

*Citation for published version:*

Niyigena, C, Amziane, S, Chateauneuf, A, Arnaud, L, Bessette, L, Collet, F, Lanos, C, Escadeillas, G, Lawrence, M, Magniont, C, Marceau, S, Pavia, S, Peter, U, Picandet, V, Sonebi, M & Walker, P 2016, 'Variability of the mechanical properties of hemp concrete', *Materials Today Communications*, vol. 7, pp. 122-133.  
<https://doi.org/10.1016/j.mtcomm.2016.03.003>

*DOI:*

[10.1016/j.mtcomm.2016.03.003](https://doi.org/10.1016/j.mtcomm.2016.03.003)

*Publication date:*

2016

*Document Version*

Peer reviewed version

[Link to publication](#)

*Publisher Rights*

CC BY-NC-ND

Creative Commons Attribution Non-Commercial No Derivatives licence

## University of Bath

**General rights**

Copyright and moral rights for the publications made accessible in the public portal are retained by the authors and/or other copyright owners and it is a condition of accessing publications that users recognise and abide by the legal requirements associated with these rights.

**Take down policy**

If you believe that this document breaches copyright please contact us providing details, and we will remove access to the work immediately and investigate your claim.

# Variability of the mechanical properties of hemp concrete

César Niyigena<sup>1</sup>; Sofiane Amziane<sup>1\*</sup>; Alaa Chateaneuf<sup>1</sup>,  
Laurent Arnaud<sup>(2\*)</sup>, Laetitia Bessette<sup>(3)</sup>, Florence Collet<sup>(4)</sup>, Christophe Lanos<sup>(4)</sup>, Gilles  
Escadeillas<sup>(5)</sup>, Mike Lawrence<sup>(6)</sup>, Camille Magniont<sup>(5)</sup>, Sandrine Marceau<sup>(7)</sup>, Sara Pavia<sup>(8)</sup>, Ulrike  
Peter<sup>(9)</sup>, Vincent Picandet<sup>(10)</sup>, Mohamed Sonebi<sup>(11)</sup>, Pete Walker<sup>(6)</sup>,

1 : Clermont université, Institut Pascal, Polytech' Clermont-Ferrand – 63174 Aubière Cédex,  
France

[cesar.niyigena@univ-bpclermont.fr](mailto:cesar.niyigena@univ-bpclermont.fr), [alaa.chateaneuf@univ-bpclermont.fr](mailto:alaa.chateaneuf@univ-bpclermont.fr)

\* Corresponding author and secretary of TC 236 BBM: [sofiane.amziane@univ-bpclermont.fr](mailto:sofiane.amziane@univ-bpclermont.fr)

2: Chair, Ecole Nationale Supérieure d'Arts et Métiers, ENSAM Cluny, France,

[laurent.arnaud@ensam.eu](mailto:laurent.arnaud@ensam.eu)

3: Centre Technique Louis VICAT, L'Isle d'Abeau, France, L'Isle d'Abeau,

[laetitia.bessette@vicat.fr](mailto:laetitia.bessette@vicat.fr)

4: LGCGM, Université Rennes 1, Rennes, France, [florence.collet@univ-rennes1.fr](mailto:florence.collet@univ-rennes1.fr),

[christophe.lanos@univrennes1.fr](mailto:christophe.lanos@univrennes1.fr)

5: Université de Toulouse, UPS, INSA, LMDC (Laboratoire Matériaux et Durabilité des  
Constructions), Toulouse, [camille.magniont@insa-toulouse.fr](mailto:camille.magniont@insa-toulouse.fr), [gilles.escadeillas@insa-toulouse.fr](mailto:gilles.escadeillas@insa-toulouse.fr)

6: BRE Centre for Innovative Construction Materials, University of Bath, Bath, UK

[m.lawrence@bath.ac.uk](mailto:m.lawrence@bath.ac.uk), [P.Walker@bath.ac.uk](mailto:P.Walker@bath.ac.uk)

7: Paris-Est, IFSTTAR, MAST, Marne-La-Vallée, France, [sandrine.marceau@ifsttar.fr](mailto:sandrine.marceau@ifsttar.fr)

8: Dept of Civil Engineering, Trinity College Dublin, Ireland, [Pavias@tcd.ie](mailto:Pavias@tcd.ie)

9: Lhoist, Lhoist Recherche et Développement S.A., Belgium, [ulrike.peter@lhoist.com](mailto:ulrike.peter@lhoist.com)

