways and roadways, it is not always a feasible option for state departments of transportation (DOTs) to completely remove and replace existing, outdated roadside guardrail systems. As a result, many guardrail systems remain in place for many years with identifiable deviations from current practice.

The design of guardrail end treatments have drastically changed and improved over the last 50 years. In early installations, guardrail ends were terminated with a blunt-end or a small spoon (i.e., fish-tale attachment), the latter of which was intended to eliminate the exposed

leading edge of the W-beam rail. However, both designs in some accident scenarios allowed W-beam rail to penetrate through vehicles during end-on impacts. This behavior initiated the development of the turned-down end terminal [12-19]. Turned-down ends were used to slope the guardrail to the ground in order to eliminate the risk of penetrating an impacting vehicle. However, these ramped ends ultimately allowed a vehicle to climb the rail and become airborne, often resulting in vehicle rollover or heavy contact into the shielded obstacle. These types of treatments have proven to be obstacles themselves. As of 1990 and according to an FHWA memorandum, all turned-down terminals were no longer to be utilized on new installations and were to be replaced on existing guardrail systems during safety improvement, obstacle elimination, or 3R projects on high-speed, high-volume facilities [20]. In 1993, the FHWA issued a technical advisory which prohibited the use of turned-down, W-beam guardrail end terminals within the designated clear zone on defined roads with operating speeds of 50 mph (80 km/h) and above and with traffic volumes in excess of 6,000 vehicles per day (vpd) [21]. However, it was noted that turned-down end terminals may remain appropriate for use on the downstream ends of the guardrail system on divided highways and in locations where end-on, high-speed accidents are unlikely. In 1994, the FHWA required that state agencies provide due care in not allowing inappropriate guardrail end terminals to remain indefinitely on the National Highway System (NHS) [22]. This guidance included a replacement strategy for blunt-end and turned-down terminals [23].

Transitions, which join together two barriers with differing stiffnesses, strengths, and geometries by gradually increasing or decreasing the lateral stiffness, are another category of guardrail systems which may include outdated features. When correctly designed, transitions redirect errant vehicles and prevent pocketing or snagging as a vehicle approaches the stiffer

barrier from the direction of a less stiff barrier. Most existing outdated transitions are found near the connection region between guardrail systems and rigid bridge rails. However, W-beam guardrail systems may have been connected directly to a bridge rail without the use of additional posts or rail elements, adequate blockouts, or a rubrail. In these scenarios, the stiffness transition could very likely be considered unsatisfactory due to the significant potential for vehicle snag or pocketing near the bridge end. Consideration should be given to replacing or upgrading these existing transitions as the opportunity becomes available [24].

Existing W-beam guardrails may also deviate from the current practice in terms of a guardrail height that does not meet current guidelines. Low guardrail height can result from poor installation, settling posts, roadway overlays, and use of outdated guardrail designs. Guardrail heights that do not meet current guidelines can affect the ability of a guardrail to contain and redirect an errant vehicle. For example, the change in vehicle fleet from large passenger sedans to taller, heavier pickup trucks, vans, and sport utility vehicles has caused the old standard 27-in. (686-mm) guardrail to fail NCHRP Report No. 350 Test Level 3 (TL-3) safety performance criteria [25]. Because of this result, FHWA issued a memo which required all newly-installed W-beam guardrail heights to be at least 27¾ in. (705 mm) to the top of the rail, and transportation agencies are recommended to adopt a 31-in. (787 mm) high guardrail system for all new installations. MASH testing has also shown some performance issues with 27¾-in. (705-mm) high guardrail designs, and the FHWA recommendation was the result of several testing programs which demonstrated improved crash-test performance at the 31-in. (787-mm) height [25].

2.2 Development of Guardrail Testing Standards

Prior to implementation, new roadside safety hardware is evaluated through the use of full-scale crash testing according to current impact safety guidelines and procedures. The full-scale crash tests allow designers to observe and evaluate the performance of the safety features for the worse-practical impact conditions. Guardrail performance is evaluated according to several measures, such as structural adequacy, occupant risk, and vehicle trajectory. Prior to 1962, there were no standardized testing criteria for designing or evaluating roadside safety devices. Thus, it was difficult to evaluate the performance of newly designed guardrails. Then, the *Proposed Full-Scale Testing Procedures for Guardrails (Circular 482)* was developed [26]. This one-page document was the first set of guidelines for testing and evaluating roadside guardrail systems. It standardized all vehicle crash testing criteria. It specified parameters such as vehicle mass, impact speed, and approach angle of the crash tests. Guardrail systems developed after this date had to pass all test criteria presented in the report in order to be implemented on highways. Healed

Since the inception of Circular 482, the roadway conditions have changed drastically. The vehicle fleet, average daily traffic (ADT), and highway design speeds have also changed, and the safety standards that are used to evaluate barrier technologies have evolved. Guardrail testing guidelines and procedures have added new and more thorough test criteria to increase the safety of the roadsides. After Circular 482 [26], there have been six testing procedures for evaluating longitudinal guardrails: NCHRP Report No. 153 (1974) [27]; Circular 191 (1978) [28]; NCHRP Report No. 230 (1981) [29]; AASHTO *Guide Specifications for Bridge Railings* (1989) [30]; NCHRP Report No. 350 (1993) [3]; and MASH (2009) [2]. Each testing standard involved more detailed testing criteria than the previous published criteria. Most updates either

demanded more test criterion or improved the methods for evaluating safety performance of hardware and/or features by including the level of roadway and vehicle type. The major changes to the full-scale crash test criteria are listed below.

Circular 482 (1962) [26]

- First document to standardize full-scale crash test criteria
- Four specifications on test article installation
- One vehicle size
- Six test conditions
- Three evaluation criteria

NCHRP Report No. 153 (1974) [27]

- First complete test matrix
- Specified parameters to be measured with methods and limits to meet
- Simple report writing formats included
- Added small car test vehicle
- Updated impact speed to 60 mph (96.6 km/h)

Circular 191 (1978) [28]

- Standardize soil for post installation
- Test vehicles updated
- Evaluation criteria changed

NCHRP Report No. 230 (1981) [29]

- Added more test vehicles
- New testing procedures added to meet available technologies
- Evaluation criteria updated
- Test matrices updated
- Basic in-service evaluation of safety features added

AASHTO *Guide Specifications for Bridge Railings* (1989) [30]

- Document specified on the testing of bridge rails
- Added pickup truck, single-unit truck, and tractor-trailer test vehicles

NCHRP Report No. 350 (1993) [3]

- Six test levels (TL-#) for different roadway conditions
- Added compact car
- ¾-ton pickup truck replaced large passenger car
- Testing matrices for more roadside features (work zone devices)
- Additional and different testing conditions
- Added computer simulation evaluation procedures
- Conversion to SI units

- Guidelines for critical impact point selection
- Enhanced measurement techniques to occupant risk values
- Optional side impact testing criteria added

MASH (2009) [2, 4]

- Small car impact angle increased from 20 to 25 degrees
- Impact speed for single-unit truck test increased from 80 km/h to 90 km/h
- Impact angle for length of need test of terminals and crash cushions increased from 20 to 25 degrees
- Impact angle for oblique end-on impacts of gating terminals and crash cushions reduced from 15 to 5 degrees
- Impact point for small vehicle tests on cable barrier changed to the mid-span of posts to evaluate the potential for under ride, while the target impact point for all other test vehicles shall be limited to 1 ft (0.3 m) upstream of the post for all test conditions
- The barrier top mounting height is recommended to be set at the maximum for small car tests and at the minimum for pickup truck tests
- Performance-based specifications for soil are used in lieu of the material-based specifications to help ensure consistency in soil strength
- Cable tension is required to be set to the value recommended for 100 degrees Fahrenheit
- Minimum installation length requirements are more clearly specified
- The size and weight of test vehicles is increased to reflect the increase in vehicle fleet size:
 - the 820C test vehicle is replaced by the 1100C
 - the 2000P test vehicle is replaced by the 2270P
 - the single-unit truck mass is increased from 8,000 kg to 10,000 kg
 - the light truck test vehicle (2270P) must have a minimum center of gravity height of 28 in.
- The option for using passenger car test vehicles older than 6 years is removed
- Windshield and occupant compartment damage evaluation uses quantitative instead of qualitative criteria
- All evaluation criteria will be pass/fail, eliminating the “marginal pass”
- Reporting the exit box evaluation criterion is required
- Language emphasizing the importance of in-service evaluation is added
- All newly designed barriers must be tested under MASH

Current vehicles are much taller and heavier than vehicles of the past as large sport utility vehicles (SUVs) and pickup trucks have become popular in society [31]. Many existing guardrail systems installed on highways are not designed to contain these larger vehicles under current impact conditions, thus guardrail systems that met past testing standards (prior to NCHRP 350) may potentially be obsolete. Along with the change in vehicle fleet, the ever-growing traffic

volumes also may affect the need for guardrail systems. Higher traffic volumes relate to higher frequencies of ran-off-road accidents. Additionally, higher posted speeds on highways can lead to more severe impacts with the safety guardrail systems. These two factors require that new guardrail installations be safer and more forgiving to errant vehicles and their motorists.

Full-scale vehicle crash testing is often used to evaluate the safety performance of a guardrail system. However, some may argue that a guardrail may also be evaluated through an in-service performance evaluation. An in-service performance evaluation provides a broad range of information on vehicle collision characteristics (e.g., number of accidents and the extent of injuries), environmental, operational, and maintenance situations for typical roadway conditions. NCHRP Report No. 490, *In-Service Performance of Traffic Barriers* [32], utilizes a step-by-step method of evaluating existing guardrail systems. This report assists in determining if and how a roadside safety feature performs in actual field conditions as compared to crash test results. An in-service performance evaluation would also provide a check against the evaluation results obtained from full-scale testing by the laboratories.

In addition to the new-feature evaluation in NCHRP Report No. 490, MASH [2] has specified a continuous in-service monitoring method for guardrail systems. After passing the brief new-feature, in-service performance evaluation (typically 3 years), a continuous monitoring system is used on a roadside safety feature to ensure the device continues to perform as designed with the changing roadway conditions. This process will provide a way to determine the effects of changing roadway variables, such as vehicle fleet, growing ADT, and roadway design speeds.

2.3 Guardrail Guidelines

After roadside safety devices have been deemed acceptable by passing all pertinent crash test criteria, they can be used on current highways. There are many different guardrail

installation guidelines that layout which systems are acceptable for specific roadway conditions based on a successfully-tested impact level. These documents are briefly described in the following sections.

2.3.1 2006 Roadside Design Guide (RDG)

The *Roadside Design Guide* (RDG) [33] was developed and published by the American Association of State Highway and Transportation Officials (AASHTO). The RDG was intended to assist highway agencies in developing cost-effective roadside safety standards, while focusing on safety treatments that can minimize the likelihood of serious injuries and fatalities when a motorist inadvertently leaves the roadway. Guardrails can pose increased risk to errant motorists themselves. As such, a guardrail system should only be implemented if the crash severity and risks are less than that provided by the obstacle itself. This guide combines current research and practical experience to create guidelines based on the guardrail risk versus the obstacle risk concept. The RDG also assists with the basic design of guardrail, including guardrail selection for particular performance or test levels, guardrail structural characteristic (e.g., deflection allowance), and guardrail placement (e.g., lateral offset, flare rate, and length of need). The Roadside Design Guide was updated in 2011 [7].

2.3.2 AASHTO Bridge Guide

The *AASHTO Load and Resistance Factor Design (LRFD) Bridge Design Specifications* [34] were developed for the design, evaluation, and rehabilitation of bridges and bridge features. These specifications employ the LRFD methodology developed from current statistical knowledge of loads and structural performance. This guide also includes a common yet not a comprehensive list of current bridge rail designs and installation practices.

2.3.3 Highway Safety Design and Operations Guide (Yellow Book)

The *Highway Safety Design and Operations Guide* (Yellow Book) [35] was developed by AASHTO. This document discusses general highway safety and defines specific roadway design elements, such as design speed, horizontal and vertical alignments, and roadsides. The Yellow Book gives a basic guide of when to implement guardrail systems on different highway functional classes.

2.3.4 A Guide to Standardized Highway Barrier Hardware (Hardware Guide)

Published jointly by AASHTO, the American Road and Transportation Builder's Association (ARTBA), and the Association of General Contractors (AGC), *A Guide to Standardize Highway Barrier Hardware*, or the Hardware Guide, contains drawings and specifications for barrier systems and their components [36]. Most systems in the Hardware Guide had been crash tested and accepted under NCHRP Report No. 350 or other accepted testing standards. This guide includes a sample of different barrier types but does not have a comprehensive list of all barriers. The barriers contained in the Hardware Guide include the most commonly-used guardrail systems in the U.S. The Hardware Guide provides specifications and materials corresponding to the guardrail elements described therein.

2.4 Crashworthy Barriers, Terminals, and Transitions

FHWA defines crashworthy devices as those that have passed all pertinent crash tests conducted under the procedures defined in NCHRP Report No. 350 or MASH. It is important to be familiar with crashworthy roadside safety systems and their components when evaluating any deviations from current practice. For this study, barriers conforming to the Test Level 3 (TL-3) impact safety criteria were considered. In this section, common crashworthy longitudinal barriers and their end treatments will be examined in order to make later comparisons to existing barrier systems with deviations from current practice.

2.4.1 Strong-Post W-Beam Guardrail

Current W-beam guardrail systems are considered to be either flexible or semi-rigid guardrail systems depending on the post size and spacing. The major components of a current typical W-beam guardrail systems include rolled steel rail sections in the shape of a “W”, steel or wood posts, and with/without blockouts. The steel W-beam thickness ranges from 14 to 10 gauge (1.90 to 3.42 mm) with a typical thickness of 12 gauge (2.66 mm).

Steel post cross sections range between W6x8.5 to W6x12 (W152x13.4 to W152x17.9). Wood posts can utilize a circular or rectangular cross section. The circular cross sections of accepted W-beam guardrail systems have a diameter between 7 in. and 8 in. (178 mm and 203 mm). A typical post rectangular cross section is 6 in. x 8 in. (152 mm x 203 mm). Most W-beam guardrail systems, which meet current guidelines, utilize a blockout to help reduce vehicle snag on posts as well as to maintain rail height. These blockouts are either wood or plastic with typical dimensions of 6 in. x 12 in. x 14¼ in. (152 mm x 305 mm x 362 mm) or 6 in. x 8 in. x 14¼ in. (152 mm x 203 mm x 362 mm).

Current guidelines require a minimum top-rail mounting height of 27¾ in. (705 mm), but it is recommended that newly installed guardrail utilize a 31-in. (787-mm) top-rail height [25]. Lap splices typically use eight ⅝-in. (16-mm) diameter steel bolts to connect two spans of W-beam guardrail at a splice location. Typical post spacing for a strong-post W-beam guardrail system is 6 ft - 3 in. (1.9 m). Typically, all steel components are galvanized to prevent and/or reduce corrosion, thus extending the design life of the guardrail.

The Midwest Guardrail System (MGS) is a non-proprietary, strong-post, W-beam guardrail [37]. On the MGS system, the splices are located between the posts, and the nominal rail height is set to 31 in. (787 mm). Originally, the MGS was cash tested and met all criteria set

forth by NCHRP Report No. 350, and was accepted as a TL-3 longitudinal guardrail [38]. The MGS was later accepted according to the MASH impact safety criteria [39-40]. The MGS is shown in Figure 1.

2.4.2 W-Beam Guardrail End Terminals

There are many different designs of W-beam guardrail end terminals which meet all current crash test standards. These terminals must provide anchorage to develop the full capacity of the guardrail and safely redirect or contain head-on impacts. Most terminals attached to W-beam guardrail are known as gating terminals, which when struck, will allow the vehicle to go behind and beyond the terminal end. W-beam end terminals can be tangent or flared. Tangent terminals denote that the end treatment and guardrail are parallel to the roadway. Tangent terminals and some flared terminals dissipate kinetic energy in head-on impacts and stop an impacting vehicle over a safe distance. Some flared terminals allow an impacting vehicle to travel much farther after contact, but the flare angle minimizes head-on impacts. Most W-beam terminals utilize breakaway wood and/or steel posts in order to be more forgiving during head-on impacts. Steel cables are often used to develop the necessary strength for a redirecting an impacting vehicle but will release during a head-on impact. An impact head is also used on most W-beam terminal types so that the rail cannot spear the impacting vehicle. There are many different types of currently-accepted W-beam terminal designs. Although some terminals perform better than others, all designs safely stop a vehicle during testing and provide adequate strength to redirect a vehicle during an impact near the terminal end.

2.4.1 W-Beam-to-Concrete Bridge Rail Transition

Most approach guardrail transitions connect a semi-rigid, W-beam guardrail to a rigid concrete bridge rail. The major concern of transitioning from a W-beam guardrail to a concrete



Figure 1. Midwest Guardrail System (MGS)

bridge rail is vehicle pocketing, where an errant vehicle deflects the semi-rigid W-beam far enough that the vehicle impacts the end of the rigid bridge rail, posing significant risk to the motorist. To mitigate this risk, the guardrail system is stiffened over a transition length. The particular stiffness of the guardrail is achieved by a combination of the following options: reducing post spacing; installing larger posts; mounting a thicker rail element; adding a nested three beam rail element to the transition; and creating a strong connection between the W-beam to the bridge rail element. To reduce the likelihood of wheel snagging on the end of the parapet, some transitions utilize a rubrail or curb. An example of a generic guardrail-to-concrete barrier transition that meets all NCHRP Report No. 350 criteria is shown in Figure 2 [41-42].

2.4.2 Cable Barrier Systems

Cable barrier systems are flexible guardrail systems and are generally more forgiving than other guardrail systems because deflection occurs over a larger span when an errant vehicle strikes the system. Cable barrier systems require a larger working width due to this large dynamic deflection. These barriers redirect impacting vehicles when enough tension is developed in the cables. The posts are weak and are designed to hold the cable in position until the system is impacted, at which point, they are easily bent or broken. A typical post is an S3x5.7 (S76x8.5) steel section, but many currently-accepted cable barrier systems have a unique post design. Typical post spacing varies from 10 to 20 ft (3.0 to 6.1 m) center-to-center. Cable barrier systems utilize either three or four ¾-in. (19-mm) diameter, 3x7 galvanized wire ropes. Top cable heights range from 27 in. to 41½ in. (686 mm to 1,054 mm).

Cable barrier systems have been installed with either low tension or high tension. Low-tension barriers are only tensioned enough to reduce the sag of the cables between posts during temperature fluctuations. The high-tension cables have been implemented to redirect an errant.



Figure 2. Generic W-Beam-to-Thrie Beam-to-Concrete Bridge Rail Transition

vehicle with less deflection and decreased maintenance. High-tension cable barrier systems are tensioned between 3 kips and 8 kips (13.4 kN and 35.6 kN). The cable-to-post connections for each system typically utilize a steel clip or rounded U-bolt. These connections are designed to release the cables from the posts to prevent development of localized stresses on the posts. A generic cable barrier system is an example of a low-tension, 3-cable median barrier [43] and is shown in Figure 3

2.4.3 Cable Barrier End Terminal

Currently-accepted cable end terminals are similar to W-beam terminals because they are designed to develop the full capacity of the guardrail and safely contain a head-on impact. The cable end terminal section is typically anchored to the ground or to multiple end posts to develop enough strength to redirect oblique impacts downstream from the end system. Many of the currently-accepted cable terminal designs have incorporated a cable release on the anchor. Similar to the W-beam terminals, these systems have both flared and tangent designs. In many of the systems, the posts near the ends are breakaway to be more forgiving to errant vehicles. An example of a breakaway end treatment is the MwRSF cable end terminal [44]. This system was successfully tested and evaluated under the NCHRP Report No. 350 criteria [45] and is shown in Figure 4.

2.5 Guardrail Height Effects

The National Crash Analysis Center (NCAC) performed a study on the G4(1S) W-beam guardrail system at varying top rail mounting heights to investigate the effect of different rail heights from the standard 27¾ in. (705 mm) top-rail height [31]. This study utilized both full-scale crash testing and finite element simulation to evaluate the safety performance of W-beam guardrail at varying rail heights. Crashes were investigated with a 2000P pickup truck impacting



Figure 3. Generic Three-Cable Low-Tension Barrier



Figure 4. Generic Low-Tension Cable End Terminal

the W-beam guardrail at 62.1 mph (100 km/h) and 25 degrees (NCHRP Report No. 350 test designation 3-11). Simulations were performed on top-rail heights of 24⁵/₈ in. (625 mm), 26¹/₈ in. (664 mm), 27³/₄ (705 mm), 29¹/₈ (740 mm), and 30⁵/₈ (778 mm). The results from the study showed that lower rail heights of 24⁵/₈ in. (625 mm) and 26¹/₈ in. (664 mm) had increased the potential for vehicle override of the W-beam guardrail system, while the 27³/₄ (705 mm), 29¹/₈ (740 mm), and 30⁵/₈ (778 mm) redirected the vehicle. Then, two full-scale crash tests were performed on a W-beam guardrail with a 25 in. (635 mm) and 27³/₄ (705 mm) to validate the simulation results. The pickup truck was redirected with a 27³/₄ (705 mm) rail height, but the 25 in. (635 mm) rail height allowed the pickup truck to override the guardrail. This occurs because as the vehicle impacts the rail the posts rotate back and down allowing a vehicle with a high center of gravity (C.G.) to climb over the guardrail element. Simulation and full-scale crash test results showed a high risk of vehicle override when the W-beam guardrail is lower than the standard height.

Another study of the Midwest Guardrail System (MGS) at higher top-rail mounting heights was also conducted to investigate guardrail performance at heights greater than the recommended 31-in. (787-mm) top-rail mounting height [46]. The MGS systems were evaluated with 34-in. (864-mm) and 36-in. (864-mm) top-rail mounting heights. Both system heights were found to satisfy MASH TL-3 evaluation criteria for test no. 3-10. This study showed little effect of a higher top-rail mounting height under 1100C impact events within the length of need.

For this research study, an additional literature search was conducted to determine performance of W-beam guardrail with low top-rail mounting heights. After evaluating around 25 full-scale W-beam crash tests, four tests were used for this research. The first full-scale crash test was performed by MwRSF with a pickup truck into a 31-in. (787-mm) tall W-beam guardrail

[47]. The Texas Transportation Institute (TTI) conducted two full-scale crash tests with pickup trucks on 27¾-in. (705 mm) and 27-in. (686-mm) tall W-beam guardrail [48-49]. The final full-scale crash test was taken from a California Transportation Agency (Caltans) research study, where a 24-in. (610-mm) tall W-beam guardrail was impacted with a sedan [50]. The impact speed, vehicle type, and impact angle were recorded for each test, as shown in Table 1. This information was vital to this research study, as explained in Chapter 7.

Table 1. Full-Scale W-beam Crash Test Information

Vehicle Type	Guardrail Height		Vehicle Weight		Angle (deg.)	Speed		Reference
	(in.)	(mm)	(lb)	(kg)		(mph)	(km/h)	
2000P	31	787	4,441	2,014	36.7	65.0	104.7	[47]
2000P	27¾	705	4,577	2,076	25.5	63.1	101.5	[48]
2000P	27	686	4,572	2,074	24.3	62.6	100.8	[49]
Sedan	24	610	4,570	2,073	25	59.0	95.0	[50]

2.6 Maintenance and Repair of Guardrail

FHWA’s *W-Beam Guardrail Repair - A Guide for Highway and Street Maintenance Personnel* informs highway officials when to repair damaged guardrail [51]. Various guardrail conditions were categorized as: (1) guardrail no longer reasonably functional; (2) guardrail should function adequately under a majority of impacts; and (3) should not impair the guardrail’s ability to perform. These functional categories come from the condition of the rail and post elements, deflection (amount out of alignment), and top-rail height. Two major conclusions from this report revealed that when the top-rail height was found to be less than or equal to 24 in. (610 mm) or the W-beam guardrail was missing 3 or more posts, the guardrail was deemed as no

longer reasonably functional. This guide also included when it is pertinent to repair many W-beam guardrail features, such as bridge rail transitions and end terminals.

Criteria for Restoration of Longitudinal Barriers was another report which provided guidance in identifying levels of damage to W-beam guardrail barriers [52]. This study evaluated commonly found barrier damage utilizing pendulum testing, full-scale crash testing, and finite element simulations. The study evaluated W-beam barrier damage such as rail tear, missing splice bolts, twisted/missing blockouts, hole in rail, post deflection, missing/broken posts, post separation from rail, and rail flattening. When evaluating each damage type, the study ranked existing systems as low, medium, and high priority to repair. This guide also included generic end terminal restoration guidance.

2.7 Previous MwRSF Benefit-to-Cost Analysis Studies

2.7.1 Low-Volume Roads

Cost-Effective Safety Treatments for Low-Volume Roads was a study conducted by MwRSF researchers to determine the best safety treatment for common low-volume roadside obstacles [53]. A field study was conducted on local low-volume roads (ADT less than 500 vpd) and common roadside obstacles observed included culverts, slopes, ditches, driveways, bridges, and trees. Treatment options, such as do nothing, remove existing system, install a W-beam guardrail system, culvert grate installation, tree removal, and adding delineation, were considered for each obstacle. Recommendations were then given based on the results of a benefit-to-cost analysis and the best treatment option was considered. This study aided the W-beam cost determination necessary for this research study, as shown in Chapters 8 and 9.

2.7.2 Culvert Treatment Guidance

Evaluation of Safety Treatments for Roadside Culverts was a study conducted by MwRSF researchers to determine the best treatment for common roadside culverts [54]. Treatment options that were evaluated included: culvert extension, guardrail installation and culvert grating. A benefit-to-cost analysis was used to determine the best treatment for various roadway conditions. Local, rural arterial, and freeways were the roadway types selected for the project. The accident costs were calculated for all scenarios of the study, thus allowing the end user to calculate the benefit-to-cost ratios. This research study aided in the determination of culvert opening treatment options, as shown in Chapter 8.

2.7.3 Roadside Grading Guidance

Roadside Grading Guidance – Phase I was a study conducted by MwRSF researchers to update the severity indices associated with foreslopes [55]. These values were assumed to be overestimated, so were updated using accident data in the state of Ohio. Once the severity indices were updated in the first phase, a benefit-to-cost analysis of roadside foreslopes was conducted in *Roadside Grading Guidance – Phase II* [56]. Treatment options for roadside slopes included: do nothing, grade the slope to be less severe and implement a guardrail to shield existing slope. Guidance was based on the treatment option which gave the greatest benefit to the end user. These research studies aided in the determination of roadside slope treatment options, as shown in Chapter 9.

3 FIELD INVESTIGATION OF EXISTING BARRIER SYSTEMS

3.1 Overview

For this study, it was necessary to gain a better understanding of the current state of existing barrier systems with known deviations from current practice. Thus, an extensive site survey was conducted in order to document many of these barrier systems found along rural arterial highways in Kansas. All system geometries, components, deviations from current practice, type of shielded obstacles, and the roadway conditions were documented during the survey using the field investigation data sheet shown in Appendix A. Each field site and barrier installation was also thoroughly photographed to aid in the subsequent analysis. The field investigation took place during the summer of 2009. Highway sites within the state of Kansas were suggested by DOT personnel and selected by MwRSF staff for this investigation. The field investigation team made an effort to visit numerous sites to obtain a wide variety of barrier types, roadway conditions and classifications, and geographical areas during the survey period. It should be noted that if a barrier system and obstacle type were nearly identical for multiple locations, then only a few similar sites were documented; since, information pertaining to different barrier systems or deviations from current practice was deemed more valuable than redundant documentation of known issues.

The types of barrier systems that were documented in the field investigation were: (1) strong-post, W-beam guardrails; (2) cable barrier systems; (3) concrete barriers; (4) channel rails; and (5) modified versions of W-beam barrier systems. These barrier systems varied in length, height, obstacle shielded, roadway offset, and condition pertaining to aged components, prior impacts, and installation practices. These real-world barrier systems are described in greater detail later in this chapter.

The highway functional classes of the roadways that were documented in the study included minor arterial, major collector, and other principal arterial, two-lane roadways without medians, as defined by Kansas DOT. Out of the 68 barriers investigated, 61 were found on minor arterial roadways. There were only 7 roadways that were documented as major collector roadways. The lane width of these highways varied from 9 to 12 ft (2.7 to 3.7 m), while the vast majority had a 12-ft (3.7-m) lane width. The shoulder width ranged from 0 to 12 ft (0 to 3.7 m), and the posted speed limit ranged between 35 and 65 mph (56.3 and 104.6 km/h), although most locations had a 65-mph (104.6-km/h) posted speed limit. The ADT on the Kansas roadways documented in the field investigation ranged from 300 to 11,000 vpd, as determined by traffic volume maps.

The barrier systems were found to shield various fixed objects or geometric features, such as culvert openings, roadside slopes, bridge rail ends, small waterways, and trees, which can be an area of concern to errant motorists and vehicles. However, the most common shielded fixed objects were culvert openings and roadside slopes. A summary of all documented systems is shown in Table 2.

All concrete box culverts included wingwalls. In the field investigation, culvert lengths varied between 6 ft and 50 ft (1.8 m and 15.2 m). The width of the culverts ranged between 5 ft and 30 ft (1.5 m and 9.1 m). The drop height of the culverts ranged between 3 ft and 14 ft (0.9 m and 4.3 m). The lateral offsets of culverts varied between 0 ft and 6 ft - 6 in. (0 m and 2.0 m) away from the edge of pavement. A summary of culvert geometries are shown in Table 3. Examples of the culvert systems found in the field investigations are shown in Figure 5.

