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Investigation of Asphalt Pavement Analyzer Testing Program in Nebraska

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16. Abstract

The asphalt pavement analyzer (APA) has been widely used to evaluate hot-mix asphalt (HMA) rutting potential in mix design and quality control – quality assurance (QC-QA) applications, because the APA testing and its data analyses are relatively simple, rapid, and easy. However, as demonstrated in many studies and also experienced by the state of Nebraska, the APA testing is in question due to its high testing variability and a lack of sufficient correlation with actual filed performance. The primary objective of this research was to find critical materials and/or mixture design factors affecting APA test results so as to eventually improve current APA testing program in Nebraska. In addition to that, development of models to predict APA rut performance with given properties of HMA mixture ingredients and mixture design characteristics were also attempted. To find variables affecting APA rut results and the extent of these variables, SP-4 mixture data from Nebraska and HMA mixture data from Kentucky were statistically analyzed using the multiple linear regression method considering six factors (binder PG, aggregate gradation, nominal maximum aggregate size, aggregate angularity, air voids in mixture, and asphalt content in mixture) as probable candidates significantly affecting APA rut results. For a detailed characterization of gradation effects, three indicators (gradation density, fineness modulus, and restricted zone) were considered and each of them was used for each statistical analysis. Analyses results demonstrated that the binder PG was the only variable that always shows significant impact on APA rut results, which is in a good agreement with other studies. Predicting models developed through the results of multiple linear regression analysis and the artificial neural network technique presented a relatively low level of model adequacy which can be observed by the coefficients of determination and crossplots between predicted APA rut values and the measured APA rut data. More data would be helpful to confirm the findings from this research and also to develop a better prediction model.

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CHAPTER 1

INTRODUCTION

The asphalt pavement analyzer (APA) has been widely used in many states as a straightforward method to evaluate hot-mix asphalt (HMA) rutting potential in mix design and quality control and quality assurance (QC/QA) applications. The APA is more advantageous than other testing methods that have been proposed from Strategic Highway Research Program (SHRP) studies, because the APA testing and resulting data analyses are relatively simple, rapid and easy to perform. However, as demonstrated in many studies including the national study, NCHRP 9-17 (Kandhal and Cooley, 2003), the APA testing is in question due to its high testing variability and a lack of sufficient correlation with actual field performance. Nebraska Department of Roads (NDOR) has employed the APA testing for several years as a supplemental tool to validate and evaluate rutting potential of Superpave mixes paved in Nebraska. NDOR typically found that the APA testing data were not sufficiently reliable to judge rutting characteristics of a Superpave mix because of the high testing variability and poor correlations with actual field performance. In many cases, APA rut depth monitored from a specific Superpave mix was not consistent with rut depths from other mixes within the same mix design criteria. Due to this fact, most state highway agencies have tried to find problems and solutions associated with current APA techniques, so that they can reach the level of confidence needed to utilize the simple APA testing to accept or reject HMA mixtures. Nebraska has also accumulated, but not fully investigated yet, APA testing data of each different mixture that has been paved and in service. Therefore, there is a pressing need for careful investigations of APA testing and resulting data to better understand why the high testing variability from specimens within the same mix design criteria has been observed.

1.1 RESEARCH OBJECTIVES

The primary objective of this research was to seek better understanding and potential improvements of the current APA testing program incorporated with Superpave specifications implemented in Nebraska. Comprehensive literature review including careful investigations of APA data available from both Nebraska (approximately 4-year data) and other states was

conducted to find critical factors affecting APA test results and to monitor sensitivity of APA results with mix design variables. This can minimize currently-observed high testing variability. In addition, development of models to predict APA rut performance with given properties of HMA mixture ingredients and mixture design characteristics was targeted. Successful research provides a form of guidelines that direct appropriate scope in use of APA techniques as a potential rutting performance-predicting indicator.

1.2 RESEARCH SCOPE

To meet the research goals, this report was performed with three phases. Phase 1 consisted of literature survey to review significant findings from other studies investigating the variability of APA test results and sensitivity of APA test results to mixture characteristics. Based on findings from the literature review, Phase 2 employed statistical approaches to determine which factors of mixture characteristics affect the APA rut performance with a high level of sensitivity. APA test data from Nebraska and another state, Kentucky were obtained and used to conduct the statistical sensitivity analysis. In Phase 3, prediction models were developed using multiple linear regression analysis and artificial neural network technique. Predicted and measured values were compared with both methods.

1.3 ORGANIZATION OF THE REPORT

This report is composed of five chapters. Following this introduction, Chapter 2 briefly summarizes findings from several other studies investigating the effects of HMA mixture and material characteristics on APA rut test results. In Chapter 3, detailed descriptions of APA data acquisition and implementation to conduct the statistical analysis, which has been employed for this study, are presented. Chapter 4 presents statistical analysis results of significant factors that affect APA rut results and the prediction models developed through the multiple linear regression analysis and the artificial neural network technique. Finally, Chapter 5 provides a summary of the findings, recommended future research, and implementation plans to the NDOR of this study.

CHAPTER 2

LITERRATURE REVIEW

To seek better understanding of current APA testing program which typically is in question due to its high testing variability, literature reviews have been conducted by primarily targeting to investigate the effects of HMA mixture and material characteristics on rut test results from APA as well as other traditional wheel-loading testers. This chapter briefly introduces some significant findings from several studies where the relationship between HMA rutting performance and related materials and/or mixture design factors has been investigated.

Kandhal and Cooley (2003) performed a National Cooperative Highway Research Program (NCHRP) study, NCHRP 9-17 project, with two objectives: to identify test conditions within the APA that produced results most related to field rutting performance and to validate the proposed APA test method as an appropriate QC/QA rut predicting tool. They selected 10 HMA mixes of known rutting performance to determine the combination of testing conditions for the APA that best predicted field rutting. These 10 mixes were selected from three full-scale pavement research projects: WesTrack (Nevada), the Minnesota Road Research (MnRoad), and the FHWA Accelerated Loading Facility (ALF) at Turner-Fairbank Highway Research Center (Virginia). Numerous APA tests were performed under different testing conditions by varying specimen geometry, APA loading hose diameter, test temperature, and air void content in mixtures. APA test results were then analyzed and correlated to field performance data. Statistical analysis results demonstrated that 5-percent air voids were more closely related to field rutting performance than 7-percent air voids, and specimens tested at a temperature corresponding to the high temperature of the standard performance grade (PG) better predicted field rutting performance than at 6°C higher of the high temperature of the standard PG. Loading hose diameter and the specimen geometry (cylinder vs. beam) did not show any significant dependency on APA rutting performance. Kandhal and Cooley also tried to validate the proposed APA test method, and they concluded that laboratory rut depths measured by the APA generally showed good correlations on individual project basis. However, there was a clear dependency of the APA results on geographic locations and traffic level.

Related to this research, some useful findings from the NCHRP study were found and are presented in Table 2-1 and Figure 2-1. Table 2-1 shows WesTrack results indicating that the high value of air voids (7%) was generally more susceptible to rutting than low air voids (4%). It is obvious that the testing temperature affects APA rut performance. Specimens were better rut-resistant at the lower temperature (64°C) than at the temperature corresponding to the 6°C higher (i.e., 70°C) of the high temperature of the standard PG. Figure 2-1 presents the effect of nominal maximum aggregate size (NMAS) on rut depths. The figure demonstrates that the larger NMAS mixtures (such as the one with 37.5-mm) had a lower laboratory rut depth than the mixtures designed with a smaller NMAS (19-mm). Therefore, the use of larger NMAS can reduce rut susceptibility.

Table 2-1. Average Rut Depths for WesTrack Sections (NCHRP 508, 2003)

Air Voids	Test Temp, °C	Hose Diameter	Specimen Type	Rut Depth @ 10,000 cycles, mm		
				Section 15	Section 19	Section 24
			Cylinder	10.13	6.12	8.17
		Standard	Beam	8.52	9.14	8.70
			Cylinder	8.30	4.19	4.05
	64 (PG)	Larger	Beam	6.46	7.99	6.39
			Cylinder	12.78	8.61	10.56
		Standard	Beam	12.65	11.14	11.69
			Cylinder	12.01	7.22	4.57
7%	70 (PG+6)	Larger	Beam	9.33	6.21	8.40
			Cylinder	6.64	7.76	8.27
		Standard	Beam	6.08	10.84	13.33
			Cylinder	6.36	5.35	7.54
	64 (PG)	Larger	Beam	5.10	5.76	7.08
			Cylinder	8.81	8.80	9.29
		Standard	Beam	11.57	13.07	14.88
			Cylinder	7.83	6.10	5.47
4%*	70 (PG+6)	Larger	Beam	5.64	6.87	5.24

^{*}Beam samples compacted to 5.0 ± 0.5% air voids.