10: Université de Bretagne Sud, 6 : EA 4250, LIMatB, F-56100 Lorient, France,

[vincent.picandet@univ-ubs.fr](mailto:vincent.picandet@univ-ubs.fr),

11: Queen's University Belfast, School of Planning, Architecture & Civil Eng., Belfast, BT7

1NN, UK, [m.sonebi@qub.ac.uk](mailto:m.sonebi@qub.ac.uk)

## 1 Abstract

The focus of this study is on statistical analysis of hemp concrete properties. The main objective is to determine statistically the variability of the three main properties, which are: material density, compressive strength and Young's modulus. The analysis is done with respect to four main parameters, namely: the testing laboratory equipment and procedure, the hemp shiv type, the batch elaboration and finally the specimen size

38 Two types of hemp shiv have been used with two batches for each type. Two cylindrical  
39 specimen sizes have been considered: 11x22 cm and 16x32 cm. All the specimens were  
40 manufactured and dried in the same laboratory in order to ensure the repeatability and  
41 homogeneity of studied material. After 90 days of drying under the same conditions, the  
42 specimens were transported to ten different laboratories for compressive testing. Before testing, a  
43 drying protocol during 48 hours was applied by all laboratories for all specimens. Then, a unique  
44 protocol for compressive testing has been applied using the compressive testing machine in each  
45 laboratory. Finally, all data have been collected for statistical analysis. In this study, the results  
46 obtained by different laboratories show low variability for compressive strength and dry density;  
47 which is not the case for Young's modulus. Three probability distributions, namely: normal, log-  
48 normal and Weibull, have been proposed to fit the experimental results.

## 49 **2 Introduction**

50 The use of plant origin aggregates is nowadays considered as an essential way in manufacturing  
51 environmentally friendly building materials. Many aggregates of this kind exist and are used in  
52 the construction industry, either in new structures or renovation of existing buildings, for  
53 example, aggregates of sunflower, hemp shiv...[1]–[3]. In contrast to aggregates of mineral  
54 origin, plant origin aggregates are renewable and carbon neutral materials. They also have other  
55 advantages such as good thermal and acoustic insulation properties. However, the major  
56 drawback is related to their low mechanical performance [4]–[6].

57 For more than one decade, the researches on these materials have not ceased to increase. A very  
58 recent study was conducted by Binici et al. [7] on the use of sunflower and waste cotton textiles  
59 for manufacturing insulation. Other researches have been also conducted on the use of the hemp  
60 shiv in insulation [8]. In the framework of the present study, the herein literature review focuses  
61 on concrete made from hemp shiv, and particularly on its mechanical behavior.

62 Several parameters influence the mechanical properties of hemp concrete. They include among  
63 others, the nature of its constituents such as the aggregate size, the type of binders, and the  
64 manufacturing method, such as the compaction energy and the molding method [9], [10].

65 The density of hemp concrete is related to quality and quantity of constituents, the aggregate  
66 size, their porosity and the energy of compaction. Considering all these parameters, different and  
67 variable density values are found in the literature. In a study conducted by Cerezo [9], several  
68 formulations were tested and specimens between 12 and 29 were manufactured for each  
69 formulation. The density distribution of each formulation was homogenous with a coefficient of  
70 variation between 1.5% and 3.5%.

71 For ten different formulations, Cerezo obtained the final average density values ranging from  
72  $256 \text{ kg/m}^3$  to  $782 \text{ kg/m}^3$ . Although she considers in her analysis that the series have a low  
73 dispersion, this is not true at all levels. This is only valid at the intra-formula level, but not for  
74 the inter-formula level, because in this latter case, considerable dispersion is observed for both  
75 final and initial mean values; which vary in the range of  $455 \text{ kg/m}^3$  to  $1140 \text{ kg/m}^3$ .

