

Article

Interaction of Swell and Sea Waves with Partially Reflective Structures for Possible Engineering Applications

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Abstract: In this work, we investigate the interaction between the combination of wind-driven and regular waves and a chamber defined by a rigid wall and a thin vertical semi-submerged barrier. A series of laboratory experiments were performed with different values of incident wave height, wave period, and wind speed. The analysis focuses on the effect of the geometry of the system characterized in terms of its relative submergence d/h and relative width B/L . Results show that for the case of $d/h = 0.58$ a resonant effect takes place inside the chamber regardless of the wind speed. Wind-driven waves have a higher influence on the variation of the wave period of the waves seaward and leeward of the plate, as well as on the phase lag. Results show that the amplification or reduction of the wave energy inside the chamber is closely related to the wave period as compared to the 1st order natural period of the chamber.

Keywords: vertical barrier; semi-submerged; wind waves; experiments; laboratory

1. Introduction

The modeling of the interactions between forcing agents, e.g., waves and wind, and maritime structures is a challenging problem with important applications in coastal engineering. In the last decades, environmentally friendly coastal structures, such as partially submerged barriers, have become of great interest due to their frequent use in environmental protection, recreation, or wave energy extraction facilities. This type of barrier can reduce the wave energy inside a harbor or a marina while allowing sediment and water exchanges [1,2]. At the same time, these structures can be designed towards wave energy extraction when considering an oscillating water column (OWC) wave energy converter (WEC). Many experimental and theoretical studies have been performed to evaluate their efficiency and hydrodynamic behavior under regular and irregular waves.

Dean, (2008), Ursell and Dean, (1947) [3,4] first studied the wave reflection of incident progressive waves on a fully and partially submerged vertical barrier. Losada et al., (1992) [5] studied the linear theory for periodic waves impinging obliquely on a vertical thin barrier, using the eigenfunction expansion method. This study was then extended to analyze oblique modulated waves [6]. Linear theory was also applied to analyze the scattering of irregular waves impinging on fixed vertical thin barriers, with a good agreement between the analytical model and experimental data by other authors [7]. Koutandos et al., (2010) [1] presented an experimental study of waves acting on a partially submerged breakwater with four different configurations, including a single fixed barrier under regular and irregular waves in shallow and intermediate water. The results showed the effect of the various configurations on transmission, reflection,

and energy dissipation, highlighting that the main governing design parameter could change depending on forcing conditions (for short waves the main parameter was the submergence of the barrier, for long waves it was the width of the chamber). Jalon et al., (2018) [8] presented an analytical model to optimize the design configuration of a vertical thin barrier concerning different criteria, such as harbor tranquility or wave energy extraction. Other types of structures, e.g., slotted-, porous-, and Jarlan-type barriers, have also been widely studied [9,10].

Regarding the experimental studies on vertical semi-submerged barriers, Kriebel et al., (1999) [11] studied their efficiency under regular and irregular waves in terms of the transmission and reflection coefficients, while Liu and Al-Banaa, (2004) [2] carried out experiments under solitary waves focusing on the wave forces acting on the barrier.

Nonetheless, in nature we find swell waves coexisting with wind-driven waves. Swell waves are described as long-period waves that have been traveling for long distances and in absence of wind can be locally analyzed by means of standard wave spectra. Wind waves are actively growing due to forcing action from the local wind, and are characterized as non-regular waves consisting of a spectrum in continuous evolution. However, most of the experimental studies on wave interaction with maritime structures such as the ones mentioned, are limited to regular and/or irregular waves generated by a paddle, without considering (1) the effect of wind forcing on both the incident and reflected swell wave trains, (2) the interaction of the driven sea waves with the structure and (3) the non-linear interaction between the different wave field components. Therefore, there is still need for data on the wave-structure interaction under wind-driven waves superimposed on swell, taking into account their intrinsic irregular and random complex nature.

The present study is based on a series of laboratory experiments of wave-structure interaction with paddle-generated regular waves in combination with wind-driven waves, due to wind blowing in the direction swell propagation. The structure consists of a thin vertical semi-submerged barrier delimiting a chamber along with an impermeable back wall. This type of structures can be used in maritime engineering applications such as harbor tranquility and energy extraction [7,8,12].

The manuscript is structured as follows. Section 2 describes the adopted methodology, the experimental facility and measurements. Section 3 contains the critical analysis of the data and the discussion. Conclusions are given in Section 4.

2. Methodology

2.1. Experimental Setup and Data Analysis

Experiments were conducted in the Atmosphere-Ocean Interaction Flume (CIAO) of the Andalusian Institute for Earth System Research, Granada, Spain. It is 16 m long, 1 m wide and is designed for an optimal water depth of 70 cm (Figure 1). This facility is dedicated to the study of the coupled processes between the ocean and the atmosphere. It is equipped with (i) two opposite piston-type wavemakers with active absorption system that allow the generation of regular and irregular waves with periods between 1–5 s, and therefore in the range of shallow to intermediate conditions, and heights up to 25 cm; (ii) a closed-circuit wind generation system (wind tunnel) with mean wind speed at start of the test section—representative of U_{10} conditions in prototype—up to 12 m/s that generate waves with an effective fetch length up to 12 m, and (iii) a current-generation system for currents of up to 75 cm/s. The system allows the generation of waves and currents following or opposing the wind direction, with all the possible combinations. The system allows for the active absorption of incoming waves by the secondary paddle and the compensation of re-reflection by the generating paddle. More information on this system and its performance can be found on Lykke Andersen et al., (2016) [13]. For a detailed description of the CIAO facility the reader is referred to Clavero et al., (2013), Nieto et al., (2015) [14,15].