Table 2. Summary of Field Investigation – Barrier, Obstacle, and Site Conditions

System No.	Barrier System Description	Obstacle Type	Lane Width		Shoulder Width		Speed Limit		Curve
			(ft)	(m)	(ft)	(m)	(mph)	(km/h)	
1	Strong-Post, W-Beam	bridge rail end	12	3.7	2	0.6	65	104.6	none
2	Strong-Post, W-Beam	bridge rail end	11	3.4	1	0.3	65	104.6	none
3	Strong-Post, W-Beam	bridge rail end	11	3.4	1	0.3	65	104.6	none
4	Strong-Post, W-Beam	bridge rail end	11	3.4	0.67	0.2	65	104.6	none
5	Strong-Post, W-Beam	bridge rail end	11	3.4	2	0.6	65	104.6	none
6	Strong-Post, W-Beam	bridge rail end	11	3.4	0	0.0	65	104.6	none
7	Strong-Post, W-Beam	bridge rail end	NA	NA	NA	NA	NA	NA	none
8	Strong-Post, W-Beam	bridge rail end	12	3.7	12	3.7	65	104.6	none
9	Strong-Post, W-Beam	bridge rail end	11	3.4	1	0.3	60	96.6	none
10	Strong-Post, W-Beam	culvert opening	12	3.7	3	0.9	65	104.6	none
11	Strong-Post, W-Beam	culvert opening	12	3.7	1	0.3	65	104.6	none
12	Strong-Post, W-Beam	culvert opening	9	2.7	3	0.9	55	88.5	yes
13	Strong-Post, W-Beam	culvert opening	9	2.7	3	0.9	55	88.5	yes
14	Strong-Post, W-Beam	culvert opening	9	2.7	2	0.6	55	88.5	yes
15	Strong-Post, W-Beam	culvert opening	12	3.7	2	0.6	65	104.6	yes
16	Strong-Post, W-Beam	culvert opening	12	3.7	NA	NA	65	104.6	none
17	Strong-Post, W-Beam	culvert opening	11	3.4	8	2.4	65	104.6	none
18	Strong-Post, W-Beam	culvert opening	11	3.4	4	1.2	65	104.6	none
19	Strong-Post, W-Beam	culvert opening	12	3.7	4	1.2	65	104.6	yes
20	Strong-Post, W-Beam	culvert opening	12	3.7	3	0.9	65	104.6	none

NA – Unable to document due to roadway conditions and/or other circumstances

Table 2. Summary of Field Investigation – Barrier, Obstacle, and Site Conditions (Continued)

System No.	Barrier System Description	Obstacle Type	Lane Width		Shoulder Width		Speed Limit		Curve
			(ft)	(m)	(ft)	(m)	(mph)	(km/h)	
21	Strong-Post, W-Beam	culvert opening	11	3.4	3.5	1.1	65	104.6	none
22	Strong-Post, W-Beam	culvert opening	NA	NA	NA	NA	NA	NA	none
23	Strong-Post, W-Beam	culvert opening	12	3.7	3	0.9	65	104.6	none
24	Strong-Post, W-Beam	culvert opening	12	3.7	2.67	0.8	65	104.6	none
25	Strong-Post, W-Beam	culvert opening	11	3.4	2	0.6	65	104.6	none
26	Strong-Post, W-Beam	culvert opening	12	3.7	2	0.6	65	104.6	none
27	Strong-Post, W-Beam	culvert opening	11	3.4	2	0.6	65	104.6	none
28	Strong-Post, W-Beam	culvert opening	12	3.7	2	0.6	55	88.5	none
29	Strong-Post, W-Beam	culvert opening	12	3.7	2.5	0.8	65	104.6	none
30	Strong/Concrete Post, W-beam	culvert opening	12	3.7	3	0.9	65	104.6	none
31	Strong/Concrete Post, W-beam	culvert opening	11	3.4	0.67	0.2	65	104.6	none
32	Strong/Concrete Post, W-beam	culvert opening	12	3.7	2.5	0.8	65	104.6	yes
33	Strong/Concrete Post, W-beam	culvert opening	12	3.7	2.5	0.8	65	104.6	none
34	Strong-Post, W-Beam	roadside slope	11	3.4	6	1.8	35	56.3	none
35	Strong-Post, W-Beam	roadside slope	12	3.7	1	0.3	45	72.4	none
36	Strong-Post, W-Beam	roadside slope	9	2.7	3	0.9	55	88.5	none
37	Strong-Post, W-Beam	roadside slope	11	3.4	4	1.2	55	88.5	none
38	Strong-Post, W-Beam	roadside slope	12	3.7	2	0.6	45	72.4	none
39	Strong-Post, W-Beam	roadside slope	11	3.4	1	0.3	55	88.5	yes
40	Strong-Post, W-Beam	roadside slope	11	3.4	1	0.3	65	104.6	none

NA – Not able to document due to roadway conditions and/or other circumstances

Table 2. Summary of Field Investigation – Barrier, Obstacle, and Site Conditions (Continued)

System No.	Barrier System Description	Obstacle Type	Lane Width		Shoulder Width		Speed Limit		Curve
			(ft)	(m)	(ft)	(m)	(mph)	(km/h)	
41	Strong-Post, W-Beam	roadside slope	12	3.7	2	0.6	50	80.5	none
42	Strong-Post, W-Beam	roadside slope	11	3.4	1	0.3	65	104.6	none
43	Strong-Post, W-Beam	roadside slope	11	3.4	0	0.0	65	104.6	none
44	Strong-Post, W-Beam	roadside slope	12	3.7	0.25	0.1	65	104.6	none
45	Strong-Post, W-Beam	roadside slope	12	3.7	3	0.9	60	96.6	none
46	Strong-Post, Modified W-Beam	culvert opening	10	3.0	1	0.3	65	104.6	none
47	Strong-Post, Modified W-Beam	culvert opening	12	3.7	3.5	1.1	65	104.6	none
48	Strong-Post, Modified W-Beam	culvert opening	11	3.4	1	0.3	65	104.6	none
49	Strong-Post, Modified W-Beam	culvert opening	11	3.4	0.5	0.2	55	88.5	none
50	Strong-Post, Modified W-Beam	roadside slope	12	3.7	1	0.3	60	96.6	none
51	Strong-Post, Modified W-Beam	roadside slope	12	3.7	2	0.6	65	104.6	none
52	Strong-Post, Modified W-Beam	roadside slope	12	3.7	1	0.3	65	104.6	none
53	Strong-Post, Modified W-Beam	roadside slope	12	3.7	0.5	0.2	65	104.6	none
54	Strong-Post, Modified W-Beam	roadside slope	11	3.4	6	1.8	65	104.6	none
55	2-Cable Low Tension	culvert opening	12	3.7	2.5	0.8	65	104.6	none
56	2-Cable Low Tension	roadside slope	12	3.7	4	1.2	65	104.6	none
57	2-Cable Low Tension	roadside slope	11	3.4	1	0.3	65	104.6	yes
58	2-Cable Low Tension	roadside slope	12	3.7	2	0.6	65	104.6	none
59	2-Cable Low Tension	roadside slope	12	3.7	3	0.9	65	104.6	none
60	2-Cable Low Tension	roadside slope	12	3.7	2.5	0.8	65	104.6	none

NA – Not able to document due to roadway conditions and/or other circumstances

Table 2. Summary of Field Investigation – Barrier, Obstacle, and Site Conditions (Continued)

System No.	Barrier System Description	Obstacle Type	Lane Width		Shoulder Width		Speed Limit		Curve
			(ft)	(m)	(ft)	(m)	(mph)	(km/h)	
61	2-Cable Low Tension	roadside slope	12.5	3.8	8	2.4	55	88.5	yes
62	2-Cable Low Tension	roadside slope	11	3.4	0.5	0.2	65	104.6	none
63	1-Cable Low Tension	culvert opening	11	3.4	1	0.3	45	72.4	none
64	Strong-Post, Channel Rail	roadside slope	12	3.7	0.5	0.2	40	64.4	yes
65	Strong-Post – Flat-Panel	roadside slope	11	3.4	6	1.8	65	104.6	none
66	Strong-Post – Flat-Panel	roadside slope	11	3.4	8	2.4	65	104.6	none
67	Strong-Post – Flat-Panel	roadside slope	11	3.4	6	1.8	65	104.6	none
68	Concrete Rail Installation	culvert opening	11	3.4	0.33	0.1	65	104.6	none

31

NA – Not able to document due to roadway conditions and/or other circumstances

Table 3. Summary of Existing Culvert Details

Culvert Site	Width		Length		Lateral Offset		Drop Height	
	(ft)	(m)	(ft)	(m)	(in.)	(mm)	(ft)	(m)
10	10	3.0	45	13.7	0	0	12	3.7
11	11	3.4	25	7.6	0	0	NA	NA
12	10	3.0	6	1.8	0	0	8	2.4
13	6	1.8	6.5	2.0	0	0	14	4.3
14	5	1.5	6.5	2.0	72	1829	NA	NA
15	8	2.4	21	6.4	10	254	NA	NA
16	10	3.0	25	7.6	12	305	NA	NA
17	30	9.1	25	7.6	22	559	NA	NA
18	30	9.1	20	6.1	12	305	NA	NA
19	30	9.1	6	1.8	76	1930	6	1.8
20	30	9.1	32	9.8	6	152	4	1.2
21	NA	NA	21	6.4	14	356	3	0.9
22	NA	NA	NA	NA	NA	NA	NA	NA
23	30	9.1	30	9.1	6	152	14	4.3
24	30	9.1	11	3.4	6	152	8	2.4
25	NA	NA	30	9.1	78	1981	NA	NA
26	NA	NA	25	7.6	12	305	NA	NA
27	30	9.1	30	9.1	6	152	NA	NA
28	30	9.1	12	3.7	0	0	NA	NA
29	30	9.1	25	7.6	NA	NA	NA	NA
30	20	6.1	25	7.6	0	0	14	4.3
31	NA	NA	NA	NA	NA	NA	NA	NA
32	NA	NA	25	7.6	6	152	NA	NA
33	NA	NA	25	7.6	6	152	NA	NA
46	30	9.1	7.5	2.3	0	0	NA	NA
47	8	2.4	22	6.7	0	0	NA	NA
48	NA	NA	30	9.1	12	305	NA	NA
49	NA	NA	13	4.0	56	1422	NA	NA
55	12	3.7	18	5.5	0	0	NA	NA
63	26	7.9	10	3.0	0	0	NA	NA
68	16	4.9	NA	NA	0	0	NA	NA

NA –Unable to document due to roadway conditions and/or other circumstances

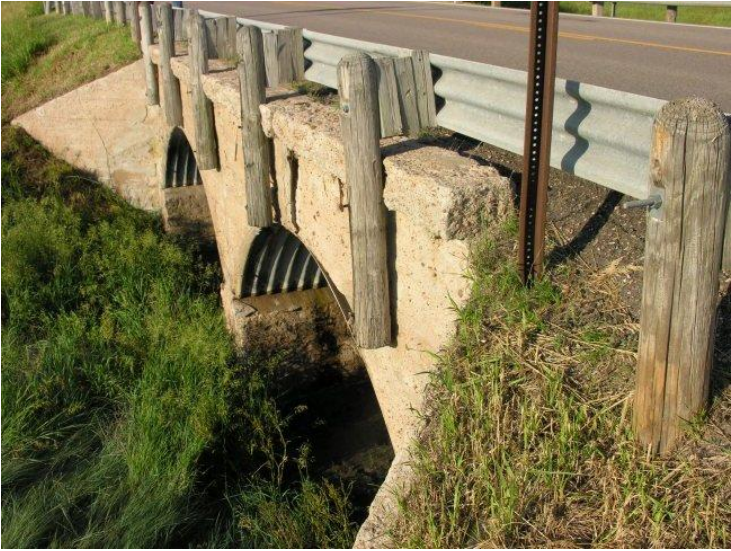


Figure 5. Examples of Shielded Culvert Systems

The roadside slopes that were documented in the field investigation varied in length, width, slope rate, drop height, and lateral offset away from the roadway. The length of the slope varied between 30 ft and 10,560 ft (9.1 m and 3,219 m). All slopes had a width greater than 30 ft (9.1 ft). The cross slope adjacent to the W-beam guardrail systems generally ranged between 5:1 and 1.5:1. The overall drop height of the slope varied between 7 ft and 15 ft (2 m and 4.6 m). The lateral offset from the face of the W-beam guardrail system to the slope break point ranged from 0 ft to 5 ft (0 m to 1.5 m). The cross slopes that were documented at existing W-beam guardrail systems are shown in Table 4. Examples of the documented roadside slopes are shown in Figure 6.

As previously noted, bridge rail ends were also documented in the field investigation. Bridge rail ends are typically placed at low lateral offsets away from the roadway edge, thus creating concern if not shielded or transitioned correctly.

For one particular site, a barrier system was used to shield both roadside trees and a small pond. Lateral tree offsets from the back of the rail of the W-beam guardrail system ranged from 5 ft to 15 ft (1.5 m to 4.6 m). The pond was laterally offset 5 ft (1.5 m) away from the back of the rail of the W-beam guardrail system. The trees and pond are shown in Figure 7.

3.2 Strong-Post W-Beam Guardrail

3.2.1 General Configurations and Concerns

W-beam guardrails were the most common feature that was documented during the field investigation (54 of the 68 documented barrier systems). The W-beam guardrail systems utilized wood posts in 46 systems, steel posts in 4 systems, and concrete posts in 4 systems. Wood posts were either round or rectangular sections with typical sizes of 7 in. (178 mm) diameter or 5½ in. x 7½ in. (140 mm x 191 mm), respectively. For the most part, the wood posts were in good

Table 4. Summary of Existing Roadside Slope Details

Slope Site	Length		Drop Height		Lateral Offset		Cross Slope
	(ft)	(m)	(ft)	(m)	(ft)	(m)	X to Y
34	6,336	1,931.2	NA	NA	3.5	1.1	2.5 to 1
35	100	30.5	6.5	2.0	0	0.0	2.5 to 1
36	NA	NA	11.5	3.5	NA	NA	NA
37	200	NA	NA	NA	0	0.0	2 to 1
38	876	267.0	12.5	3.8	2	0.6	2 to 1
39	500	152.4	NA	NA	3	0.9	2.5 to 1
40	639	194.8	12.5	3.8	0	0.0	NA
41	90	27.4	14	4.3	0	0.0	NA
42	404	123.1	13.5	4.1	5	1.5	2.5 to 1
43	300	91.4	NA	NA	0	0.0	4 to 1
44	400	121.9	12	3.7	0	0.0	NA
45	400	121.9	8	2.4	0	0.0	2.5 to 1
50	30	9.1	5.5	1.7	0	0.0	2.5 to 1
51	350	106.7	11	3.4	0	0.0	2.5 to 1
52	50	15.2	6	1.8	0	0.0	2.5 to 1
53	200	61.0	10.5	3.2	0	0.0	2 to 1
54	76	23.2	12.5	3.8	14	4.3	2.5 to 1
56	300	91.4	NA	NA	0	0.0	5 to 1
57	454	138.4	11	3.4	0	0.0	2.5 to 1
58	30	9.1	12.5	3.8	0	0.0	5 to 1
59	501	152.7	11	3.4	0.5	0.2	5 to 1
60	605	184.4	15	4.6	0	0.0	3 to 1
61	5,280	1,609.3	11.5	3.5	0	0.0	NA
62	402	122.5	8	2.4	0	0.0	3.5 to 1
64	10,560	3,218.7	21	6.4	4	1.2	3.5 to 1
65	64	19.5	11	3.4	4	1.2	3 to 1
66	64	19.5	7.5	2.3	4	1.2	3 to 1
67	273	83.2	13.5	4.1	5	1.5	3 to 1
Average	890.3	279.8	11.2	3.4	1.6	0.5	3.0 to 1
Max.	10,560	3,219	21	6	14	4	5 to 1
Min.	30	9	6	2	0	0	2 to 1

NA – Not able to document due to roadway conditions and/or other circumstances



Figure 6. Examples of Shielded Roadside Slopes



Figure 7. Examples of Shielded Roadside Trees and Pond

condition with some weathering and decay below the ground line. The steel and concrete posts had cross sections of W6x9 (W152x13.4) and 10 in. x 7 in. (254 mm x 178 mm), respectively. Nearly all of the systems utilized wood blockouts. However, two guardrail systems utilized steel I-beam blockouts, and 15 guardrail systems did not use blockouts.

The W-beam rail sections were generally in good condition, with some systems containing early stages of corrosion (i.e., rust) and a few systems damaged due to prior impacts. Out of the 54 systems, 9 had a modified rail element. The upper and lower edges of the modified W-beam were vertical rather than horizontal. Most guardrail systems utilized a splice with a 12½ in. (318 mm) lap and eight ⅝-in. (16 mm) diameter splice bolts, but nine systems utilized only three ⅝ in. (16 mm) bolts, instead of eight. All splice locations were centered at post locations. The W-beam guardrail systems were anchored at the ends with various types of end terminals. Spoon (blunt-end) terminals were used on 49 of the W-beam guardrail systems, while the other five W-beam guardrail systems utilized turned-down end terminals.

The guardrail systems were laterally offset away from the roadway edge by 1½ ft to 13 ft (0.5 m to 4.0 m) with a common offset of 6 ft (1.8 m). The W-beam guardrail systems shielded culvert openings, roadside slopes, bridge rail ends, small waterways, and trees. A summary of the documented W-beam guardrail systems is shown in Table 5. Sample photographs of the existing W-beam guardrail systems are shown in Figures 8 through 12.

3.2.2 Existing W-Beam Guardrail Height

In the field investigation, the maximum and minimum top rail heights were measured for each guardrail system. These height measurements were taken from the top of the rail to the ground as well as from the top of the rail to the roadway surface at the edge of travel lane, as shown in Figures 13 and 14. When compared to the recommended 31-in. (787-mm) top-rail

Table 5. Summary of Existing W-Beam Guardrail Systems – Barrier, Terminal, and Roadway Details

System No.	Post Material	Blockout Material	Terminal Type	Barrier Length (with Terminals)		Lateral Barrier Offset (roadway to barrier)		Post Spacing	
				(ft)	(m)	(in.)	(mm)	(in.)	(mm)
1	wood	wood	spoon	255	77.7	NA	NA	75	1,905
2	steel	none	spoon	NA	NA	NA	NA	NA	NA
3	wood	wood	spoon	63	19.2	41	1,041	75	1,905
4	wood	none	spoon	NA	NA	NA	NA	NA	NA
5	wood	wood	spoon	89	27.1	NA	NA	75	1,905
6	wood	wood	Turn-down	124	37.8	30	762	75	1,905
7	wood	wood	spoon	NA	NA	NA	NA	NA	NA
8	wood	wood	Turn-down	148	45.1	144	3,658	75	1,905
9	wood	wood	spoon	50	15.2	50	1,270	75	1,905
10	wood	wood	spoon	162.5	49.5	NA	NA	75	1,905
11	wood	wood	spoon	125	38.1	NA	NA	75	1,905
12	wood	wood	spoon	250	76.2	71	1,803	75	1,905
13	wood	wood	spoon	162.5	49.5	74	1,880	75	1,905
14	wood	wood	spoon	137.5	41.9	51	1,295	75	1,905
15	steel	steel	spoon	200	61.0	NA	NA	75	1,905
16	wood	wood	spoon	201	61.3	NA	NA	75	1,905
17	wood	wood	spoon	180	54.9	48	1,219	150	3,810
18	wood	wood	spoon	764	232.9	48	1,219	75	1,905
19	wood	wood	Turn-down	150	45.7	126	3,200	75	1,905
20	wood	wood	spoon	177	53.9	4	102	75	1,905
21	wood	wood	spoon	177	53.9	NA	NA	75	1,905
22	wood	wood	Turn-down	150	45.7	NA	NA	NA	NA
23	wood	wood	spoon	128	39.0	99	2,515	75	1,905
24	wood	wood	spoon	188	57.3	NA	NA	75	1,905

NA – Not able to document due to roadway conditions and/or other circumstances

Table 5. Summary of Existing W-Beam Guardrail Systems – Barrier, Terminal, and Roadway Details (continued)

System No.	Post Material	Blockout Material	Terminal Type	Barrier Length (with Terminals)		Lateral Barrier Offset (roadway to barrier)		Post Spacing	
				(ft)	(m)	(in.)	(mm)	(in.)	(mm)
25	wood	wood	spoon	190	57.9	138	3,505	75	1,905
26	wood	wood	spoon	210	64.0	96	2,438	75	1,905
27	wood	wood	spoon	125.5	38.3	54	1,372	75	1,905
28	wood	wood	spoon	151	46.0	53	1,346	150	3,810
29	wood	none	spoon	477	145.4	104	2,642	150	3,810
30	concrete	none	spoon	25	7.6	119	3,023	75	1,905
31	concrete	none	spoon	NA	NA	NA	NA	NA	NA
32	wood/ concrete	none	spoon	132	40.2	118	2,997	75	1,905
33	wood/ concrete	none	spoon	138	42.1	118	2,997	75	1,905
34	steel	none	spoon	6,336	1,931.2	18	457	150	3,810
35	wood	wood	spoon	100	30.5	50	1,270	150	3,810
36	wood	wood	spoon	NA	NA	NA	NA	NA	NA
37	wood	none	spoon	200	61.0	63	1,600	150	3,810
38	steel	steel	spoon	896	273.1	68	1,727	75	1,905
39	wood	wood	spoon	501	152.7	65	1,651	75	1,905
40	wood	wood	spoon	739	225.2	56	1,422	75	1,905
41	wood	wood	spoon	155	47.2	63	1,600	75	1,905
42	wood	wood	spoon	90	27.4	NA	NA	75	1,905
43	wood	wood	spoon	503.5	153.5	104	2,642	75	1,905
44	wood	wood	spoon	400	121.9	49	1,245	75	1,905
45	wood	wood	spoon	551	167.9	52	1,321	75	1,905
46	wood	wood	spoon	125	38	NA	NA	75	1,905
47	wood	wood	spoon	100	30	NA	NA	75	1,905
48	wood	wood	spoon	137	42	26	660	75	1,905

NA – Not able to document due to roadway conditions and/or other circumstances

Table 5. Summary of Existing W-Beam Guardrail Systems – Barrier, Terminal, and Roadway Details (continued)

System No.	Post Material	Blockout Material	Terminal Type	Barrier Length (with Terminals)		Lateral Barrier Offset (roadway to barrier)		Post Spacing	
				(ft)	(m)	(in.)	(mm)	(in.)	(mm)
49	steel/wood	none	spoon	27	8	56	1,422	150	3,810
50	wood	none	spoon	425	130	20	508	150	3,810
51	wood	none	spoon	350	107	47	1,194	150	3,810
52	wood	none	spoon	53	16	60	1,524	150	3,810
53	wood	none	spoon	190	58	59	1,499	150	3,810
54	wood	none	spoon	76	23	48	1,219	150	3,810

NA – Not able to document due to roadway conditions and/or other circumstances

mounting height, the W-beam heights found in the field investigation are very low. The mean, standard deviation, and range of the guardrail heights at the face of the rail and relative to roadway are shown in Table 6. Examples of W-beam guardrail found with low rail height are shown in Figure 15.

3.2.1 W-Beam Guardrail End Terminals

As noted previously, the W-beam guardrail end treatments found at the selected sites were the spoon (blunt-end) and turned-down (sloped-end) terminals. These terminal types are not acceptable according to the TL-3 safety performance criteria found in NCHRP Report No. 350 or MASH. A fishtail or spoon terminal acts as a blunt-end which can penetrate into the occupant compartment of errant vehicles. As observed in the field investigation, many of these blunt-end terminals lacked the proper tensile anchorage to adequately contain and redirect errant vehicles



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Figure 8. Examples of Existing W-Beam Guardrail Systems



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Figure 9. Examples of Existing W-Beam Guardrail Systems



Figure 10. Examples of Existing W-Beam Guardrail Systems



Figure 11. Examples of Existing W-Beam Guardrail Systems



Figure 12. Examples of Existing W-Beam Guardrail Systems

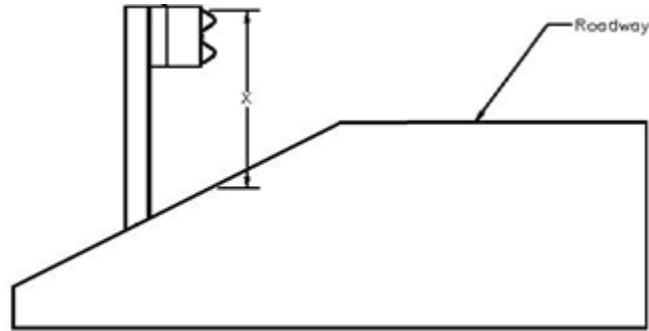


Figure 13. Guardrail Height Measured to the Ground at Rail Face

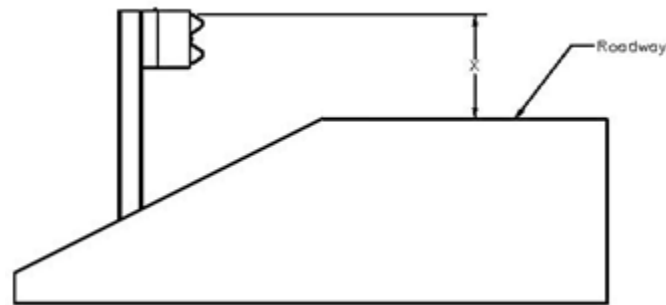


Figure 14. Guardrail Height Measured to the Ground at Roadway Edge

Table 6. Summary of Guardrail Heights from Field Investigation

	Guardrail Height							
	Ground at Face of Guardrail				Ground at Roadway Edge			
	Minimum		Maximum		Minimum		Maximum	
	(in.)	(mm)	(in.)	(mm)	(in.)	(mm)	(in.)	(mm)
Average	21.8	555	26	659	10.4	264	16.9	428
Range	11 to 32	279 to 813	17 to 52	432 to 1,321	-16 to 26	-406 to 660	6 to 30	152 to 762
Standard Deviation	4.8	122	5.5	141	7.8	199	5.3	134



Figure 15. Examples of Low Heights for Existing W-beam Guardrail Systems

which impact the guardrail system away from the ends. The turned-down terminal was developed to eliminate the potential for the rail to penetrate into the passenger compartment of an impacting vehicle, which was a significant improvement over the blunt-end. However, the sloped end acted as a ramp and allowed impacting vehicles to climb the rail, become airborne, and rollover. In some cases, the airborne vehicles impacted the vertical obstacles that were shielded by the guardrail under high-speed crashes. An errant vehicle impacting either of these non-crashworthy terminals may likely cause a more severe accident than striking the unshielded obstacle itself.

3.2.2 W-Beam Guardrail to Bridge Rail Transition

W-beam guardrail-to-bridge rail transitions were included in the field investigation and were found to deviate from current practice at many of the old sites. Some existing W-beam guardrails were not connected to the bridge rail ends. In most cases, an errant vehicle could likely contact the end of the rigid bridge rail. This heavy contact and inadequate vehicle redirection would likely result in snag on the bridge rail end with large decelerations and increased occupant risk. Approach guardrail transitions have been developed and successfully crash tested by using reduced post spacing, stronger or longer posts, stacked or nested rail elements, and gradual changes in lateral guardrail stiffness and strength. Examples of W-beam guardrail-to-bridge rail transitions that were found in the field investigation are shown in Figure 16.

3.2.3 Insufficient Length of Need

Guardrails are intended to protect motorists from roadside obstacles or portion of an obstacle, even when vehicles inadvertently leave the roadway upstream of the obstacle and would be unable to avoid that obstacle. The section which shields errant motorists from the



Figure 16. Examples of Existing W-beam Guardrail to Bridge Rail Transitions

obstacle is known as the length of need. Guardrail length of need consists of two guardrail sections: the length of the crashworthy terminal section capable of redirecting or containing the errant vehicle and the remaining guardrail that is required to meet the length of need. Many of the guardrails found in the field investigation had a much shorter length of need than the current recommended criteria. Some culverts only had guardrails on top of them, thus producing no upstream guardrail to shield errant vehicles from the obstacle.

3.2.1 Existing W-Beam Guardrail System Damage

State and federal agencies have limited funds and resources to repair all damage observed in a guardrail system. It is important to know what types of damage need immediate attention. System damage can be caused by prior vehicle crashes, maintenance equipment (snow plows and mowers), and corrosion to name a few. The system damage found in the field investigation included missing posts, missing blockouts, missing splice bolts, minor and major rail damage, minor corrosion of steel guardrail hardware, and weathering of wood posts. FHWA's *W-Beam Guardrail Repair-A Guide for Highway and Street Maintenance Personnel* informs highway officials when to repair damaged guardrail [51]. This guidance is helpful when evaluating a guardrail installation that does not deviate from current practice in any other way. The following sections describe the guardrail damage found in the field investigation. Engineering judgment should be used to evaluate when to repair, remove, or replace the existing guardrail system if there is damage or other deviations from current practice. When a system is damaged extensively, the entire guardrail system is often updated to the current guidelines. This practice should also be considered when a system is found with different levels of guardrail damage.

3.2.1.1 W-Beam Rail Damage

Damage on rail caused by previous impacts will most likely require repair unless the damage is minor. Scratches, small dents, and kinks can be considered to be minor in many circumstances. Significant damage can be characterized by tears, cuts, folds, and bends to name a few. Again, the *W-Beam Guardrail Repair Guide* and engineering judgment should be used when considering which of these systems would require repair and which are still crashworthy. Examples of rail damage found on existing W-beam guardrail systems are shown in Figure 17.

3.2.1.2 Missing Hardware

Missing splice bolts was another type of rail damage documented in the field investigation. Missing splice bolts and other small components were frequently observed on the W-beam guardrail systems. Out of the 54 W-beam guardrail systems, 13 systems had missing bolts at one or more splice locations. Splices are considered to be a weak point of a guardrail system, and missing splice bolts increase the risk of rail rupture at the splice location. This finding will increase the potential for vehicles to penetrate the rail and interact with the obstacle, which the rail was designated to shield. Missing splice bolts can be caused by improper construction, inspection, and maintenance practices. In the field investigation, many of the guardrail splices were missing four bolts.

3.2.1.1 Post Damage

Missing posts are common deviations from current practice in existing guardrail systems. Posts can be missing and/or ineffective because of prior impacts, snowplow damage, rotting wood, insect damage, frost uplift, and faulty construction. A system with one or two missing posts may function adequately under a majority of vehicle impacts [51-52]. Thus systems with three or more missing posts should be considered for repair. This finding is not to say that



Figure 17. Examples of Rail Damage in Existing W-Beam Guardrail Systems

a system with a missing post doesn't need repair. Existing guardrail systems with missing posts may need to be repaired for the guardrail to act as intended. Examples of this deficiency are shown in Figure 18.

Many wood posts found in the field investigation were weathered or rotted. This type of system damage can occur due to normal environmental conditions. Although these posts with superficial damage may appear weaker, they potentially may retain much of their structural integrity and possibly may not need repair. When significant rotting of wood material is found on multiple posts, repair or replacement of the components and/or guardrail system is necessary. Examples of weathered or rotted wood posts are shown in Figure 19.