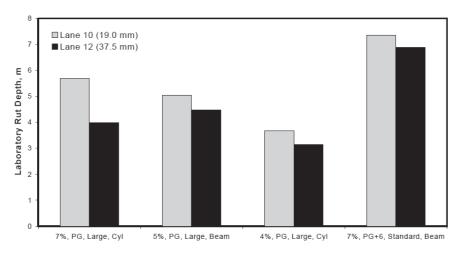
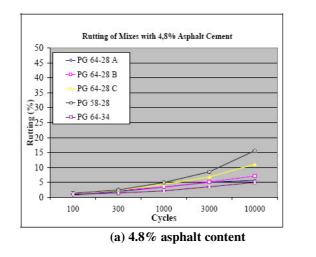


Figure 2-1. Effect of Nominal Maximum Aggregate Size on Rutting (NCHRP 508, 2003)

Uzarowski *et al.* (2004) tested accelerated performance of Canadian asphalt mixes using three different wheel rut testers: French laboratory tester, Hamburg wheel tester and APA. They controlled asphalt content, binder PG, and cycles of wheel on each testing. Despite the different methods, the test results revealed similar patterns. First of all, every test showed good correlation between field observation and testing results. Also, the high level of asphalt content (5.6%) was more susceptible to rutting performance than the low asphalt contents (4.8%). Figure 2-2 shows how the modified binder resisted to rutting better than neat binder using the French laboratory wheel-loading tester.



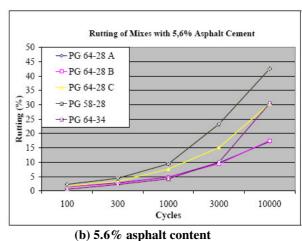


Figure 2-2. Relationship between Binder PG and Rutting at Different Asphalt Content (Uzarowski *et al.*, 2004)

Cross and Purcell (2001) investigated effects of fine aggregate angularity (FAA) on voids in mineral aggregates (VMA) and rutting in Kansas HMA mixtures. For the evaluation, two gradations (coarse and fine) using 100% crushed limestone were used. To change the FAA of the mix, natural sand and chat were used instead of crushed limestone. Among the three materials, limestone and natural sand showed an increasing trend of FAA with increasing mixture VMA. However, chat did not show the same increasing pattern. Increasing the FAA resulted in less rutting in the limestone and natural sand mixtures, but there was no clear relationship between the FAA of the chat mixes and rutting.

Lee *et al.* (1999) found relationship between FAA and asphalt mixture performance. They used PURWheel designed by Purdue University to evaluate HMA rut potential at different FAA values. They indicated that the specimen with the high value of FAA showed less susceptibility than the low value in rutting. The mixture with a FAA value of 45 performed better than the others.

Stiady *et al.* (2001) studied effects of aggregate properties (NMAS), coarse aggregate type (granite and limestone), fine aggregate angularity, and gradation types using PURWheel. As shown in Figure 2-3, there was a significant relationship between FAA and permanent deformation, but a FAA value too high (greater than 45) did not show better performance in the mixtures. They also concluded that a NMAS of 9.5-mm and 19-mm had no difference statistically.

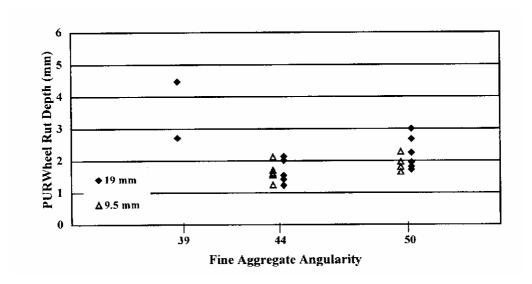


Figure 2-3. PURWheel Rut Depths at Different FAA Values (Stiady et al., 2001)

Kandhal and Mallick (2001) evaluated APA testing for HMA mixture design by using the test data and statistical method. The focus of their study was to find the effect of mix gradations on HMA rutting performance. They used three aggregates (granite, limestone, and gravel) and three types of aggregate gradations: above-restricted zone (RZ), through-RZ, and below-RZ. Permanent deformation was significantly affected by the gradation and the type of the aggregate as shown in Table 2-2. Analysis of variance (ANOVA) indicates the significant effect of aggregate type, gradation, and course type as well as interaction of aggregate and gradation.

Table 2-2. Analysis of Variance for Rut Depths of Mixes (Kandhal and Mallick, 2001)

Source	DF	Mean Square	F Value	Pr>F
Aggregate	2	51.59	45.64	0.0001
Asphalt	1	34.96	30.94	0.0001
Gradation	2	24.35	21.54	0.0001
Course	1	57.56	50.92	0.0001
Aggregate*Gradation	4	33.79	29.90	0.0001

Tarefder *et al.* (2003) attempted to identify the most significant factors which have been known to affect rut potential of HMA mixture using APA testing. They tested three sets (set A, B, and

C) with seven factors (binder PG, temperature, load, hose pressure, asphalt content, moisture in test specimen, and type of specimen), six factors (gradation, temperature, load, hose pressure, asphalt content, and moisture in test specimen), and five factors (gradation, temperature, load, hose pressure, and moisture in test specimen) and analyzed them using the statistical method. Table 2-3 summarizes the results of ANOVA indicating that binder's PG, testing temperature, moisture in specimen, and aggregate gradation were commonly observed factors affecting mixtures' APA rutting performance significantly. Wheel load, asphalt content and loading hose pressure were less significant.

Table 2-3. Statistical Results (Tarefder *et al.*, 2003)

Test	Factor	$\mathrm{df}_x = (n_x - 1)$	SS_x	Variance, $V_x = \frac{SS_x}{df_x}$	$F_x = \frac{V_x}{V_{cor}}$ (Statistics)	$F_{\text{Table}} = F(1,3)_{0.05}$	SS'_x	% Contribution, P_x
Set A	PG	1	20.9	20.7	102.6	10.1	20.3	58.5
	Specimen	1	7.3	7.3	36.1	10.1	6.7	19.4
	Temperature	1	2.9	2.9	14.1	10.1	2.3	6.5
	Moisture	1	1.9	1.9	9.5	10.1	1.3	3.8
	Error	3	0.6	0.2				
	Total	7	34.6	5.0				88.2
Set B	Temperature	1	7.4	7.4	21.5	10.1	7.1	25.2
	Gradation	1	7.0	7.0	20.4	10.1	6.7	23.8
	AC	1	5.0	5.0	14.4	10.1	4.6	16.4
	Moisture	1	3.8	3.8	11.0	10.1	3.4	12.2
	Error	2	0.7	0.4				
	Total	6	28.1	5.6				77.6
Set C	Temperature	1	28.9	28.8	22.1	18.5	27.6	47.6
	Gradation	1	18.0	18.0	13.8	18.5	16.7	28.8
	Moisture	1	5.5	5.5	4.2	18.5	4.1	7.2
	Error	2	2.6	1.3				
	Total	5	57.9	11.6				83.6

Mohammad *et al.* (2001) evaluated aggregate contributions to rutting susceptibility of asphalt mixtures. Three types of mixtures were used in the research: stone mastic asphalt (SMA), coarse matrix high binder (CMHB) and dense-graded wearing course. Three types of aggregate (siliceous limestone, sandstone, and novaculite) were used in SMA, and crushed limestone was used in the other mixtures. PG 70-22M modified binder was used in all the mixtures. SMA made of sandstone aggregate was the best-performing mixture, and the dense-graded and the CMHB generally showed better performance in rutting performance than the other SMA mixtures.

More recently, Shu, *et al.* (2006) investigated the effects of coarse aggregate angularity (CAA) and binder PG grade on rutting performance of HMA mixtures. APA was used to evaluate the rut depth of mixtures. Two types of binder performance grade (64-22 and 76-22) with varying CAA values were investigated. The test results showed that CAA significantly affected rutting performance of HMA mixtures when the binder grade was critical to the environment.

CHAPTER 3

RESEARCH METHODOLOGY

To accomplish the research objectives, the statistical method based on the multiple linear regression analysis was selected for this study. NDOR has performed APA testing for four years and accumulated testing data. Using APA data, it was possible to identify materials and/or mixture design factors affecting APA rutting and the extent that each factor affects APA rutting in HMA through statistical analysis. Among many advantages of the use of statistical approaches, one is easy adaptation of the same approach to other available data. A successfully developed statistical approach for a set of data from the state of Nebraska can be directly applied to other data obtained from another state, Kentucky. Another advantage of the statistical approach is that this method requires much less time and costs than other methods such as laboratory testing.

The multiple linear regression analysis selected to identify factors significantly affecting APA rut results can also provide prediction models relating the APA rut depth to materials and/or mixture design factors considered. The APA rut values predicted by the multiple linear regression technique were then compared to values from another technique, the artificial neural network, which has been widely employed in developing prediction models with many variables. This chapter briefly explains the statistical method (multiple linear regression analysis) and the artificial neural network technique that were employed for this research. After the brief introduction to the multiple linear regression analysis and the artificial neural network technique, target APA data selected for this study are presented. The type of Superpave mix and materials and/or mixture design factors to be considered for the analyses are determined and also presented in this chapter.

3.1 MULTIPLE LINEAR REGRESSION ANALYSIS

Linear regression is defined as the method which finds the statistical model that defines the experimental data (Draper and Smith, 1998). The model consists of one independent variable and one dependent variable in simple linear regression. Simple linear regression analysis is a

method to find a relationship between these two variables. The multiple linear regression analysis is used when experimental data has several independent variables. The general purpose of multiple linear regression analysis is to investigate the linear relationship between several independent (or predictor) variables and a dependent (or response) variable.

The regression result is presented through the analysis of variance (ANOVA) table. ANOVA is a tool expressing test results based on the F-ratio which is defined as a test of standard deviation of populations. A typical format and entities in an ANOVA table from the multiple linear regression analysis with n number of data and p number of independent variables in the model is presented in Table 3-1.