A rigid thin barrier, with a thickness of 1.9 cm and a submergence d was installed vertically (see Figure 2). This barrier was located at 11 m from the wavemaker and spaced a distance B (width of the chamber) from

a vertical rigid back wall. The x - and z -axes refer to the stream-wise and vertical direction, respectively, with the origin at the mean water level in the section where the thin barrier is located.

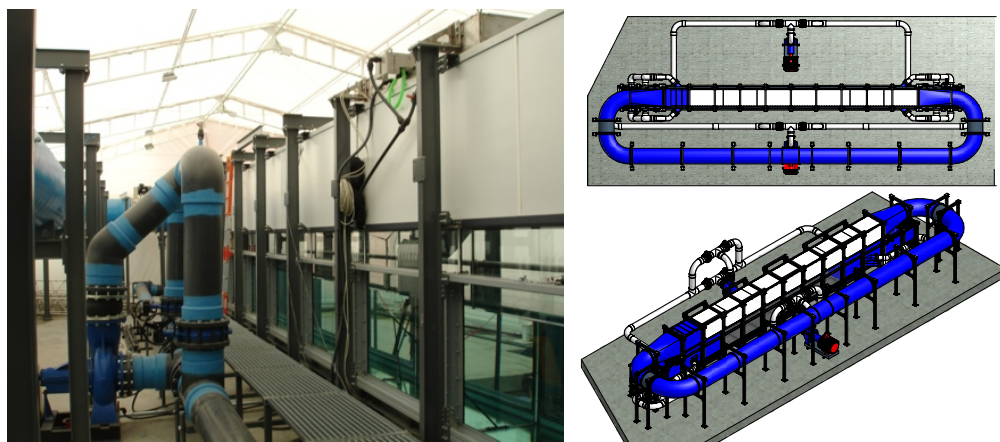


Figure 1. Atmosphere-Ocean Interaction Flume (CIAO). Schematics provided by VTI.

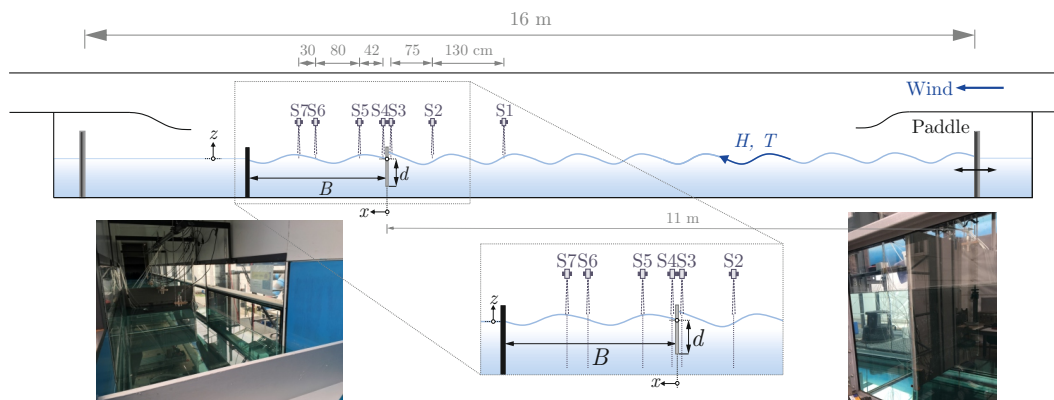


Figure 2. The experimental setup of the flume. The system consists of a semi-submerged rigid thin barrier separated a distance B (chamber width) from a vertical rigid back wall. S1–S7 are ultrasound distance gauges for free surface elevation measurements. Tests were done with a combination of regular waves (paddle-generated) and wind-driven waves traveling in the same direction.

Water level measurements were obtained using seven UltraLab ULS 80D acoustic wave gauges installed at different sections along the flume, with a maximum repetition rate equal to 75 Hz, a declared space resolution of 0.18 mm and a reproducibility of $\pm 0.15\%$. The overall accuracy is 0.5 mm. Free surface measurements were taken for 185 s.

Measurements of wind speed were collected before the installation of the system (vertical plate or back wall) using a Pitot tube at $x = 0$ m (plate section) and at different heights above the water level. The signal was acquired for ≈ 60 s with a data rate of 1 kHz. Figure 3 shows the wind speed profiles at different rotation rates of the fan used in the tests, with U_{ref} representing the average speed in the vertical profile [16].

A combination of paddle (with H^{input} , T^{input} , representing the r.m.s. wave height and period given as inputs to wave generation software), and wind waves (U_{ref} , representing the reference wind speed) were generated for a chamber with a constant width $B = 2.5$ m and different plate submergences d . Each regular wave experiment was performed (i) in the absence of wind, and (ii) in combination with wind waves using different U_{ref} . Regular waves were generated using only one of the wavemakers with the active absorption on.