3.2.1.2 Blockout Damage

Many blockouts found in the field investigation were weathered, rotting, rotated off center, or absent from the system at various post locations, with the most critical state being missing blockouts. Blockouts offset the W-beam rail element laterally away from the posts to mitigate the amount of wheel snag on the posts as well as maintain rail height. Missing blockouts may cause a guardrail to deviate from the expected guardrail performance. Blockouts can be missing from a system because of prior impacts, snowplow damage, material rotting, insect damage, and/or faulty construction. The performance of a guardrail system with a missing blockout may be comparable to a system with no missing blockouts [52]. For this reason, w-beam guardrail systems with missing blockouts do not require immediate repair. Systems with missing blockouts from the field investigation are shown in Figure 20.

FHWA's *W-Beam Guardrail Repair Guide* should be used for all damaged guardrails when no other deviations from current practice are found, such as low top-rail heights and outdated end treatments. Engineering judgment and the results provided in Chapters 8 and 9



Figure 18. Examples of Missing and Inadequate Posts



Figure 19. Examples Weathered and Decaying Post in Existing Guardrail Systems



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Figure 20. Examples Missing Blockouts in Existing Systems

should be used to determine whether to replace, remove, repair, or do nothing to the existing guardrail system if a guardrail installation has both system damage, as described in this section, and other deviations from current practice. The assessment of repairing damaged guardrail should include obstacle exposure, obstacle severity, severity of guardrail damage, guardrail hardware utilized, and agency resources.

3.3 Cable Barrier Systems

Out of the 68 barrier systems documented during the field investigation, 9 were cable barrier systems. The cable barrier systems were either two-cable, low-tension systems (8 systems) or single-cable, low-tension systems (1 system). The cables were generally in good condition. All of the cable barrier systems had wood posts, but two systems incorporated a combination of concrete and wood posts. The round and rectangular wood posts had typical cross sections of 7 in. (178 mm) diameter and 5½ in. x 7½ in. (140 mm x 191 mm), respectively. For the most part, the wood posts were in good condition with some weathering and decay below the ground line. The concrete posts had a cross section of 6 in. x 6 in. (152 mm x 152 mm). The post spacing for the cable barrier systems was 12 ft - 6 in. (3.8 m) for 8 systems and 10 ft (3.0 m) for 1 system. All systems used a large steel cable-to-post bracket. The longer barrier systems utilized 400-ft (121.9-m) cable segments, which were not connected to each other. The cable systems were used to shield roadside slopes and culvert openings. A summary of the cable barrier systems that were documented during the field investigation is shown in Table 7. Photographs of various documented cable barrier systems are shown in Figures 21 through 23.

In general, cable barrier systems redirect errant vehicles through the use of various mechanisms, including post bending or fracture, axial stretch of the cables, work done by frictional losses between the vehicle and barrier components, and other losses as the vehicle

Table 7. Summary of Existing Cable Barrier Systems - Design Details

System No.	Post Material	Terminal Type	Barrier Length		Barrier Offset		Post Spacing	
			(ft)	(m)	(in.)	(mm)	(in.)	(mm)
55	wood	none	100	30.5	10	254	150	3,810
56	wood	none	300	91.4	128	3,251	150	3,810
57	concrete/ wood	none	454	138.4	59	1,499	120	3,048
58	wood	none	153	46.6	127	3,226	150	3,810
59	wood	none	501	152.7	12	305	150	3,810
60	wood	none	605	184.4	9	229	150	3,810
61	wood	none	5,280	1,609.3	114	2,896	150	3,810
62	wood	none	402	122.5	78	1,981	150	3,810
63	wood	none	298	90.8	97	2,464	150	3,810

traverses the terrain. The documented cable barrier systems had many deviations from typical cable barrier systems. Most cables had kinks, slack (non-tensioned) spans, and corroded components. The concrete posts would become blunt obstacles to motorists, if impacted. The end sections of the existing barrier systems had two major concerns: (1) they did not have sufficient anchorage to produce enough strength on the ends of the cable systems to redirect an errant vehicle and (2) the end posts were exposed to errant vehicles, presenting a blunt end obstacle. Missing posts were also found within the systems. The use of only 1-cable or 2-cable systems may pose a risk of not being able to adequately contain or redirect an impacting vehicle.

3.4 Miscellaneous Barrier Systems

Out of the 68 documented barrier systems, 5 were classified as “Miscellaneous Barrier Systems”. These systems included channel rail, flat panel, and concrete systems, which are shown in Table 8.



Figure 21. Examples of Cable Barrier Systems



Figure 22. Examples of Cable Barrier Systems



Figure 23. Examples of Cable Barrier Systems

Table 8. Miscellaneous Barrier Parameters from Field Investigation

System No.	System Description	Post Material	Blockout Material	Barrier Length		Barrier Offset		Post Spacing	
				(ft)	(m)	(in.)	(mm)	(in.)	(mm)
64	Strong-Post Channel Rail	steel	spoon	10,560	3,219	0	0	150	3,810
65	Steel, Flat-Panel	wood	steel	64	20	4	102	192	4,877
66	Steel, Flat-Panel	wood	steel	64	20	7	178	192	4,877
67	Steel, Flat-Panel	wood	steel	273	83	66	1,676	192	4,877
68	Concrete Post and Rail	concrete	NA	NA	NA	NA	NA	48	1,219

NA – Not able to document due to roadway conditions and/or other circumstances

3.4.1 Steel, Flat-Panel Systems

Three of the 68 documented barrier systems were steel, flat-panel barriers. This guardrail system utilized a steel panel rail with an average thickness of 0.126 in. (3.2 mm). The flat-panel system used rectangular 5-in. x 7-in. (127-mm x 178-mm) wood posts with circularly looped, steel tube blockouts. The rail was spliced at each post with two steel ½-in. (13-mm) diameter pins. The upstream and downstream end treatments of all flat-panel systems were blunt ends with little or no anchorage. All three flat-panel systems were shielding slopes. Examples of the flat-panel systems are shown in Figure 24.

3.4.2 Channel Rail System

One documented barrier was regarded as a channel rail. The barrier appeared to be in good condition. The steel channel barrier was very similar to a typical W-beam guardrail and utilized steel W6x9 (W152x13.4) posts. Post spacing for the channel rail was 12 ft - 6 in. (3.8 m). Two steel brackets separated the rail from the posts. The upstream and downstream end treatments of the channel rail were blunt ends with no anchorage. Rail splices were located at



Figure 24. Examples of Flat-Panel Systems

each post location with twelve $\frac{5}{8}$ -in. (16-mm) splice bolts. The steel channel rail shielded the slope of a dam. Photographs of the channel rail system are shown in Figure 25.

3.4.3 Concrete Post and Rail System

One concrete rail with concrete posts over a culvert was discovered in the field investigation. The barrier was in good condition with minor cracks. The posts were 12 in. x 9 in. x 39 in. (305 mm x 229 mm x 991 mm) with a 48-in. (1,219-mm) post spacing. The barrier was not equipped with an end treatment. In some situations this barrier may pose a more severe obstacle than the obstacle it was shielding. Photographs of the concrete post and rail system are shown in Figure 26.



Figure 25. Examples of Channel Rail Systems



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Figure 26. Examples of Concrete Post and Rail System

4 ROADSIDE SAFETY ANALYSIS PROGRAM (RSAP)

4.1 RSAP Overview

RSAP provides a benefit-to-cost analysis procedure for use in developing general guidelines and best practices for upgrading existing barrier systems [10]. RSAP utilizes a probability-based approach to predict vehicle encroachments, impacts, and severities. RSAP predicts the benefits of reducing injuries and fatalities along with the costs of installation and forecasted repairs to the safety devices utilizing the Monte Carlo simulation technique. The Monte Carlo technique generates average impact conditions, such as impact speed and angle, for a particular set of roadway conditions. From this impact severity, accident costs for a particular roadside condition can be determined. The benefits are defined as a reduction in injuries and fatalities in terms of a unit of cost. If the benefits of a particular system outweigh its material and installation costs, then that barrier alternative is recommended for use at that particular site. RSAP is also able to examine multiple alternatives at once, making it possible to select the optimum solution from various treatment options. The general formulation for the B/C method provided in RSAP is shown in Equation 1.

$$B/C Ratio_{2-1} = \frac{AC_1 - AC_2}{DC_2 - DC_1} \quad (1)$$

Where,

B/C Ratio₂₋₁ = Incremental B/C ratio for Alternative 2 to Alternative 1

AC₁, AC₂ = Annualized societal crash cost for Alternative 1 and Alternative 2, respectively

DC₁, DC₂ = Annualized direct costs for Alternatives 1 and Alternative 2, respectively

The encroachment module used in RSAP was based on a study conducted by Cooper in the late 1970's [9]. This study was performed by collecting encroachment data from off-road tire tracks. The results of the Cooper data are shown in Figure 27. There were two significant

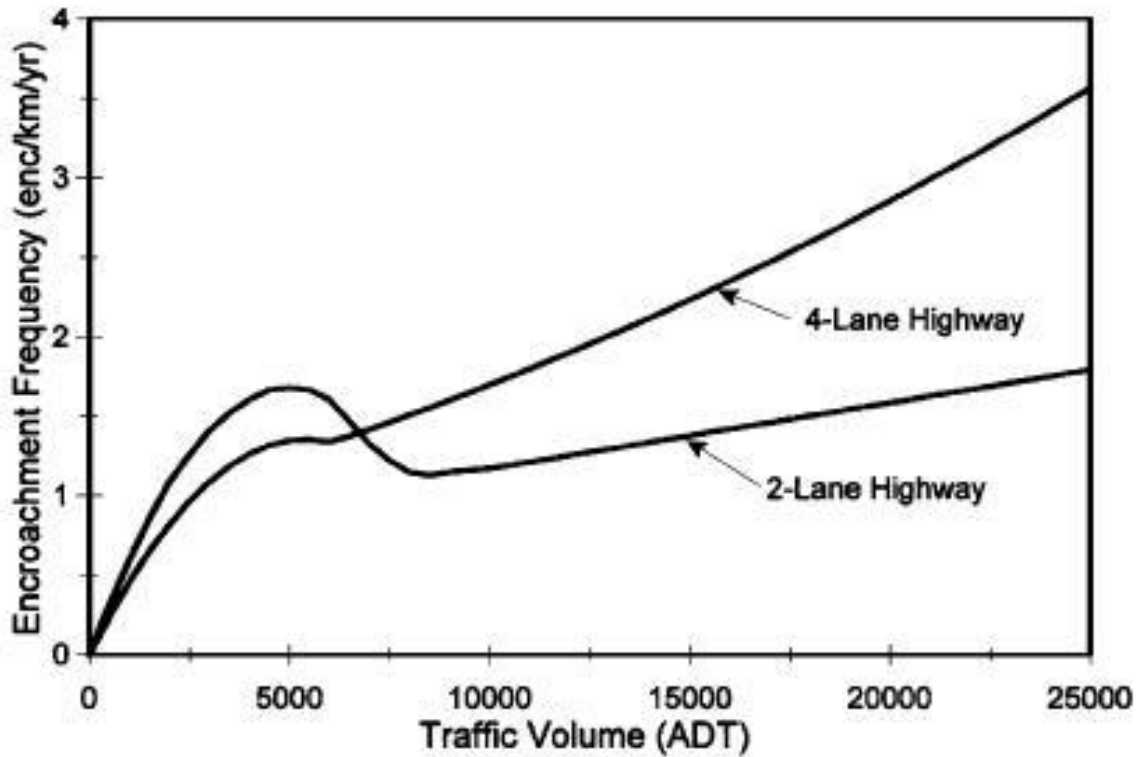


Figure 27. Encroachment Rates from Cooper [9]

concerns from this study. First, there were no recorded encroachments less than 13.1 ft (4 m) laterally due to paved shoulders. The re-analysis of the Cooper encroachment data on the extent of lateral encroachment involved fitting a regression model to lateral extent data beyond 13.1 ft (4 m). The results of the lateral extent data regression is shown in Figure 28. From these results, it was estimated to increase the encroachment frequencies by a ratio of 2.466 on two-lane undivided highways [10]. A separate study was used to distinguish controlled and uncontrolled encroachments [57]. A controlled encroachment occurs when a driver purposefully drives off the travelway for a particular reason, such as pulling over to look at a map. This consideration would then reduce the amount of uncontrolled encroachments. It was estimated that encroachment frequency was multiplied by a factor of 0.60 to account for this issue.

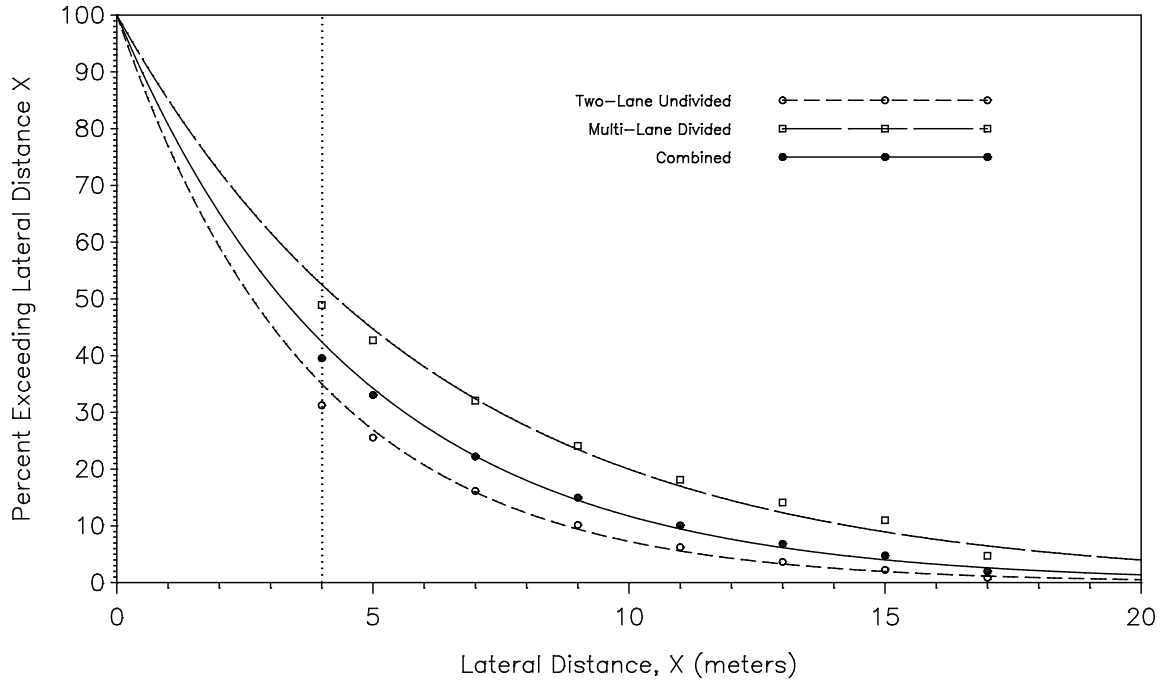


Figure 28. RSAP Lateral Extent of Encroachment Distribution [9]

From the encroachment module, an impact into a roadside feature may be predicted during the crash prediction module. This can be determined by the trajectory (i.e., speed, angle, and location) of the errant vehicle from the roadway and location of the defined obstacle. If an obstacle was in the path of an encroaching vehicle, an impact was predicted. Each obstacle is defined with a containment value. In RSAP, this value can determine if the errant vehicle has enough energy to penetrate through an obstacle or barrier and interact with objects placed behind. This was a very important occurrence when modeling barriers with deviations from current practice.

When RSAP generates a predicted accident from the encroachment probability, it must also have an associated calculated cost of the accident. This is done using the severity of the crash (i.e. severity level). The severity level is found by developing a link between vehicular

impact conditions and the Severity Index (SI) of the obstacle or barrier. SI is a scale of crash severity ranging from 0 (no damages) to 10 (100 percent fatality rate). RSAP attempts to assign an SI value for each predicted impact based upon the predicted speed, impact angle, and the obstacle struck. The SI values are based on percentages of injury levels of impacts as incorporated into RSAP, as shown in Table 9. Finally a benefit-to-cost module was developed. This was based on the results of the preceding modules (encroachment, crash prediction, and severity modules). The benefit-to-cost module compares the direct and accident costs from a number of alternatives to develop a guideline based on the input data.

Table 9. Injury Level Percentages for Each Severity Index [10]

Severity Index (SI)	Injury Level (%)						
	None	PDO1	PDO2	C	B	A	K
0	100.0	-	-	-	-	-	-
0.5	-	100.0	-	-	-	-	-
1	-	66.7	23.7	7.3	2.3	-	-
2	-	-	71.0	22.0	7.0	-	-
3	-	-	43.0	34.0	21.0	1.0	1.0
4	-	-	30.0	30.0	332.0	5.0	3.0
5	-	-	15.0	22.0	45.0	10.0	8.0
6	-	-	7.0	16.0	39.0	20.0	18.0
7	-	-	2.0	10.0	28.0	30.0	30.0
8	-	-	-	4.0	19.0	27.0	50.0
9	-	-	-	-	7.0	18.0	75.0
10	-	-	-	-	-	-	100.0

Where,

- PDO1 = Property Damage Only (Level 1)
- PDO2 = Property Damage Only (Level 2)
- C = Possible or Minor Injury
- B = Moderate Injury
- A = Severe Injury
- K = Fatal Injury

4.2 RSAP Considerations

The RSAP program is currently being updated in NCHRP Project No. 22-27. During the research effort to update the current RSAP program, Dr. Malcolm Ray found many discrepancies, bugs, and errors in the RSAP code. Discrepancies occurred when information from the RSAP Engineering Manual [10] or the RSAP User Manual [58] differed from the actual program. Bugs are faulty programming logic. Errors are mistakes made in the code. These issues may lead to inaccurate results. A complete list of the discovered discrepancies, bugs, and errors are shown in the draft report of NCHRP Project No. 22-27 [59]. However, the discovered problems were determined to be insignificant in the scope of this project. As such, the original RSAP program was utilized for this study after addressing some of the known concerns.

RSAP (Version 2003.04.01) [10] incorporates two integrated programs, the Main Analysis Program and the User Interface Program. This user interface provides a user-friendly environment for data input and review of the program results from data files. One of these files is called “road.dat,” which contains parameters to model the roadway, such as functional class, number of lanes, lane width, speed limit, segment length, and horizontal/vertical curve information. The functional class is determined by a two-digit number, which was then used by the Main Analysis Program to determine the speed and angle of the vehicle encroachments. The functional class selected in the user interface differs from the Main Analysis Program, as shown in Table 10. Rural arterials were the only functional class used in this project, which was determined later in this report. Thus, this problem was found to be insignificant in the scope of this project.

Table 10. Functional Class Code Differences

Functional Class	User Interface	Analysis Program
Freeway	22	21
Urban Arterial	25	12
Urban Local	24	15
Rural Arterial	22	22
Rural Local	21	25

5 CONSTANT RSAP MODELING PARAMETERS

5.1 Societal Costs

RSAP has two predefined sets of accident crash costs from the RDG and FHWA. These costs are intended to associate a dollar value to societal costs for an accident resulting in a certain injury level. The RDG accident costs are not considered to be comprehensive and do not include all factors, such as a person's willingness to pay to improve safety (i.e. avoid injury or fatality). The FHWA values are based on the 1994 U.S. dollar. However, adjustments have been made in a previous study, namely the 2009 FHWA's *Highway Safety Improvement Program Manual*, as shown in Table 11 [60]. These values were incorporated into RSAP for this study.

Table 11. FHWA's 2009 Comprehensive Accident Costs [60]

Accident Type	Accident Costs (\$)
Fatal	4,008,900
Severe Injury	216,000
Moderate Injury	79,000
Minor Injury	44,900
Property Damage Only	7,400

5.2 Highway Modeling

5.2.1 Sensitivity Analysis

The roadway sections implemented into RSAP were modeled to represent the rural Kansas highways that were documented in the field investigation. Three steps were used to best determine how each roadway feature was modeled. First, the results from the field investigation were analyzed to determine the common roadway features found. Next, a sensitivity analysis was performed in RSAP to conclude if the roadway feature differences had a substantial effect on the

accident cost. This analysis was completed setting all variables pertaining to the roadway, obstacle, and barrier constant in RSAP to a standard base condition and then changing one roadway parameter to see how or if it affected the results. The variables that were subjected to the sensitivity analysis were chosen using information found in the field investigation and team discussion. The roadway conditions were modeled with a TL-1 W-beam guardrail and a culvert opening model on rural arterial highway to generate accident costs. The predefined TL-1 W-beam guardrail better modeled the existing low to rail height W-beam systems. The baseline conditions for the roadway sensitivity analysis are shown in Table 12. The roadway variables examined in the sensitivity analysis and results are shown in Table 13. If the feature parameters had little difference to the baseline, only a few or one value was used for that variable in the final RSAP set. The last step in modeling the RSAP runs was a team discussion. In the discussion, the final roadway constraints were determined based on the field investigation, sensitivity analysis, and engineering judgment, as described in this section.

5.2.1 Highway Type

All roadways documented in the field investigation were two-lane roadways without medians. Around 90 percent of the roadways were minor arterial roadways, as defined by Kansas DOT. For these reasons, two-lane undivided, minor arterial roadways were the highway type selected for the RSAP analysis.

5.2.2 Lane Widths

As previously noted, lane widths were typically 12 ft (3.7 m). However, some roadways had lane widths of 9 ft (2.7 m). Distributions of lane widths found in the field investigation are shown in Figure 29. The sensitivity analysis showed little variation in the results when changing

the typical lane width of 12 ft (3.7 m) to 10 ft (3.0 m) and 11 ft (3.4 m) (both less than 10 percent change). For this reason, only roadways with 12 ft (3.7 m) lane widths were considered.

Table 12. Roadway Sensitivity Analysis – Baseline Conditions

Feature	Design Parameter	Base Condition
Highway	Area Type	Rural
	Functional Class	Minor Arterial
	Highway Type	Two-Way, Undivided
	Number of Lanes	2
	Lane Width	12 ft (3.7 m)
	Speed Limit	55 mph (88.5 km/h)
	ADT	5,000
	Percent Trucks	10
	Traffic Growth Factor	0
	Encroachment Rate Adjustment Factor	1
Barrier	System Type	TL-1 Guardrail
	Terminal Type	Blunt End
	Lateral Offset	3 ft (0.9 m)
	Length of Need	221 ft (67.4 m)
Culvert	Drop Height	13 ft (4.0 m)
	Length	30 ft (9.1 m)
	Lateral Offset	4 ft (1.2 m)
Fill Slope	Slope Rate	4 to 1
	Drop Height	7 ft (2.0 m)
	Length	40 ft (12.2 m)
	Lateral Offset	4 ft (1.2 m)

5.2.1 Shoulders

All roadways had paved surfaces in the field investigation. Only one documented barrier type had a paved shoulder adjacent to the roadway. The width of grass and gravel shoulders was documented. After conducting a sensitivity analysis of different shoulder widths, it was found that they did not significantly influence the results. Therefore, shoulders were omitted from the

B/C analysis. These values were just considered as part of the lateral offset of the existing W-beam guardrail system in the RSAP analysis from the roadway.

Table 13. Roadway Sensitivity Analysis - Results

Road Parameters	Base Condition	Changed Condition	Estimated Annual Crash Costs (USD)	Percentage Change
Base	Base	None	\$14,326	NA
ADT	5,000	1,000	\$5,041	-64.8%
	5,000	25,000	\$15,299	+6.8%
Horizontal Curve	No Curve	5 Degree Right	\$19,536	+36.4%
	No Curve	5 Degree Left	\$33,156	+131.4%
Lane Width	12 ft (3.7 m)	10 ft (3.0 m)	\$15,614	+9.0%
	12 ft (3.7 m)	11 ft (3.4 m)	\$15,242	+6.4%
Shoulder Width	2.5 ft (0.8 m)	0 ft (0.0 m)	\$14,326	0.0%
	2.5 ft (0.8 m)	12 ft (3.7 m)	\$14,326	0.0%
Vertical Grade	No Grade	3% Downgrade	\$15,630	+9.1%

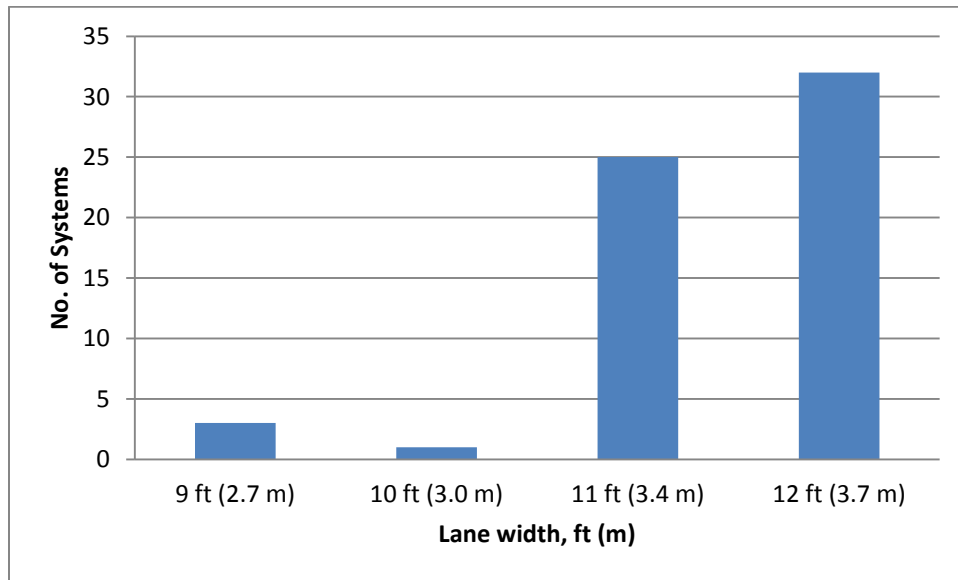


Figure 29. Lane Width found in Field Investigation

5.2.2 Speed Limit

The speed limit is another input to consider in RSAP. As previously noted, the posted speed limits found on these roadways varied from 35 mph to 65 mph (56.3 km/h to 104.6 km/h), as shown in Table 14. Although most roadways had a 65-mph (104.6-km/h) speed limit, the speed data in RSAP was based on the Cooper encroachment study, which was completed when the national speed limit was set at 55 mph (88.5 km/h) [9]. As a result, speeds above 55 mph (88.5 km/h) are not considered to be modeled correctly. Thus, all RSAP models were set with a 55 mph (88.5 km/h) speed limit.

Table 14. Distribution of Speed Limits Found in the Field Investigation

Speed Limit	mph	65	60	55	50	45	40	35
	km/h	104.6	96.6	88.5	80.5	72.4	64.4	56.3
No. of Systems		43	3	8	1	3	2	1

5.2.3 Average Daily Traffic (ADT)

As previously noted, the ADT on the roadways documented in the field investigation ranged from 300 to 11,000 vpd. The ADT has a big influence of the accident frequency in RSAP, as shown from the sensitivity analysis results (64.8 percent change from 5,000 to 1,000 vpd). After completing the sensitivity analysis and team discussion, ADTs of 500, 1,000, 5,000, 10,000, and 25,000 vpd were chosen for the RSAP analysis based on the significant changes in the sensitivity analysis.

5.2.4 Other Roadway RSAP Parameters

The nominal percentage of trucks was set to 2 percent. Traffic growth factor was set to zero, and the encroachment rate adjustment factor was left unchanged at the default value of 1.

Default values of 25 years and 4 percent were used for the design life and discount rate, respectively.

5.3 Segment Modeling

5.3.1 Segment Length

The length of the evaluated road was 3,281 ft (1,000 m) long. This would allow for a longitudinal provision for the clear area on either side of the downstream and upstream guardrail terminals.

5.3.2 Vertical Grade

There were vertical grades reported in the field investigation, but no values were recorded. From results of the sensitivity analysis, the change from flat ground to a 3 percent down grade was under 10 percent. After team discussion, it was determined to leave vertical grade out of the RSAP analysis, and only flat ground was considered.

5.3.3 Horizontal Curvature

The final criteria to consider in segment modeling were horizontal curves. Although only 9 of the 68 barriers in the field investigation had a horizontal curve, it was still determined by the sensitivity analysis and group discussion that implementing a curve for the RSAP analysis was needed. RSAP only analyzes traffic in one direction, so it is important to find which direction of curvature would make the most severe roadside conditions. Left-hand curves were more severe than right-hand curves due to increased encroachment frequency, as shown in the sensitivity analysis (5 degree left-hand turn resulted in a 131.4 percent increase in accident costs). So a typical 5-degree left curve, or 1,146-ft (349-m) radius curve, and a straight roadway segment were used in the RSAP models.

6 BARRIER AND OBSTACLE SELECTION

6.1 Introduction

RSAP has the ability to evaluate many different roadway conditions, barriers, and obstacles. In order to best evaluate existing guardrail systems and keep the RSAP evaluation matrix manageable, the amount of variables was limited to only those deemed most critical. Thus, the most prominent and severe features found in the field investigation were selected to be evaluated in RSAP.

6.2 Obstacle Selection

The selection of a representative obstacle was based on the number of occurrences, the severity of the obstacle, and the relative distance between the feature and the edge of roadway. It was important to select obstacles which would encompass most situations, yet still keep the RSAP evaluation matrix manageable in size. Common roadside obstacles that were shielded by existing barriers on Kansas DOT highways included culvert openings, roadside slopes, bridge rail ends, small waterways, and trees.

The trees and waterway obstacles were only documented at one guardrail location. In light of the limited exposure in the field investigation, these two obstacles were omitted from further analysis.

All documented bridge approach guardrail (i.e. transitions) utilized a W-beam guardrail connected to a concrete bridge rail. These stiffness transition systems had many deviations from current practice for W-beam guardrail transitions. Blunt-end terminals were the only end treatments found at the locations of the bridge approach guardrails that were included in the field investigation. The approach guardrail normally included two steel posts bolted to a bridge curb, which were used to extend the W-beam rail past the end of the concrete bridge rail. However, the

W-beam rail was rarely appropriately anchored to the concrete bridge parapet. No W-beam guardrail stiffening was used, such as reduced post spacing or increased post size. For these reasons, it can be expected that most high-speed impacts into these approach barriers would result in high severity crashes. The analysis of bridge transitions was left out of the RSAP analysis. Due to the deficiencies, it was recommended that all non-crashworthy transition and end terminal systems be upgraded with systems that have met current impact safety guidelines.