76 In parallel, F. Collet [11] has determined the density of two kinds of hemp concrete (batch A for  
77 one hemp and B for another) by using three different methods: weighing and dimension  
78 measurement, pycnometer and mercury porosimeter. The first step of its study is to determine  
79 the representative elementary volume. The density variation obtained between the samples of  
80 5 cm and 20 cm edges was of approximately 4%. Thanks to this low variation in the average  
81 density between samples, she concluded that samples of 5 cm edge are representative of hemp  
82 concrete. However, the differences in results were observed with respect to the used  
83 measurement method. For the pycnometer test, the density for batch A is  $390 \text{ kg/m}^3$  and  
84  $425 \text{ kg/m}^3$  for batch B. The test of mercury porosimeter gave a dry density of  $609 \text{ kg/m}^3$  and  
85  $664 \text{ kg/m}^3$  for batches A and B, respectively. With the method of weighing and measuring  
86 dimensions for two different series in batch A, she got  $408 \text{ kg/m}^3$  and  $406 \text{ kg/m}^3$  with 6.6% and  
87 2.7% of coefficient of variation for the first and second series, respectively. Finally, in batch B,

88 the mean value density for sample of 5cm edge cube is  $438 \text{ kg/m}^3$  with a standard deviation  
89 equal to 5.7%.

90 Another study has been conducted by Nguyen [10] on two types of hemp shives: the first with  
91 pure shiv particles (CP), while the other one contains fibres (CF). It is shown that there is no  
92 difference between the two shives in terms of density. For specimens tested under the same  
93 conditions, the observed difference was less than 2%. Results obtained were in the range of  
94  $450 \text{ kg/m}^3$  to  $800 \text{ kg/m}^3$  at 90 days. This dispersion is mainly based on three main parameters of  
95 formulation and manufacturing process, namely the binder/aggregate ratio; the water/binder ratio  
96 and the compaction strength.

97

98 Nguyen [10] also highlighted parameters influencing the compressive strength. Because of the  
99 low rigidity of particles, hemp concrete has a very ductile behavior in both compression and  
100 tension. Based on test results, he obtained a compressive strength, for a strain equal to 7.5% after  
101 28 days, which varies between 0.2 MPa and 3.6 MPa. On her side, Cerezo [9] obtained the  
102 compressive strength ranging between 0.25 and 1.15 MPa. For low binder content, the  
103 compressive strength is around 0.25 MPa. For intermediate dosage, it varies between 0.4 and  
104 0.8 MPa and for high binder content, it is 1.15 MPa. She concluded that mechanically, hemp  
105 concrete is characterized by an elastic-plastic behavior, and that this material must be used with a  
106 support structure to meet structural requirements.

107 Other parameters may also influence the mechanical behavior of hemp concrete such as drying  
108 conditions, the age of hemp concrete and the size of hemp particles [12]. Taking into account  
109 these parameters, Arnaud and Gourlay [12] obtained compressive strength, which varies between  
110 0.35 MPa and 0.85 MPa for the age of 21 days to 24 months. Increasing the energy of  
111 compaction during the manufacturing process may enhance the maximum compressive strength.  
112 However it has been proven that the compressive strength is limited to 3 MPa for a compaction  
113 pressure between 0.6 MPa and 1 MPa [13]. Nguyen [10] obtained a compressive strength beyond

114 3.5 MPa at 28 days by using a compaction stress maintained during 48 hours before demoulding  
115 the hemp concrete fresh paste.

116

117 Young's modulus values found in the literature have also high variability and the methods used  
118 for its calculation are also different. According to Cerezo [9], the Young's modulus is defined as  
119 the slope at the origin of the strength-strain curve by considering the validity of the small strain  
120 assumption. Young's modulus varies from 1 to 3 MPa for low binder content; 32 to 95 MPa for  
121 intermediate dosages and 100 to 160 MPa for high dosage. For various formulations,  
122 Nguyen [10] obtained, at 90 days, the Young's modulus between 25 MPa and 176 MPa; using  
123 pure hemp particles. According to this study, the Young's modulus of a given specimen is  
124 calculated based on the strongest increase in the ratio strength/strain recorded at the beginning of  
125 the loading stage.

126 The results in the literature show that the values for properties of hemp concrete have a great  
127 variability and are sensitive to many factors. The literature shows also that there is a lack on  
128 consideration of the accuracy of testing instruments used and the variability of results due to  
129 experimentations. For example on one hand, Mounanga et al. [14] studied the influence of the  
130 composition and method of implementation on the development of mechanical properties of  
131 hemp concrete. On the other hand an analysis of the variability on the self-compacting concrete  
132 was led by Almeida Filho et al. [15]. In this last study, in order to reduce the impact of statistical  
133 errors, they used results from 10 to 24 specimens for each type of formulation.