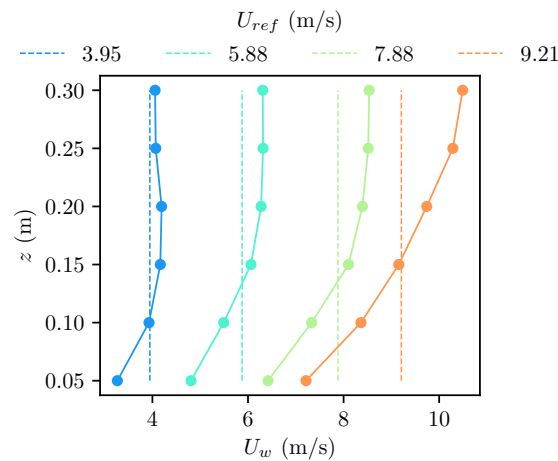


Figure 3. Wind speed profiles (symbols) and reference speed U_{ref} (dashed vertical lines) for the different rotation rates of the fan.

Table 1 lists the values of the parameters of the experiments. H_{rms0} and T_{z0} represent the r.m.s. wave height and the mean wave period obtained by the statistical analysis of the surface elevation data in the section of the wave gauge S1 ($x = -2.1$ m) for the experiment without wind ($U_{ref} = 0$ m/s). The statistical analysis consists on applying the zero-upcrossing technique to the surface elevation time series obtaining the individual wave heights and periods of the signal. The mean wave period T_{z0} is always coincident with the input period T^{input} , whereas H_{rms0} is not coincident with the imposed value H^{input} . L_0 represents the wavelength (experiment without wind) obtained by means of the linear dispersion equation $\omega_0^2 = gk_0 \tanh(k_0 h)$ where $\omega_0 = \frac{2\pi}{T_{z0}}$ is the angular frequency, $k_0 = \frac{2\pi}{L_0}$ is the wavenumber and $h = 0.7$ m is the water depth and T_{z0} is the mean wave period obtained from the experiment without wind. The two first natural oscillation periods of the system were approximated using Merian’s formula $T_n = \frac{2B}{n\sqrt{gh}}$ with $n = 1, 2, \dots$, and are equal to $T_1 = 1.9$ s and $T_2 = 0.95$ s, respectively. Figure 4 presents the signal of the wave gauge S1 for the different experiments and different wind conditions.

Table 1. Parameters of the experiments. H^{input} and T^{input} are the input values given to the wave generation software. H_{rms0} , T_{z0} and L_0 represent the r.m.s. wave height, mean wave period and wavelength, respectively, obtained by the statistical analysis of the surface elevation data of S1 for the experiment without wind. The column H_{rms0} depicts the mean value \pm the error—when applicable— between two repetitions of the same case. d/h is the relative submergence, B is the chamber width and U_{ref} represents the reference wind speed.

Test	H^{input} (cm)	H_{rms0} (cm)	$T^{input} = T_{z0}$ (s)	d/h	B/L_0	T_{z0}/T_1	U_{ref} (m/s)
R1a	7	6.4 ± 0.5	1.85	0.33	0.6	0.97	0, 3.95, 5.88, 7.88, 9.21
R1b	7	5	1.85	0.58	0.6	0.97	0, 3.95, 5.88, 7.88, 9.21
R1c	7	8.9 ± 0.15	1.85	0.71	0.6	0.97	0, 3.95, 5.88, 7.88, 9.21
R2a	6	6.3	1.65	0.58	0.7	0.87	0, 3.95, 5.88, 7.88, 9.21
R3a	7	2.7	2.78	0.58	0.4	1.46	0, 3.95, 5.88, 7.88, 9.21
R4a	7	6.7	4.76	0.58	0.2	2.5	0, 3.95, 5.88, 7.88, 9.21
R5a	6	2.8 ± 0.25	3	0.58	0.3	1.58	0, 3.95, 5.88, 7.88, 9.21
R6a	6	5.8 ± 1.4	4.7	0.58	0.2	2.47	0, 3.95, 5.88, 7.88, 9.21

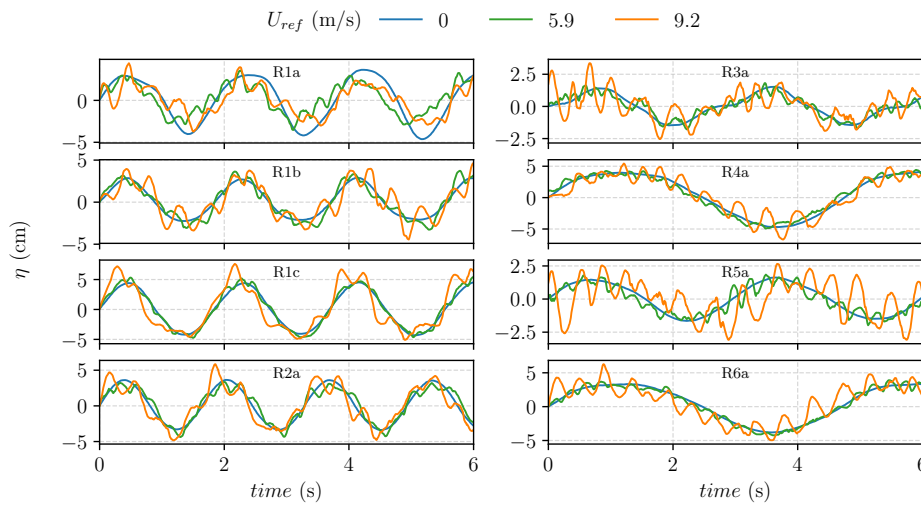


Figure 4. Free surface elevation of the wave gauge S1 for the experiments presented on Table 1. In each panel, different colors of the lines refer to different wind speeds.