From the field investigation, culvert openings and roadside slopes were the most prominent obstacles that were shielded by an existing barrier system with documented deviations from typical practice. Both obstacle types were found near the traveled way and are easily modeled using predefined features within RSAP. The culvert structures varied in length, drop height, lateral offset, and width. The roadside slopes varied in length, slope rate, drop height, lateral offset, and width. The high frequency, high severity, and small lateral offset away from the roadway edge to culvert openings and roadside slopes made them prime candidates for consideration in an RSAP analysis to evaluate the cost-effectiveness of various safety treatments.

6.3 Barrier Selection

Existing outdated barriers may still provide substantial benefit to the motorist population. These existing barriers still provide some level of vehicle containment for errant vehicles and delineation of known objects or areas of concern near the roadway. The existing barriers were selected for RSAP analysis based on the number of specific systems documented in the field, the condition of each system, and the ability to model the various systems in RSAP. The various barrier systems documented in the field investigation were W-beam guardrail, cable barrier system, flat-panel guardrail, modified W-beam guardrail, channel rail system, and roadside concrete barriers. Many of the documented systems provided little or no vehicle containment,

thus allowing a high probability of penetrating the existing barrier and interacting with the obstacle as well. Thus, the best practice, when feasible, may be to remove these barriers (cable, flat-panel, and the concrete post and rail systems) and replace them with a crashworthy system meeting current design and safety guidelines.

Cable barrier systems are not a predefined feature in RSAP. They are assumed to have the same severity and containment level as a typical W-beam guardrail system. The existing cable barrier systems had slack cables, kinks, faulty transitions, strong-posts, outdated cable brackets, and other deviations from a current crashworthy, cable barrier system. No cable barrier systems had crashworthy terminal ends. The existing cable barrier systems would provide very little containment and redirection for an errant vehicle due to the slack cable segments, only one or two cable wire ropes, and lack of anchorage at many of the ends. Thus, cable barrier systems were not selected for evaluation in RSAP; since, cable barrier systems are modeled in a similar manner to that of W-beam guardrails. In addition, extensive deviations from current practice were found in these cable barrier systems. Thus, the existing cable barrier systems should be considered for removal or replacement as no further RSAP analysis was completed. However, designers can utilize the barrier selection guidelines developed herein to determine the proper treatment of these special cases.

Likewise, the flat-panel systems and the concrete post and rail systems found in the field investigation have become obsolete. Thus, these barriers could not be upgraded but instead are recommended to be removed. However, just like the obsolete cable barrier systems, designers can utilize the barrier selection guidelines developed herein to determine the proper treatment of these cases.

Strong-post, W-beam guardrail systems were the most common documented barrier system. Most of these systems had the ability to contain and redirect an errant vehicle, and therefore provided some safety and societal benefits to motorists. Due to the common occurrence of the strong-post, W-beam guardrail system and the modeling ability in RSAP, W-beam guardrail systems were ideal for this investigation. Additionally, the older versions of modified W-beam and channel rail systems were of similar conditions and appeared to provide similar strengths and capacities. Thus, modeling recommendations for the W-beam analysis would apply to these systems as well.

7 W-BEAM GUARDRAIL CONTAINMENT LEVEL – PERFORMANCE LIMITS

7.1 Problem

As stated previously, a major concern for existing W-beam guardrail systems is the top rail mounting height. An insufficient top rail height can allow vehicles to climb, override, or penetrate a guardrail system. These behaviors pose a major concern; since, a guardrail's primary function is to shield those obstacles located behind them. Thus, guardrail height was an important parameter to model and consider in the RSAP analysis. There are two means of raising the guardrail height: (1) replace the guardrail system with a guardrail system which meets all current criteria or (2) reset the rail to the original design height. However, if the guardrail system presented other deviations from current practice, raising the rail may not be a viable option. Thus, replacement was the only option considered herein.

For the RSAP analysis, it was necessary to first determine the guardrail heights for examination. The chosen heights were representative of guardrail installations found in the field, which had potential to redirect an errant vehicle. After evaluating the existing systems encountered in the field investigation, four guardrail heights - 31-in. (787-mm), 27 in. (686 mm), 25 in. (635 mm), and 22 in. (559 mm) - were selected for further investigation and evaluation in RSAP.

7.2 Low Rail Height Options for RSAP Modeling

The next step was to determine how to model different guardrail heights in RSAP. Options included changing the defined mounting height, severity index, and containment limit. The containment limit is defined as the maximum impact severity (IS) that a guardrail system can withstand without allowing an errant vehicle to penetrate or override the guardrail system.

RSAP uses barrier mounting heights to predict rollovers associated with heavy trucks. All other vehicles are unaffected by the change in the guardrail height. Thus, changing the defined mounting height in RSAP would not accurately model the performance of the guardrail systems found in the field investigation.

Changing the severity index for each guardrail height could make lower guardrails more severe in an impact event, potentially representing a higher potential for override or rollover. However, the research team could not obtain any data that would objectively measure the change in guardrail system performance associated with a low rail height.

Changing the containment limit based on vehicle type could accurately model existing guardrail systems with a low top mounting height. However, accurately identifying the effect of guardrail height versus vehicle size would be insurmountable.

The final option was to change the containment limit of the guardrail based on different guardrail heights alone. This option would not consider the full variation in vehicle properties found in the vehicle feet. This option required a review of full-scale W-beam crash tests on different guardrail heights. From this investigation, the results would need to be correctly implemented into RSAP. It was found that changing the containment limit of guardrail with different rail heights would be the best means for modeling the 31-in. (787-mm), 27-in. (686-mm), 25-in. (635-mm), and 22-in. (559-mm) guardrail heights in RSAP. The defined guardrail heights would also be changed to simulate rollover of heavy truck vehicles.

7.3 Containment Limit

As stated previously, the containment limit is the maximum kinetic energy that a guardrail system can withstand during the successful containment and/or redirection of an impacting vehicle. This value is then compared to the impact severity (IS). The IS value is a

portion of the kinetic energy of the impacting vehicle which is calculated by taking the lateral velocity vector squared and multiplying it by one-half and the mass of the vehicle, as shown in Equation 2. Any vehicle impact condition with an IS value greater than the set containment limit has the potential to penetrate/override the defined guardrail system.

$$IS = \frac{1}{2} * m(V * \sin\theta)^2 \quad (2)$$

where,

- IS* = Impact Severity (ft-lbf, Joules)
- m* = Mass of impacting vehicle (lbm, kg)
- V* = Velocity of impacting vehicle (ft/s, m/s)
- θ* = Angle of encroachment (deg)

To determine values of the containment limit for the three guardrail heights, a literature search was performed as described in Section 2.5. These values were generated from previously crash tests and/or simulations of W-beam guardrail systems. From each test, the speed, impact angle, and the mass of the vehicle were used to determine the IS of the impact giving the containment limit for its respected guardrail height. Only the highest IS value for its respective height was taken into consideration. No failed tests were included in the determination of the containment limit determination.

It should be noted that two of the six points used to find the best fit line were determined by the use of computer simulations to generate more data points. A 2270P vehicle model impacted a W-beam guardrail at both 22-in. (559-mm) and 25-in. (635-mm) rail heights with a 25-degree impact angle and varying speeds. The 25-in. (635-mm) guardrail height contained the impacting vehicle at 43.5 mph (70 km/h), thus resulting in a containment limit value of 57,000 ft-lbf (77,000 J). The 22-in. (559-mm) guardrail height failed to completely contain the vehicle at 43.5 mph (70 km/h), because the tire of the vehicle rode on top of the rail element. This

simulation was deemed to be “marginal,” so 37.3 mph (60 km/h) was used to determine the containment limit of 42,000 ft-lbf (56,000 J). The simulation results are shown in Appendix B.

The containment limit values for the selected guardrail tests are shown in Table 15, and the resulting values used in RSAP are shown in Table 16. The containment limit values were graphed in Figure 30 and were based on a best-fit linear regression line from the data points. From the slope of the best-fit line, containment limit values were found for the 31-in. (787-mm), 27 in. (686 mm), 25 in. (635 mm), and 22 in. (559 mm) top guardrail heights.

Table 15. Full-Scale W-beam Crash Test Information

Vehicle Type	Guardrail Height		Vehicle Weight		Angle (deg.)	Speed		Containment Limit		Reference
	(in.)	(mm)	(lb)	(kg)		(mph)	(km/h)	(ft-lbf)	(Joules)	
2000P ¹	31	787	4,441	2,014	36.7	65.0	104.7	224,000	304,000	[47]
2000P ¹	27¾	705	4,577	2,076	25.5	63.1	101.5	113,000	153,000	[48]
2000P ¹	27	686	4,572	2,074	24.3	62.6	100.8	102,000	138,000	[49]
2270P ²	25	635	5,004	2,270	25	43.5	70.0	57,000	77,000	Appendix B
Sedan ¹	24	610	4,570	2,073	25	59.0	95.0	95,000	129,000	[50]
2270P ²	22	559	5,004	2,270	25	37.3	60.0	42,000	57,000	Appendix B

- 1 – Full-Scale Crash Test
- 2 – Crash Test Simulation

Table 16. Containment Limit Values Used in RSAP

Guardrail Height		Containment Limit	
(in.)	(mm)	(ft-lbf)	(Joules)
31	787	196,000	266,000
27	686	122,000	165,000
25	635	84,000	114,000
22	559	29,000	39,000

Containment Level vs Guardrail Height

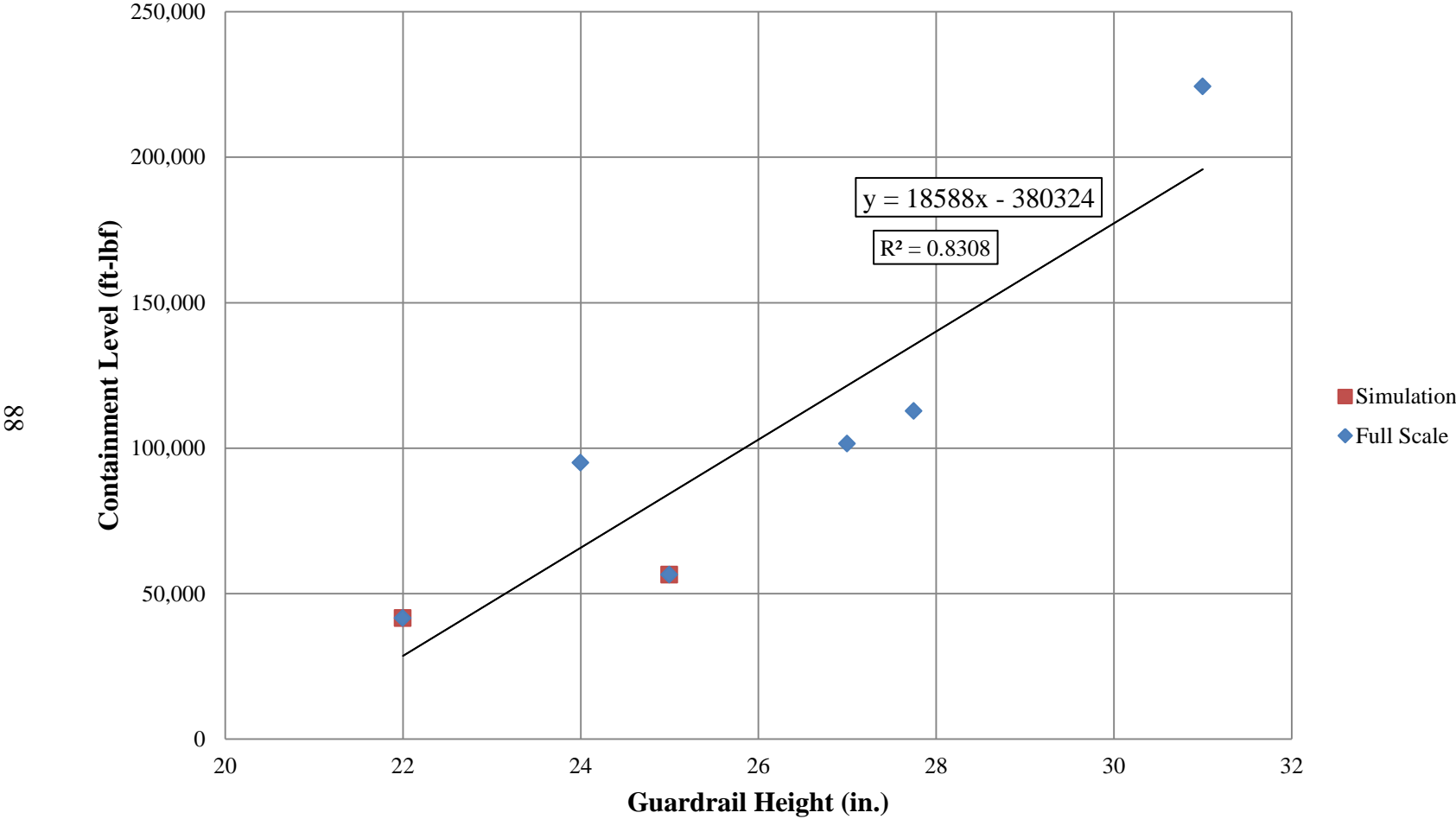


Figure 30. Containment Limit from Selected Guardrail Crash Tests and Simulations

8 ANALYSIS OF EXISTING W-BEAM GUARDRAILS SHIELDING CULVERTS

8.1 Introduction

As noted previously, most W-beam guardrail systems that were documented in the field investigation were utilized to shield errant motorists from culvert openings. The existing W-beam guardrails utilized wood, concrete, or steel posts across the culvert. Most of the guardrail systems utilized wood posts which were placed in front of the culvert edge. W-beam guardrail systems, which utilize robust reinforced concrete posts, potentially increase motorist risk due to being more rigid obstacle above the culvert. The steel posts and wood posts were attached to the back side of the culvert with the use of two horizontal bolts embedded in the concrete head wall. The majority of the systems had low rail heights and blunt-end guardrail terminals. Therefore, it was deemed necessary to evaluate the cost-effectiveness of safety treatments for existing W-beam guardrails used to shield culvert openings.

8.2 Investigation of Existing Guardrail Systems Shielding Culverts

The existing W-beam guardrail and culvert systems were modeled in RSAP with a wide range of design parameters, as depicted in Table 17. First, a sensitivity analysis was performed in RSAP to determine if various parameters had a substantial effect on the accident cost. This process was completed by setting all roadway, culvert, and guardrail system variables constant in RSAP to represent the base condition. A rural, arterial, two-lane, undivided highway, an ADT of 5,000 vpd, and a straight roadway segment were used to represent the roadway conditions for the sensitivity analysis. The baseline conditions for the sensitivity analysis are shown in Table 18. Then, one guardrail or culvert parameter was changed to investigate if and how it affected the results. Several variables were subjected to a sensitivity analysis and were based on the project team's discussion and engineering judgment. These design parameters and results are shown

Table 17. RSAP Variables Considered for W-Beam Guardrail Shielding Culverts

Features	Design Parameters
Roadway	ADT, Lane Width, Number of Lanes, Highway Type, Speed Limit, Shoulder Width
Guardrail System	System Length, Guardrail Height, Terminal Type, Lateral Offset
Culvert	Drop Height, Width, Length, Lateral Offset

Table 18. Sensitivity Analysis for W-Beam Guardrail Shielding Culverts – Baseline Conditions

Feature	Design Parameter	Base Condition
Highway	Area Type	Rural
	Functional Class	Minor Arterial
	Highway Type	Two-Way, Undivided
	Number of Lanes	2
	Lane Width	12 ft (3.7 m)
	Speed Limit	55 mph (88.5 km/h)
	ADT	5,000
	Percent Trucks	10
	Traffic Growth Factor	0
	Encroachment Rate Adjustment Factor	1
Guardrail System	System Type	TL-1 Guardrail
	Terminal Type	Blunt End
	Lateral Offset	3 ft (0.9 m)
	Length of Need	221 ft (67.4 m)
Culvert	Drop Height	13 ft (4.0 m)
	Length	30 ft (9.1 m)
	Lateral Offset	4 ft (1.2 m)
Fill Slope	Slope Rate	4 to 1
	Drop Height	7 ft (2.0 m)
	Length	40 ft (12.2 m)
	Lateral Offset	4 ft (1.2 m)

in Table 19. If the feature parameters had little difference to the baseline condition, then only a few or one value was used for that variable in the final RSAP simulation matrix. The final W-beam constraints were determined based on the field investigation, sensitivity analysis, and engineering judgment.

Table 19. Sensitivity Analysis for W-beam Guardrail Shielding Culverts - Results

Design Parameter	Base Condition	Changed Condition	Estimated Annual Crash Costs (USD)	Percentage Change
Base	Base	none	\$14,326	NA
End Treatment	Blunt-End	Turned-Down	\$11,400	-20.4%
Terminal Flare	No Flare	1:25	\$13,984	-2.4%
Culvert Length ₁	30 ft (9.1 m)	10 ft (3.0 m)	\$13,631	-4.9%
	30 ft (9.1 m)	50 ft (15.2 m)	\$14,981	+4.6%
Culvert Drop Height	13 ft (4.0 m)	7 ft (2.1 m)	\$14,258	-0.5%
	13 ft (4.0 m)	26 ft (7.9 m)	\$14,362	+0.2%
Guardrail Face Lateral Offset	4 ft (1.2 m)	2 ft (0.6 m)	\$16,041	+12.0%
	4 ft (1.2 m)	7 ft (2.1 m)	\$11,865	-17.2%
Guardrail Length of Need	221 ft (67.4 m)	190 ft (57.9 m)	\$15,254	+6.5%
	221 ft (67.4 m)	250 ft (76.2 m)	\$14,709	-2.7%

1 – Culvert Length is the opening length measured parallel with the travelway.

8.2.1 End Terminal Modeling

Blunt-end and turned-down terminals were included in the RSAP model for the existing guardrails. Although blunt-end terminals made up over 90 percent of the systems found in the field investigation, turned-down terminals were also considered to be an important feature for analysis with RSAP based on the sensitivity analysis. Both, turned-down and blunt-end terminals were predefined features in RSAP.

8.2.2 Guardrail Lateral Offset Modeling

The lateral offsets of the W-beam guardrail found in the field investigation varied from 2 ft (0.6 m) to 12 ft (3.7 m), as measured from edge of traveled way to face of the guardrail system. Of the 42 W-beam lateral offsets that were documented, 36 ranged between 2 ft (0.6 m) and 7 ft (2.1 m). After the RSAP sensitivity analysis, 2-ft (0.6-m), 4-ft (1.2-m) and 7-ft (2.1-m) lateral offsets were chosen for the guardrails which shielded culverts. All guardrail parameters that were varied in the RSAP analysis are summarized in Table 20.

Table 20. RSAP Parameters for W-Beam Guardrail Shielding Culvert Obstacles

Guardrail Height		Lateral Offset from Travelway		Tangent End Terminal	
(in.)	(mm)	(ft)	(m)		
22	559	2	0.6	Spoon	Turned-Down
25	635	4	1.2		
27	686	7	2.1		

8.2.1 Changes to Predefined W-Beam Feature in RSAP

8.2.1.1 Severity of Guardrail

Most minor impacts with guardrail go unreported to state agencies. As presented in NCHRP No. 665, RSAP default accident severities are too high due to 26% of guardrail impacts being unreported [61]. In order to resolve this issue, NCHRP No. 665 developed an adjustment factor on guardrail impacts of 1.0 to 0.7.

8.2.1.2 Repair Cost for TL-3 Guardrail System

In RSAP (Version 2003.04.01) [10], there is a predefined repair cost for all barrier types. An error exists in the guardrail input file (si7.dat) where the repair costs for the TL-3 guardrail

system appeared to be off by an order of 10. This value was adjusted to eliminate this problem. Guardrail repair costs were found to have little influence on the total cost.

8.3 Culvert Modeling

Although this research primarily focused on the guardrail system, an accurate representation of the culvert system was also included in order to determine when a guardrail system should be upgraded. Culvert geometries were determined based on information obtained from the field investigation and the RSAP sensitivity analysis. To efficiently and accurately model culvert obstacles in RSAP, the sizes and shapes of the culverts were matched to predefined features in RSAP.

The selected predefined intersecting slope drop-offs in RSAP were 7, 13, and 26 ft (2.1, 4.0, and 7.9 m) deep. Although a drop height less than 26 ft (7.9 m) would give a better representation of existing culverts found in the field investigation, it would have required interpolation between the predefined heights to generate representative impact severities. Since the actual severities of these drop heights are not specified in RSAP, the predefined heights provided in the RSAP module were utilized. After a review of the dimensions observed in the field investigation and completion of a sensitivity analysis, three culvert lengths, three lateral offsets, and three culvert drop heights were chosen for the RSAP analysis. A summary of the culvert modeling values is given in Table 21.

8.4 Fill Slope Details

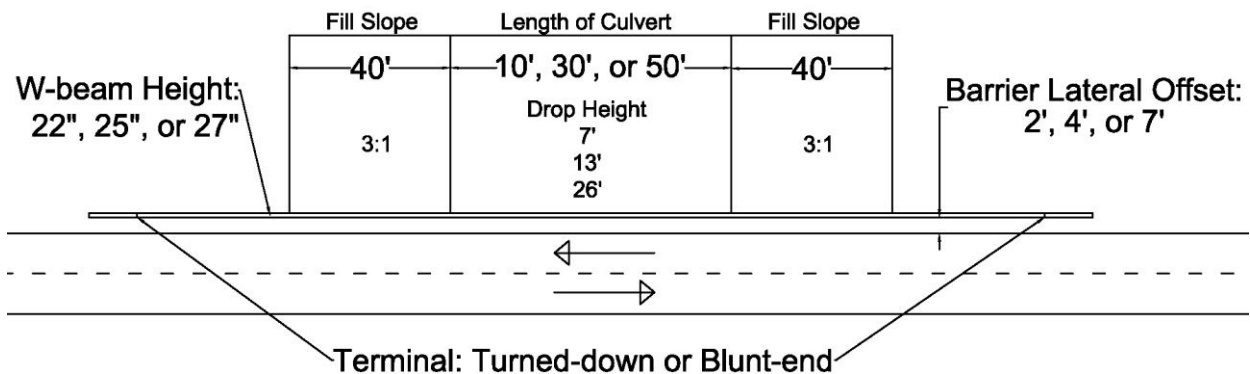
Fill Slopes are often associated with culvert structures and can present risks to motorists, such as vehicle rollover. In the field investigation, fill slopes near culverts were no steeper than 2:1, but most of these fill slopes were flatter than 3:1. For these reasons, only a fill slope of 3:1 was modeled in RSAP. The fill slopes were placed on both sides of the culvert opening. The

Table 21. Culvert Parameters Evaluated in RSAP

Culvert Length		Drop Heights		Culvert Lateral Offset	
(ft)	(m)	(ft)	(m)	(ft)	(m)
10	3.0	7	2.1	3	0.9
30	9.1	13	4.0	5	1.5
50	15.2	26	7.9	8	2.4

widths of the fill slopes were set to the same 40 ft (12.2 m), because it was found that changes did not greatly influence the results and thus allowed for a more simplified RSAP model. A sketch of the existing W-beam guardrail system for shielding culvert openings and that modeled in RSAP is shown in Figure 31.

Figure 31. RSAP Parameter Model of Existing W-Beam Guardrail for Shielding Culvert Openings



8.5 Safety Treatment Options

The safety treatment options only included removal and/or upgrades to the existing guardrail system without changes to the culvert and nearby sloped terrain. Thus, roadside grading, culvert extensions, and/or culvert grates were not considered in the RSAP analysis.

Three treatment options that were considered are: (1) do nothing; (2) remove the existing

guardrail system; and (3) remove existing guardrail system and install an approved guardrail system. These treatment options are discussed in greater detail in the following sections.

8.5.1 Do Nothing

The first safety treatment option was the “do nothing” option to the existing W-beam guardrail system. For this option, the existing guardrail system would remain in place, despite any deviations from current practice. Thus, the existing guardrail system would remain if deemed suitable for shielding the obstacle or if the cost associated with its removal and replacement exceeded the benefit, or a reduction in accident costs.

8.5.2 Remove Existing Guardrail System Only

The second safety treatment option was to remove the existing guardrail and end terminal systems. If the culvert drop-off has a large lateral offset away from the roadway edge and has a low drop height, an exposed culvert opening may be an acceptable alternative. As stated previously, protective guardrail systems should only be installed when crashes into the guardrail system are less severe than crashes into the roadside obstacle. However, many existing guardrail systems were very old and believed to pose greater risk than that provided by the obstacles themselves. For these scenarios, system removal was recommended.

The removal of existing W-beam guardrail was estimated to cost \$5.00 per linear foot (\$16.40 per linear meter) [53]. Additional costs exist for traffic control as well as material and construction team mobilization. Thus, a contingency cost was used to cover all extra costs that were also considered for the removal of the existing W-beam guardrail. These supplementary costs of 10 percent, 7.5 percent, and 15 percent, respectively, were added to the final cost of the guardrail system removal. Guardrail modeling details, costs, and sample calculations for removal of existing W-beams shielding culverts are shown in Appendix C. These costs only considered

the removal of existing W-beam guardrail with steel or wood posts. There should be extra consideration when concrete posts exist, which would increase removal costs.

Delineation of the culvert obstacle is highly recommended if removal of the existing guardrail system is recommended. Delineation is a cost-effective means of reducing accident frequency. It should be noted that delineation cannot reduce the severity of vehicle run-off-the-road accidents, but it should reduce the frequency of them. Delineation has been proven to reduce the frequency of all vehicle accidents by 30 percent [62-63]. Because the benefit of delineation could not be quantified, it was not considered in the RSAP analysis. It should be noted that the existing barriers would provide some level of delineation of known objects or areas of concern near the roadway.

8.5.3 Remove Existing Guardrail System and Install Crashworthy W-Beam Guardrail

The third safety treatment option was to remove the existing guardrail and end terminal systems, which deviate from current practice, and replace them with crashworthy W-beam guardrail and end treatment systems that meet current impact safety guidelines. This alternative was implemented when a guardrail system, including guardrail end terminals, was needed to shield a culvert opening. The new guardrail and end terminal systems were modeled with the same width, length, and lateral offset as the existing guardrail systems, with the only differences being the 31-in. (787-mm) top-rail height and two crashworthy end terminals. The containment index of 196,000 ft-lbf (266,000 J) for a 31-in. (787-mm) tall guardrail was incorporated in RSAP, as described in Chapter 7.

Two different W-beam guardrail systems were considered for replacing the existing guardrail system on the culverts. The first system was an unsupported, W-beam guardrail system

known as the MGS Long Span [40, 64]. The MGS Long Span is a W-beam guardrail system used for the protection of low-fill culverts. This system utilizes a long unsupported span which allows the low-fill culverts to be free from guardrail attachments. The second option was install a W-beam guardrail in front of the culvert headwall. This option would be available if the culvert headwall extended far enough from the roadway for a typical W-beam guardrail to be installed.

Two generic TL-3 crashworthy terminals were modeled for consideration of the replacement guardrail terminals. The length of the terminal was assumed to be 37.5 ft (11.4 m). The terminal length modeled in RSAP was 12.5 ft (3.8 m) because beyond this point (i.e., post no. 3), the terminal can redirect errant vehicles and contribute to the system's length of need.

The cost to install a TL-3 W-beam guardrail system was assumed to be \$18.16 per linear foot (\$59.58 per linear meter) [53]. This cost was multiplied by the total length of rail minus two 37.5-ft (11.4-m) generic TL-3 terminal segments. The cost to install a generic terminal was estimated to be to be \$2,100 for the 37.5 ft (11.4 m) guardrail length. The cost to remove the existing guardrail system must also be under consideration for this alternative. The traffic control, transportation, and contingency costs are the same as for the removal of the guardrail system with 10, 7.5, and 15 percent of the total cost, respectively. Guardrail modeling details, costs, and sample calculations for replacing existing W-beams shielding culverts are shown in Appendix C.

8.6 RSAP Simulation Results and Guidelines for W-Beam Guardrail Shielding Culverts

There were 4,860 scenarios simulated that were for existing W-beam guardrail systems that were used to shield culvert obstacles. The complete RSAP B/C tables for the recommendations of existing W-beam guardrail systems shielding culverts are shown in Appendix D. As expected, for most of the 22-in. (559-m) top-rail height systems, replacement

was recommended, but for 27-in. (686-mm) top-rail height systems, replacement was less frequently recommended. Existing guardrail systems utilizing turned-down terminals were less likely to be replaced than those with blunt-end terminals. W-beam guardrail with a 22-in. (559-mm) mounting height and ADT higher than 500 vpd called for guardrail systems to be replaced in most cases. When the ADT is lower than 1,000, 25-in. and 27-in. (635-mm and 686-mm) tall W-beam guardrail systems were not recommended for replacement in most instances. Existing W-beam guardrail systems found on curves were recommended to be removed or replaced in most cases due to the greater amount of impacts caused by the horizontal curvature of the roadway. Guidelines were determined from the benefit-to-cost analyses of the W-beam guardrail shielding culverts and are shown in Tables 22 through 27.