134 As other materials, the variability performance of hemp concrete has two origins: intrinsic  
135 variability of the studied material itself and uncertainty caused by insufficient information with  
136 respect to these mechanical performance [16]. It is of course fundamental, even though not  
137 necessarily easy, to distinguish between these two sources through appropriate statistical

138 modeling. For this reason, a statistical study is required to assess the certainty and variability of  
 139 the results for the mechanical properties of hemp concrete.

140 In the present study, a statistical analysis of the results taking into account two types of hemp  
 141 shives, four types of batches, and two specimen sizes, is carried out in order to define the  
 142 probability distributions fitting the experimental results. The considered characteristics are:  
 143 density, maximum compressive strength and Young's modulus. The mechanical compression  
 144 tests were conducted in ten different laboratories, which allow analyzing the impact of the  
 145 laboratory on the estimation of material characteristics.

### 146 **3 Material and methods**

147 The specimens in this study, were manufactured using two hemp shives with the same binder,  
 148 prompt natural cement (PNC) and citric acid. The characterization results for bulk density, water  
 149 absorption and particle size distribution, are given for both shives in section 3.1. The protocols  
 150 and methods related to manufacturing, mixing process and compressive testing are given in  
 151 section 3.2. The compressive tests have been made using different machines under the same  
 152 protocol, and the experimental results were collected for statistical analysis. The considered  
 153 parameters during the mixing and manufacturing process are provided in Table 1.

Specimen sizes		Specimen 11cmX22cm				Specimen 16cmX32cm				Total per laboratoire
Type of batch		I	II	III	IV	I	II	III	IV	
Laboratory name	<b>Institut Pascal (A)</b>		3		3		3			9
	<b>Belfast (B)</b>		2	2						4
	<b>Trinity (C)</b>	2			2					4
	<b>LMDC Toulouse (D)</b>	2		2						4
	<b>Bath univ (E)</b>		3		3					6
	<b>LGCGM Rennes (F)</b>		3		3				3	9
	<b>Vicat (G)</b>	3	1	6				3		13
	<b>IFSTTAR (H)</b>		3		4					7
	<b>LiMATB Lorient (I)</b>	3		3		3				9
	<b>Lhoist (J)</b>	2		2						4

<b>Total per batch</b>	<b>12</b>	<b>15</b>	<b>15</b>	<b>15</b>	<b>3</b>	<b>3</b>	<b>3</b>	<b>3</b>	<b>69</b>
<b>Total per specimen size</b>	<b>57</b>				<b>12</b>				

Table 1: Summary datas for tested specimens

### 3.1 Raw material characterization

#### 3.1.1 Shives

The shives used in this study are from the same producer, but they were stored in two separate places. One bag with the reference 13 0173 KANABAT at the ENTPE laboratory, noted S1 shiv, and the other one at Vicat laboratory with the reference 13 0174 KANABAT, noted S2 shiv. Samples, of about 1 kg each, have been taken and characterization tests were conducted according to the protocol proposed in [17], [18]. The characterization tests were made in laboratories A and G; they include among others: bulk density, water absorption and particle size distribution by two methods: mechanical sieving and image analysis.

##### 3.1.1.1 Bulk density (kg/m<sup>3</sup>)

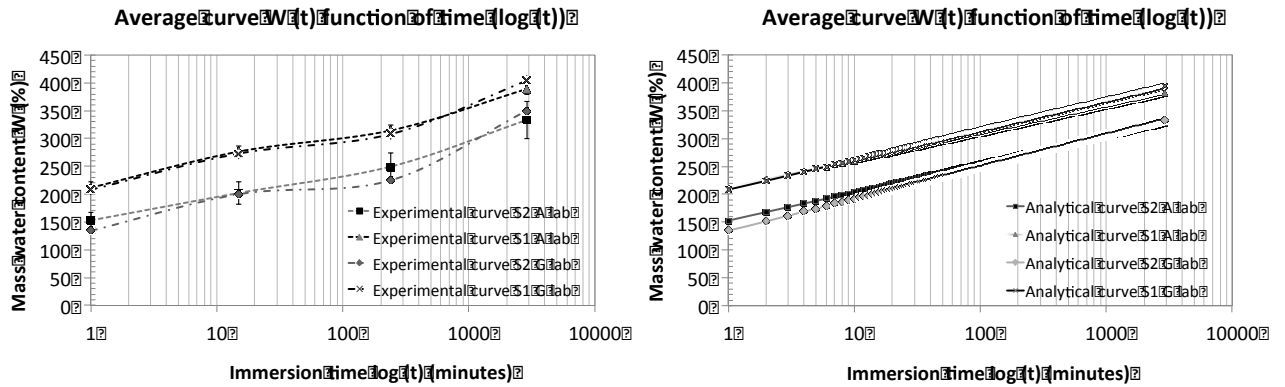
Tests of bulk density were conducted according to the protocol in [17]. The results obtained for the two types of hemp shives show that there is slight difference, with 143.6 kg/m<sup>3</sup> for S1 and 147.5 kg/m<sup>3</sup> for S2. Observed differences may be due to errors in manual handling or to the accuracy of the used method. Whatever, these differences are acceptable as they are below 2.7%.