Table 3-1. ANOVA Table from the Multiple Linear Regression Analysis

Source	Degree of Freedom	Sum of Squares	Mean Square (MS)	F-Ratio
	(\mathbf{DF})	(SS)		
Regression Model	p	SSR^1	$MSR^4 = SSR/p$	MSR/MSE
Error	n-p-1	SSE^2	$MSE^5 = SSE/(n-p-1)$	
Total	<i>n</i> -1	$SSTO^3$		

Note:

 SSR^1 = regression sum of squares

 SSE^2 = error sum of squares

 $SSTO^3$ = total sum of squares

 MSR^4 = mean square due to regression, and

 MSE^5 = mean square due to error.

The components in the regression sum-of-squares (SSR, SSE, and SSTO) in the third column can be defined as follows:

$$SSR = \sum_{i=1}^{n} (\hat{Y}_i - \overline{Y})^2$$
 [3.1]

$$SSE = \sum_{i=1}^{n} (Y_i - \hat{Y}_i)^2 = \sum_{i=1}^{n} e_i^2$$
 [3.2]

$$SSTO = \sum_{i=1}^{n} (Y_i - \overline{Y})^2$$
 [3.3]

where Y_i = observed values,

 \hat{Y}_i = fitted values,

 \overline{Y} = mean of fitted values, and

 e_i = residuals.

The mean square due to error (MSE) and the mean square due to regression (MSR) are given in the fourth column of the ANOVA table. The F-ratio in the fifth column is simply calculated by dividing MSR by MSE and provides a statistic for testing whether or not the independent variables explain some of the variation in the response variable (dependent variable). The significance of the test results is justified by comparing the F-ratio to a significance level (α) which is typically equal to either 0.01, 0.05, or 0.10. The significance level is decided by the statistician who performs the analysis. When the calculated F-ratio is equal or greater than $F(\alpha; p, n-p-1)$, it means that there is at least one independent variable that explains the variation in the dependent variable. Alternatively, many statistical software calculate the F-value = Probability $[F(p, n-p-1) \ge F]$ where F is the calculated F-ratio. If this F-value is small (less than the significance level α), one can conclude that with the data there is sufficient evidence to say that at least one independent variable contributes to the variation in the dependent variable. Thus, the resulting model from the multiple linear regression analysis is considered a significant model where a meaningful relationship between a dependent variable and independent variables exists.

As an example, an ANOVA table resulting from a real multiple regression analysis was produced and is shown in Table 3-2. With α level of 0.05 selected, the ANOVA results indicate that there exists at least one independent variable that contributes to variation in the dependent variable. There is a significant relationship between variables, because the P-value (i.e., $\Pr > F$ as presented in the table) is less than the specified significance level (α value), 0.05.

Table 3-2. ANOVA Table Resulting from a Real Multiple Regression Analysis

Analysis of Variance

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	9	0,00019450	0,00002161	2,39	0,0188
Error	81	0,00073324	0,00000905		
Corrected Total	90	0,00092774			

If the testing analysis is significant, it is now able to produce a multiple linear regression model relating variables. Equation [3-4] is the typical form of the model produced from the multiple linear regression analysis.

$$Y_{i} = \beta_{o} + \beta_{1}X_{i1} + \beta_{2}X_{i2} + ... + \beta_{p}X_{ip} + \varepsilon_{i}$$
 $i = 1, 2, ..., n$ [3-4]

where Y_i = dependent variable,

 β_o = intercept,

 β_p = parameters (coefficients) of independent variables,

 X_{ip} = independent variables,

p = total number of independent variables,

n = total number of data, and

 $\varepsilon_i = \text{error}.$

Table 3-3 presents typical results from the multiple linear regression analysis. The table shows parameter estimate (β_p) of each variable (X_{ip}) and its level of significance based on the t-ratio values. Similar to the F-ratio, the t-ratio is used to assess the significance of individual regression coefficient (β_p) multiplied to each variable in the model. In the case of t-test, statistical software calculate the P-value = 2*Probability $[t(n-p-1) \ge |t|]$ where t is the computed value of the t-statistic, with the significance level, α . If the P-value is less than or equal to α , one can conclude that the corresponding independent variable (X_{ip}) has a significant impact on the response. These t-tests are also called partial t-tests, since they assess the partial (or

additional) significance of the variables X_{ip} , over and above the impact of all other variables in the model. The sign of each parameter estimate in Table 3-3 indicates the trend of relationship between Y_i and X_{ip} . A positive sign in a parameter estimate infers proportionality between the dependent variable and the independent variable corresponding to the parameter estimate. The parameters shown in Table 3-3 identify the prediction model (Equation [3-4]).

Table 3-3. Parameter Estimate from the Multiple Linear Regression Analysis

Parameter Estimates

Variable	Label	DF	Parameter Estimate	Standard Error	t Value	Pr > [t]
Intercept NMA PG airvoid Binder	Intercept NMA PG airvoid Binder	1 1 1 1	0,05868 -0,00312 -0,00191 -0,00090993 -0,00071804	0,02352 0,00133 0,00059569 0,00060056 0.00103	2,49 -2,34 -3,20 -1,52 -0,70	0,0146 0,0216 0,0020 0,1336 0,4867

Regression analysis typically provides a measure of the strength of the relationship between dependent and independent variables. One measure that has been widely used to quantify the strength of the relationship is called the coefficient of correlation, R-value. The R-value lies between -1 and +1, therefore R^2 -value (called coefficient of determination) is more frequently used to give the proportion of the total variability in the dependent variable that is accounted for by the independent variables. The R^2 can be calculated by the following expression:

$$R^2 = 1 - \frac{SSE}{SSTO}$$
 [3-5]

If the \mathbb{R}^2 is equal to 1.0, all measured values are predicted by the regression model. In other words, the developed model explains the relationship among variables perfectly. On the other hand, \mathbb{R}^2 value of zero indicates that no measured data agrees with the prediction model.

Since \mathbb{R}^2 -value usually can be made larger by including a large number of predictor variables, it is sometimes suggested that a modified measure be used that adjusts for the number of independent variables in the model. The adjusted coefficient of determination, denoted by adj. \mathbb{R}^2 , modifies \mathbb{R}^2 value by dividing each sum of squares by its associated degrees of freedom:

$$adj.R^{2} = 1 - \left(\frac{n-1}{n-p-1}\right) \frac{SSE}{SSTO}$$
 [3-6]

As the number of independent variables increases, the value of \mathbb{R}^2 also increases. However, the adj. \mathbb{R}^2 may actually become smaller when another independent variable is introduced into the model, because any decrease in SSE may be more than that is offset by the loss of a degree of freedom in the denominator (n-p) in Equation [3-6]. Therefore, the adj. \mathbb{R}^2 has been known as a better indicator than the \mathbb{R}^2 -value to measure the strength of the relationship between variables. As an example, the values of \mathbb{R}^2 and adj. \mathbb{R}^2 from the multiple linear regression analysis are presented in Table 3-4.

Table 3-4. R^2 and adj. R^2 Values from the Multiple Linear Regression Analysis

Root MSE	0,00301	R-Square	0,2096
Dependent Mean	0,00255	Adj R-Sq	0,1218
Coeff Var	118,12363		

3.2 ARTIFICIAL NEURAL NETWORK APPROACH

The artificial neural network is a kind of statistical, mathematical, or computational methodology. It was developed from inspiration of biological neurons in the human brain. McCulloch and Pits (1943) first introduced the concept of artificial neurons. The neural network concept gained popularity after the development of inexpensive computer emulation. Currently, various fields such as finance, medicine, environmental science, and transportation engineering often use the artificial neural network. The advantages of the artificial neural network approach are that it

detects trends from complicated data and it can also do prediction and forecasting. Figure 3-1 shows the basic structure of the artificial neural network approach. The components of the neural network are input variables, one or more output variables, and one or more hidden layers relating input and output variables through networking. The input variables are transformed by a special function such as a logistic or sigmoidal function to account for nonlinearity in the model.

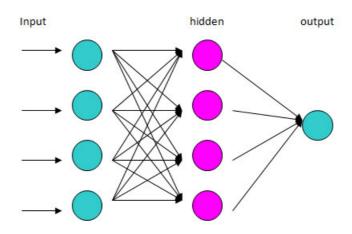


Figure 3-1. Basic Structure of the Artificial Neural Network

Among several artificial neural network algorithms, the back-propagation algorithm was adopted in this study because it has been widely used for prediction. The back-propagation algorithm was first introduced by Rumelhart *et al.* (1986). The back-propagation paradigm usually uses a sigmoidal function for transformation from linearity to nonlinearity.

3.3 APA DATA FOR ANALYSES

To accomplish the research objectives, the APA data of *SP-4* mix type were chosen for two reasons: 1) the number of *SP-4* data was the highest in the NDOR APA database (total 91 *SP-4* mix APA data), and 2) *SP-4* mix type was one of the primary mixes frequently paved in Nebraska. With the *SP-4* mix APA database, materials and/or mixture variables to be considered as candidates affecting APA rut results needed to be determined. Based on the literature review, six factors (performance grade of binder, aggregate gradation, nominal maximum aggregate size,

aggregate angularity, air voids, and asphalt content) were selected as illustrated in Figure 3-2. Performance grade (PG) of binder represents the binder mechanical property which clearly affects mixture rut potential. Size and shape factors of aggregates such as the gradation, angularity, and the nominal maximum aggregate size (NMAS) were included in the analysis. For the mixture side, two variables (air voids and asphalt content) were selected as primary factors because they are crucial indicators identifying mixture volumetric characteristics and were also expected to affect APA rut depth.