Table 22. Guidelines for 22-in. (559-mm) Tall W-Beam Guardrail Shielding Culverts (B/C 2:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Culvert Length (ft)	Drop Height (ft)	ADT Recommendation		
					Do Nothing	Remove Existing System	Remove Existing Guardrail and Install Crashworthy Barrier
Blunt-End	Straight	≥2	≥10	≥7			≤25,000
	5 degree	≥2	≥10	≥7			≤25,000
Turned-Down	Straight	2-3.9	≥10	≥7			≤25,000
	Straight	4-6.9	10-49.9	7-25.9	≤500		501-25,000
	Straight	≥4	≥50	≥26			≤25,000
	Straight	≥7	≥10	≥7	≤500		501-25,000
	5 degree	≥2	≥10	≥7			≤25,000

Table 23. Guidelines for 22-in. (559-mm) Tall W-Beam Guardrail Shielding Culverts (B/C 4:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Culvert Length (ft)	Drop Height (ft)	ADT Recommendation		
					Do Nothing	Remove Existing System	Remove Existing Guardrail and Install Crashworthy Barrier
Blunt-End	Straight	2-6.9	≥10	≥7	≤500		501-25,000
		≥7	10-49.9	≥7	≤500		501-25,000
		≥7	≥50	≤7	≤1,000		1,001-25,000
		≥7	≥50	>7	≤500		501-25,000
	5 Degree	≤2	≤10	≥7		≤500	501-25,000
		≤2	>10	≥7			≤25,000
>2		≤10	≥7			≤25,000	
Turned-Down	Straight	≤2	10-49.9	≥7	≤1,000		1,001-25,000
		≤2	≥50	≤7	≤1,000		1,001-25,000
		≤2	≥50	>7	≤500		501-25,000
		2-3.9	10-49.9	≥7	≤1,000		1,001-25,000
		2-3.9	≥50	≤7	≤1,000		1,001-25,000
		2-3.9	≥50	>7	≤500		501-25,000
		≥4	≥10	≥7	≤1,000		1,001-25,000
	5 Degree	≥2	≥10	≥7			≤25,000

Table 24. Guidelines for 25-in. (635-mm) Tall W-Beam Guardrail Shielding Culverts (B/C 2:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Culvert Length (ft)	Drop Height (ft)	ADT Recommendation		
					Do Nothing	Remove Existing System	Remove Existing Guardrail and Install Crashworthy Barrier
Blunt-End	Straight	≥2	≥10	≥7	≤500		501-25,000
	5 Degree	≥2	≥10	≥7			≤25,000
Turned-Down	Straight	2-3.9	≥10	≥7	≤500		501-25,000
		≥4	≥10	≥7	≤1,000		1,001-25,000
	5 Degree	2-6.9	≥10	≥7			≤25,000
		≥7	≤10	≤7	≤500		501-25,000
		≥7	>10	>7			≤25,000

Table 25. Guidelines for 25-in. (635-mm) Tall W-Beam Guardrail Shielding Culverts (B/C 4:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Culvert Length (ft)	Drop Height (ft)	ADT Recommendation		
					Do Nothing	Remove Existing System	Remove Existing Guardrail and Install Crashworthy Barrier
Blunt-End	Straight	≥2	≥10	≥7	≤500		501-25,000
	5 Degree	≥2	≥10	≥7	≤500		501-25,000
Turned-Down	Straight	≥2	≥10	≥7	≤1,000		1,001-25,000
	5 Degree	2-6.9	≥10	≥7	≤500		501-25,000
		≥7	≥10	≥7	≤1,000		1,001-25,000

Table 26. Guidelines for 27-in. (686-mm) Tall W-Beam Guardrail Shielding Culverts (B/C 2:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Culvert Length (ft)	Drop Height (ft)	ADT Recommendation		
					Do Nothing	Remove Existing System	Remove Existing Guardrail and Install Crashworthy Barrier
Blunt-End	Straight	≥2	≥10	≥7	≤500		501-25,000
	5 Degree	≥2	≥10	≥7			≤25,000
Turn-Down	Straight	≥2	≥10	≥7	≤1,000		1,001-25,000
	5 Degree	≥2	≥10	≥7	≤500		501-25,000

Table 27. Guidelines for 27-in. (686-mm) Tall W-Beam Guardrail Shielding Culverts (B/C 4:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Culvert Length (ft)	Drop Height (ft)	ADT Recommendation		
					Do Nothing	Remove Existing System	Remove Existing Guardrail and Install Crashworthy Barrier
Blunt-End	Straight	≥2	≥10	≥7	≤1,000		1,001-25,000
	5 Degree	2-3.9	≥10	≥7	≤500		501-25,000
		4-6.9	≤10	≥7	≤1,000		1,001-25,000
		4-6.9	≥10	≥7	≤500		501-25,000
		≥7	≥10	≥7	≤1,000		1,001-25,000
Turn-Down	Straight	≥2	≥10	≥7	≤25,000		
	5 Degree	≥2	≥10	≥7	≤1,000		1,001-25,000

8.7 Discussion

While W-beam guardrail was the most commonly found guardrail system in the field investigation, culverts were the most represented roadside obstacle shielded by these existing W-beam guardrail systems. The documented culverts had drop heights over 14 ft (4.3 m) and were over 50 ft (15.2 m) in length. Culverts are used to move water perpendicularly under the roadway and mitigate erosion. To keep expenses low, culvert structures are constructed with the headwall close to the roadway edge. This configuration often generates a low lateral offset for the guardrail system shielding these culverts. If the guardrail system isn't properly designed, installed, and maintained, it could create a severe obstacle close to the roadway. For these reasons, existing guardrail systems with known deviations from current practice also may create an unsafe condition.

Some of the culverts found in the field investigation were shielded with W-beam guardrail which utilized concrete posts that were attached to the top of the concrete headwall. The concrete post and rail systems were essentially rigid and would likely be fixed objects with increased risk to motorists when positioned at small lateral offsets away from the roadway edge. As noted previously, MwRSF researchers examined W-beam systems with concrete posts attached to rural culvert structures in a report titled, *Cost-Effective Safety Treatments for Low-Volume Roads* [53]. From this study, it was recommended all concrete posts be removed on roadways with ADTs in excess of 50 vpd. Note that, the traffic volumes modeled for this project were always greater than or equal to 500 vpd. Thus, it is recommended deficient W-beam guardrail systems with concrete posts found on culverts be removed and analyzed as an unprotected culvert opening. With this in mind, guardrail improvement recommendations will follow very closely to a culvert without an existing guardrail system, and the engineer can refer

to the RDG to determine the best practice on whether to keep the obstacle unshielded or to install a guardrail system which meets current design and safety guidelines. Again, it is recommended that at the very least, the concrete post system should be removed on these highway types. For these reasons, culvert rails with concrete posts were not considered in the final RSAP testing matrix.

Delineation should be considered in addition to all treatment options, especially if the existing guardrail system was removed and not replaced. Delineation can aid in reducing the frequency of run-off-road accidents but does not reduce accident severity unless an alerted driver slows down before an impacting event. It should be noted that the existing barriers would provide some level of delineation of known objects or areas of concern near the roadway. It should be noted that the existing barriers would provide some level of delineation of known objects or areas of concern near the roadway.

8.8 Limitations of Culvert Model

This research was limited due to the fact that it was not feasible or able to model and analyze all existing guardrail systems and deviations from current practice. This recommendation only included existing strong-post, W-beam guardrail systems. Cable, flat-panel, and concrete rails were not included in this analysis of existing guardrail systems shielding culverts. These systems would be difficult to accurately model in RSAP.

The W-beam guardrail systems in the analysis only included those with steel and wood posts. Concrete posts were not included in the analysis. Concrete posts on top of culverts would require extra removal equipment beyond that needed for steel and wood posts, which would add to the total cost to transport and time to remove.

Guardrail height and outdated terminals were the only deviations from current practice that were included in the RSAP analysis. Although these deviations were the most prominent and most severe, there were many other conditions that were documented during the field investigation which were not evaluated in this study. These deviations include rail damage, damaged and missing posts and blockouts, and insufficient length of need.

The only functional class modeled in RSAP was rural arterial highways. However, other functional classes were documented but not evaluated.

The RSAP analysis recommendations were based on costs at the time of the research study. Injury, fatality, installation, material, and other costs will continue to increase over time. If one cost increases faster than others, it may change the results of the B/C analysis. If material and installation costs increase with injury and fatality costs remaining constant, it may become less beneficial to install a new guardrail system.

There are two common culvert treatments that were not evaluated in this study: (1) culvert grate installation or (2) extending the headwall. Culvert grates can be installed on typical culvert sizes and have been found to be passably traversable by errant vehicles [65]. Extending the culvert to a farther offset, such as outside the clear zone, is another treatment option. This alternative would require that fill material be easily obtainable to remain economically viable. This study focused on upgrading existing guardrail systems. Thus, these two alternatives were not considered for this project, although they may be the best treatment options.

Culverts are either found on flat ground or on a sag section of the roadway where the water can flow through a valley. Vertical sag curves on the roadway may increase the severity for all roadside features located nearby due to the increased speed caused by the downward acceleration of a vehicle. Sag segments were not considered in the RSAP analysis. Thus,

conservative recommendations were made when treating an existing guardrail system for shielding a culvert in a sag segment.

The guardrail system lateral offsets were modeled as 2 ft, 4 ft, and 7 ft (0.6 m, 1.2 m, and 2.1 m). Although these offsets considered most of the systems found in the field investigation, there were also offsets found outside of this range. Systems with lateral offsets greater than 7 ft (2.1 m) were found in many instances, which increased to 12 ft (3.7 m). These systems may provide different results but were not included in this analysis.

9 ANALYSIS OF EXISTING W-BEAM GUARDRAILS SHIELDING ROADSIDE SLOPES

9.1 Introduction

The existing W-beam guardrail systems that were documented in the field investigation were also found to shield various roadside slopes. Most of these roadside slopes were considered to be foreslopes or fill slopes. Once again, existing W-beam guardrail systems deviated from typical practice due to low rail heights and the use of blunt-end terminals. Therefore, it was necessary to determine the cost-effectiveness of treatments based on the existing W-beam guardrails that were used to shield foreslopes. As previously noted, the existing W-beam guardrails utilized either wood or steel posts.

9.2 Investigation of Existing Guardrail Systems Shielding Slopes

The existing W-beam guardrail system and roadside slope were modeled in RSAP with a wide range of design parameters, as depicted in Table 28. First, a sensitivity analysis was performed in RSAP to determine if various parameters had a substantial effect on the accident cost. This process was completed by setting all roadway, roadside slope, and guardrail system variables constant in RSAP to represent the base condition. A rural, arterial, two-lane, undivided highway, an ADT of 5,000 vpd, and a straight roadway segment were used to represent the roadway conditions for the sensitivity analysis. The baseline conditions for the sensitivity analysis are shown in Table 29. Then, one guardrail or roadside slope parameter was changed to investigate if and how it affected the results. Several variables were subjected to a sensitivity analysis and were based on the project team's discussion and engineering judgment. These design parameters and results are shown in Table 30. If the feature parameters had little difference to the baseline condition, then only a few or one value was used for that variable in

the final RSAP simulation matrix. The final W-beam constraints were determined based on the field investigation, sensitivity analysis, and engineering judgment.

Table 28. RSAP Variables Considered for W-Beam Guardrail Shielding Slopes

Feature	Design Parameters
Roadway	ADT, Lane Width, Number of Lanes, Highway Type, Speed Limit
Guardrail System	Length of Need, Guardrail Height, Terminal Type, Lateral Offset
Slope	Slope Rate, Drop Height, Width, Length, Lateral Offset

Table 29. Sensitivity Analysis for W-Beam Guardrail Shielding Slopes– Baseline Conditions

Feature	Design Parameter	Base Condition
Highway	Area Type	Rural
	Functional Class	Minor Arterial
	Highway Type	Two-Way, Undivided
	Number of Lanes	2
	Lane Width	12 ft (3.7 m)
	Speed Limit	55 mph (88.5 km/h)
	ADT	5,000
	Percent Trucks	10
	Traffic Growth Factor	0
	Encroachment Rate Adjustment Factor	1
Guardrail System	System Type	TL-1 Guardrail
	Terminal Type	Blunt End
	Lateral Offset	3 ft (0.9 m)
	Length of Need	221 ft (67.4 m)
Roadside Slope	Slope Rate	2 to 1
	Drop Height	13 ft (4.0 m)
	Length	350 ft (106.7 m)
	Lateral Offset	4 ft (1.2 m)
Transition Slope	Slope Rate	4 to 1
	Drop Height	7 ft (2.0 m)
	Length	40 ft (12.2 m)
	Lateral Offset	4 ft (1.2 m)

Table 30. Sensitivity Analysis for W-Beam Guardrail Shielding Slopes – Results

Design Parameter	Base	Change	Estimated Annual Crash Costs (USD)	Percentage Change
Base	Base	NA	\$14,958	NA
End Treatment	Blunt-End	Turned-Down	\$11,497	-23.1%
Terminal Flare	No Flare	1:25	\$14,489	-3.1%
Slope Drop Height	13 ft (4.0 m)	7 ft (2.1 m)	\$13,585	-9.2%
	13 ft (4.0 m)	20 ft (6.1 m)	\$15,398	+2.9%
Slope Length	350 ft (106.7 m)	150 ft (45.7 m)	\$12,723	-14.9%
	350 ft (106.7 m)	650 ft (198.1 m)	\$18,556	+24.1%
Lateral Guardrail system Offset	4 ft (1.2 m)	2 ft (0.6 m)	\$16,735	+11.9%
	4 ft (1.2 m)	7 ft (2.1 m)	\$12,338	-17.5%
Guardrail Length of Need	221 ft (67.4 m)	190 ft (57.9 m)	\$14,519	-2.9%
	221 ft (67.4 m)	250 (76.2 m)	\$14,843	-0.8%

Existing W-beam guardrail systems were modeled by finding a set of parameters which best reflected what was found in the field investigation. Critical parameters for modeling existing W-beam guardrail systems were length of need, rail height, terminal type, and lateral offset. The W-beam guardrail which shielded slopes had the same parameters that were determined for culverts, as noted in Section 8.2. Length of need, guardrail height, lateral guardrail system offset, and terminal type were all modeled with the same values as used for culverts and are shown in Table 31.

Table 31. RSAP Parameters for W-Beam Guardrail Shielding Roadside Slope

Guardrail Height		Lateral Offset from Travelway		Tangent End Terminal	
(in.)	(mm)	(ft)	(m)		
22	559	2	0.6	Spoon	Turned-Down
25	635	4	1.2		
27	686	7	2.1		

9.3 Slope Modeling

Although this research primarily focused on the guardrail system, an accurate modeling of the roadside slope was also included in order to depict the nature of the roadside terrain for which an existing guardrail system was shielding. Slope geometries were determined based on information obtained from the field investigation, an RSAP sensitivity analysis, and a team discussion. To efficiently and accurately model the slopes in RSAP, the slope geometries were matched to predefined foreslopes in RSAP.

In RSAP, the Severity Index (SI) of the slopes was based on a survey of highway safety officials to rank the severity of accidents on a scale of 1 to 10. The predefined SI values for foreslopes in RSAP are believed to have a bias toward high-speed impacts [10]. As a result, the SI values were overestimated. A previous study by MwRSF developed new SI values for slopes based on actual accident data [55-56]. These values were implemented in the RSAP runs for this study.

Slopes were modeled using the dimensions observed in the field investigation, sensitivity analysis, and group discussion. Ultimately, three slope rates, three slope lengths, three slope drop heights, and three lateral offsets were chosen for the RSAP analysis. A summary of the slope modeling values is shown in Table 32.

Table 32. Slope Parameters Evaluated in RSAP

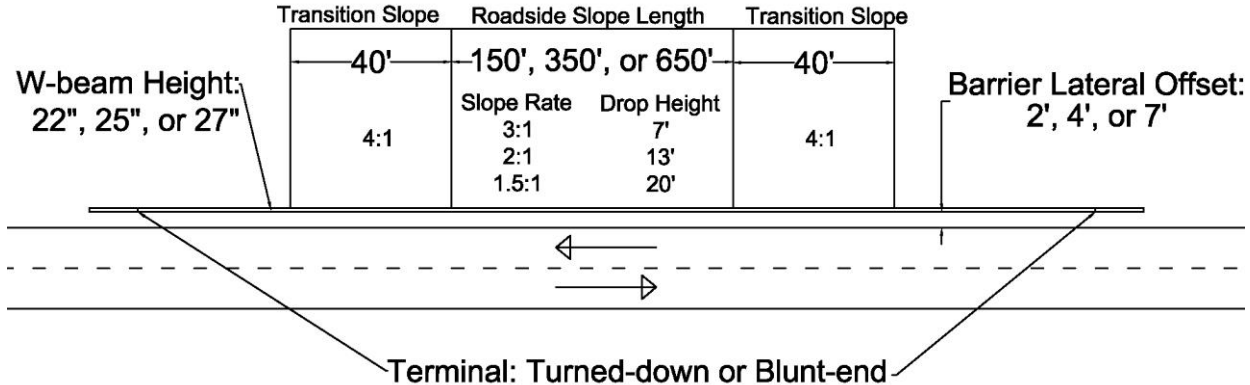
Slope Rate	Length		Drop Height		SBP Lateral Offset	
	(ft)	(m)	(ft)	(m)	(ft)	(m)
3:1	150	45.7	7	2.1	3	0.9
2:1	350	106.7	13	4.0	5	1.5
1.5:1	650	198.1	20	6.1	8	2.4

SBP – Slope Break Point

9.4 Transition Slope Modeling

A transition slope was considered to provide a better model of existing slopes observed in the field investigation. This transition slope was modeled as a recoverable foreslope which was on the upstream and downstream ends of the primary roadside slope in order to a transition from a non-recoverable slope rate to flat ground. A 4:1 slope transition spanning 40 ft (12.2 m) on each end of the primary roadside slope was considered for the RSAP analysis. A sketch of the existing W-beam guardrail system for shielding slopes and that modeled in RSAP is shown in Figure 32.

Figure 32. RSAP Parameter Model of Existing W-Beam Guardrail for Shielding Roadside Slopes



9.5 Safety Treatment Options

The safety treatment options only included removal and/or upgrades to the existing guardrail system without changes to the existing slope. Thus, roadside grading was not considered in the analysis. If slope grading is found to be an applicable treatment option, MwRSF’s prior roadside grading guidance [55-56] should be followed for specific roadside

conditions. Three treatment options were considered: (1) do nothing; (2) remove the existing guardrail system; and (3) remove existing guardrail system and install an approved guardrail system. These treatment options are discussed in greater detail in the following sections.

9.5.1 Do Nothing

The first safety treatment was the “do nothing” option to the existing W-beam guardrail system. For this option, the existing guardrail system would remain in place, despite any deviations from current practice. Thus, the existing guardrail system would remain if deemed suitable for shielding the obstacle or if the cost associated with its removal and replacement exceeded the benefit, or a reduction in accident costs.

9.5.2 Remove Existing Guardrail System Only

The second safety treatment option was to remove the existing guardrail system. As stated previously, most existing guardrail systems shielding slopes had low rail heights and blunt-end terminals, and in most cases will pose a more severe obstacle than the slope it is shielding. It is in these scenarios that this treatment option may be chosen.

The removal of existing W-beam guardrail was estimated to cost \$5.00 per linear foot (\$16.40 per linear meter) [53]. Additional costs exist for traffic control as well as material and construction team mobilization. Thus, a contingency cost which was used to cover all extra costs that were considered for the removal of the existing W-beam guardrail. These supplementary costs of 10 percent, 7.5 percent, and 15 percent, respectively, were added to the final cost of the guardrail system removal. Guardrail modeling details, costs, and sample calculations for removal of existing W-beams shielding roadside slopes are shown in Appendix E.

Delineation should be considered if removal of the existing guardrail system is recommended. Delineation is a cost-effective means of reducing accident frequency. It should be

noted that delineation cannot reduce the severity of vehicle run-off-the-road accidents, but it should reduce the frequency of them. Delineation has been proven to reduce the frequency of all vehicle accidents by 30 percent [62-63]. Because the benefit of delineation could not be quantified, it was not considered in the RSAP analysis. It should be noted that if the roadside slope is excessive in length, the use of delineation may become less cost-effective. Delineation should be considered for short, untreated slopes on roadways with horizontal or vertical curves. It should be noted that the existing barriers would provide some level of delineation of known objects or areas of concern near the roadway.

9.5.3 Remove Existing Guardrail System and Install Crashworthy W-Beam Guardrail

The third safety treatment option was to remove the existing guardrail and end terminal systems, which deviate from current practice, and replace them with crashworthy W-beam guardrail and end treatment systems that meet current impact safety guidelines. This alternative would be implemented when a guardrail system, including guardrail end terminals, is needed to shield a critical roadside slope. The new guardrail and end terminal systems were modeled with the same width and lateral offset as the existing guardrail systems, with the only differences being the 31-in. (787-mm) top-rail height and two crashworthy end terminals. The containment index of 196,000 ft-lbf (266,000 J) for a 31-in. (787-mm) tall guardrail was incorporated into RSAP, as described in Chapter 7.

Two generic TL-3 crashworthy terminals were modeled for consideration of the replacement guardrail terminals. The length of the terminal was assumed to be 37.5 ft (11.4 m). The terminal length modeled in RSAP was 12.5 ft (3.8 m) because beyond this point (i.e., post no. 3), the terminal can redirect errant vehicles and contribute to the system's length of need.

The cost to install a TL-3 W-beam guardrail system was assumed to be \$18.16 per linear foot (\$59.58 per linear meter) [53]. This cost was multiplied by the total length of rail minus two 37.5-ft (11.4-m) generic TL-3 terminal segments. The cost to install a generic terminal was estimated to be to be \$2,100 for the 37.5 ft (11.4 m) guardrail length. The cost to remove the existing guardrail system must also be under consideration for this alternative. The traffic control, transportation, and contingency costs are the same as for the removal of the guardrail system with 10, 7.5, and 15 percent of the total cost, respectively. Guardrail modeling details, costs, and sample calculations for replacing existing W-beams shielding culverts are shown in Appendix E.

9.6 RSAP Simulation Results and Guidelines for W-Beam Guardrail Shielding Slopes

There were 14,580 scenarios that were simulated for existing W-beam guardrail systems that were used to shield slopes. The complete RSAP B/C tables for the recommendations of existing W-beam guardrail systems shielding slopes are shown in Appendix F. As expected, most of the 22-in. (559-mm) top-rail height systems are recommended for removal and replacement with fewer 27-in. (686-mm) top-rail heights needing replacement. Existing guardrail systems utilizing turned-down terminals were less likely to be replaced than those with blunt-end treatments. The 25-in. and 27-in. (635-mm and 686-mm) tall W-beam guardrail systems only need replacement when the ADT is higher than 1,000 vpd in most cases. Roadside slopes that are 3:1 or flatter and configured with low drop heights were usually recommended for removal. Existing W-beam guardrail systems found on curves were recommended to be removed or replaced in most cases due to the greater amount of impacts caused by the horizontal curvature of the roadway. Guidelines were determined from the benefit-to-cost analyses of the W-beam guardrail shielding roadside slopes and are shown in Tables 33 through 38.

Table 33. Guidelines for 22-in. (559-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 2:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Blunt End	Straight	<4	≤150	<2:1	≥7			≤25,000
				2:1 - 2.9:1	≤7		≤500	501-25,000
					>7			≤25,000
			≥3:1	≥7		≤25,000		
			151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
		>350	<3:1	≥7			≤25,000	
					≤7		≤25,000	
				≥3:1	7.1-13		≤1,000	1,001-25,000
				>13		≤500	501-25,000	
			≤150	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
		151-350	<3:1	≥7			≤25,000	
			≥3:1	≥7		≤25,000		
		>350	<3:1	≥7			≤25,000	
			≥3:1	≤7		≤25,000		
			>7		≤1,000	1,001-25,000		
		≥7	≤150	<3:1	≥7			≤25,000
	≥3:1			≥7		≤25,000		
	151-350		<3:1	≥7	≤500		501-25,000	
			≥3:1	≥7		≤25,000		
	>350		<3:1	≥7	≤500		501-25,000	
			≥3:1	≤7		≤25,000		
		>7		≤1,000	1,001-25,000			
5 Degree	<4	≤150	<3:1	≥7			≤25,000	
			≥3:1	≥7		≤25,000		
		151-350	<3:1	≥7			≤25,000	
			≥3:1	≥7		≤25,000		
		>350	<3:1	≥7			≤25,000	
			≥3:1	≥7		≤25,000		

Table 33. Guidelines for 22-in. (559-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 2:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Blunt End	5 Degree	4-6.9	≤150	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
		≥7	≤150	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
Turned-Down	Straight	<4	≤150	<2:1	≤13	≤500		501-25,000
				>13			≤25,000	
			2:1 - 2.9:1	≥7	≤500		501-25,000	
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			4-6.9	≤150	<3:1	≥7	≤500	
		≥3:1			≥7		≤25,000	
		151-350		<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
		>350		<2:1	≤13	≤500		501-25,000
				>13			≤25,000	
		2:1 - 2.9:1		≥7	≤500		501-25,000	
				≥3:1	≤7		≤25,000	

Table 33. Guidelines for 22-in. (559-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 2:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Turned-Down	Straight	4-6.9	>350	≥3:1	>7		≤1,000	1,001-25,000
		≥7	≤150	<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
			151-350	<2:1	≥7	≤500		501-25,000
				≥3:1	≤13		≤25,000	
				>13		≤10,000	10,001-25,000	
			>350	<3:1	≥7	≤500		501-25,000
		≥3:1		≤7		≤25,000		
			>7		≤1,000	1,001-25,000		
		5 Degree	<4	≤150	<3:1	≥7		
	≥3:1				≥7		≤25,000	
	151-350			<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
	>350			<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
	4-6.9		≤150	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
	>350		<3:1	≥7			≤25,000	
			≥3:1	≥7		≤25,000		
	≥7		≤150	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
		151-350	<3:1	≥7			≤25,000	
≥3:1			≥7		≤25,000			
>350		<3:1	≥7			≤25,000		
		≥3:1	≤13		≤25,000			
	>13		≤500	501-25,000				

Table 34. Guidelines for 22-in. (559-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 4:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation			
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier	
Blunt End	Straight	<4	≤150	<2:1	≥7	≤500		501-25,000	
				2:1 - 2.9:1	≤7		≤1,000	1,001-25,000	
					>7	≤500		501-25,000	
			≥3:1	≥7		≤25,000			
			151-350	<3:1	≥7	≤500		501-25,000	
				≥3:1	≥7		≤25,000		
			>350	<3:1	≥7	≤500		501-25,000	
				≥3:1	≤13		≤25,000		
					>13		≤1,000	1,001-25,000	
			4-6.9	≤150	<2:1	≥7	≤500		501-25,000
					2:1 - 2.9:1	≤7	≤500	501-1,000	1,001-25,000
						>7	≤500		501-25,000
		≥3:1		≥7		≤25,000			
		151-350		<3:1	≥7	≤500		501-25,000	
				≥3:1	≥7		≤25,000		
		>350		<3:1	≥7	≤500		501-25,000	
				≥3:1	≤13		≤25,000		
					>13		≤1,000	1,001-25,000	
		≥7		≤150	<2:1	≥7	≤1,000		1,001-25,000
					2:1 - 2.9:1	≤7	≤500	501-1,000	1,001-25,000
						>7	≤1,000		1,001-25,000
			≥3:1	≥7		≤25,000			
			151-350	<3:1	≥7	≤1,000		1,001-25,000	
				≥3:1	≥7		≤25,000		
>350	<3:1		≥7	≤1,000		1,001-25,000			
	≥3:1		≤13		≤25,000				
			>13		≤1,000	1,001-25,000			
5 Degree	<4		≤150	<2:1	≤7		≤500	501-25,000	
				>7			≤25,000		

Table 34. Guidelines for 22-in. (559-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 4:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Blunt End	5 Degree	<4	≤150	2:1 - 2.9:1	≤7		≤25,000	
					7.1-13		≤500	501-25,000
					>13			≤25,000
			151-350	≥3:1	≥7		≤25,000	
					≥7		≤25,000	
					≥7		≤25,000	
		>350	<3:1	≥7		≤25,000		
				≥7		≤25,000		
				≥7		≤25,000		
		4-6.9	≤150	<2:1	≤7		≤500	501-25,000
					>7			≤25,000
				2:1 - 2.9:1	≤7		≤25,000	
					7.1-13		≤500	501-25,000
					>13			≤25,000
				≥3:1	≥7		≤25,000	
			≥7			≤25,000		
			≥7			≤25,000		
			151-350	<3:1	≥7		≤25,000	
					≥7		≤25,000	
					≥7		≤25,000	
			>350	<3:1	≥7		≤25,000	
		≥7				≤25,000		
		≥7				≤25,000		
		≥7	≤150	<2:1	≥7			≤25,000
≥7					≤25,000			
2:1 - 2.9:1	≤7				≤25,000			
	7.1-13				≤500	501-25,000		
	>13					≤25,000		
≥3:1	≥7				≤25,000			
	≥7			≤25,000				
	≥7			≤25,000				
151-350	<3:1		≥7		≤25,000			
			≥7		≤25,000			
			≥7		≤25,000			
>350	<3:1		≥7		≤25,000			
		≥7		≤25,000				
		≥7		≤25,000				
Turned-Down	Straight	<4	≤150	<3:1	≥7	≤1,000	1,001-25,000	
				≥3:1	≥7		≤25,000	

Table 34. Guidelines for 22-in. (559-mm) Tall W-Beam Guardrail Shielding Roadside Slopes
(B/C 4:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Turned-Down	Straight	<4	151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			>350	<2:1	≥7	≤500		501-25,000
				2:1 - 2.9:1	≤7	≤1,000		1,001-25,000
					>7	≤500		501-25,000
				≥3:1	≤13		≤25,000	
			>13		≤1,000	1,001-25,000		
		4-6.9	≤150	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
		≥7	≤150	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7	≤1,000		1,001-25,000
	≥3:1			≥7		≤25,000		
	5 Degree	<4	≤150	<2:1	≤7		≤500	501-25,000
				>7			≤25,000	
			2:1 - 2.9:1	≥7		≤500	501-25,000	
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7		≤25,000	
≥3:1				≥7		≤25,000		
>350		<3:1	≥7		≤25,000			
		≥3:1	≥7		≤25,000			
4-6.9		≤150	<2:1	≤7		≤500	501-25,000	

Table 34. Guidelines for 22-in. (559-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 4:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Turned-Down	5 Degree	4-6.9	≤150	<2:1	>7			≤25,000
				2:1 - 2.9:1	≥7		≤500	501-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
				<3:1	≥7			≤25,000
		>350	≥3:1	≥7		≤25,000		
			<2:1	≥7			≤25,000	
		≥7	≤150	2:1 - 2.9:1	≤13		≤500	501-25,000
					>13			≤25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7			≤25,000
≥3:1	≥7				≤25,000			