In addition to the statistical analysis by means of the zero-upcrossing technique, data were processed by spectral analysis using the python Scipy Signal processing functions. The spectra are obtained with the Welch method, using a Hanning window with 50% overlap resulting in 17 degrees of freedom and a spectral resolution of 0.039 Hz.

The model investigated in this work can be replicated in real-world applications guaranteeing Froude similarity, commonly adopted for the design of maritime structures. We notice that using a Froude-based similarity and working with the same fluid viscosity for both model and prototype, inevitable violates the Reynolds number scaling. In this case, a partial dynamic similarity is achieved using Froude scaling, with length and pressure scale of λ and a time scale of $\sqrt{\lambda}$. Assuming a geometrical scale of 1/14, experiment R1a, e.g., scales to an incident wave height of $H^{input} = 1$ m, $T^{input} = 7$ s, $L = 59$ m in water depths of $h = 10$ m.

2.2. Expected Wave Characteristics

Jalon et al., (2018) [8] presented an analytical model to optimize the design configuration of a vertical thin barrier concerning different criteria, such as harbor tranquility or wave energy extraction using a prototype setup as the once presented in this work. Their results showed the presence of nodal and antinodal frequencies of the spectra on both the leeward and seaward side depending on the geometrical configuration and the incident wave characteristics focusing on the relative submergence (d/h) and relative width (B/L) parameters. For values of $d/h > 0.25$, maximum values of the reflection coefficient were found for very small values of B/L and slightly larger than 0.5 for which also maximum capture coefficients were found.

For B/L close to 0.6, the reflection coefficient increases with d/h (experiments R1a-R1c—Table 1) up to values of $d/h \approx 0.7$ when it starts decreasing again. In these cases, the highest capture coefficient is obtained for $d/h \approx 0.6$. At $x = 0$, their results showed the presence of quasi-nodes (quasi-antinodes) in the seaward (leeward) sides for the configurations resembling experiments R1a and R2a and the quasi-antinodes on both sides experiment R1c.

For d/h close to 0.6, and varying B/L (experiments R1b, R3a, R4a) the reflection coefficient increases with B/L while the transmission coefficient presents similar values for B/L of 0.2 (R4a) and 0.4 (R3a) and larger values for $B/L = 0.6$ (R1b). At $x = 0$, their results showed the presence of quasi-antinodes (quasi-nodes) in the seaward (leeward) sides for the configurations resembling experiments R3a and R4a and the opposite for experiment R1b.

3. Results and Discussion

The key element of the analysis is the influence of the geometry of the system and of the wind-generated waves on the measured variables. Therefore, we analyzed the results with respect to the relative submergence d/h and to the different U_{ref} . During the experiments, the relative submergence d/h was modified by varying the submergence of the plate d whereas the water depth h was kept constant. To test different values of the relative width B/L , the chamber width B remained fixed and different configurations of wave periods with and without the combination of wind were tested, changing the wavelength of the incident wave L_0 .

All the results presented henceforth are dimensionless with respect to the corresponding values of the measurements in absence of wind and at Section S1.

Figures 5–8 show the results for the different R1-experiments, corresponding to the same initial theoretical values $H^{input} = 7$ cm, $T^{input} = 1.85$ s and wind speeds, but with a different relative submergence d/h (see Table 1). The mean wave period of the regular waves ($T_{z0} = 1.85$ s) is very close to the 1st order natural period of the chamber, with $T_{z0}/T_1 = 0.97$.

Figure 5 depicts, in the first and second row, the dimensionless r.m.s. wave height and wavelength for the gauges S2 (seaward region) and S5, S6 and S7 (leeward region) for each experiment. In the leeward region, the ratio H_{rms}/H_{rms0} is related to the transmission coefficient. The third row shows the mean phase lag defined as $\Phi = \omega t_{max}$, with ω being the angular frequency. t_{max} represents the time difference of the maximum surface elevation position between the signal of each sensor and the signal of sensor S1 within a wave period. For its calculation, the signal of each sensor is divided into waves of period T^{input} and the position of the maximum surface elevation is found.

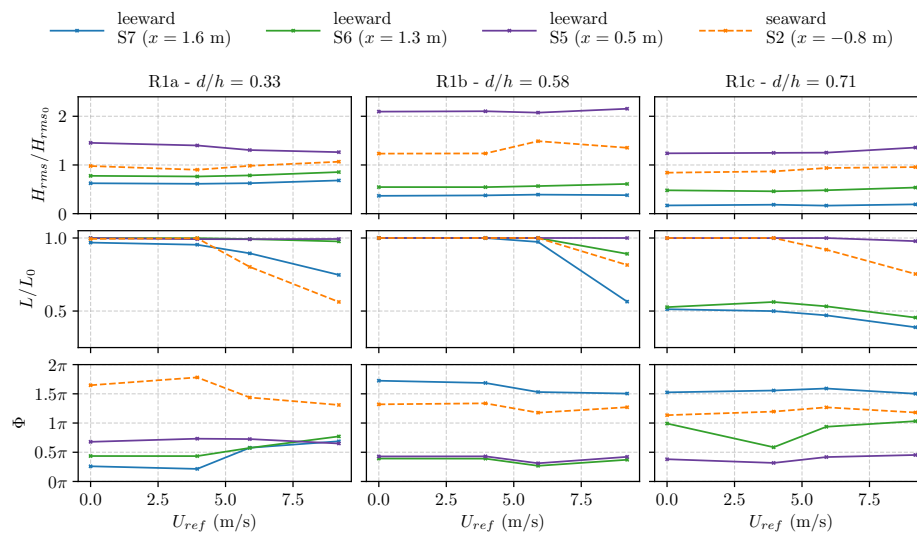


Figure 5. R1a-b-c-experiments with constant input values $H^{input} = 7$ cm, $T^{input} = 1.85$ s and different relative submergences d/h . The first row shows the r.m.s. wave height H_{rms} , the second row shows the wavelength L , the third row shows the phase lag Φ . Each panel shows the results for S5-S6-S7 sensors (violet, green, and blue lines, respectively) in the leeward region, and for sensor S2 (orange dashed line) in the seaward region. Values are dimensionless with respect to H_{rms0} , L_0 measured in Section S1 without wind.