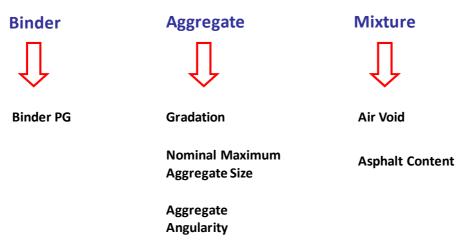


Figure 3-2. Factors Selected for Analyses

From the APA test database of the *SP-4* mix, it was observed that two nominal maximum aggregate sizes (0.375 inch and 0.5 inch) and three binder performance grades (PG 64-22, 64-28, and 70-28) were used. For a more detailed analysis, aggregate angularity factor was categorized into three variables: coarse aggregate angularity value with one or more fractured faces (denoted by CAA1), coarse aggregate angularity value with two or more fractured faces (denoted by CAA2), and fine aggregate angularity (FAA). In the case of aggregate gradation, the gradation factor needed to be quantified in numbers to be implemented in the statistical analyses (multiple linear regression analysis and artificial neural network for this study). In an attempt to quantify the characteristics of gradation, three alternatives were attempted. First, the gradation curve of each mixture was plotted on the 0.45 power chart as shown in Figure 3-3, and then the gradation

curve was divided into a coarse aggregate part and a fine aggregate part based on sieve No.4 (4.75-mm mesh size which is corresponding to 2.016 on the 0.45 power chart). The gradation curve was compared to the maximum density line which is represented by a straight line on the 0.45 power chart as illustrated in Figure 3-3. Then, the areas formed between the gradation curve and the maximum density line were calculated to quantify density characteristics (coarse aggregate density signified by CAD and fine aggregate density signified by FAD) of the gradation. In other words, when the gradation curve is closer to the maximum density line which is an indication of denser mix, the calculated area becomes smaller.

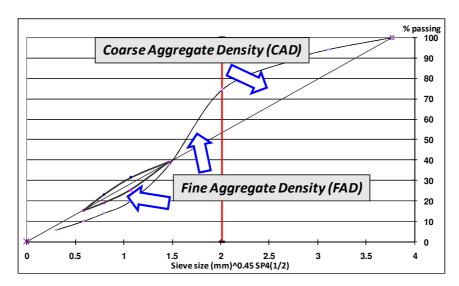


Figure 3-3. A Gradation Curve with Its Density Characteristics on the 0.45 Power Chart

The second alternative selected to represent the gradation characteristics was the fineness modulus (signified by FM). The fineness modulus is defined as an empirical factor obtained by adding the total percentages of a sample of the aggregate retained on each of a specified series of sieves and dividing that sum by 100 (Mamlouk and Zaniewski, 2006). As the value of the fineness modulus increases, the amount of coarse aggregate increases: in other words, a greater fineness modulus of an aggregate blend means a coarser mix. Therefore, the value of fineness modulus can be a potentially good indicator that represents gradation characteristics of the mix.

The third alternative to represent the gradation characteristics was the use of the restricted zone (signified by RZ). The restricted zone forms a band residing along the maximum density gradation between an intermediate sieve and the 0.3-mm sieve as shown in Figure 3-3. There has been a common belief from the asphalt community that a humped gradation indicates an over-sanded mixture which often results in compaction problems during construction and reduced resistance to rutting. However, the concept of the restricted zone related to mixture rutting potential has been almost discarded from the Superpave specification today as many studies (Watson *et al.*, 1997; Kandhal and Mallick, 2001; Hand and Epps, 2001; Kandhal and Cooley, 2002; Sebaaly *et al.*, 2004) demonstrated no clear relationship between the restricted zone and mixture rutting potential. Nevertheless, the effect of restricted zone as an indicator representing gradation characteristics was considered in this study, since the research conclusion supporting elimination of the restricted zone criteria has often been made for mixes with coarse aggregate angularity of about 100 which is not true for the *SP-4* mix.

With all independent variables selected, a data sheet for statistical analyses was developed and is shown in Figure 3-4 for an illustration purpose. The data sheet including all samples (total 91 samples) from Nebraska can be seen in Appendix A. Similar to the Nebraska data sheet, data sheets for the state of Kentucky were also generated and are attached in Appendix B. Figure 3-4 presents specific values of independent variables of each mixture and its APA rut result (shown in the last column in the figure) as the dependent variable. In the case of the variable, NMAS, the numbers 1 and 2 were used to represent 0.375-inch and 0.5-inch NMAS, respectively for the purpose of statistical analyses. Similarly, the numbers 1, 2, and 3 were assigned to binder PG (1 for PG 64-22, 2 for PG 64-28, and 3 for PG 70-28) and to the RZ (1 for above-RZ, 2 for through-RZ, and 3 for below-RZ), respectively. For other variables, real experimental values were used. Instead of using APA rut depth, the rut ratio was calculated by dividing total rut depth by the number of loading cycles. This is because the APA test automatically stopped when the wheel loading reached 8,000 cycles before 12-mm rut depth or when the total rut depth exceeded 12mm. To provide an identical measure of mixture rut potential for both cases, the rut ratio was calculated and used. One more thing to be noted from the figure is that only one alternative among three on the gradation factors was used for analyses. For example, if the concept of gradation densities (CAD and FAD) was used, the other two gradation-related variables (FM and RZ) were excluded in the analyses.

No.	NMAS	PG	Airvoid	Binder	CAAI	CAA2	FAA	CAD	FAD	FM	RZ	Rutratio
1	2	2	4.2	5.9	94	90	45.7	20.273	11.9	4.28	2	0.0002
2	2	1	3.9	5.14	89	86	45.4	15.941	8.533	4.477	2	0.0002
3	2	2	4.8	5.7	99	94	50.I	24.814	13.903	4.114	1	0.0003
4	1	2	4	6.83	95	90	46.9	10.02	10.707	3.813	2	0.0003
•												
						•						
						•						
89	2	2	3.6	5.41	88	80	45.1	24.382	15.456	4.123	2	0.012
90	2	1	5	5.56	91	82	45.5	21.819	12.669	4.441	2	0.013
91	2	2	4	5.11	95	89	45.2	19.451	9.234	4.498	2	0.0181

Figure 3-4. Data Sheet Developed for Statistical Analyses

CHAPTER 4

ANALYSIS RESULTS AND DISCUSSION

To find variables affecting APA rut results and the extent of these variables, *SP-4* APA data from NDOR and data from another state (Kentucky) were analyzed through the Statistical Analysis Software (SAS) using the multiple linear regression analysis. In addition to the analysis of significance, prediction models were developed using the results of multiple linear regression analysis and the artificial neural network technique. The predicted rutting values from the multiple linear regression analysis were compared to the values from the artificial neural network.

4.1 ANALYSIS OF SIGNIFICANCE

Table 4-1 presents statistical analysis results. As can be seen, three cases were considered using the Nebraska data due to the use of different gradation-related alternatives (gradation density signified by CAD and FAD, fineness modulus (FM), and restricted zone (RZ)). For the data from Kentucky, the gradation effects were investigated by using the gradation density indicators (CAD and FAD) only. Table 4-1 shows overall significance of the test results justified by the F-ratio and P-value of each case. By comparing the P-value (i.e., Pr > F) to a given α value (0.01, 0.05, or 0.10), one can decide if the resulting model from the multiple linear regression analysis is considered as a model where there is at least one independent variable that affects the variation of the dependent variable (i.e., APA rut results). Table 4-1 also presents significance of each individual regression coefficient and its parameter estimate by providing the t-test results which are useful to assess the significance of each independent variable in the model. As mentioned previously, if the P-value (i.e., Pr > |t|) is less than the significance level (α value) specified, then the independent variable considered is a significant factor affecting the APA rut depth. To maintain consistency of the analysis, the same value of α level (0.05) was applied to all cases.