Table 35. Guidelines for 25-in. (635-mm) Tall W-Beam Guardrail Shielding Roadside Slopes
(B/C 2:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Blunt End	Straight	<4	≤150	<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7	≤500		501-25,000
				≥3:1	≤7		≤25,000	
			>7		≤1,000	1,001-25,000		
		4-6.9	≤150	<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤500		501-25,000
				≥3:1	≤7		≤25,000	
				>7		≤1,000	1,001-25,000	
	>350		<3:1	≥7	≤500		501-25,000	
		≥3:1	≥7		≤25,000			
	≥7	≤150	<3:1	≥7	≤500		501-25,000	
			≥3:1	≥7		≤25,000		
		151-350	<3:1	≥7	≤500		501-25,000	
			≥3:1	≤13		≤25,000		
			>13		≤10,000	10,001-25,000		
		>350	<3:1	≥7	≤500		501-25,000	
	≥3:1		≤7		≤25,000			
		>7		≤1,000	1,001-25,000			
	5 Degree	<4	≤150	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
151-350			<3:1	≥7			≤25,000	
			≥3:1	≥7		≤25,000		
>350		<3:1	≥7			≤25,000		
		≥3:1	≥7		≤25,000			
4-6.9		≤150	<3:1	≥7			≤25,000	
			≥3:1	≥7		≤25,000		

Table 35. Guidelines for 25-in. (635-mm) Tall W-Beam Guardrail Shielding Roadside Slopes
(B/C 2:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Blunt End	5 Degree	4-6.9	151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
		≥7	≤150	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7			≤25,000
				≥3:1	≤13		≤25,000	
					>13		≤500	501-25,000
				Turned-Down	Straight	<4	≤150	<3:1
≥3:1	≥7		≤25,000					
151-350	<2:1	≤7	≤1,000					1,001-25,000
		>7	≤500					501-25,000
	2:1 - 2.9:1	≥7	≤1,000					1,001-25,000
		≥3:1	≥7					≤25,000
>350	<2:1	≤7	≤1,000					1,001-25,000
		>7	≤500					501-25,000
	2:1 - 2.9:1	≥7	≤1,000					1,001-25,000
		≥3:1	≤7					≤25,000
	7.1-13						≤10,000	10,001-25,000
	>13	501-1,000	≤500				1,001-25,000	
4-6.9	≤150	<3:1	≥7	≤1,000		1,001-25,000		
		≥3:1	≥7		≤25,000			
	151-350	<2:1	≤7	≤1,000		1,001-25,000		
			>7	≤500		501-25,000		

Table 35. Guidelines for 25-in. (635-mm) Tall W-Beam Guardrail Shielding Roadside Slopes
(Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation			
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier	
Turned-Down	Straight	4-6.9	151-350	2:1 - 2.9:1	≥7	≤1,000		1,001-25,000	
				≥3:1	≥7		≤25,000		
			>350	<2:1	≤7	≤1,000		1,001-25,000	
					>7	≤500		501-25,000	
				2:1 - 2.9:1	≥7	≤1,000		1,001-25,000	
				≥3:1	≤7		≤25,000		
					7.1-13		≤1,000	1,001-25,000	
					>13	501-1,000	≤500	1,001-25,000	
			≥7	≤150	<3:1	≥7	≤1,000		1,001-25,000
					≥3:1	≥7		≤25,000	
				151-350	<2:1	≤7	≤1,000		1,001-25,000
						>7	≤500		501-25,000
		2:1 - 2.9:1			≥7	≤1,000		1,001-25,000	
		≥3:1			≤13		≤25,000		
			>13		≤1,000	1,001-25,000			
		>350	<2:1	≤7	≤1,000		1,001-25,000		
				>7	≤500		501-25,000		
			2:1 - 2.9:1	≥7	≤1,000		1,001-25,000		
			≥3:1	≤7		≤25,000			
				7.1-13		≤1,000	1,001-25,000		
				>13	501-1,000	≤500	1,001-25,000		
		<4	≤150	<3:1	≥7			≤25,000	
				≥3:1	≥7		≤25,000		

Table 35. Guidelines for 25-in. (635-mm) Tall W-Beam Guardrail Shielding Roadside Slopes
(B/C 2:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Turned-Down	5 Degree	<4	151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
		4-6.9	≤150	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
		≥7	≤150	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7			≤25,000
				≥3:1	≥7		≤25,000	

Table 36. Guidelines for 25-in. (635-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 4:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Blunt End	Straight	<4	≤150	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
		7.1-13			≤500	501-10,000	10,001-25,000	
		>13			≤500	501-1,000	1,001-25,000	
		4-6.9	≤150	<2:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
		7.1-13			≤500	501-25,000		
		>13			≤500	501-1,000	1,001-25,000	
		≥7	≤150	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
			>350	>7	≤500	501-25,000		
				<3:1	≥7	≤1,000		1,001-25,000
		≥3:1	≤7		≤25,000			
			>7	≤1,000		1,001-25,000		
5 Degree	<4	≤150	<2:1	≥7	≤500		501-25,000	
			2:1 - 2.9:1	≤7	≤500	501-25,000		
				>7	≤500		501-25,000	
		≥3:1	≥7		≤25,000			
		151-350	<3:1	≥7	≤500		501-25,000	
			≥3:1	≤7		≤25,000		
>7	≤1,000				1,001-25,000			

Table 36. Guidelines for 25-in. (635-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 4:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation			
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier	
Blunt End	5 Degree	<4	>350	<3:1	≥7	≤500		501-25,000	
				≥3:1	≥7		≤25,000		
		4-6.9	≤150	<2:1	≥7	≤500		501-25,000	
				2:1 - 2.9:1	≤7	≤500	501-25,000		
					>7	≤500		501-25,000	
				≥3:1	≥7		≤25,000		
			151-350	<3:1	≥7	≤500		501-25,000	
				≥3:1	≥7		≤25,000		
		>350	<3:1	≥7	≤500		501-25,000		
			≥3:1	≥7		≤25,000			
		≥7	≤150	<2:1	≥7	≤500		501-25,000	
				2:1 - 2.9:1	≤7	≤500	501-25,000		
				>7	≤500		501-25,000		
	≥3:1			≥7		≤25,000			
	151-350		<3:1	≥7	≤500		501-25,000		
			≥3:1	≥7		≤25,000			
	>350		<3:1	≥7	≤500		501-25,000		
			≥3:1	≥7		≤25,000			
	Turned-Down	Straight	<4	≤150	<3:1	≥7	≤1,000		1,001-25,000
					≥3:1	≥7		≤25,000	
151-350				<3:1	≥7	≤1,000		1,001-25,000	
				≥3:1	≤7		≤25,000		
				>7	≤500	501-25,000			
>350				<3:1	≥7	≤1,000		1,001-25,000	
			≥3:1	≤7		≤25,000			
			>7	≤1,000		1,001-25,000			
4-6.9			≤150	<3:1	≥7	≤1,000		1,001-25,000	
				≥3:1	≤7		≤25,000		
			>7	≤500	501-25,000				

Table 36. Guidelines for 25-in. (635-mm) Tall W-Beam Guardrail Shielding Roadside Slopes
(B/C 4:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Turned-Down	Straight	4-6.9	151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
					>7	≤500	501-25,000	
			>350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
					>7	≤1,000		1,001-25,000
		≥7	≤150	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
					>7	≤500	501-25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
					>7	≤500	501-25,000	
	>350	<3:1	≥7	≤1,000		1,001-25,000		
		≥3:1	≤7		≤25,000			
			>7	≤1,000		1,001-25,000		
	5 Degree	<4	≤150	<2:1	≥7	≤500		501-25,000
				2:1 - 2.9:1	≤7		≤25,000	
					>7	≤500	501-25,000	
			≥3:1	≥7		≤25,000		
			151-350	<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
		>350		<3:1	≥7	≤500		501-25,000
		>350	≥3:1	≥7		≤25,000		
			≤150	<2:1	≥7	≤500		501-25,000
2:1 - 2.9:1				≤7	≤500	501-25,000		
		>7		≤500		501-25,000		
≥3:1		≥7		≤25,000				
151-350	<3:1	≥7	≤500		501-25,000			
	≥3:1	≥7		≤25,000				

Table 36. Guidelines for 25-in. (635-mm) Tall W-Beam Guardrail Shielding Roadside Slopes
(B/C 4:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Turned-Down	5 Degree	4-6.9	>350	<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
Turned-Down	5 Degree	≥7	≤150	<2:1	≥7	≤500		501-25,000
				2:1 - 2.9:1	≤7	≤500	501-25,000	
					>7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	

Table 37. Guidelines for 27-in. (686-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 2:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Blunt End	Straight	<4	≤150	<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤500		501-25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
		>350	≥3:1	>7	≤1,000		1,001-25,000	
			4-6.9	≤150	<3:1	≥7	≤500	
		≥3:1			≥7		≤25,000	
		151-350		<3:1	≤13	≤1,000		1,001-25,000
				>13	≤500		501-25,000	
		2:1 - 2.9:1		≥7	≤1,000		1,001-25,000	
				≥3:1	≥7		≤25,000	
		>350	<3:1	≥7	≤1,000		1,001-25,000	
			≥3:1	≤7		≤25,000		
		>350	≥3:1	>7	≤1,000		1,001-25,000	
			≥7	≤150	<3:1	≥7	≤1,000	
		≥3:1			≥7		≤25,000	
		151-350		<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
		>350		≥3:1	>7	≤500	501-25,000	
				<3:1	≥7	≤1,000		1,001-25,000
		>350	≥3:1	≤7		≤25,000		
			≥3:1	>7	≤1,000		1,001-25,000	
5 Degree	<4	≤150	<3:1	≥7			≤25,000	
			≥3:1	≥7		≤25,000		
		151-350	<3:1	≥7		≤25,000		
			≥3:1	≥7		≤25,000		
		>350	<3:1	≥7			≤25,000	
			≥3:1	≥7			≤25,000	

Table 37. Guidelines for 27-in. (686-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 2:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation			
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier	
Blunt End	5 Degree	<4	>350	>3:1	≥7		≤25,000		
				<3:1	≥7		≤25,000		
			4-6.9	≤150	≥3:1	≥7		≤25,000	
					<3:1	≥7		≤25,000	
				151-350	<3:1	≥7		≤25,000	
					≥3:1	≥7		≤25,000	
			>350	<3:1	≥7		≤25,000		
				≥3:1	≥7		≤25,000		
			≥7	≤150	<3:1	≥7	≤500		501-25,000
					≥3:1	≥7		≤25,000	
	151-350	<3:1		≥7	≤500		501-25,000		
		≥3:1		≥7		≤25,000			
	>350	<3:1	≥7	≤500		501-25,000			
		≥3:1	≤13		≤25,000				
		>13		≤500	501-25,000				
Turned-Down	Straight	<4	≤150	<3:1	≥7	≤1,000		1,001-25,000	
				≥3:1	≥7		≤25,000		
			151-350	<3:1	≥7	≤1,000		1,001-25,000	
				≥3:1	≤7		≤25,000		
					7.1-13	≤500	501-25,000		
				>13	≤1,000		1,001-25,000		
		>350	<3:1	≥7	≤1,000		1,001-25,000		
			≥3:1	≤7		≤25,000			
				>7	≤1,000		1,001-25,000		
		4-6.9	≤150	<3:1	≥7	≤1,000		1,001-25,000	
				≥3:1	≤13		≤25,000		
				>13	≤500	501-25,000			
	151-350		<3:1	≥7	≤1,000		1,001-25,000		
			≥3:1	≤7		≤25,000			
	>7		≤1,000		1,001-25,000				
	>350	<3:1	≥7	≤1,000		1,001-25,000			

Table 37. Guidelines for 27-in. (686-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 2:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Turned-Down	Straight	4-6.9	>350	≥3:1	≤7		≤25,000	
					>7	≤1,000	1,001-25,000	
		≥7	≤150	<3:1	≥7	≤1,000	1,001-25,000	
				≥3:1	≤7	≤25,000		
			151-350	<3:1	≥7	≤1,000	1,001-25,000	
				≥3:1	≤7	≤25,000		
			>350	<3:1	≥7	≤1,000	1,001-25,000	
				≥3:1	≤7	≤25,000		
		<4	≤150	<3:1	≥7	≤500	501-25,000	
				≥3:1	≥7	≤25,000		
			151-350	<3:1	≥7	≤500	501-25,000	
				≥3:1	≥7	≤25,000		
	>350		<3:1	≥7	≤500	501-25,000		
			≥3:1	≥7	≤25,000			
	5 Degree	4-6.9	≤150	<3:1	≥7	≤500	501-25,000	
				≥3:1	≥7	≤25,000		
			151-350	<3:1	≥7	≤500	501-25,000	
				≥3:1	≥7	≤25,000		
			>350	<3:1	≥7	≤500	501-25,000	
				≥3:1	≤13	≤25,000		
		>13	≤500	501-25,000				
		≥7	≤150	<3:1	≥7	≤500	501-25,000	
				≥3:1	≥7	≤25,000		
			151-350	<3:1	≥7	≤500	501-25,000	
≥3:1				≥7	≤25,000			

Table 37. Guidelines for 27-in. (686-mm) Tall W-Beam Guardrail Shielding Roadside Slopes
(B/C 2:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Turned-Down	5 Degree	≥7	>350	<3:1	≥7	≤500		501-25,000
				≥3:1	≤13		≤25,000	
					>13	≤500		501-25,000

Table 38. Guidelines for 27-in. (686-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 4:1)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Blunt End	Straight	<4	≤150	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
			≥3:1	>7	≤500	501-25,000		
				<3:1	≥7	≤1,000		1,001-25,000
		≥3:1	≤7		≤25,000			
			>7	≤1,000		1,001-25,000		
		4-6.9	≤150	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
			≥3:1	>7	≤500	501-25,000		
				<3:1	≥7	≤1,000		1,001-25,000
		≥3:1	≤7		≤25,000			
			>7	≤1,000		1,001-25,000		
		≥7	≤150	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
			≥3:1	7.1-13	≤1,000	1,001-25,000		
				>13	≤1,000	1,001-25,000		
		>350	<3:1	≥7	≤25,000			
			≥3:1	≤7		≤25,000		
	7.1-13			≤1,000	1,001-25,000			
	>13		≤10,000	10,001-25,000				
	5 Degree	<4	≤150	<2:1	≤7	≤1,000		1,001-25,000
					>7	≤500		501-25,000
2:1 - 2.9:1				≤7	≤500	501-1,000	1,001-25,000	
				>7	≤500		501-25,000	

Table 38. Guidelines for 27-in. (686-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 4:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Blunt End	5 Degree	<4	≤150	≥3:1	≥7		≤25,000	
			151-350	<2:1	≤7	≤1,000	1,001-25,000	
					>7	≤500	501-25,000	
				2:1 - 2.9:1	≤7	≤1,000	1,001-25,000	
					>7	≤500	501-25,000	
			≥3:1	≥7		≤25,000		
		>350	<3:1	≥7	≤500	501-25,000		
			≥3:1	≥7		≤25,000		
		4-6.9	≤150	<3:1	≥7	≤1,000	1,001-25,000	
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000	1,001-25,000	
				≥3:1	≥7		≤25,000	
	>350		<3:1	≥7	≤1,000	1,001-25,000		
			≥3:1	≥7		≤25,000		
	≥7	≤150	<3:1	≥7	≤1,000	1,001-25,000		
			≥3:1	≥7		≤25,000		
		151-350	<3:1	≥7	≤1,000	1,001-25,000		
			≥3:1	≥7		≤25,000		
		>350	<3:1	≥7	≤1,000	1,001-25,000		
			≥3:1	≤7		≤25,000		
		>7		≤500	501-25,000			
Straight		Turned-Down	<4	≤150	<3:1	≥7	≤25,000	
					≥3:1	≤7		≤25,000
	>7			≤500		501-25,000		
	151-350			<3:1	≥7	≤25,000		
				≥3:1	≤7		≤25,000	
					>7	≤1,000	1,001-25,000	
			>350	<3:1	≥7	≤25,000		
	≥3:1			≤7		≤25,000		
			>7	≤25,000				

Table 38. Guidelines for 27-in. (686-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 4:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Turned-Down	Straight	4-6.9	≤150	<3:1	≥7	≤25,000		
				≥3:1	≤7		≤25,000	
				>7	≤500	501-25,000		
			151-350	<3:1	≥7	≤25,000		
				≥3:1	≤7		≤25,000	
				>7	≤1,000	1,001-25,000		
		>350	<3:1	≥7	≤25,000			
			≥3:1	≤7		≤25,000		
			>7	≤25,000				
		≥7	≤150	<3:1	≥7	≤25,000		
				≥3:1	≤7		≤25,000	
					7.1-13	≤500	501-25,000	
			>13	≤1,000	1,001-25,000			
	151-350		<3:1	≥7	≤25,000			
			≥3:1	≤7		≤25,000		
			7.1-13	≤1,000	1,001-25,000			
		>13	≤10,000	10,001-25,000				
	>350	<3:1	≥7	≤25,000				
		≥3:1	≤7		≤25,000			
			>7	≤25,000				
	5 Degree	<4	≤150	<2:1	≥7	≤1,000		1,001-25,000
				2:1 - 2.9:1	≤7	≤500	501-25,000	
					>7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
151-350			<3:1	≥7	≤1,000		1,001-25,000	
			≥3:1	≥7		≤25,000		
>350		<3:1	≥7	≤1,000		1,001-25,000		
		≥3:1	≤13		≤25,000			
			>13	≤500	501-25,000			

Table 38. Guidelines for 27-in. (686-mm) Tall W-Beam Guardrail Shielding Roadside Slopes (B/C 4:1) (Continued)

End Terminal Type	Roadway Horizontal Curvature	Guardrail Lateral Offset (ft)	Slope Length (ft)	Slope Rate	Drop Height (ft)	ADT Recommendation		
						Do Nothing	Remove Existing System	Remove Existing Barrier and Install Crashworthy Barrier
Turned-Down	5 Degree	4-6.9	≤150	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			>350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
		≥7	≤150	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≥7		≤25,000	
			151-350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
			>350	<3:1	≥7	≤1,000		1,001-25,000
				≥3:1	≤7		≤25,000	
			>350	>7	≤500	501-25,000		
				>7	≤500	501-25,000		

9.7 Discussion

Roadside slopes are found on virtually all high-speed roadways and often present a severe obstacle. They must be properly evaluated and considered for guardrail implementation in accordance with the RDG. Many existing guardrail systems found on current highways that shield slopes are more severe than the slope they are shielding. These systems were documented and evaluated by RSAP to make recommendations for treatment. Guardrail implementation was recommended for most slopes between a 1.5 and 2:1. For 3:1 slopes, many guardrails were recommended for removal.

Delineation should be considered in addition to all treatment options, especially if the existing guardrail system is removed and not replaced. Delineation can aid in reducing the number and speed of impacts. It should be repeated that delineation can reduce the frequency of run-off-road accidents but does not reduce the severity of the accident unless it alerts the driver to slow down before the impacting event. It should be noted that the existing barriers would provide some level of delineation of known objects or areas of concern near the roadway.

9.8 Limitations of the Slope Model

The slope model used in RSAP was simplified to include a typical 4:1 transition slope to the steeper slopes of 3:1, 2:1, and 1.5:1. Although, this configuration does not truly model the existing slopes which would have more of a transition zone, this simplified model accurately modeled the existing slopes with a less intricate RSAP model.

RSAP (Version 2003.04.01) does not consider the driver behavior on slopes. Drivers are more likely to attempt a corrective maneuver when the vehicle is encroaching on a foreslope than they are to continue in a straight line (which RSAP models). This corrective maneuver would increase the propensity for rollover; however, RSAP does not incorporate rollover into the calculation of the average severity index of a foreslope. Rollovers on foreslopes are incorporated by adding to the SI values of foreslopes instead of determining an actual probability of rollover [56, 58].

10 SUMMARY, CONCLUSIONS, AND RECOMMENDATIONS

10.1 Summary

The primary function of a guardrail is to prevent errant vehicles from impacting a roadside obstacle or encroaching into an area of concern. Guardrails are intended to shield an obstacle (based on judgment), yet fatalities and serious injuries continue to occur from vehicles impacting these safety devices. These severe injury and fatality crashes may also result from crashes into outdated guardrail installations that did not satisfy current safety performance criteria. Existing guardrail installations can also be found to not meet current guidelines in many ways, such as outdated barrier types, antiquated end treatments, low rail heights, improper installations, variable post spacing, or inadequate lengths of need.

The objective of this research study was to develop guidelines for upgrading existing guardrail installations that have deviations from current practice. Common deviations from current practice include non-typical barrier types, antiquated end treatments, low rail heights, improper installations, and inadequate lengths-of-need. As such, there existed a need for an economic analysis to determine the best safety treatment for existing W-beam guardrail systems with deviations from current practice.

A field investigation was performed on rural minor arterial highways in the state of Kansas. All system geometries, components, deviations from current practice, shielded obstacles, and the roadway conditions were documented. Each field site and barrier installation was also thoroughly photographed to aid in the subsequent analysis. The types of barrier systems that were documented in the field investigation were: (1) strong-post, W-beam guardrail; (2) cable barrier systems; (3) concrete barriers; (4) channel rails; and (5) modified versions of W-beam

guardrail systems. These barrier systems varied in length, height, obstacle shielded, roadway offset, and condition pertaining to aged components, prior impacts, and installation practices.

Strong-post, W-beam guardrail systems were the most common documented barrier system and were the only barrier type selected for the RSAP analysis. Most of these systems had the ability to contain and redirect an errant vehicle and therefore provided some benefit to motorists. The existing W-beam guardrail systems were found with various deviations from current practice, but the most prominent deviations were low-rail height and antiquated end treatments (i.e. blunt-end and turned-down systems).

From the field investigation, culvert openings and roadside slopes were the most prominent obstacles that were shielded by existing barrier systems. Both obstacle types were found near the traveled way and were easily modeled using predefined features within RSAP. The culvert structures varied in length, drop height, lateral offset, and width. Due to the roadside slopes varied in length, slope rate, drop height, lateral offset, and width. The high frequency, high severity, and small lateral offset away from the roadway edge to culvert openings and roadside slopes, it was deemed appropriate to evaluate the cost-effectiveness of various safety treatments.

10.2 Conclusions

10.2.1 Containment Level Study

The containment level study was conducted to better model existing W-beam guardrails with low rail heights. This study utilized previous crash tests and vehicle simulations to generate a graph of containment limit verses rail height for varying-height W-beam guardrail systems. From this graph, containment limit values were found for the 31-in. (787-mm), 27-in. (686-mm), 25-in. (635-mm), and 22-in. (559-mm) guardrail heights. Revised containment limits were

determined when a guardrail system was able to contain and redirect an errant vehicle at a lower rail height.

10.2.2 Existing W-Beam Guardrail Shielding Culverts

The existing guardrail, culvert openings, and roadway conditions were modeled using data obtained from a field investigation conducted on Kansas highways. Three treatment options were examined during the analysis. The baseline treatment was based on a “do nothing” option to the existing guardrail. For this option, the existing guardrail system and a culvert opening with different lengths, offsets, and drop heights. The first safety treatment alternative was to remove the existing guardrail. The removal of the existing guardrail system was estimated to cost \$5.00 per linear foot (\$16.40 per linear meter). The estimated range of the total cost to remove the existing guardrail system ranged between \$1,863.24 and \$4,346.54, which included traffic control, mobilization, and a contingency cost. The second safety treatment alternative was to remove the existing guardrail system and install a guardrail system that meets current safety and design criteria. In this case, the cost to install a new W-beam guardrail systems was estimated to be \$18.16 per linear foot (\$59.58 per linear meter) with an end terminal installation cost of \$4,200 (for two generic terminals). The estimated range of total costs to remove the existing guardrail system and install a new W-beam guardrail system to shield culverts ranged between \$12,392.61 and \$23,897.58, which included traffic control, mobilization, and contingency costs. The complete RSAP B/C tables for the recommendations of existing W-beam guardrail shielding culverts are shown in Appendix D. Guidelines from the benefit-to-cost analyses of the W-beam guardrail shielding culverts are shown in Section 8.6.

10.2.3 Existing W-beam Guardrail Systems Shielding Roadside Slopes

The second analysis was performed to evaluate existing W-beam guardrails shielding slopes and determine the cost-effectiveness of treating these systems with different safety alternatives. The W-beam guardrail system, roadside slope, and roadway conditions were modeled using information obtained from a field investigation conducted on Kansas highways. Three treatment options were examined during the analysis. The baseline option was to “do nothing” to the existing guardrail system. For this option, the existing guardrail system and roadside slope was modeled with different slope rates, lengths, lateral offsets, and drop heights. The first safety treatment alternative was to remove the existing guardrail. The removal of the existing guardrail system was estimated to cost \$5.00 per linear foot (\$16.40 per linear meter). The range of the total cost to remove the existing guardrail system ranged between \$2,790.50 and \$8,320.52, which included traffic control, mobilization, and contingency costs. The second safety treatment alternative was to remove the existing guardrail system and install a new W-beam guardrail system that meets current safety and design criteria. In this case, the cost to install a new W-beam guardrail system was estimated to be \$18.16 per linear foot (\$59.58 per linear meter) with end terminal installation cost of \$4,200 (for two generic terminals). The estimated range of the total cost to remove and install a crashworthy W-beam guardrail system ranged between \$16,688.55 and \$41,913.93, which included traffic control, mobilization, and contingency costs. The complete RSAP B/C tables for the recommendations of existing W-beam guardrail shielding slopes are shown in Appendix F. Guidelines from the benefit-to-cost analyses of the W-beam guardrail shielding slopes are shown in Section 9.6.

10.3 Recommendations

10.3.1 Existing Cable Barrier Systems

Out of the 68 barrier systems that were documented in the field investigation, 9 were low-tension cable barrier systems. Most cables had kinks, slack (non-tensioned) spans, concrete posts, antiquated end treatments, and rusted components. The concrete posts will present blunt obstacles to motorists, if impacted. The end sections of the existing barrier systems had two major concerns. First, they did not have sufficient anchorage to produce enough strength on the ends of the cable systems to redirect an errant vehicle. Second, the end posts were exposed to errant vehicles, presenting a blunt-end obstacle. Missing posts were also found in some of the systems. The use of only 1-cable and 2-cable systems will pose a risk to motorists if the barrier is unable to safely contain or redirect a vehicle. The existing cable barrier systems found in the field investigation had very little, if any, containment capacity for capturing an errant vehicle due to the slack cable segments, only 1 or 2 cables, and lack of end anchorage at many of the end terminals. Cable barrier systems were not selected to be evaluated in RSAP; because, they are not a predefined feature in RSAP, and extensive deviations from current practice were found in these systems. Thus, the existing cable barrier systems should be considered for removal or replacement. No further RSAP analysis was conducted for the cable barrier systems.

10.3.2 Flat-Panel Rail

Three of the 68 barrier systems that were documented consisted of steel, flat-panel barriers. This barrier utilized a steel panel rail and wood posts. The flat-panel rail found in the field investigation had a high potential to trip an errant vehicle because of the low top mounting height of the rail element. The upstream and downstream end treatments of all flat-panel systems were blunt-ends with little or no anchorage. For these reasons, flat-panel barriers were not

considered in the RSAP analysis. Removal of these barriers is recommended with a consideration of replacement with a new barrier that meets all current guidelines.

10.3.3 Existing Concrete Barriers

One concrete rail with concrete posts over a culvert was discovered in the field investigation. The barrier was not equipped with an end treatment. The concrete barrier found in the field investigation should be removed due to the fact it would act as a rigid object to impacting vehicles likely as severe as most culverts. Removal of this barrier is necessary on high-speed roadways. System replacement should be considered if the culvert opening is determined to require shielding.

10.3.4 Existing W-Beam Guardrail

W-beam guardrails were the most common guardrail systems that were documented in the field investigation, representing 45 of the 68 documented systems. Spoon (blunt-end) terminals were used on 40 of the W-beam guardrail systems, while the other five systems utilized turned-down terminals. The main deviations from current practice that were found with W-beam guardrail systems were low rail height and deficient end treatments. A number of systems had missing posts and blockouts. Other deviations from current practice include deficient bridge rail connections, non-crashworthy end treatments, and system damage. Strong-post, W-beam guardrails were the only guardrail systems considered for the RSAP analysis, because of their ability to be modeled and their high frequency within the sample. These guardrail systems were found to shield a number of obstacles which were predominantly culvert openings or slopes. Modified W-beam and channel rails were very comparable to the existing W-beam guardrails that were documented. For this reason, they were added to the analysis.

10.3.4.1 Shielding Culverts

There were 4,860 scenarios that were simulated for existing W-beam guardrails used to shield culvert obstacles. As expected and for most of the 22-in. (559-mm) tall guardrail systems, guardrail system replacement was recommended. For 27-in. (686-mm) tall guardrail systems, guardrail system replacement was less frequently recommended. Existing guardrail systems utilizing turned-down terminals were less likely to be replaced than those systems configured with blunt-end terminals. For W-beam guardrail systems with a 22-in. (559-mm) mounting height and ADT greater than 500 vpd, guardrail system replacement was recommended in most cases. When the ADT was lower than 1,000 vpd, both 25-in. and 27-in. (635-mm and 686-mm) tall W-beam guardrail systems were not recommended for replacement in most instances. Existing W-beam guardrail systems found on curves were recommended to be removed or replaced in most cases due to the greater number of impacts caused by the horizontal curvature of the roadway. The complete RSAP B/C tables for the recommendations of existing W-beam guardrail systems shielding culverts are shown in Appendix D.

10.3.4.2 Shielding Slopes

There were 14,580 scenarios that were simulated for existing W-beam guardrails used to shield roadside slopes. As expected, most of the 22-in. (559-mm) tall guardrail systems are recommended for removal and replacement, although fewer 27-in. (686-mm) tall guardrail systems needed replacement. Existing guardrail systems which utilized turned-down terminals were less likely to be replaced than those systems configured with blunt-end treatments. The 25-in. and 27-in. (635-mm and 686-mm) tall W-beam guardrail systems only needed replacement when the ADT was higher than 1,000 vpd in most cases. Roadside slopes of 3:1 or flatter with low drop heights were usually recommended for removal. Existing W-beam guardrail systems

found on curves were recommended to be removed or replaced in most cases due to the greater number of impacts caused by the horizontal curvature of the roadway. The complete RSAP B/C tables for the recommendations of existing W-beam guardrail systems shielding slopes are shown in Appendix F.

11 LIMITATIONS AND FUTURE WORK

11.1 Limitations

This research had some limitations due to the fact that it was not possible to model and analyze all existing guardrail systems and all deviations from current practice. These RSAP recommendations did not include guardrail systems beyond the existing strong-post W-beam guardrail systems. Cable, flat-panel, and concrete rails were not included in the analysis of existing guardrail systems that shielded culvert openings. These systems would be difficult to accurately model in RSAP.