It can be observed that for each individual sensor and experiment the wave height is constant regardless of the wind speed, showing that wind-driven waves have almost no influence in the amplification or reduction of the periodic wave inside the chamber, when $T_{z0}/T_1 \approx 1$. In the case of the highest plate submergence, $d/h = 0.71$ (R1c), the minimum value of the wave height is observed. This result was expected, given the high reflection of the incident wave and the low energy transmitted inside the chamber. The highest values are obtained for the experiment R1b with $d/h = 0.58$. In this case,

S5 gives a value of $H_{rms}/H_{rms0} \approx 2$, showing a resonant behavior inside the chamber. However, a high variation between the different sensors inside the chamber is observed in this test, with values in the range 0.5–2 between the sensors S7 and S5. It can be attributed to the partial standing pattern inside the chamber that exhibits a spatial variation of the total wave amplitude with values depending on the relative distance of each gauge to quasi-nodes and quasi-antinodes. Also, the phase lag shows a similar variability, with $0.2\pi < \Phi < 0.5\pi$ for sensor S5 and $1.5\pi < \Phi < 1.7\pi$ for sensor S7. We notice the presence of quasi-antinodes and surface elevations in phase opposition ($\Phi \approx \pi$) which is critical for the study of the loads acting on the plate [8]. These results are in agreement with the results in Jalon et al., (2019) [8], where the same structural configuration is studied analytically. The maximum values of the capture coefficient (a proxy for the transmission coefficient), with the presence of quasi-antinodes, are obtained for $B/L \approx 0.6$ (R1a-b-c-experiments).

The main influence of wind-driven waves is observed in the estimations of L/L_0 . As observed, L/L_0 decreases for increasing wind speeds and is more evident for Section S2, where the wind is expected to have a higher influence. A change of the phase lag can be observed for varying U_{ref} for the tests R1a and R1b, whereas for tests R1c, with $d/h = 0.71$, the phase lag is almost constant; however, as already highlighted, it varies considerably for different sections. This can be attributed to the fact that higher reflections are likely to be associated with higher submergences showing partial standing oscillations. Therefore, a reduction of the non-linear interaction between wind waves and the periodic component is expected.

Figure 6 depicts the dimensionless surface elevation η/A_{rms0} , where A_{rms0} is the r.m.s. wave amplitude obtained from S1 in the absence of wind. It can be observed that the highest relative amplitude inside the chamber is given for tests with $d/h = 0.33 - 0.58$, with almost double of the seaward wave amplitude at the Section S5 (which is very close to a quasi-antinode), for all wind speeds. The deformation of the regular wave induced by wind action in the seaward region is transmitted inside the chamber, where an asymmetry between crests and troughs is observed. This could be due to the amplification modes of the system and to the re-reflection, both controlled by geometry. In passing, we notice that free surface oscillations are strongly affected by the forcing term and by the damping, the latter mainly due to dissipation [17]. For limited dissipation, a blow up of the oscillations is expected for multiple harmonic in the forcing term. In this respect, the free surface oscillations in the chamber can vary significantly if energy is extracted.

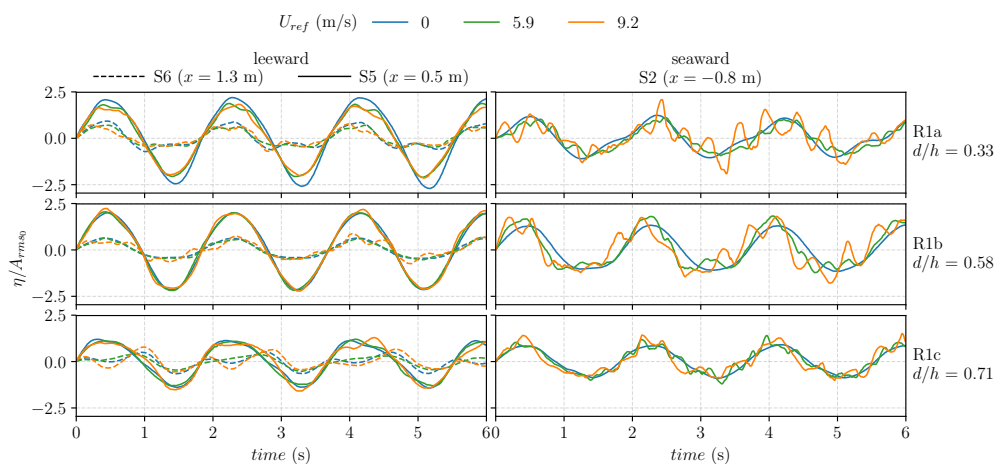


Figure 6. R1a-b-c-experiments with constant nominal values $H^{input} = 7$ cm, $T^{input} = 1.85$ s and different relative submergences d/h . Free surface elevation is dimensionless with respect to the amplitude measured at Section S1 in the absence of wind. The left column shows the results for the two sensors S5–S6 in the leeward region (continuous and dashed lines, respectively), the right column shows the results for sensor S2 in the seaward region. In each panel, different colors of the lines refer to different wind speeds.