Table 4-1. Statistical Analysis Results

			Nebraska I	Data	Kentucky Data				
	Case 1		Case 2		Case 3		Case 4 F -ratio = 6.60		
	F-ratio = 2	2.39	F-ratio = 2	2.05	F-ratio = 2	2.02			
	Pr > F = 0.0	Pr > F = 0.0506		Pr > F = 0.0539		Pr > F = 0.0037			
Variables	Parameter	Pr > t	Parameter	Pr > t	Parameter	Pr > t	Parameter	Pr > t	
	Estimate		Estimate		Estimate		Estimate		
Intercept	0.05868	0.0146	0.03953	0.1140	0.05570	0.0290	0.03820	0.0808	
NMAS	-0.00312	0.0216	-0.00138	0.1182	-0.00106	0.1752	-0.000053	0.8164	
PG	-0.00191	0.0020	-0.00164	0.0069	-0.00165	0.0066	-0.000258	0.0005	
% Air	-0.00091	0.1336	-0.000887	0.1522	-0.00096	0.1212	-0.000281	0.6337	
% Binder	-0.00072	0.4867	-0.000779	0.4619	-0.00127	0.1985	0.000068	0.7084	
CAA1	0.00005	0.7499	0.000070	0.6289	0.000061	0.6748	-0.000372	0.0735	
CAA2	-0.00002	0.8312	-0.000078	0.4783	-0.00007	0.5263	***	***	
FAA	-0.00095	0.0948	-0.00648	0.2484	-0.00078	0.1820	0.000017	0.7804	
CAD	0.000257	0.0365	***	***	***	***	0.000008	0.6337	
FAD	-0.000215	0.0448	***	***	***	***	0.000005	0.8225	
FM	***	***	0.00146	0.3016	***	***	***	***	
RZ	***	***	***	***	-0.00073	0.3469	***	***	

For Case 1, test statistic infers that there existed a meaningful relationship between the APA rut results and at least one independent variable, since the P-value (Pr > F) was less than the specified α value (0.05). Among the nine variables (excluding the intercept), four variables (NMAS, PG, CAD, and FAD) were found to be significant at the α value of 0.05. By analyzing the parameter estimate of the four significant variables, the NMAS had the largest effect on rutting because the absolute quantity of parameter estimate of the NMAS was the largest. The negative sign of the parameter estimate indicates that 0.5 inch was less susceptible than 0.375 inch of NMAS in rutting performance. The second largest variable affecting APA rutting was binder performance grade (PG), which was -0.00191 in the analysis. As with the NMAS, the value of the parameter estimate of PG had a negative sense. Among the three types of PG (62-22, 64-28 and 70-28) used in the analysis, 70-28 was the best in rutting performance followed by 64-28 and 64-22, respectively. Gradation also affected APA rutting. When compared with parameter estimate of NMAS and PG, coefficients to gradation were relatively small values (CAD of 0.000257 and FAD of -0.000215) inferring that the gradation affects rutting performance in a less significant way. In the case of CAD, the parameter estimate was in positive sign. The more the value of CAD increased, the more that APA rut depth increased.

FAD had a negative value, which implied the more fine aggregate density decreased, the more the depth of permanent deformation increased.

To investigate the gradation effects in a more detailed way, the same multiple linear regression analysis was repeated with different data sets where samples in each data set were categorized by their FAD values. Figure 4-1 presents the analysis results which clearly demonstrated that as the FAD increased, the *P*-value tended to decrease. It can be inferred that APA rut depth was more likely affected by the gradation characteristics such as the FAD. In other words, if the FAD values of mixtures are less than approximately 27 (as shown in Figure 4-1), APA rut results among mixtures were expected to produce the same value statistically when the significance level is 0.05. In fact, there was only one gradation presented in Figure 4-2 where the FAD value was greater than 27.

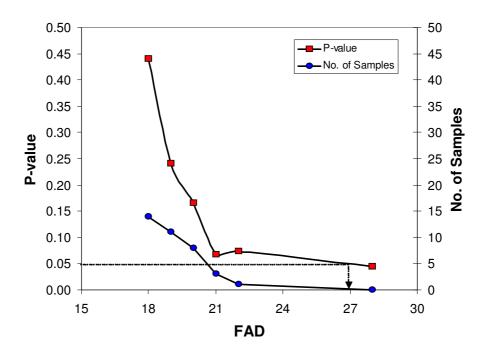


Figure 4-1. Variation of *P*-Value with Different FAD in Gradation

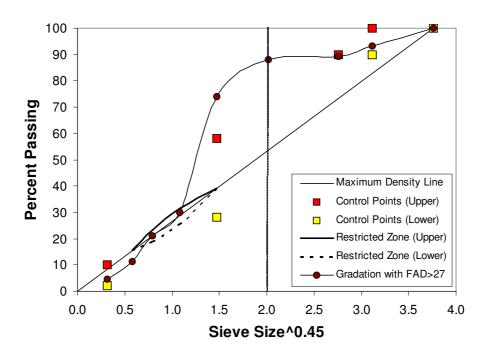


Figure 4-2. A Gradation Curve with FAD Value Greater Than 27

Similar to the case of FAD, the multiple linear regression analysis was also repeated for different data sets grouped by varying CAD values. As expected and clearly shown in Figure 4-3, the *P*-value decreased as the CAD of mixtures increased. From the figure, it can be concluded that mixtures with the CAD value less than approximately 25 are likely producing the same APA rut results at the significance level of 0.05, if other variables of the mixture remain constant. Among the total 91 samples, only four gradations experienced the CAD value greater than 25. The four gradations are plotted in Figure 4-4. As demonstrated in Figures 4-1 to 4-4, majority of mixtures were designed with the FAD and CAD less than their critical values, therefore the effect of gradation may be trivial in practice even if the statistical analysis produced the significance of gradation characteristics based on its density.

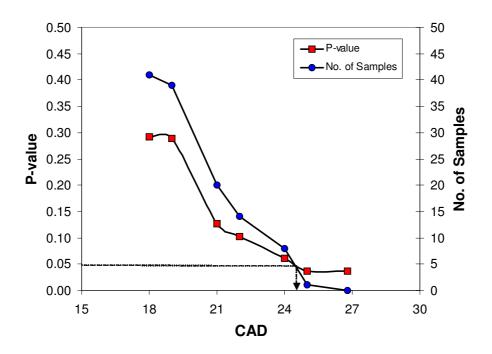


Figure 4-3. Variation of *P*-Value with Different CAD in Gradation

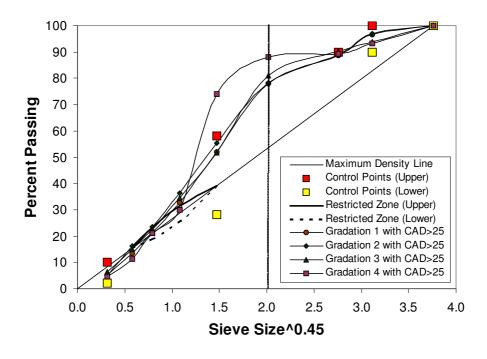


Figure 4-4. Four Gradation Curves with CAD Value Greater Than 25

In Case 2 using the fineness modulus instead of gradation densities, the *P*-value from the *F*-test was 0.0506 which is a little greater than but very close to the α value (0.05). Among the independent variables included, the fineness modulus did not show significance ($\mathbf{Pr} > |t|$: 0.3016), and binder PG was the only significant variable ($\mathbf{Pr} > |t|$: 0.0069) found from this analysis.

In Case 3 based on the concept of restricted zone, the statistical analysis results were similar to the results from Case 2 which are presented in Table 4-1. The P-value ($\mathbf{Pr} > \mathbf{F}$) was 0.0539 which is greater than but still close to the specified significance level ($\alpha = 0.05$). Among the variables included in the model, only binder PG showed its significance in the APA rutting. The restricted zone did not show any significant effects on rutting potential, which is in a good agreement with general findings from many other studies (Watson $et\ al.$, 1997; Kandhal and Mallick, 2001; Hand and Epps, 2001; Kandhal and Cooley, 2002; Sebaaly $et\ al.$, 2004) that have demonstrated the insignificance of the restricted zone to the HMA rutting potential.

As mentioned earlier, APA data sets were also obtained from Kentucky in an attempt to compare analysis results from the state of Nebraska to the results from a different state. Table 4-1 includes the analysis results (Case 4) using the Kentucky data (total 21 samples). There were three binder performance grades (PG 64-22, 70-22 and 76-22), and the values of CAA1 and CAA2 were identical with the range of 98-100. Due to the redundancy in CAA, the CAA2 was excluded in the analysis. Test statistic shown in Table 4-1 infers that there exists a meaningful relationship between the APA rut results and at least one independent variable, since the *P*-value (0.0037) is less than the α value (0.05). Among the eight independent variables, binder PG was the only significant variable (**Pr** > |t|: 0.0005), which was the same result found from Cases 2 and 3. Binder PG produced a negative effect in performance as observed from all previous cases (Case 1 to 3) using Nebraska data.

4.2 DEVELOPMENT OF PREDICTION MODELS

Prediction models were developed using the results of the multiple linear regression analysis and the artificial neural network technique. The predicted APA rutting values from the multiple linear regression models were compared to the values from the artificial neural network.

4.2.1 Prediction Models from the Multiple Linear Regression Analysis

As discussed in Chapter 3, a multiple linear regression model (Equation [3-4]) relates variables by providing parameter estimate of each independent variable. The strength of relationship between the dependent variable and independent variables is measured by the R^2 and/or adj. R^2 values which are defined by Equations [3-5] and [3-6] in the previous chapter. For the development of multiple regression models to predict APA rut results with given materials and mixture design variables, Nebraska data were used, and a total of nine independent variables (NMAS, binder PG, % air voids, % binder content, CAA1, CAA2, FAA, CAD, and FAD) were considered for the model. Table 4-2 presents resulting model parameters and coefficients of determination (R^2 and adj. R^2) of each model.