The W-beam guardrail systems used in the RSAP analysis only included those guardrail systems with steel and wood posts. Concrete posts were not included in the analysis. Concrete posts on top of culverts would require extra removal equipment as compared to steel and wood posts, which would add to the total cost to transport and time to remove.

Guardrail height and outdated terminals were the only deviations from current practice that were modeled in the RSAP analysis. Although these deviations were likely the most prominent and most severe, there were other deviations and conditions that were documented during the field investigation which were not evaluated in this study. These other deviations include rail damage, damaged and missing posts and blockouts, and insufficient length of need.

The only functional class modeled in RSAP was rural minor arterial highways. Although 90 percent of all roadways in the field investigation were minor arterial highways, there were other functional classes that were documented but not evaluated.

The RSAP recommendations were based on costs available at the time of the research study. Injury, fatality, installation, material, and other costs will continue to increase over time. These changes may alter the B/C results in the future.

There are two typical treatments for culverts that were not evaluated in this report: (1) culvert grate installation or (2) extending the headwall. Culvert grates can be installed on typical culvert sizes and have been found to be passably traversable by errant vehicles [65]. Extending the culvert to a farther offset, such as outside the clear zone, is another treatment option. This alternative would require that fill material be easily obtainable for this option to be economically viable. This study focused on upgrading existing guardrail systems, so these two alternatives were not considered for this project, although they may be the preferred treatment alternatives.

Culverts are either found on flat ground or on a sag section of the roadway where the water can flow through a valley. Vertical sag curves on the roadway may increase the potential for vehicle encroachments. Due to the increased speed caused by the downward acceleration of a vehicle, sag segments were not considered in the RSAP analysis. Thus, conservative recommendations were made when treating an existing guardrail system that is used to shield a culvert opening in a sag segment.

The guardrail system lateral offsets were modeled as 2 ft, 4 ft, and 7 ft (0.6 m, 1.2 m, and 2.1 m). Although these offsets represent most of the systems found in the field investigation, there were also systems found outside of this range. Systems with offsets greater than 7 ft (2.1 m) were found in many instances, which increased to 12 ft (3.7 m). Although, these systems may produce different results, they were not included in this analysis.

This version of RSAP [10] does not consider the driver behavior on slopes. Drivers are more likely to attempt a corrective maneuver when the vehicle is encroaching on a foreslope than when continuing in a straight line (which RSAP models). This corrective maneuver would increase the propensity for rollover. However, RSAP does not incorporate rollover into the calculation of the average severity index of a foreslope. Rollovers on foreslopes are incorporated

by adding to the SI values of foreslopes instead of determining an actual probability of rollover [58, 56].

It should be repeated that cable barrier systems were not considered in this RSAP analysis. However, they may remain viable when replacing existing barrier systems. In RSAP, there is no predefined cable barrier system, so the W-beam and cable barrier systems are modeled the same. The only differences in modeling the two barriers are the maximum deflection and terminal types, which may be assumed to generate a similar or reduced severity for each type of barrier. Cable barrier systems should be considered for treating on slopes when found to cost less and/or when a more forgiving barrier is needed for containing errant vehicles. Additional deflection distance should be considered when implementing cable barrier systems.

Roadside soil grading was not evaluated as a treatment option in this study. This treatment could lead to slope flattening (i.e., changing a 2:1 slope to a 6:1 slope). As the slope flattens, general vehicle instability and the potential for a rollover are also reduced. This treatment would require the transportation of soil material and the possible purchase of right-of-way land adjacent to the roadway. This study was focused on upgrading existing guardrail systems, thus roadside grading was not considered even though it may be a preferred treatment practice for certain cases. If slope grading were selected, the prior published roadside grading guidance [55-56] should be followed based on specific roadside conditions.

11.2 Recommendations for Future Work

The only functional class of roadway considered in this study was rural minor arterials. In RSAP, the functional class plays a major roll when determining vehicle speeds and encroachment probabilities. In the future, it may be interesting to investigate similar safety treatments over different functional classes of roadways.

The majority of lateral barrier offsets ranged from 2 ft to 7 ft (0.6 m to 2.1 m) in the field investigation. As a result, lateral offsets greater than 7 ft (2.1 m) were not considered. RSAP encroachment predictions drop significantly as lateral offsets increase. Thus, lateral offsets of 10 ft (3.0 m) could vary from the evaluated 7 ft (2.1 m). It may be interesting to evaluate these RSAP scenarios with larger lateral offsets.

The recommended guardrail system upgrade that was used in the RSAP analysis corresponded to an MGS with a 31 in. (787 mm) top mounting height. It should be noted that no guardrail system upgrades considered the addition of blockouts or raising the rail to the standard 27¾ in. (705 mm) height.

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13 APPENDICES

Appendix A. Field Investigation Form

FIELD INVESTIGATION DATA SHEET

Field Team Members: _____ Date: _____

I. DISTRICT NO.: _____

II. AREA NO.: _____

III. COUNTY NAME: _____

IV. ROADWAY:

Highway No.: _____ Mile Marker: _____ Location: _____ (e/w or n/s)

No. Lanes: _____
Lane Width: _____
Shoulder Width: _____
Median Width: _____

Grade: _____ (Flat, Shallow, Steep)
_____ (Up, Down, Sag, Crest)
Curve: _____ (None, Mild, Sharp)
_____ (Left, Right, NA)

Turn Lanes: _____ (Left, Right, Both, None)
Lane Markings: _____ (Paint, Thermoplastic)
Marking Condition: _____ (Excellent, Good, Fair, Poor)
Roadway Lighting: _____ (None, Spot, Continuous)
Speed Limit: _____

V. BARRIER SYSTEM:

A. Description/Identification and Measurements:

System Description: _____
(Strong post / weak post, rail type, etc.)

Barrier Length: _____ (First post to last post)
Barrier Width: _____ (Front rail face to back of post)

Lateral Barrier Offset: (Front rail face to edge of traveled way or lane)
US: _____
Mid: _____
DS: _____

Figure A-1. Field Investigation Form (1 of 4)

Approach Slope (rail face to shoulder edge) lateral offset is from roadway

Lateral Offset	2 ft	4 ft	6 ft	8 ft	10 ft	12 ft	Rail ___ ft
elev. US							
elev Mid							
elev. DS							

Backside Slope: Measured to 10 ft behind post or 30 ft from edge of roadway, whichever is greater.

Lateral Offset	Rail ___ ft	5 ft	___	10 ft	___	15 ft	___	20 ft	___	25 ft	___	30 ft
elev. US												
elev Mid												
elev. DS												

B. Rail:

Guardrail Element Type: _____
Guardrail Depth (vertical): / *US* / *Mid* / *DS*
Guardrail Width (horizontal): / *US* / *Mid* / *DS*
Guardrail Thickness: / *US* / *Mid* / *DS*

Guardrail Element Surface Condition: _____
Guardrail Splices: _____ (Type - Describe)
Guardrail Splice Bolts: _____ (number and size)
Top Rail Mounting Height (High):* _____ (*taken at face of rail)
Top Rail Mounting Height (Low): * _____ (*taken at face of rail)
Top Rail Mounting Height (High):** _____ (** relative to edge of roadway)
Top Rail Mounting Height (Low):** _____ (** relative to edge of roadway)

C. Posts:

No. of Posts: _____
Post Spacing: _____
Post Material: _____
Post Shape: _____
Post Size: _____
Post Orientation: _____ (Description, angle)*
* - Vertical, lean forward or backward, rotated upstream or downstream; degrees
Post Condition: _____
(Evaluate post condition 0-3" below grade at back side of at least 3 posts)

Figure A-2. Field Investigation Form (2 of 4)

D. Blockouts:

Blockout Material: _____
Blockout Shape: _____
Blockout Size: _____
Blockout Orientation: _____(Vertical, rotated US or DS)
Blockout Condition: _____

E. Guardrail End Terminals:

Crashworthy Terminal (US End): _____(Y or N; Describe)
Crashworthy Terminal (DS End): _____(Y or N; Describe)

Terminal Geometry (US End): _____(Flared, tangent, or parabolic)
Terminal Geometry (DS End): _____(Flared, tangent, or parabolic)

Terminal Length (US End): _____
Terminal Length (DS End): _____
End Offset (US End): _____
End Offset (DS End): _____

Cable Anchorage (US End): _____(Y or N; Describe)
Cable Anchorage (DS End): _____(Y or N; Describe)

VI. HAZARD DESCRIPTION AND MEASUREMENTS:

Hazard Type: _____
Hazard Width: _____
Hazard Length: _____
Distance from Rail Face to Front Face of Hazard: _____
US Guardrail End to Start of Hazard: _____
DS Guardrail End to End of Hazard: _____(Reverse traffic)

VII. OTHER DOCUMENTATION:

Site Photographs: _____(Photo No. X to Photo No. Y)*
* - Photographs from all vantage points for barrier system, hazard, and combinations thereof.

VIII. Additional Notes/Comments:
(damage to rail, missing elements / parts, incorrect installations, substandard systems)

Figure A-3. Field Investigation Form (3 of 4)

IX. Field Sketch (if necessary)

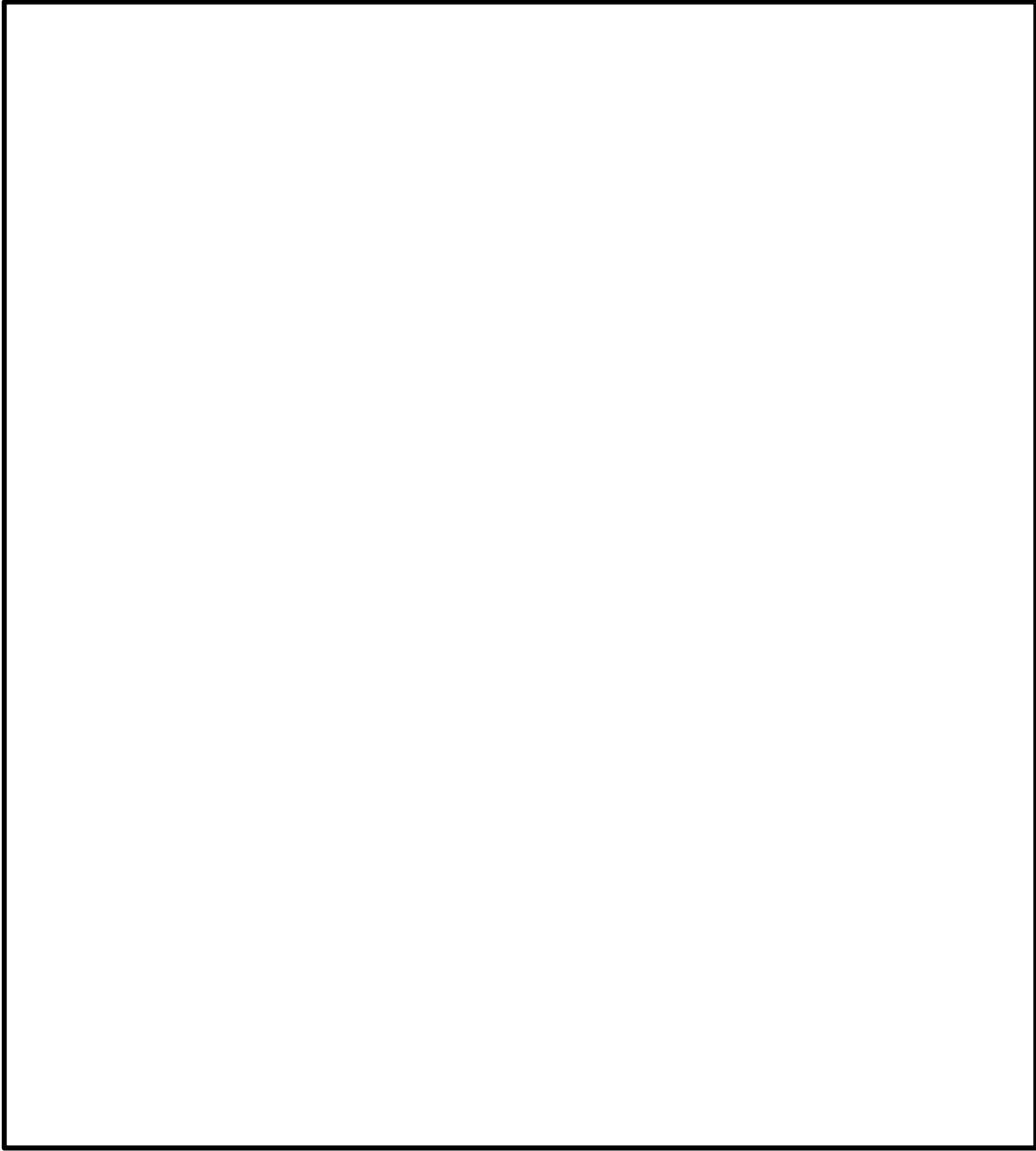
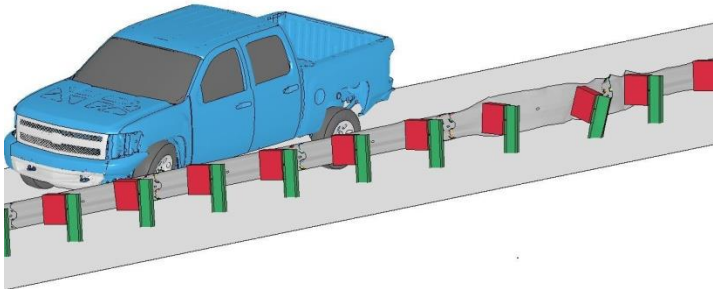
A large, empty rectangular box with a black border, intended for a field sketch. The box is oriented vertically and occupies most of the page's width and height.

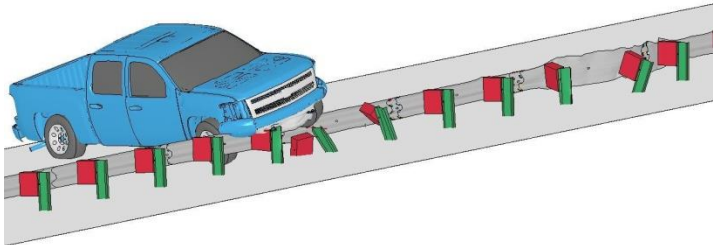
Figure A-4. Field Investigation Form (4 of 4)

Appendix B. LS-DYNA Computer Simulations of Variable-Height Guardrail Systems

37.3 mph
(60 km/h)



43.5 mph
(70 km/h)



62.1 mph
(100 km/h)

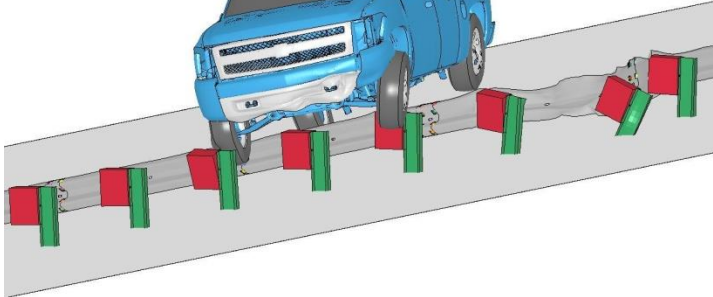
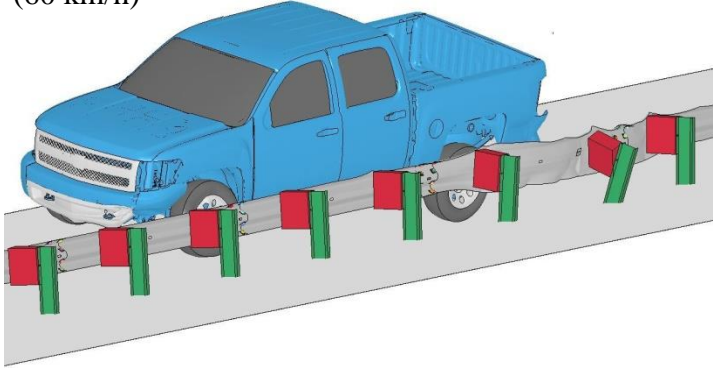
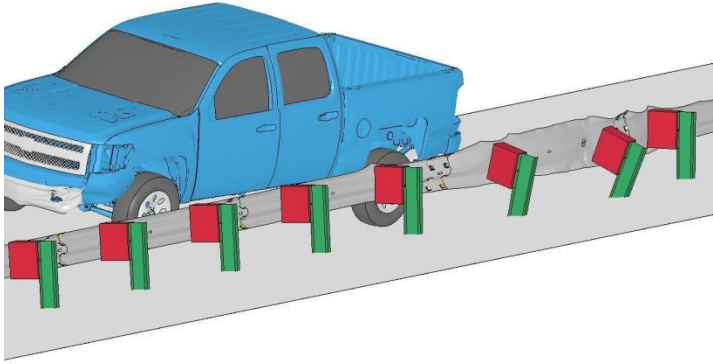


Figure B-1. Simulation Results for a 2270p Pickup Impacting 22-in. (589-mm) Rail Height.

37.3 mph
(60 km/h)



43.5 mph
(70 km/h)



62.1 mph
(100 km/h)

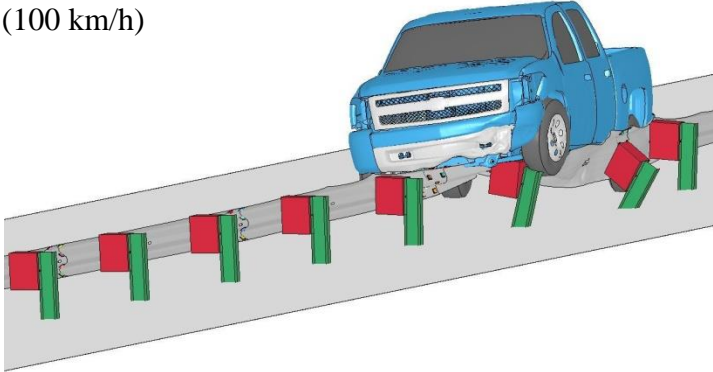


Figure B-2. Simulation Results for a 2270p Pickup Impacting 25-in. (635-mm) Rail Height.

**Appendix C. Guardrail Modeling and Costs for Upgrading Existing W-Beam Guardrail
Shielding Culvert Openings**

Sample Calculations.

Table C-1. Interpolated Runout Lengths (L_R) [66-67]

L_R	Traffic Volume (ADT)							
	Under 1,000		1,000-5,000		5,000-10,000		Over 10,000	
Speed	(ft)	(m)	(ft)	(m)	(ft)	(m)	(ft)	(m)
65 mph (104.6 km/h)	195	66.5	220	67.0	255	77.5	310	94.5

Table C-2. Clear-zone Distances (L_C) Interpolated Values [33]

L_C	L_C Given Traffic Volume (ADT)							
	Under 750		750-1,500		1,500-6,000		Over 6,000	
Speed	(ft)	(m)	(ft)	(m)	(ft)	(m)	(ft)	(m)
65 mph (104.6 km/h)	19	5.8	25	7.6	30	9.1	32	9.8

First row of Table C-3:

Segment length = SGL = 3,280.84 ft

ADT = 500 vpd

Slope Length = CL = 10 ft

Lateral Offset = OFF = 2 ft

Runout Length = L_R = 195 ft (Table C-1)

Clear-zone distance = L_C = 19 ft (Table C-2)

Terminal Length = TL = 12.5 ft

Guardrail Removal Cost = GRRC = \$5 per linear foot

TL-3 Terminal Cost = \$2,100 (37.5 ft)

TL-3 Guardrail Cost = \$18.16 per linear foot

Added Costs:

Traffic Control = 10%

Mobilization = 7.5%

Contingency = 15%

$$\text{Culvert Starting Location} = \frac{SGL}{2} - \frac{CL}{2} = \frac{3280.84 \text{ ft}}{2} - \frac{10}{2} = 1635.4 \text{ ft}$$

$$\text{Length of Need (LON)} = \frac{L_C - OFF}{\frac{L_C}{L_R}} = \frac{19 \text{ ft} - 2 \text{ ft}}{19 \text{ ft}/195 \text{ ft}} = 174.5 \text{ ft}$$

$$W - \text{beam Length} = CL + LON \times 2 = 10 \text{ ft} + 174.5 \text{ ft} \times 2 = 358.9 \text{ ft}$$

$$\begin{aligned} \text{Barrier Starting Location} &= \text{Culvert Starting Location} - LON = 1635.4 \text{ ft} - 174.5 \text{ ft} \\ &= 1460.9 \text{ ft} \end{aligned}$$

$$\begin{aligned} \text{Total Length of Barrier (TL)} &= CL + LON \times 2 + TL \times 2 = 10 \text{ ft} + 174.5 \text{ ft} \times 2 + 12.5 \times 2 \\ &= 383.9 \text{ ft} \end{aligned}$$

$$\text{Total Cost to Remove} = TL \times GRRC \times (1 + 0.1 + .075 + 0.15)$$

$$= 383.9 \text{ ft} \times \frac{\$5}{\text{lf}} \times (1 + 0.1 + .075 + 0.15) = \$2,543.65$$

Total Cost to Remove & Replace

$$\begin{aligned} &= \text{Total Cost to Remove} + ((TL - 37.5 \times 2) \times \$18.16 + \$2,100 \times 2) \\ &\times (1 + 0.1 + .075 + 0.15) = \$15,542.54 \end{aligned}$$

Table C-3. Guardrail Shielding Culverts Modeling and Cost (English Units)

ADT	Culvert Length (ft)	Lateral Offset (ft)	Culvert Starting Location (ft)	Length of Need (ft)	W-beam Length (ft)	Barrier Starting Location (ft)	Terminal Length (ft)	Total Length of Barrier (ft)	Total Cost to Remove (USD)	Total Cost to Remove & Replace (USD)
500	10	2	1635.4	174.5	358.9	1460.9	12.5	383.9	\$2,543.65	\$15,542.54
		4	1635.4	153.9	317.9	1481.5	12.5	342.9	\$2,271.68	\$14,282.76
		7	1635.4	123.2	256.3	1512.3	12.5	281.3	\$1,863.72	\$12,393.09
	30	2	1625.4	174.5	378.9	1450.9	12.5	403.9	\$2,676.15	\$16,156.28
		4	1625.4	153.9	337.9	1471.5	12.5	362.9	\$2,404.18	\$14,896.50
		7	1625.4	123.2	276.3	1502.3	12.5	301.3	\$1,996.22	\$13,006.83
	50	2	1615.4	174.5	398.9	1440.9	12.5	423.9	\$2,808.65	\$16,770.02
		4	1615.4	153.9	357.9	1461.5	12.5	382.9	\$2,536.68	\$15,510.24
		7	1615.4	123.2	296.3	1492.3	12.5	321.3	\$2,128.72	\$13,620.57
1,000	10	2	1635.4	202.4	414.8	1433.0	12.5	439.8	\$2,913.68	\$17,256.49
		4	1635.4	184.8	379.6	1450.6	12.5	404.6	\$2,680.48	\$16,176.31
		7	1635.4	158.4	326.8	1477.0	12.5	351.8	\$2,330.68	\$14,556.04
	30	2	1625.4	202.4	434.8	1423.0	12.5	459.8	\$3,046.18	\$17,870.23
		4	1625.4	184.8	399.6	1440.6	12.5	424.6	\$2,812.98	\$16,790.05
		7	1625.4	158.4	346.8	1467.0	12.5	371.8	\$2,463.18	\$15,169.78
	50	2	1615.4	202.4	454.8	1413.0	12.5	479.8	\$3,178.68	\$18,483.97
		4	1615.4	184.8	419.6	1430.6	12.5	444.6	\$2,945.48	\$17,403.79
		7	1615.4	158.4	366.8	1457.0	12.5	391.8	\$2,595.68	\$15,783.52
5,000	10	2	1635.4	238.0	486.0	1397.4	12.5	511.0	\$3,385.38	\$19,441.41
		4	1635.4	221.0	452.0	1414.4	12.5	477.0	\$3,160.13	\$18,398.05
		7	1635.4	195.5	401.0	1439.9	12.5	426.0	\$2,822.25	\$16,833.01
	30	2	1625.4	238.0	506.0	1387.4	12.5	531.0	\$3,517.88	\$20,055.15
		4	1625.4	221.0	472.0	1404.4	12.5	497.0	\$3,292.63	\$19,011.79
		7	1625.4	195.5	421.0	1429.9	12.5	446.0	\$2,954.75	\$17,446.75
	50	2	1615.4	238.0	526.0	1377.4	12.5	551.0	\$3,650.38	\$20,668.89
		4	1615.4	221.0	492.0	1394.4	12.5	517.0	\$3,425.13	\$19,625.53
		7	1615.4	195.5	441.0	1419.9	12.5	466.0	\$3,087.25	\$18,060.49
10,000	10	2	1635.4	239.1	488.1	1396.4	12.5	513.1	\$3,399.45	\$19,506.62
		4	1635.4	223.1	456.3	1412.3	12.5	481.3	\$3,188.28	\$18,528.47
		7	1635.4	199.2	408.4	1436.2	12.5	433.4	\$2,871.52	\$17,061.25
	30	2	1625.4	239.1	508.1	1386.4	12.5	533.1	\$3,531.95	\$20,120.36
		4	1625.4	223.1	476.3	1402.3	12.5	501.3	\$3,320.78	\$19,142.21
		7	1625.4	199.2	428.4	1426.2	12.5	453.4	\$3,004.02	\$17,674.99
	50	2	1615.4	239.1	528.1	1376.4	12.5	553.1	\$3,664.45	\$20,734.10
		4	1615.4	223.1	496.3	1392.3	12.5	521.3	\$3,453.28	\$19,755.95
		7	1615.4	199.2	448.4	1416.2	12.5	473.4	\$3,136.52	\$18,288.73
25,000	10	2	1635.4	290.6	591.3	1344.8	12.5	616.3	\$4,082.66	\$22,671.21
		4	1635.4	271.3	552.5	1364.2	12.5	577.5	\$3,825.94	\$21,482.09
		7	1635.4	242.2	494.4	1393.2	12.5	519.4	\$3,440.86	\$19,698.41
	30	2	1625.4	290.6	611.3	1334.8	12.5	636.3	\$4,215.16	\$23,284.95
		4	1625.4	271.3	572.5	1354.2	12.5	597.5	\$3,958.44	\$22,095.83
		7	1625.4	242.2	514.4	1383.2	12.5	539.4	\$3,573.36	\$20,312.15
	50	2	1615.4	290.6	631.3	1324.8	12.5	656.3	\$4,347.66	\$23,898.69
		4	1615.4	271.3	592.5	1344.2	12.5	617.5	\$4,090.94	\$22,709.57
		7	1615.4	242.2	534.4	1373.2	12.5	559.4	\$3,705.86	\$20,925.89

Table C-4. Guardrail Shielding Culverts Modeling and Cost (Metric Units)

ADT	Culvert Length (m)	Lateral Offset (m)	Culvert Starting Location (m)	Length of Need (m)	W-beam Length (m)	Barrier Starting Location (m)	Terminal Length (m)	Total Length of Barrier (m)	Total Cost to Remove (USD)	Total Cost to Remove & Replace (USD)
500	3.0	0.6	498.5	53.2	109.4	445.3	3.8	117.0	\$2,543.65	\$15,542.54
		1.2	498.5	46.9	96.9	451.6	3.8	104.5	\$2,271.68	\$14,282.76
		2.1	498.5	37.5	78.1	460.9	3.8	85.7	\$1,863.72	\$12,393.09
	9.1	0.6	495.4	53.2	115.5	442.2	3.8	123.1	\$2,676.15	\$16,156.28
		1.2	495.4	46.9	103.0	448.5	3.8	110.6	\$2,404.18	\$14,896.50
		2.1	495.4	37.5	84.2	457.9	3.8	91.8	\$1,996.22	\$13,006.83
	15.2	0.6	492.4	53.2	121.6	439.2	3.8	129.2	\$2,808.65	\$16,770.02
		1.2	492.4	46.9	109.1	445.5	3.8	116.7	\$2,536.68	\$15,510.24
		2.1	492.4	37.5	90.3	454.8	3.8	97.9	\$2,128.72	\$13,620.57
1,000	3.0	0.6	498.5	61.7	126.4	436.8	3.8	134.1	\$2,913.68	\$17,256.49
		1.2	498.5	56.3	115.7	442.1	3.8	123.3	\$2,680.48	\$16,176.31
		2.1	498.5	48.3	99.6	450.2	3.8	107.2	\$2,330.68	\$14,556.04
	9.1	0.6	495.4	61.7	132.5	433.7	3.8	140.1	\$3,046.18	\$17,870.23
		1.2	495.4	56.3	121.8	439.1	3.8	129.4	\$2,812.98	\$16,790.05
		2.1	495.4	48.3	105.7	447.1	3.8	113.3	\$2,463.18	\$15,169.78
	15.2	0.6	492.4	61.7	138.6	430.7	3.8	146.2	\$3,178.68	\$18,483.97
		1.2	492.4	56.3	127.9	436.1	3.8	135.5	\$2,945.48	\$17,403.79
		2.1	492.4	48.3	111.8	444.1	3.8	119.4	\$2,595.68	\$15,783.52
5,000	3.0	0.6	498.5	72.5	148.1	425.9	3.8	155.8	\$3,385.38	\$19,441.41
		1.2	498.5	67.4	137.8	431.1	3.8	145.4	\$3,160.13	\$18,398.05
		2.1	498.5	59.6	122.2	438.9	3.8	129.8	\$2,822.25	\$16,833.01
	9.1	0.6	495.4	72.5	154.2	422.9	3.8	161.8	\$3,517.88	\$20,055.15
		1.2	495.4	67.4	143.9	428.1	3.8	151.5	\$3,292.63	\$19,011.79
		2.1	495.4	59.6	128.3	435.8	3.8	135.9	\$2,954.75	\$17,446.75
	15.2	0.6	492.4	72.5	160.3	419.8	3.8	167.9	\$3,650.38	\$20,668.89
		1.2	492.4	67.4	150.0	425.0	3.8	157.6	\$3,425.13	\$19,625.53
		2.1	492.4	59.6	134.4	432.8	3.8	142.0	\$3,087.25	\$18,060.49
10,000	3.0	0.6	498.5	72.9	148.8	425.6	3.8	156.4	\$3,399.45	\$19,506.62
		1.2	498.5	68.0	139.1	430.5	3.8	146.7	\$3,188.28	\$18,528.47
		2.1	498.5	60.7	124.5	437.8	3.8	132.1	\$2,871.52	\$17,061.25
	9.1	0.6	495.4	72.9	154.9	422.6	3.8	162.5	\$3,531.95	\$20,120.36
		1.2	495.4	68.0	145.2	427.4	3.8	152.8	\$3,320.78	\$19,142.21
		2.1	495.4	60.7	130.6	434.7	3.8	138.2	\$3,004.02	\$17,674.99
	15.2	0.6	492.4	72.9	161.0	419.5	3.8	168.6	\$3,664.45	\$20,734.10
		1.2	492.4	68.0	151.3	424.4	3.8	158.9	\$3,453.28	\$19,755.95
		2.1	492.4	60.7	136.7	431.7	3.8	144.3	\$3,136.52	\$18,288.73
25,000	3.0	0.6	498.5	88.6	180.2	409.9	3.8	187.8	\$4,082.66	\$22,671.21
		1.2	498.5	82.7	168.4	415.8	3.8	176.0	\$3,825.94	\$21,482.09
		2.1	498.5	73.8	150.7	424.7	3.8	158.3	\$3,440.86	\$19,698.41
	9.1	0.6	495.4	88.6	186.3	406.8	3.8	193.9	\$4,215.16	\$23,284.95
		1.2	495.4	82.7	174.5	412.8	3.8	182.1	\$3,958.44	\$22,095.83
		2.1	495.4	73.8	156.8	421.6	3.8	164.4	\$3,573.36	\$20,312.15
	15.2	0.6	492.4	88.6	192.4	403.8	3.8	200.0	\$4,347.66	\$23,898.69
		1.2	492.4	82.7	180.6	409.7	3.8	188.2	\$4,090.94	\$22,709.57
		2.1	492.4	73.8	162.9	418.6	3.8	170.5	\$3,705.86	\$20,925.89