A similar behavior can be observed for all three d/h values tests in Section S2 in the seaward region, where the wind generates short waves traveling alongside the long regular wave. This effect is less noticeable in the test with $d/h = 0.71$ (R1c), where a higher reflection of the periodic wave components is expected.

These results are mirrored in the power spectrum $S' = S(f)f_p/m_0$ shown in Figure 7, dimensionless with respect to the peak frequency f_p and to the zeroth-order moment m_0 obtained from data at Section S1 in the absence of wind. At high frequencies, the scenario is controlled by wind-driven waves, energy increases, and the resonant frequencies become less noticeable. Inside the chamber the peak frequency in all sections almost corresponds to the frequency of the regular waves, which is very close to the natural period of the chamber, $T_{z0}/T_1 = 0.97$. The energy is higher for sensor S5 closest to the plate and the energy in the chamber increases with respect to the seaward region, which is associated with the peak frequency of regular waves and to the first resonant period, more so for the case with the highest relative submergence $d/h = 0.71$; there is an energy decrease at higher frequencies with respect to the seaward side. This is attributed to non-linear interaction between wind-driven waves, periodic waves and resonant waves controlled by the geometry of the system and to the filtering effect of the screen, more efficient for high frequency components. For clear cut results, it will be necessary to further explore the wave profile and its variation with different geometrical configurations and with different forcing conditions.

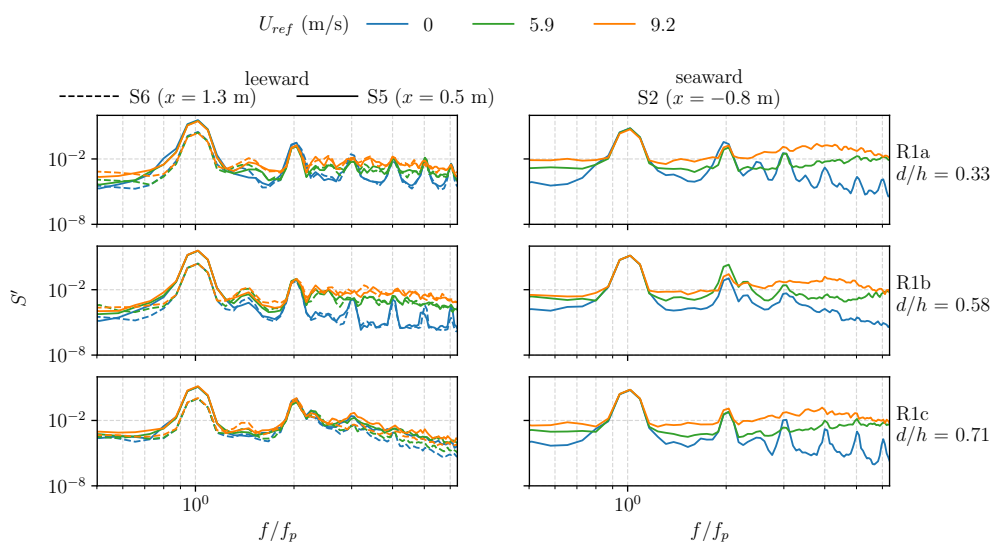


Figure 7. Dimensionless power spectrum S' . For caption see Figure 6.

Figure 8 shows the dimensionless phase-averaged surface level for the R1a-b-c-experiments, with and without wind-driven waves and for different sensors. It can be observed that both the predominant regular wave phase, and the wave profile, change from the seaward to the leeward region. In the case of the highest relative submergence $d/h = 0.71$ (R1c), the transmission of the longer periodic wave is expected to be lower, and the changes could be due to the wind acting on the free surface. Sensor 5, at section $|x|/L = 0.12$, shows a wave profile similar to the incident wave, with an increment in wave height due to its position in a quasi-antinode. Sensor 6 shows a reduction of the wave height and some changes in wave profile. As expected, due to the shading effect of the plate, the influence of wind-driven waves is limited to the seaward region, with a more evident variation of the wave profile for the case of $d/h = 0.58$ (R1b) and $U_{ref} = 5.9$ m/s.

To analyze the effect of the dimensionless parameter B/L in the resonant behavior of the chamber for different wind conditions, different regular wave (paddle-generated) and wind-wave configurations were tested for a fixed relative submergence $d/h = 0.58$. This analysis is especially relevant for the design of OWC devices that have performance directly related to the oscillation of the water column.