Table 4-2. Prediction Models from the Multiple Linear Regression Analysis

	Nebraska Data									
	Model 1		Model 2		Model 3 F -ratio = 3.26 $Pr > F = 0.0020$					
	F-ratio = 2.39	9	F-ratio = 4.4	11						
	Pr > F = 0.018	38	Pr > F = 0.00	001						
	$R^2 = 0.21$		$R^2 = 0.33$		$R^2 = 0.27$					
	Adj. $R^2 = 0.12$	2	$Adj. R^2 = 0.2$	25	$Adj. R^2 = 0.19$					
Variables	Parameter Estimate	Pr > t	Parameter Estimate	$\Pr > t $	Parameter Estimate	$\Pr > t $				
Intercept	0.05868	0.0146	22.01186	0.0033	0.04856	0.0097				
NMAS	-0.00312	0.0216	-0.96337	0.0218	-0.00290	0.0063				
PG	-0.00191	0.0020	-0.77936	< 0.0001	-0.00192	< 0.0001				
% Air	-0.00091	0.1336	-0.33716	0.0729	-0.00055	0.2440				
% Binder	-0.00072	0.4867	-0.34853	0.2756	-0.00081	0.3102				
CAA1	0.00005	0.7499	0.00276	0.9500	0.000001	0.9928				
CAA2	-0.00002	0.8312	-0.00512	0.8794	0.000016	0.8499				
FAA	-0.00095	0.0948	-0.48916	0.0062	-0.000740	0.0946				
CAD	0.000257	0.0365	0.05383	0.1532	0.000178	0.0622				
FAD	-0.000215	0.0448	-0.05232	0.1127	-0.000153	0.0673				

The first model (Model 1) was developed using all 91 data. As shown in Table 4-2, the value of the \mathbb{R}^2 and the adj. \mathbb{R}^2 was 0.21 and 0.12, respectively. In order to look for more appropriate models that produce a higher correlation between variables, several attempts such as the data transformation and the diagnostics of outliers were made. Model 2 was developed through the data transformation by taking a natural log function (LN function) on the response variable (Y). Simple transformations of either the response variable (dependent variable), the predictor

variables (independent variables), or of both, are often used to make the regression model more appropriate to the data. In investigating several different transformation functions, such as 1/Y, log Y, \sqrt{Y} , and other powers of Y, the following transformed model was selected because the model produced the best performance.

$$LN(Y_i) = \beta_o + \beta_1 X_{i1} + \beta_2 X_{i2} + ... + \beta_p X_{ip} + \varepsilon_i$$
 $i = 1, 2, ..., n$ [4-1]

As a result of the data transformation, the adequacy of the model improved with higher values of the R^2 (0.33) and the adj. R^2 (0.25). Another method which typically improves the adequacy of the fitted model is checking the presence of outliers and removing them from the data. Outliers are extreme observations that are numerically distant from the rest of the data. When there are outliers in a data set, a statistical analysis will return values not representing the overall data. Among various diagnostics for outliers, the Cook's D technique was employed in this study. The Cook's D technique measures precision of estimation and detects influential points that pull regression towards their direction. Through this analysis, two observations (sample No. 89 and 91) were found as influential points. After two observations were removed from the data set, the multiple linear regression analysis was performed and resulted in R^2 (0.27) and the adj. R^2 (0.19). Figures 4-5 to 4-7 present cross-plots between measured APA rut results and the predicted values from each model. As the coefficients of determination increased, cross-plots were closer to the line of equality.

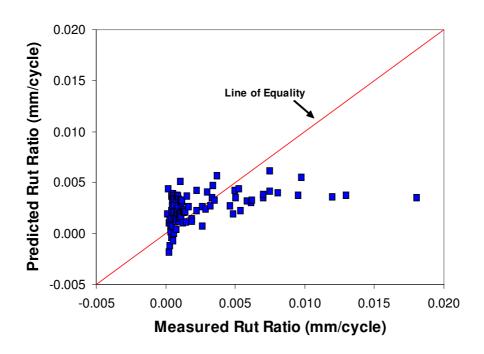


Figure 4-5. Cross-Plots between Measured and Predicted Using Model 1

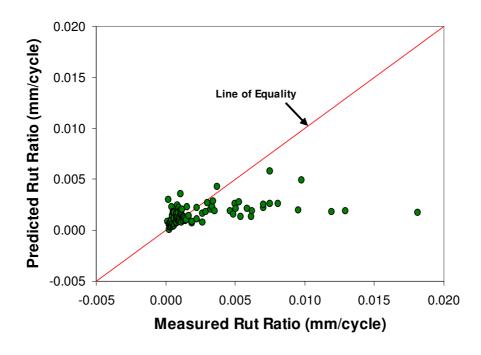


Figure 4-6. Cross-Plots between Measured and Predicted Using Model 2

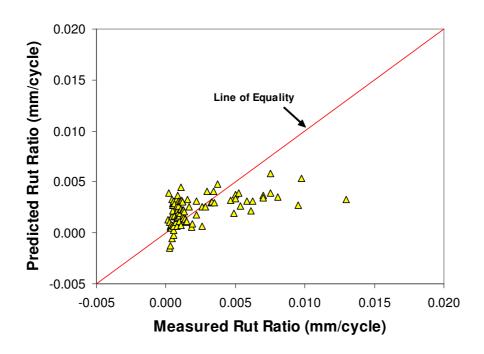


Figure 4-7. Cross-Plots between Measured and Predicted Using Model 3

4.2.2 Prediction Models from the Artificial Neural Network

As illustrated in Figure 4-8, the artificial neural network consists of three steps (Lacroix *et al.*, 2008). The first step is the training stage, the second step is the validation stage, and the last step is the testing stage. The first and second stages are the network developing step and the last stage is to predict the response variable based on the network developed.

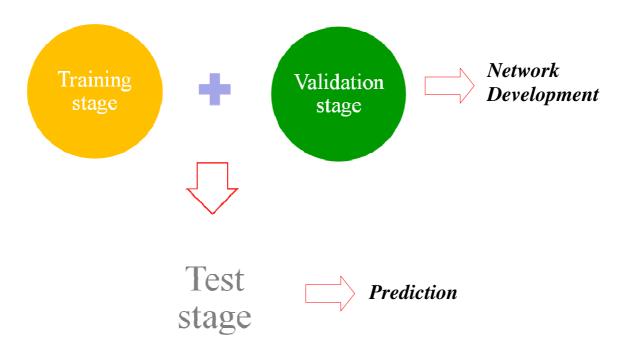


Figure 4-8. Process of the Artificial Neural Network Modeling

For the artificial neural network modeling, the same set of data (91 samples of the *SP-4* mix) was used to compare predictions from the artificial neural network modeling to the predictions obtained from the multiple regression modeling (Model 1 in Table 4-2). With the 91 samples, 56 data were used for training, 15 data were used for validation, and 20 data were used for testing. Cross-plots relating the measured APA rut results to the predictions from each stage are shown in Figures 4-9 to 4-11. Figure 4-12 was also developed to compare a predicting power between two methods (the artificial neural network modeling vs. the multiple linear regression analysis) by plotting both predictions to the measured APA data on the same graph. Even if there is no equivalent indicator that can be used to quantitatively estimate the predicting power between two modeling approaches, it can be inferred that there is no huge difference between two methods. Both methods did not provide a high level of model accuracy, which might be due to a lack of data involved.

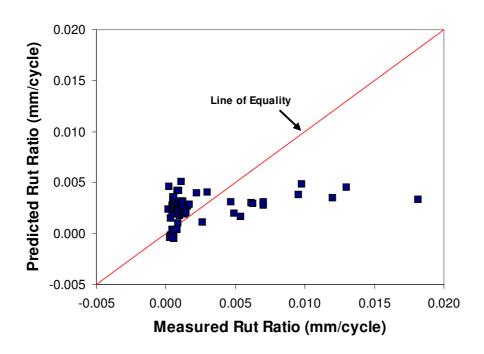


Figure 4-9. Cross-Plots between Measured and Predicted (Training Stage)

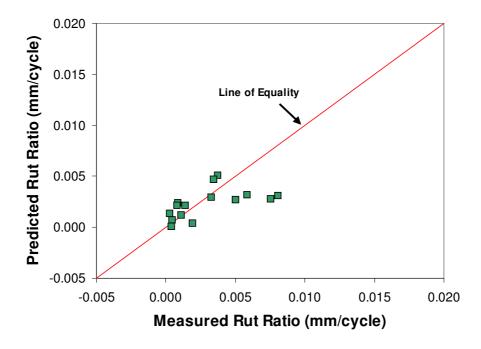


Figure 4-10. Cross-Plots between Measured and Predicted (Validation Stage)

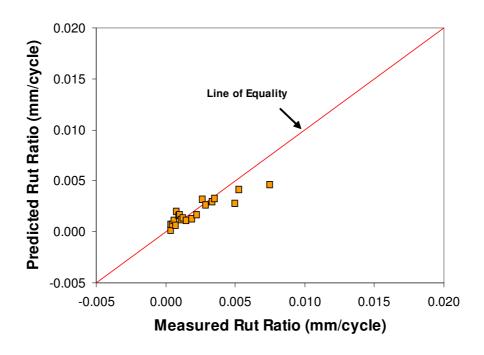


Figure 4-11. Cross-Plots between Measured and Predicted (Testing Stage)