##### 3.1.1.2 Water absorption

Tests of water absorption were conducted according to the protocol in [17]. The water absorption capacity of these aggregates is determined gravimetrically by applying the expression:  $W(t) = \frac{M(t)-M_0}{M_0} \times 100$ , where  $W(t)$  is the water absorption ratio at time  $t$ ,  $M(t)$  the soaked hemp shive aggregate mass at time  $t$ , and  $M_0$  is the initial oven-dried aggregate mass. The water absorption  $W$  is calculated after soaking for 48 hours using the expression:  $W = IRA + K_1 \times \text{Log}(t)$ , where  $K_1$  is a kind of diffusion rate in shiv cells.  $IRA$  represents the characteristic factor of the



176 external water adsorption on the shiv surface, and is related to the first minute measurement. The  
 177 results are shown in Figure 1.



178 **Figure 1: Water absorption curves (experimental and analytical) for S1 and S2, immersion time in log scale**

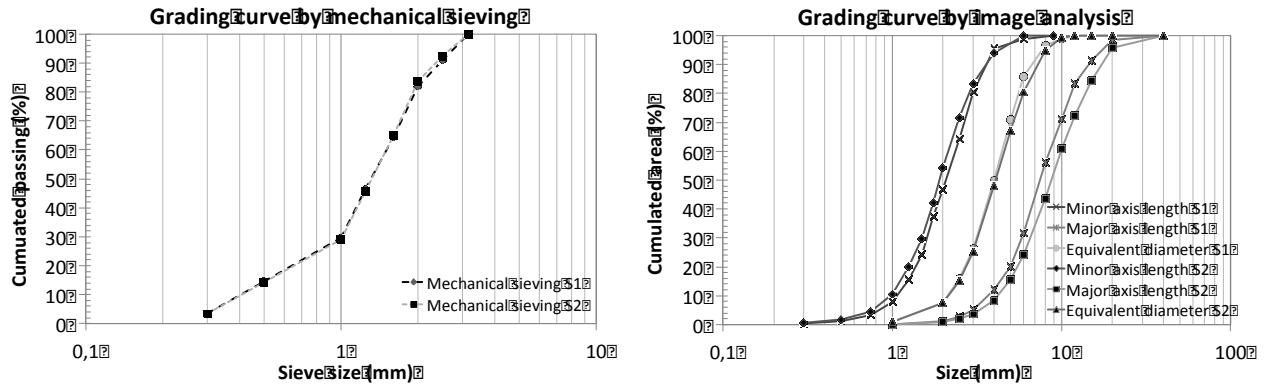
179 For comparison purpose, the test was conducted in two separate laboratories: A and G. The  
 180 results are almost identical for both laboratories especially for initial mass water absorption but  
 181 with a slight difference in the case of S2. In terms of initial water absorption, they are different,  
 182 with initial mass water absorption around 150% and 200% for S2 and S1, respectively.  
 183

### 184 **3.1.1.3 Particle size distribution**

185 The particle size distributions are analyzed by using two methods: mechanical sieving and image  
 186 analysis. The first method is the reference technique when dealing with characterization of  
 187 mineral aggregates [19]. For the analysis of vegetal origin aggregates, square mesh sieves are of  
 188 limited interest because they do not take into account the elongation of aggregates [20].

189 By the sieving method, Nozahic [17] made a comparative study on hemp shiv and sunflower  
 190 aggregates and realized that the two types are almost similar in size. He concluded that  
 191 mechanical sieving technique is not yet suitable neither for determining the size of a  
 192 lignocellulosic particle aggregate, nor for comparison of two different kinds of aggregates. His  
 193 conclusion is in accordance with the results of mechanical sieving obtained in the present study  
 194 and shown in Figure 2 where both S1 and S2 are almost identical.