Appendix D. Guidelines for Existing W-Beam Guardrail Shielding Culvert Openings

Table D-1. 22-in. Tall W-beam Guardrail with Blunt-End Terminal Shielding Culvert (B/C=2:1)

22 in. (559 mm) W-beam with Blunt End Terminal Shielding a Culvert (B/C 2:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft (15 m) Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
	26	7.9	500									
1,000												
5,000												
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
	26	7.9	500									
1,000												
5,000												

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	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table D-2. 22-in.Tall W-beam Guardrail with Blunt-End Terminal Shielding Culvert (B/C=4:1)

22 in. (559 mm) W-beam with Blunt End Terminal Shielding a Culvert (B/C 4:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft (15 m) Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
26	7.9	500										
		1,000										
		5,000										
		10,000										
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
	26	7.9	500									
			1,000									
			5,000									
			10,000									
25,000		500										
		1,000										
		5,000										
		10,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

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Table D-3. 22 in. Tall W-beam Guardrail with Turned-Down Terminal Shielding Culvert (B/C=2:1)

22 in. (559 mm) W-beam with Turned Down End Terminal Shielding a Culvert (B/C 2:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft [15 m] Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
26	7.9	500										
		1,000										
		5,000										
		10,000										
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
	26	7.9	500									
			1,000									
			5,000									
			10,000									
25,000		500										
		1,000										
		5,000										
		10,000										

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	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table D-4. 22-in.Tall W-beam Guardrail with Turned-Down Terminal Shielding Culvert (B/C=4:1)

22 in. (559 mm) W-beam with Turned Down End Terminal Shielding a Culvert (B/C 4:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft [15 m] Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
	26	7.9	500									
1,000												
5,000												
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
	13	4.0	500									
			1,000									
			5,000									
	26	7.9	500									
			1,000									
			5,000									

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	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table D-5. 25-in. Tall W-beam Guardrail with Blunt-End Terminal Shielding Culvert (B/C=2:1)

25 in. (635 mm) W-beam with Blunt End Terminal Shielding a Culvert (B/C 2:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft [15 m] Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
26	7.9	500										
		1,000										
		5,000										
		10,000										
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
	26	7.9	500									
			1,000									
			5,000									
			10,000									
25,000		500										
		1,000										
		5,000										
		10,000										

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	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table D-6. 25-in. Tall W-beam Guardrail with Blunt-End Terminal Shielding Culvert (B/C=4:1)

25 in. (635 mm) W-beam with Blunt End Terminal Shielding a Culvert (B/C 4:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft [15 m] Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
26	7.9	500										
		1,000										
		5,000										
		10,000										
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
	26	7.9	500									
			1,000									
			5,000									
			10,000									
25,000		500										
		1,000										
		5,000										
		10,000										

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	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table D-8. 25-in. Tall W-beam Guardrail with Turned-Down Terminal Shielding Culvert (B/C=4:1)

25 in. (635 mm) W-beam with Turned Down End Terminal Shielding a Culvert (B/C 4:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft [15 m] Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
26	7.9	500										
		1,000										
		5,000										
		10,000										
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
	26	7.9	500									
			1,000									
			5,000									
			10,000									
25,000	7.9	500										
		1,000										
		5,000										
		10,000										
25,000	7.9	25,000										
		25,000										
		25,000										
		25,000										

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	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table D-9. 27-in. Tall W-beam Guardrail with Blunt-End Terminal Shielding Culvert (B/C=2:1)

27 in. (686 mm) W-beam with Blunt End Terminal Shielding a Culvert (B/C 2:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft [15 m] Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
26	7.9	500										
		1,000										
		5,000										
		10,000										
		25,000										
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
	13	4.0	500									
			1,000									
			5,000									
	26	7.9	500									
			1,000									
			5,000									

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	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table D-10. 27-in. Tall W-beam Guardrail with Blunt-End Terminal Shielding Culvert (B/C=4:1)

27 in. (686 mm) W-beam with Blunt End Terminal Shielding a Culvert (B/C 4:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft [15 m] Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
26	7.9	500										
		1,000										
		5,000										
		10,000										
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
	26	7.9	500									
			1,000									
			5,000									
			10,000									
25,000	7.9	500										
		1,000										
		5,000										
		10,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table D-11. 27-in. Tall W-beam Guardrail with Turned-Down Terminal Shielding Culvert (B/C=2:1)

27 in. (686 mm) W-beam with Turned Down End Terminal Shielding a Culvert (B/C 2:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft [15 m] Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
26	7.9	500										
		1,000										
		5,000										
		10,000										
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
	26	7.9	500									
			1,000									
			5,000									
			10,000									
25,000		500										
		1,000										
		5,000										
		10,000										

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	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table D-12. 27-in. Tall W-beam Guardrail with Turned-Down Terminal Shielding Culvert (B/C=4:1)

27 in. (686 mm) W-beam with Turned Down End Terminal Shielding a Culvert (B/C 4:1)												
Horizontal Curve	Drop Height		ADT	10 ft [3 m] Culvert Length Along Road			30 ft [9 m] Culvert Length Along Road			50 ft [15 m] Culvert Length Along Road		
	(ft)	(m)		2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset	2 ft [0.6 m] Offset	4 ft [0.6 m] Offset	7 ft [2.1 m] Offset
Straight Segment	7	2.1	500									
			1,000									
			5,000									
10,000												
25,000												
13	4.0	500										
		1,000										
		5,000										
		10,000										
		25,000										
26	7.9	500										
		1,000										
		5,000										
		10,000										
		25,000										
5° Curved Segment	7	2.1	500									
			1,000									
			5,000									
			10,000									
			25,000									
	13	4.0	500									
			1,000									
			5,000									
			10,000									
			25,000									
	26	7.9	500									
			1,000									
			5,000									
			10,000									
			25,000									

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" Tall W-beam Guardrail with acceptable end treatment

**Appendix E. Guardrail Modeling and Costs for Upgrading Existing W-Beam Guardrail
Shielding Slopes**

Sample Calculations.

Table E-1. Interpolated Runout Lengths (L_R) [66-67]

L_R	Traffic Volume (ADT)							
	Under 1,000		1,000-5,000		5,000-10,000		Over 10,000	
Speed	(ft)	(m)	(ft)	(m)	(ft)	(m)	(ft)	(m)
65 mph (104.6 km/h)	195	66.5	220	67.0	255	77.5	310	94.5

Table E-2. Clear-zone Distances (L_C) Interpolated Values [33]

L_C	L_C Given Traffic Volume (ADT)							
	Under 750		750-1,500		1,500-6,000		Over 6,000	
Speed	(ft)	(m)	(ft)	(m)	(ft)	(m)	(ft)	(m)
65 mph (104.6 km/h)	19	5.8	25	7.6	30	9.1	32	9.8

First row of Table E-3:

Segment length = SGL = 3,281 ft

ADT = 500 vpd

Slope Length = SL = 150 ft

Lateral Offset = OFF = 2 ft

Runout Length = L_R = 195 ft (Table C-1)

Clear-zone distance = L_C = 19 ft (Table C-2)

Terminal Length = TL = 12.5 ft

Guardrail Removal Cost = GRRC = \$5 per linear foot

TL-3 Terminal Cost = \$2,100 (37.5 ft)

TL-3 Guardrail Cost = \$18.16 per linear foot

Added Costs:

Traffic Control = 10%

Mobilization = 7.5%

Contingency = 15%

$$\text{Slope Starting Location} = \frac{SGL}{2} - \frac{SL}{2} = \frac{3281 \text{ ft}}{2} - \frac{150}{2} = 1565.4 \text{ ft}$$

$$\text{Length of Need (LON)} = \frac{L_C - OFF}{\frac{L_C}{L_R}} = \frac{19 \text{ ft} - 2 \text{ ft}}{19 \text{ ft}/195 \text{ ft}} = 174.5 \text{ ft}$$

$$W - \text{beam Length} = SL + LON \times 2 = 150 \text{ ft} + 174.5 \text{ ft} \times 2 = 498.9 \text{ ft}$$

$$\begin{aligned} \text{Barrier Starting Location} &= \text{Slope Starting Location} - LON = 1565.4 \text{ ft} - 174.5 \text{ ft} \\ &= 1390.9 \text{ ft} \end{aligned}$$

$$\begin{aligned} \text{Total Length of Barrier (TL)} &= SL + LON \times 2 + TL \times 2 = 150 \text{ ft} + 174.5 \text{ ft} \times 2 + 12.5 \times 2 \\ &= 523.9 \text{ ft} \end{aligned}$$

$$\text{Total Cost to Remove} = TL \times GRRC \times (1 + 0.1 + .075 + 0.15)$$

$$= 523.9 \text{ ft} \times \frac{\$5}{\text{lf}} \times (1 + 0.1 + .075 + 0.15) = \$3,471.15$$

Total Cost to Remove & Replace

$$\begin{aligned} &= \text{Total Cost to Remove} + ((TL - 37.5 \times 2) \times \$18.16 + \$2,100 \times 2) \\ &\times (1 + 0.1 + .075 + 0.15) = \$19,838.72 \end{aligned}$$

Table E-3. Guardrail Shielding Slope Modeling and Cost (English Units)

ADT	Slope Length (ft)	Lateral Offset (ft)	Slope Starting Location (ft)	Length of Need (ft)	W-beam Length (ft)	Barrier Starting Location (ft)	Terminal Length (ft)	Total Length of Barrier (ft)	Total Cost to Remove (USD)	Total Cost to Remove & Replace (USD)
500	150	2	1565.4	174.5	498.9	1390.9	12.5	523.9	\$3,471.15	\$19,838.72
		4	1565.4	153.9	457.9	1411.5	12.5	482.9	\$3,199.18	\$18,578.94
		7	1565.4	123.2	396.3	1442.3	12.5	421.3	\$2,791.22	\$16,689.27
	350	2	1465.4	174.5	698.9	1290.9	12.5	723.9	\$4,796.15	\$25,976.12
		4	1465.4	153.9	657.9	1311.5	12.5	682.9	\$4,524.18	\$24,716.34
		7	1465.4	123.2	596.3	1342.3	12.5	621.3	\$4,116.22	\$22,826.67
	650	2	1315.4	174.5	998.9	1140.9	12.5	1023.9	\$6,783.65	\$35,182.22
		4	1315.4	153.9	957.9	1161.5	12.5	982.9	\$6,511.68	\$33,922.44
		7	1315.4	123.2	896.3	1192.3	12.5	921.3	\$6,103.72	\$32,032.77
1,000	150	2	1565.4	202.4	554.8	1363.0	12.5	579.8	\$3,841.18	\$21,552.67
		4	1565.4	184.8	519.6	1380.6	12.5	544.6	\$3,607.98	\$20,472.49
		7	1565.4	158.4	466.8	1407.0	12.5	491.8	\$3,258.18	\$18,852.22
	350	2	1465.4	202.4	754.8	1263.0	12.5	779.8	\$5,166.18	\$27,690.07
		4	1465.4	184.8	719.6	1280.6	12.5	744.6	\$4,932.98	\$26,609.89
		7	1465.4	158.4	666.8	1307.0	12.5	691.8	\$4,583.18	\$24,989.62
	650	2	1315.4	202.4	1054.8	1113.0	12.5	1079.8	\$7,153.68	\$36,807.71
		4	1315.4	184.8	1019.6	1130.6	12.5	1044.6	\$6,920.48	\$35,788.67
		7	1315.4	158.4	966.8	1157.0	12.5	991.8	\$6,570.68	\$34,195.72
5,000	150	2	1565.4	238.0	626.0	1327.4	12.5	651.0	\$4,312.88	\$23,737.59
		4	1565.4	221.0	592.0	1344.4	12.5	617.0	\$4,087.63	\$22,694.23
		7	1565.4	195.5	541.0	1369.9	12.5	566.0	\$3,749.75	\$21,129.19
	350	2	1465.4	238.0	826.0	1227.4	12.5	851.0	\$5,637.88	\$29,874.99
		4	1465.4	221.0	792.0	1244.4	12.5	817.0	\$5,412.63	\$28,831.63
		7	1465.4	195.5	741.0	1269.9	12.5	766.0	\$5,074.75	\$27,266.59
	650	2	1315.4	238.0	1126.0	1077.4	12.5	1151.0	\$7,625.38	\$38,868.95
		4	1315.4	221.0	1092.0	1094.4	12.5	1117.0	\$7,400.13	\$37,884.65
		7	1315.4	195.5	1041.0	1119.9	12.5	1066.0	\$7,062.25	\$36,408.20
10,000	150	2	1565.4	239.1	628.1	1326.4	12.5	653.1	\$4,326.95	\$23,802.80
		4	1565.4	223.1	596.3	1342.3	12.5	621.3	\$4,115.78	\$22,824.65
		7	1565.4	199.2	548.4	1366.2	12.5	573.4	\$3,799.02	\$21,357.43
	350	2	1465.4	239.1	828.1	1226.4	12.5	853.1	\$5,651.95	\$29,940.20
		4	1465.4	223.1	796.3	1242.3	12.5	821.3	\$5,440.78	\$28,962.05
		7	1465.4	199.2	748.4	1266.2	12.5	773.4	\$5,124.02	\$27,494.83
	650	2	1315.4	239.1	1128.1	1076.4	12.5	1153.1	\$7,639.45	\$38,930.47
		4	1315.4	223.1	1096.3	1092.3	12.5	1121.3	\$7,428.28	\$38,007.69
		7	1315.4	199.2	1048.4	1116.2	12.5	1073.4	\$7,111.52	\$36,623.52
25,000	150	2	1565.4	290.6	731.3	1274.8	12.5	756.3	\$5,010.16	\$26,967.39
		4	1565.4	271.3	692.5	1294.2	12.5	717.5	\$4,753.44	\$25,778.27
		7	1565.4	242.2	634.4	1323.2	12.5	659.4	\$4,368.36	\$23,994.59
	350	2	1465.4	290.6	931.3	1174.8	12.5	956.3	\$6,335.16	\$33,104.79
		4	1465.4	271.3	892.5	1194.2	12.5	917.5	\$6,078.44	\$31,915.67
		7	1465.4	242.2	834.4	1223.2	12.5	859.4	\$5,693.36	\$30,131.99
	650	2	1315.4	290.6	1231.3	1024.8	12.5	1256.3	\$8,322.66	\$41,915.94
		4	1315.4	271.3	1192.5	1044.2	12.5	1217.5	\$8,065.94	\$40,794.13
		7	1315.4	242.2	1134.4	1073.2	12.5	1159.4	\$7,680.86	\$39,111.41

Table E-4. Guardrail Shielding Slope Modeling and Cost (Metric Units)

ADT	Slope Length (m)	Lateral Offset (m)	Slope Starting Location (m)	Length of Need (m)	W-beam Length (m)	Barrier Starting Location (m)	Terminal Length (m)	Total Length of Barrier (m)	Total Cost to Remove (USD)	Total Cost to Remove & Replace (USD)
500	45.7	0.6	477.1	53.2	152.1	424.0	3.8	159.7	\$3,471.15	\$19,838.72
		1.2	477.1	46.9	139.6	430.2	3.8	147.2	\$3,199.18	\$18,578.94
		2.1	477.1	37.5	120.8	439.6	3.8	128.4	\$2,791.22	\$16,689.27
	106.7	0.6	446.7	53.2	213.0	393.5	3.8	220.7	\$4,796.15	\$25,976.12
		1.2	446.7	46.9	200.5	399.7	3.8	208.1	\$4,524.18	\$24,716.34
		2.1	446.7	37.5	181.8	409.1	3.8	189.4	\$4,116.22	\$22,826.67
	198.1	0.6	400.9	53.2	304.5	347.8	3.8	312.1	\$6,783.65	\$35,182.22
		1.2	400.9	46.9	292.0	354.0	3.8	299.6	\$6,511.68	\$33,922.44
		2.1	400.9	37.5	273.2	363.4	3.8	280.8	\$6,103.72	\$32,032.77
1,000	45.7	0.6	477.1	61.7	169.1	415.4	3.8	176.7	\$3,841.18	\$21,552.67
		1.2	477.1	56.3	158.4	420.8	3.8	166.0	\$3,607.98	\$20,472.49
		2.1	477.1	48.3	142.3	428.9	3.8	149.9	\$3,258.18	\$18,852.22
	106.7	0.6	446.7	61.7	230.1	385.0	3.8	237.7	\$5,166.18	\$27,690.07
		1.2	446.7	56.3	219.3	390.3	3.8	227.0	\$4,932.98	\$26,609.89
		2.1	446.7	48.3	203.2	398.4	3.8	210.9	\$4,583.18	\$24,989.62
	198.1	0.6	400.9	61.7	321.5	339.2	3.8	329.1	\$7,153.68	\$36,807.71
		1.2	400.9	56.3	310.8	344.6	3.8	318.4	\$6,920.48	\$35,788.67
		2.1	400.9	48.3	294.7	352.7	3.8	302.3	\$6,570.68	\$34,195.72
5,000	45.7	0.6	477.1	72.5	190.8	404.6	3.8	198.4	\$4,312.88	\$23,737.59
		1.2	477.1	67.4	180.4	409.8	3.8	188.1	\$4,087.63	\$22,694.23
		2.1	477.1	59.6	164.9	417.6	3.8	172.5	\$3,749.75	\$21,129.19
	106.7	0.6	446.7	72.5	251.8	374.1	3.8	259.4	\$5,637.88	\$29,874.99
		1.2	446.7	67.4	241.4	379.3	3.8	249.0	\$5,412.63	\$28,831.63
		2.1	446.7	59.6	225.9	387.1	3.8	233.5	\$5,074.75	\$27,266.59
	198.1	0.6	400.9	72.5	343.2	328.4	3.8	350.8	\$7,625.38	\$38,868.95
		1.2	400.9	67.4	332.8	333.6	3.8	340.5	\$7,400.13	\$37,884.65
		2.1	400.9	59.6	317.3	341.4	3.8	324.9	\$7,062.25	\$36,408.20
10,000	45.7	0.6	477.1	72.9	191.5	404.3	3.8	199.1	\$4,326.95	\$23,802.80
		1.2	477.1	68.0	181.7	409.1	3.8	189.4	\$4,115.78	\$22,824.65
		2.1	477.1	60.7	167.2	416.4	3.8	174.8	\$3,799.02	\$21,357.43
	106.7	0.6	446.7	72.9	252.4	373.8	3.8	260.0	\$5,651.95	\$29,940.20
		1.2	446.7	68.0	242.7	378.7	3.8	250.3	\$5,440.78	\$28,962.05
		2.1	446.7	60.7	228.1	385.9	3.8	235.7	\$5,124.02	\$27,494.83
	198.1	0.6	400.9	72.9	343.9	328.1	3.8	351.5	\$7,639.45	\$38,930.47
		1.2	400.9	68.0	334.1	332.9	3.8	341.8	\$7,428.28	\$38,007.69
		2.1	400.9	60.7	319.6	340.2	3.8	327.2	\$7,111.52	\$36,623.52
25,000	45.7	0.6	477.1	88.6	222.9	388.6	3.8	230.5	\$5,010.16	\$26,967.39
		1.2	477.1	82.7	211.1	394.5	3.8	218.7	\$4,753.44	\$25,778.27
		2.1	477.1	73.8	193.4	403.3	3.8	201.0	\$4,368.36	\$23,994.59
	106.7	0.6	446.7	88.6	283.8	358.1	3.8	291.5	\$6,335.16	\$33,104.79
		1.2	446.7	82.7	272.0	364.0	3.8	279.7	\$6,078.44	\$31,915.67
		2.1	446.7	73.8	254.3	372.8	3.8	261.9	\$5,693.36	\$30,131.99
	198.1	0.6	400.9	88.6	375.3	312.4	3.8	382.9	\$8,322.66	\$41,915.94
		1.2	400.9	82.7	363.5	318.3	3.8	371.1	\$8,065.94	\$40,794.13
		2.1	400.9	73.8	345.8	327.1	3.8	353.4	\$7,680.86	\$39,111.41

Appendix F. Guidelines for Existing W-Beam Guardrail Shielding Slopes

Table F-1. 22-in. Tall W-beam with Blunt-End Shielding Slope (B/C=2:1)

22 in. (559 mm) W-beam with Blunt End Terminal Shielding a Slope on Straight Road Segment (B/C 2:1)												
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length			
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1	
7 ft [2.1 m]	2 ft [0.6 m]	500	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		1,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		5,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		10,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		25,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
	4 ft [1.2 m]	500	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		1,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		5,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		25,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
7 ft [2.1 m]	7 ft [2.1 m]	500	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		1,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		5,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		10,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		25,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
13 ft [4.0 m]	2 ft [0.6 m]	500	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		1,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		5,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		10,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		25,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
	4 ft [1.2 m]	4 ft [1.2 m]	500	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light
			1,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light
			5,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light
			25,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light
7 ft [2.1 m]	7 ft [2.1 m]	500	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		1,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		5,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		10,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		25,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
20 ft [6.1 m]	2 ft [0.6 m]	500	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		1,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		5,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		10,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		25,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
	4 ft [1.2 m]	4 ft [1.2 m]	500	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light
			1,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light
			5,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light
			25,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light
7 ft [2.1 m]	7 ft [2.1 m]	500	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		1,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		5,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		10,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	
		25,000	Dark	Dark	Light	Dark	Dark	Light	Dark	Dark	Light	

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-2. 22-in.Tall W-beam with Blunt-End Shielding Slope (B/C=4:1)

22 in. (559 mm) W-beam with Blunt End Terminal Shielding a Slope on Straight Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-3. 22-in. Tall W-beam with Blunt-End on Curve Shielding Slope (B/C=2:1)

22 in. (559 mm) W-beam with Blunt End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-4. 22-in. Tall W-beam with Blunt-End on Curve Shielding Slope (B/C=4:1)

22 in. (559 mm) W-beam with Blunt End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000	■			■			■		
		5,000	■			■			■		
		10,000	■			■			■		
		25,000	■			■			■		
	4 ft [1.2 m]	500									
		1,000	■			■			■		
		25,000	■			■			■		
	7 ft [2.1 m]	500									
1,000		■			■			■			
25,000		■			■			■			
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000	■			■			■		
		5,000	■			■			■		
		10,000	■			■			■		
		25,000	■			■			■		
	4 ft [1.2 m]	500									
		1,000	■			■			■		
		25,000	■			■			■		
	7 ft [2.1 m]	500									
1,000		■			■			■			
25,000		■			■			■			
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000	■			■			■		
		5,000	■			■			■		
		10,000	■			■			■		
		25,000	■			■			■		
	4 ft [1.2 m]	500									
		1,000	■			■			■		
		25,000	■			■			■		
	7 ft [2.1 m]	500									
1,000		■			■			■			
25,000		■			■			■			

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-5. 22-in. Tall W-beam with Turned-Down Shielding Slope (B/C=2:1)

22 in. (559 mm) W-beam with Turned Down End Terminal Shielding a Slope on Straight Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-6. 22-in. Tall W-beam with Turned-Down Shielding Slope (B/C=4:1)

22 in. (559 mm) W-beam with Turned Down End Terminal Shielding a Slope on Straight Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-7. 22-in. Tall W-beam with Turned-Down on Curve Shielding Slope (B/C=2:1)

22 in. (559 mm) W-beam with Turned Down End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-8. 22-in. Tall W-beam with Turned-Down on Curve Shielding Slope (B/C=4:1)

22 in. (559 mm) W-beam with Turned Down End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-9. 25-in. Tall W-beam with Blunt-End Shielding Slope (B/C=2:1)

25 in. (635 mm) W-beam with Blunt End Terminal Shielding a Slope on Straight Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-10. 25-in. Tall W-beam with Blunt-End Shielding Slope (B/C=4:1)

25 in. (635 mm) W-beam with Blunt End Terminal Shielding a Slope on Straight Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-11. 25-in. Tall W-beam with Blunt-End on Curve Shielding Slope (B/C=2:1)

25 in. (635 mm) W-beam with Blunt End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-12. 25-in. Tall W-beam with Blunt-End on Curve Shielding Slope (B/C=4:1)

25 in. (635 mm) W-beam with Blunt End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-13. 25-in. Tall W-beam with Turned-Down Shielding Slope (B/C=2:1)

25 in. (635 mm) W-beam with Turned Down End Terminal Shielding a Slope on Straight Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-14. 25-in. Tall W-beam with Turned-Down Shielding Slope (B/C=4:1)

25 in. (635 mm) W-beam with Turned Down End Terminal Shielding a Slope on Straight Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-15. 25-in. Tall W-beam with Turned-Down on Curve Shielding Slope (B/C=2:1)

25 in. (635 mm) W-beam with Turned Down End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	7 ft [2.1 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-16. 25-in. Tall W-beam with Turned-Down on Curve Shielding Slope (B/C=4:1)

25 in. (635 mm) W-beam with Turned Down End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-17. 27-in. Tall W-beam with Blunt-End Shielding Slope (B/C=2:1)

27 in. (686 mm) W-beam with Blunt End Terminal Shielding a Slope on Straight Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-18. 27-in. Tall W-beam with Blunt-End Shielding Slope (B/C=4:1)

27 in. (686 mm) W-beam with Blunt End Terminal Shielding a Slope on Straight Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-19. 27-in. Tall W-beam with Blunt-End on Curve Shielding Slope (B/C=2:1)

27 in. (686 mm) W-beam with Blunt End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-20. 27-in. Tall W-beam with Blunt-End on Curve Shielding Slope (B/C=4:1)

27 in. (686 mm) W-beam with Blunt End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-21. 27-in. Tall W-beam with Turned-Down Shielding Slope (B/C=2:1)

27 in. (686 mm) W-beam with Turned Down End Terminal Shielding a Slope on Straight Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-22. 27-in. Tall W-beam with Turned-Down Shielding Slope (B/C=4:1)

27 in. (686 mm) W-beam with Turned Down End Terminal Shielding a Slope on Straight Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-23. 27-in. Tall W-beam with Turned-Down on Curve Shielding Slope (B/C=2:1)

27 in. (686 mm) W-beam with Turned Down End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 2:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000									
		10,000									
		25,000									
	4 ft [1.2 m]	500									
		1,000									
		5,000									
		10,000									
7 ft [2.1 m]	500										
	1,000										
	5,000										
	10,000										
	25,000										

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

Table F-24. 27-in. Tall W-beam with Turned-Down on Curve Shielding Slope (B/C=4:1)

27 in. (686 mm) W-beam with Turned Down End Terminal Shielding a Slope on 5° Curved Road Segment (B/C 4:1)											
Drop Height	Offset	ADT	150 ft [46 m] Slope Length			350 ft [107 m] Slope Length			650 ft [198 m] Slope Length		
			1.5:1	2:1	3:1	1.5:1	2:1	3:1	1.5:1	2:1	3:1
7 ft [2.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000	■	■	■	■	■	■	■	■	■
		10,000	■	■	■	■	■	■	■	■	■
		25,000	■	■	■	■	■	■	■	■	■
	4 ft [1.2 m]	500									
		1,000									
		5,000	■	■	■	■	■	■	■	■	■
		10,000	■	■	■	■	■	■	■	■	■
7 ft [2.1 m]	500										
	1,000										
	5,000	■	■	■	■	■	■	■	■	■	
	10,000	■	■	■	■	■	■	■	■	■	
	25,000	■	■	■	■	■	■	■	■	■	
13 ft [4.0 m]	2 ft [0.6 m]	500									
		1,000									
		5,000	■	■	■	■	■	■	■	■	■
		10,000	■	■	■	■	■	■	■	■	■
		25,000	■	■	■	■	■	■	■	■	■
	4 ft [1.2 m]	500									
		1,000									
		5,000	■	■	■	■	■	■	■	■	■
		10,000	■	■	■	■	■	■	■	■	■
7 ft [2.1 m]	500										
	1,000										
	5,000	■	■	■	■	■	■	■	■	■	
	10,000	■	■	■	■	■	■	■	■	■	
	25,000	■	■	■	■	■	■	■	■	■	
20 ft [6.1 m]	2 ft [0.6 m]	500									
		1,000									
		5,000	■	■	■	■	■	■	■	■	■
		10,000	■	■	■	■	■	■	■	■	■
		25,000	■	■	■	■	■	■	■	■	■
	4 ft [1.2 m]	500									
		1,000									
		5,000	■	■	■	■	■	■	■	■	■
		10,000	■	■	■	■	■	■	■	■	■
7 ft [2.1 m]	500										
	1,000										
	5,000	■	■	■	■	■	■	■	■	■	
	10,000	■	■	■	■	■	■	■	■	■	
	25,000	■	■	■	■	■	■	■	■	■	

	Do Nothing
	Remove deficient system
	Remove deficient system and install 31" tall W-beam guardrail with a crashworthy end treatment

END OF DOCUMENT