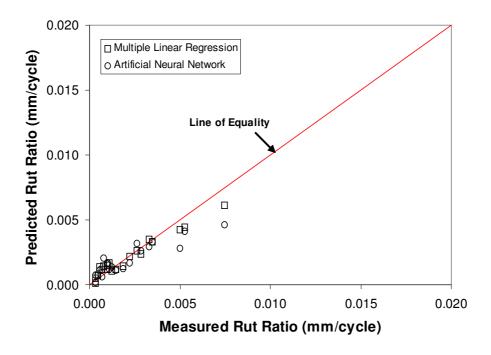


Figure 4-12. Multiple Linear Regression vs. Artificial Neural Network

CHAPTER 5

CONCLUDING REMARKS

Based on this study, the following conclusions and suggested follow-up studies can be drawn:

5.1 CONCLUSIONS

- To find variables affecting APA rut results and the extent of these variables, a total of 91 SP-4 mixture data from Nebraska and 21 samples from Kentucky were statistically analyzed using the multiple linear regression method. Based on literature review, six factors (binder PG, aggregate gradation, nominal maximum aggregate size, aggregate angularity, air voids in mixture, and asphalt content in mixture) were considered as probable candidates significantly affecting APA rut results. For characterizing gradation effects, three indicators (gradation density, fineness modulus, and restricted zone) were considered and each of them was used for each statistical analysis.
- A common variable found from repeated multiple regression analyses by merely varying gradation indicators included was the binder PG. The binder PG was the variable that always shows significant impact on APA rut results from both Nebraska mixtures and Kentucky data.
- In the case of considering gradation effects by including gradation density factors, aggregate gradation was the significant factor together with the binder PG and the nominal maximum aggregate size. However, the effect of gradation is trivial in actual practice, because the significance of aggregate gradation resulting from the statistical analysis was because of minor number of mixtures in the data set.
- The trivial effect of aggregate gradation was verified by considering different gradation indicators (fineness modulus or restricted zone). Neither one showed any significance.
 Only binder PG showed its significance in the APA rutting.

Predicting models were also developed using the results of multiple linear regression
analysis and the artificial neural network technique. Adequacy of the models was
investigated by observing coefficients of determination and cross-plotting predicted APA
rut values to the measured APA rut data. Both methods generally did not provide a high
level of model adequacy, which might be from a lack of data involved.

5.2 RECOMMENDED FURTHER STUDIES

- A total of 91 *SP-4* data were used for the statistical analyses in this study. Even if the 91 data produced outcomes that were expected and were good agreements with other studies, more data would be helpful to derive better understanding. With more APA data gathered, the analyses can be conducted again.
- APA test results have generally shown poor correlations with actual field performance.
 Further research investigating the correlations using Nebraska data and finding out any significant factors of APA tests and results to the field rutting performance would be recommended. Similar statistical analyses employed for this study can be conducted.

5.3 NDOR IMPLEMENTATION PLAN

The findings of this research project complement the past findings of NDOR laboratory personnel following limited years of APA testing of various asphalt mixtures in-house. Although currently a common mixture characteristic has not been identified to be capable of predicting in-field performance using APA, NDOR will continue to perform APA testing in 2008. NDOR has also agreed to become involved in an "Aggregate Imaging System" study sponsored by FHWA. This new study has the potential to provide aggregate surface texture information that may bring more meaning to the APA research findings.

REFERENCES

Cross, S. A. and Purcell, E. M., (2001). "Effects of Fine Aggregate Angularity on VMA and Rutting of Kansas HMA Mixtures." American Society for Testing and Materials, West Conshohocken, PA.

Draper, N. R. and Smith, H., (1998). "Applied Regression Analysis." 3rd Edition, John Wiley & Sons, Inc.

Hand, A. J. and Epps, A. L., (2001). "Impact of Gradation Relative to Superpave Restricted Zone on Hot-Mix Asphalt Performance." *Transportation Research Record*, 1767, pp. 158-166.

Kandhal, P. S. and Mallick, R. B., (2001). "Effect of Mix Gradation on Rutting Potential of Dense-Graded Asphalt Mixtures." *Transportation Research Record*, 1767, pp. 146-151.

Kandhal, P. S. and Cooley, L. A. Jr., (2002). "Coarse-versus Fine-Graded Superpave Mixtures – Comparative Evaluation of Resistance to Rutting." *Transportation Research Record*, 1789, pp. 216-224.

Kandhal, P. S. and Cooley, L. A. Jr., (2003). "Accelerated Laboratory Rutting Tests: Evaluation of the Asphalt Pavement Analyzer." NCHRP Report No. 508. National Research Council, Washington, D.C.

Lacroix, A., Kim, Y. R., and Ranjithan, S. R., (2008). "Backcalculation of the Dynamic Modulus from the Resilient Modulus of Asphalt Concrete Using an Artificial Neural Network." Presented at the 87th Transportation Research Board Annual Meeting, Washington, D.C.

Lee, C., White, T. D., and West, T. R., (1999). "Effect of Fine Aggregate Angularity on Asphalt Mixture Performance." Joint Transportation Research Program, FHWA/IN/JTRP-98/20.

Mamlouk, M. S. and Zaniewski, J. P., (2006). "Materials for Civil and Construction Engineers." 2nd Edition, Prentice Hall.

McCulloch, W. S. and Pitts, W., (1943). "A Logical Calculus of the Ideas Immanent in Nervous Activity." *Bull. Math. Biophys.*, 5, pp. 115-133.

Mohammad, L. N., Huang, B., and Tan, Z. Z., (2001). "Evaluation of Aggregare Contribution to Rutting Susceptibility of Asphalt Mixture." American Society for Testing and Materials, West Conshohocken, PA.

Rumelhart, D. E. and McClelland, J. L., (1986). "Parallel Distributed Processing: Explorations in the Microstructures of Cognition." Bradford Books.

Sebaaly P. E., Hand, A. J., McNamara, W. M., Weitzel, D., and Epps, J. A., (2004). "Filed and Laboratory Performance of Superpave Mixtures in Nevada." *Transportation Research Record*, 1891, pp. 76-84.

Shu, X., Huang, B., Chen, X., and Robison, L., (2006). "Effect of Coarse Aggregate Angularity on Rutting Performance of HMA." Geotechnical Special Publication, No. 154, Pavement Mechanics and Performance, pp. 126-133.

Stiady, J., Hand, A., and White, T., (2001). "Quantifying Contributions of Aggregate Characteristics to HMA Performance Using PURWheel Laboratory Tracking Device." American Society for Testing and Materials, West Conshohocken, PA.

Tarefder, R. A., Zaman, M., and Hobson, K., (2003). "A Laboratory and Statistical Evaluation of Factors Affecting Rutting." *International Journal of Pavement Engineering*, 4(1), pp. 59-68.

Uzarowski, L., Paradis, M., and Lum, P., (2004). "Accelerated Performance Testing of Canadian Asphalt Mixes Using Three Different Wheel Rut Testers." Paper Presented at the 2004 Annual Conference of the Transportation Association of Canada, Quebec City.

Watson, D. E., Johnson, A., and Jared, D., (1997). "The Superpave Gradation Restricted Zone and Performance Testing with Georgia Loaded Wheel Tester." *Transportation Research Record*, 1583, pp. 106-111.

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APPENDIX A

Table A-1. Datasheet of Nebraska *SP-4* Mixtures

No.	NMAS	PG	Air void	% Binder	CAA1	CAA2	FAA	CAD	FAD	FM	RZ	Rut ratio
1	2	2	4.2	5.9	94	90	45.7	20.273	11.9	4.28	2	0.000156
2	2	1	3.9	5.14	89	86	45.4	15.941	8.533	4.477	2	0.000222
3	2	2	4.8	5.7	99	94	50.1	24.814	13.903	4.114	1	0.000251
4	1	2	4	6.83	95	90	46.9	10.02	10.707	3.813	2	0.00028
5	2	3	4.2	5.15	91	86	45.2	20.061	9.591	4.501	2	0.000302
6	2	3	4.5	5.6	94	90	45.3	13.094	10.39	4.751	3	0.000324
7	2	2	4.7	5.1	93	86	45.4	26.771	28.263	3.884	2	0.000353
8	2	2	4.1	5.43	98	98	45	18.883	20.515	4.08	2	0.000378
9	1	2	4.3	6.79	93	91	48	12.111	10.908	3.689	1	0.00038
10	2	2	3.8	5.51	94	90	45.5	19.554	13.403	4.235	2	0.00043
11	2	3	5	5.22	92	88	45.3	16.594	9.496	4.4	2	0.000434
12	1	2	3.8	5.36	94	83	45.3	9.002	10.706	4.218	3	0.00044
13	1	2	4.8	6.13	95	88	45.2	14.26	20.737	3.518	2	0.000459
14	2	2	3.7	5.46	93	80	45.4	24.99	16.418	4.046	2	0.000504
15	1	2	5.4	5.3	96	85	45.4	10.679	10.317	3.858	2	0.000509
16	1	2	5.4	5.3	96	85	45.4	10.679	10.579	3.867	2	0.000509
17	1	3	5.4	5.3	96	85	45.4	10.679	10.579	3.867	2	0.000509
18	2	2	4	5.06	96	87	45.3	22.11	10.74	4.278	2	0.000529
19	1	3	5.2	5.83	98	94	45.5	13.699	19.188	3.542	2	0.000549
20	2	2	4	5.28	97	95	45.2	20.588	9.061	4.468	2	0.00056
21	1	2	4.4	6.41	91	81	46.9	13.484	19.575	3.542	2	0.00058
22	1	2	3.9	5.99	98	95	45.3	13.743	14.581	3.74	2	0.000581
23	2	3	4.3	5.45	95	94	45.3	22.925	11.111	4.25	2	0.00059
24	1	2	4.3	5.35	89	85	45.5	7.769	9.089	4.178	2	0.000606
25	2	2	4.1	5.17	87	82	45.5	15.83	8.696	4.495	2	0.000635
26	2	2	4.1	5.19	99	96	45.6	18.48	16.737	4.194	2	0.000718
27	2	2	5.3	5.4	99	96	45	17.517	16.083	4.342	2	0.000774
28	1	2	5	5.67	85	81	45.2	12.231	15.685	3.795	2	0.000775
29	2	2	4.2	5.04	99	94	45.4	20.319	9.794	4.48	2	0.000799
30	2	1	6	5.21	96	94	45.3	19.329	9.437	4.632	3	0.000808
31	2	2	6	5.21	96	94	45.3	19.329	9.437	4.632	3	0.000808
32	1	2	3.9	6.31	86	81	45.5	14.42	16.993	3.592	2	0.000837
33	1	2	3.9	6.31	86	81	45.5	14.42	17.027	3.591	2	0.000837
34	2	1	4.2	5.04	93	84	45.1	24.096	21.629	3.96	2	0.000843
35	2	2	4.1	5.2	96	95	45.2	22.908	18.909	4.037	2	0.000848