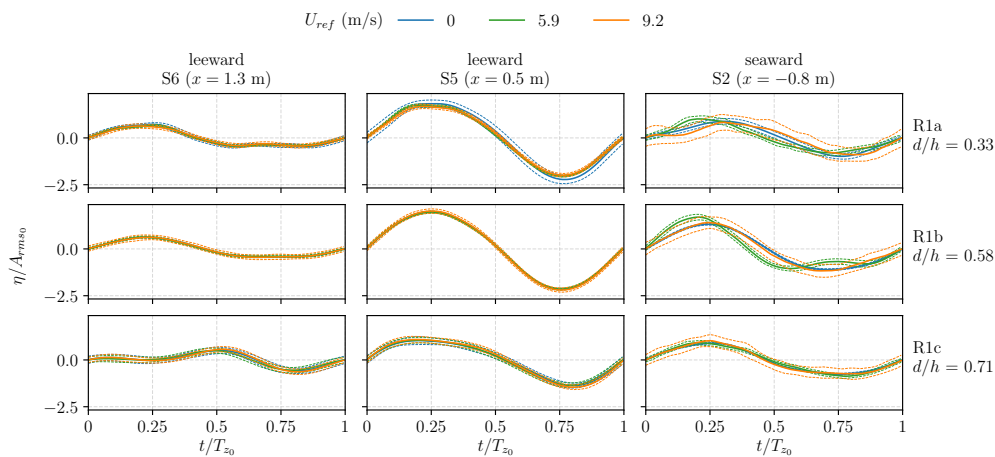


Figure 8. Phase-averaged dimensionless free surface elevations. For caption see Figure 6. The dashed lines correspond to the phased-averaged value \pm one standard deviation.

Figure 9 shows the same variables shown in Figure 5, but the comparison is made between different regular wave experiments with the same relative submergence $d/h = 0.58$. Each column represents two sets of experiments with initial theoretical values of $H^{input} = 6-7$ cm and similar values of T_{z0}/T_1 .

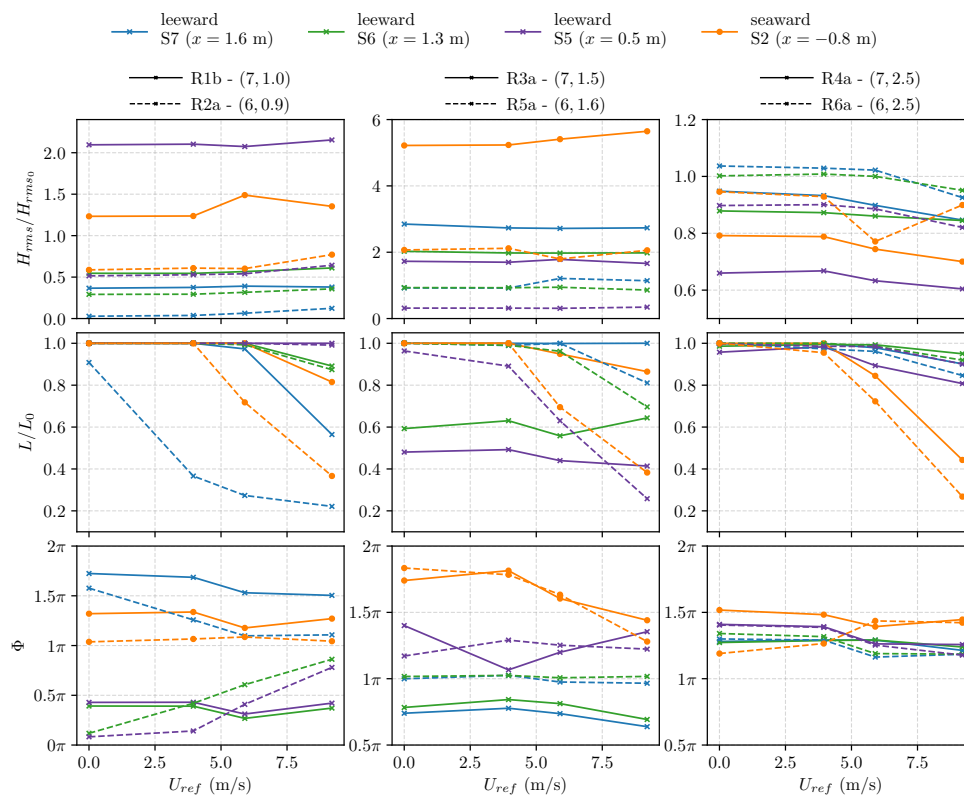


Figure 9. Experiments with the same relative submergence $d/h = 0.58$. For caption see Figure 5.

Values of $H_{rms}/H_{rms0} \approx 3$ can be observed in the leeward region for tests with $T_{z0}/T_1 \approx 1.5$, while at Section S2 in the seaward region, results $H_{rms}/H_{rms0} \approx 5 - 6$. We notice that the scaling value of wave height H_{rms0} is significantly reduced with respect to the nominal value (Table 1). For example, R3a was carried out using a nominal wave height of $H^{input} = 7$ cm while the statistical analysis of S1 for this experiment without wind gave a value of $H_{rms0} = 2.7$ cm. In this case, at Section S2, the closest to

the plate in the seaward region, very high values of r.m.s. wave height were obtained (higher reflection), with consequent very high values of H_{rms}/H_{rms0} . It can also be observed that for experiments R4a and R6a, corresponding to the experiments with higher wave periods of the regular wave, there is a noticeable stronger influence of wind-driven waves than for experiments with lower wave periods, characterized by a constant value of H_{rms}/H_{rms0} for the different wind speeds. A reduction of the wavelength L/L_0 can be observed for increasing wind speed for almost all tests. In the seaward region, there is an almost five-fold reduction with the highest wind. In the leeward region, this influence changes depending on T_{z0}/T_1 , e.g., for $T_{z0}/T_1 \approx 2.5$ there is a negligible change of the wavelength with wind speed.

Another relevant indicator of the system behavior is the phase lag. For tests with $T_{z0}/T_1 \leq 1.5$ the phase lag varies greatly from one sensor to the other for the same experiment, whereas for tests with $T_{z0}/T_1 \approx 2.5$ results $\pi \leq \Phi \leq 1.5\pi$ regardless of wind speed. Hence, in some tests, the relation between the period of the incident wave height and the resonant wave periods of the system plays a major role than the wind speed.

Figure 10 shows H_{max} and H_{rms} of sensors S2, S5, and S6. Each point represents a different configuration of regular wave (paddle-generated) and wind speed and they were ordered according to the relation T_z/T_1 (x-axis) where T_z is the mean wave period of each sensor and case and T_1 the 1st order natural period of the chamber. The first row presents the results for S2 (seaward region) where the highest values of H/H_{rms0} are achieved. This is due to the fact that as previously explained, in this position the regular periodic wave component is predominant over the shorter wind waves, therefore the highest values of wave height are obtained. For the sensors S5-S6 in the leeward region, an amplification of wave energy is observed for values T_z/T_1 close to 1 for all wind speeds. For most of the experiments in the absence of wind, the same values of H_{max} and H_{rms} are obtained while in tests with wind, H_{max}/H_{rms0} presents higher values than H_{rms}/H_{rms0} . This is expected and due to the fact that the r.m.s. is a statistic that takes into account all the wave heights in the signal, therefore, taking into account the shorter wind-driven waves, its value is expected to be lower than H_{max} .

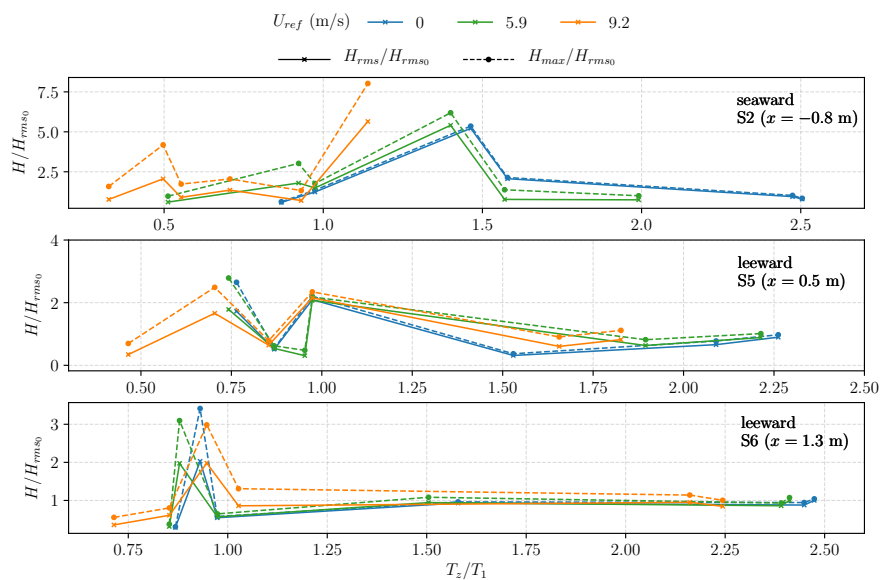


Figure 10. Dimensionless maximum and r.m.s. wave heights for different sensors (row-wise). In each panel, different points correspond to the experiments with the same relative submergence d/h and different configurations of regular waves (paddle-generated) with and without the combination of wind waves. In each panel, the different colored lines represent the results for experiments with and without wind.

4. Conclusions

In the present study, the hydrodynamic interaction of regular waves, in combination with short wind-driven waves, with a vertical fixed semi-submerged barrier was experimentally investigated. The study was focused on the influence of the seaward wave characteristics and certain geometric characteristics of the structure, with special attention on the role of wind waves providing new insight of the influence of wind-driven waves on the incident wave characteristics and in the wave field inside the chamber as it still remains a topic to be further investigated in maritime and coastal engineering. A series of conclusions can be derived.

When analyzing a regular wave configuration with a wave period similar to the 1st order natural period of the chamber, an amplification of wave energy in the seaward region is measured for a relative submergence of $d/h = 0.58$, in agreement with the results obtained by Jalon et al., (2019) [8]. In the case of higher submergences, the fixed plate operates in a highly reflective manner and a reduction of the wave energy inside the chamber can be observed. Therefore, the submergence of the plate is a key parameter for the design of these types of structures, either for harbor protection (energy reduction), or WEC devices (energy amplification).

Wind-driven waves have a high influence on the wave symmetry and phase lag between the seaward and leeward regions. This is an important factor to take into account since it could lead to strong forces on the barrier when the phase lag equals π .

The influence of wind waves is more noticeable for the experiments where $T_{z0}/T_1 \geq 1.5$, in which the short-crested waves proved to have a higher influence on the resulting wave periods and heights in the seaward region. Therefore, the influence of wind waves on the behavior of the structure depends greatly on the period of the predominant periodic component (swell).

The overall behavior of the system depends on the relative values of periodic waves, wind waves, and resonant periods of the chamber. The analysis has been conducted without extracting energy from the system, since no OWC device was inserted in the experimental layout. In real conditions, with an OWC device active, we expect some minor modifications, with a general reduction of the wave height in both leeward and seaward regions. In this respect, the present analysis can be confidently applied for verifying the structures in the most critical conditions.

As a future extension, this research could involve (i) the analysis of irregular waves, where is expected that wind waves effects depend on the frequency range; (ii) the analysis of grouping effects, where a sequence of grouped waves can force the system with presently unnoticed effects.

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