No.	NMAS	PG	Air void	% Binder	CAA1	CAA2	FAA	CAD	FAD	FM	RZ	Rut ratio
36	2	2	5.2	5.42	94	88	45.3	20.232	11.326	4.313	2	0.000876
37	2	2	4	5.41	85	80	45.3	24.295	16.26	4.074	2	0.000906
38	2	2	3.7	5.7	89	86	45.2	21.71	9.824	4.318	2	0.000909
39	1	2	3.5	5.6	92	88	45.1	9.968	13.865	3.809	2	0.000932
40	2	2	3.9	5.4	92	91	45.4	12.149	9.208	4.775	3	0.00097
41	2	2	4.6	5.2	92	89	45.4	22.208	15.653	4.185	2	0.000997
42	1	2	4.2	5.32	87	81	45.5	5.912	13.352	4.418	3	0.00101
43	2	1	4.1	5.1	95	91	45.8	12.599	8.159	4.739	3	0.001059
44	1	2	4.5	5.04	99	92	45.2	3.363	10.645	4.37	3	0.001075
45	2	1	4	5.24	89	87	45.2	20.7	10.831	4.456	2	0.001093
46	2	3	3.8	5.12	96	89	45.1	19.311	10.417	4.307	2	0.001108
47	1	2	3.9	5.42	96	88	45.5	9.002	10.759	4.222	3	0.001119
48	1	1	4.3	6.4	97	96	45.9	14.127	20.838	3.474	2	0.001154
49	1	2	4	5.68	86	83	45.1	8.992	10.605	4.059	2	0.001196
50	2	2	4	5.2	95	83	45.5	20.727	11.112	4.462	2	0.001214
51	2	2	4.3	5.2	85	80	45.3	20.041	12.807	4.31	2	0.001255
52	1	2	5	6.29	92	89	45.2	9.454	14.536	3.79	2	0.001267
53	1	2	4.1	6.3	92	87	45.2	13.718	17.691	3.624	2	0.001286
54	2	1	4.6	5.86	94	90	45.4	20.875	20.479	4.053	2	0.00133
55	2	2	4.2	5.54	92	81	45.1	19.427	14.43	4.277	2	0.001363
56	2	3	3.7	5.12	90	82	45.2	21.088	9.708	4.378	2	0.001425
57	2	2	4	5.25	94	93	45.1	14.672	13.961	4.264	2	0.001497
58	2	1	3.8	5.48	95	91	45.2	21.679	19.623	4.032	2	0.001548
59	1	2	4.2	5.8	87	81	45.9	11.725	11.509	4.126	3	0.001658
60	2	3	3.7	5.23	90	86	45.5	22.422	12.541	4.46	2	0.001879
61	2	2	3.8	5.34	96	89	45.6	20.875	20.479	4.053	2	0.001915
62	2	2	3.5	5.13	93	87	45.2	20.728	9.392	4.415	2	0.002222
63	2	2	4.6	5.56	96	93	45	24.882	18.575	3.993	1	0.00223
64	2	1	4.5	5.6	94	90	45.3	13.094	10.39	4.751	3	0.002627
65	2	2	4.5	5.6	94	90	45.3	13.094	10.39	4.751	3	0.002627
66	2	1	4.5	5.24	94	92	45	12.945	13.6	4.337	2	0.002861
67	1	1	5.1	6	91	83	45.1	15.58	18.348	3.577	2	0.002977
68	1	2	4.4	5.1	85	81	45.1	7.537	10.89	4.221	2	0.003246
69	1	2	3.5	5.4	93	81	45.3	7.079	10.372	4.223	3	0.003329
70	2	1	4.7	4.82	91	80	45.5	21.366	11.786	4.331	2	0.003428
71	1	2	4.2	6.12	88	83	45.1	13.208	12.819	3.908	2	0.003503
72	2	1	4.6	4.6	91	80	45.1	21.697	10.571	4.36	2	0.003707

No.	NMAS	PG	Air void	% Binder	CAA1	CAA2	FAA	CAD	FAD	FM	RZ	Rut ratio
73	1	1	5.2	6.32	90	87	45.2	12.367	17.933	3.703	2	0.004646
74	2	1	3.7	6	93	89	45.1	11.039	13.934	4.992	3	0.004876
75	1	2	3.5	5.56	99	95	45.2	10.327	10.54	4.15	3	0.005015
76	1	2	4.1	5.31	99	93	45.3	6.851	7.884	4.13	2	0.005025
77	2	1	4.3	5.47	95	92	45	20.212	13.118	4.205	2	0.005272
78	1	2	5	5.39	96	94	45.5	10.107	11.797	4.18	2	0.005374
79	1	2	3.9	5.41	87	83	45.3	9.002	10.664	4.234	3	0.005859
80	2	2	4.2	5.31	96	87	45.5	20.676	10.563	4.543	2	0.006128
81	1	2	4.4	5.63	94	89	45.2	9.573	9.728	4.169	3	0.006239
82	1	2	4.2	5.29	92	90	45.2	10.33	10.98	4.111	2	0.007023
83	1	2	3.9	5.32	92	90	45.1	9.889	10.246	4.11	2	0.007023
84	1	1	4.1	5.13	97	91	45.1	12.335	12.384	3.949	2	0.007506
85	1	2	4.1	5.13	97	94	45.1	12.335	12.384	3.949	2	0.007506
86	2	1	4.2	5.04	93	84	45.1	24.096	21.629	3.96	2	0.00806
87	2	2	3.5	5.49	88	84	45	22.798	13.005	4.471	2	0.009532
88	1	1	4.1	5.53	90	87	45.1	13.112	13.753	3.883	2	0.009756
89	2	2	3.6	5.41	88	80	45.1	24.382	15.456	4.123	2	0.011962
90	2	1	5	5.56	91	82	45.5	21.819	12.669	4.441	2	0.012956
91	2	2	4	5.11	95	89	45.2	19.451	9.234	4.498	2	0.018115

APPENDIX B

Table B-1. Datasheet of Kentucky Mixtures

NO.	NMAS	PG	Air void	% Binder	CAA1	CAA2	FAA	CAD	FAD	Rut ratio
1	1	1	3.8	6.3	100	100	45	4.73103	11.1229	0.00082
2	1	1	4	6.2	98	98	46	3.93326	9.11786	0.00116
3	1	1	4	6.2	98	98	46	3.93326	9.11786	0.00165
4	1	2	4.2	5.9	100	100	46	12.6013	13.8411	0.00046
5	1	3	4	5.4	100	100	45	3.10271	16.4924	0.00027
6	1	3	4.1	6	100	100	46	2.74071	9.78786	0.00018
7	1	3	4.2	5.4	100	100	47	3.03526	6.51035	0.00023
8	1	3	4	5.9	100	100	46	6.53026	10.0281	0.00018
9	1	3	3.9	6	100	100	46	4.85926	11.7491	0.00027
10	1	3	4.1	6.2	100	100	46	6.33526	8.1569	0.00026
11	2	1	4	5.8	99	99	48	15.6455	8.03541	0.00151
12	2	1	4.1	5.6	100	100	45	15.8132	11.4804	0.00063
13	2	1	4	5.5	100	100	45	14.3012	11.4804	0.00057
14	2	3	4.1	5.7	100	100	46	19.4815	8.05308	0.00032
15	2	3	4	5.8	100	100	45	16.8695	14.1575	0.00047
16	2	3	4.3	5.5	100	100	48	22.0185	12.8929	0.00023
17	2	3	4	5.8	99	99	48	15.6455	8.03541	0.00044
18	2	3	4	5.8	99	99	48	15.6455	8.03541	0.00057
19	2	3	4.1	6.3	100	100	45	18.4055	13.0956	0.00032
20	2	3	4.1	5.6	100	100	45	23.1715	11.5372	0.00033
21	2	3	4.1	5.6	100	100	48	13.8955	9.28995	0.00025

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