195



196 **Figure 2: Grading curve by mechanical and image analysis methods for S1 and S2.**

197 However, the second method of image analysis brings clearly richer information than the  
 198 previous method. This latter has been used and published for the first time in 1996 [21]. In our  
 199 study, we have used a similar approach to the work in [10], [12], [18] but with the ImageJ  
 200 software [22] and a sample of 3g has been considered for each hemp shiv. The comparison of  
 201 both S1 and S2, illustrated in Figure 2, reveals, in contrast to sieving method, significant  
 202 differences between both axes. The obtained specific surface areas are  $13187\text{mm}^2$  and  
 203  $13913\text{mm}^2$  for S2 and S1, respectively.

## 204 3.2 Preparation of compression test specimens

### 205 3.2.1 Mix proportioning

206 In construction, hemp concrete has several applications, such as: filling wooden frame walls,  
 207 roofing insulation, etc. To each application correspond a given number of specifications such as  
 208 minimum compressive strength and Young's modulus [23], which can be met by specific  
 209 formulations. For the purpose of the herein study, it has been decided to use the formulation for  
 210 wall application [23]; as the objective is not to analyse the formulation, any other one may have  
 211 been used. The quantities in kilograms per batch of 80 liters are detailed in Table 2.

Shiv (kg)	PNC (kg)	Citric Acid	Water (kg)	Ratio Water/PNC	Ratio Shiv/PNC
8	20	0.06	19,2	0.96	0.4

212 **Table 2: Tested formula for wall application per batch**

### 213 **3.2.2 Mixing of hemp concrete**

214 Each constituent is weighed in buckets. The shiv is put in the mixer, then the PNC with Citric  
215 Acid is introduced; they are then mixed with 40% of the mixing speed for few minutes. Water is  
216 added and the mixing retaining. The mixing speed is increased to 50% then kept until  
217 homogeneous mixture is obtained. Finally, for the use, the mixer is emptied into a wheelbarrow.

#### 218 **3.2.2.1 Casting method for specimens**

219 The mold is filled by 5 or 6 layers; two consecutive layers must be compacted using a suitable  
220 tool. For the last layer, the upper surface is kept smooth and the specimen is weighed. A cover is  
221 put and the specimen is kept returned for a period of at least 72 hours after which the cover and  
222 the bottom are removed. The specimen is then kept at 20°C and 55% of relative humidity for 90  
223 days. To ensure that the tested specimens are identical, they were manufactured the same day  
224 and were dried for 90 days under the same conditions at the laboratory G. After this drying  
225 period, samples were transported to ten different laboratories for compression testing.

#### 226 **3.2.2.2 Protocol of the compressive test**

227 Tests were done under the same conditions, the detailed below protocol, was carefully followed  
228 by all laboratories. Specimens were dried under an oven at 50°C for 48 hours before the  
229 compressive test.

- 230 1. Weighing the specimen with the mold; then remove the mold using a cutter: remove the  
231 sample ends then cut just the surface of the mold; and mark it with the same reference on  
232 the mold;
- 233 2. weighing the specimen without the mold; then put it in an oven at 50°C until a  
234 stabilization of weight equal to +/-2%; and left it in a sealed plastic bag until the test day;
- 235 3. before the test, measure three diameters (at top, bottom and middle) and the height every  
236 120°;

- 237 4. no surfacing of the sample and a perfect parallel plates is made before the starting of the  
238 test;
- 239 5. The test must be displacement controlled at the rate of 3mm/min for loading stage. The  
240 unloading stage should be 6mm/min or free if it is not possible to control it;
- 241 6. Applying three load cycles depending on specimen size:
- 242 **1<sup>st</sup> cycle:** loading is done from 0 to 1% of relative deformation and unloading until zero  
243 load or zero displacement; 2<sup>nd</sup> and 3<sup>rd</sup> cycles are the same as the 1<sup>st</sup>, the strain is always  
244 increased by 1% for each cycle. The final loading: from 0 until the total failure load of  
245 the specimen (maximum of 20% of strain) and unloading until zero load (when possible)  
246 or zero displacement.
- 247 Voluntarily for some specimens, in the case of lab C: I-11-7; I-11-8; IV-11-11; IV-11-12; and I  
248 lab: I-11-2; III-11-11; I-16-1; the compressive tests were done with a monotonic loading.

### 249 **3.3 Mechanical analysis of hemp concrete properties**

#### 250 **3.3.1 Young's modulus (Floating modulus on loading stage)**

251 For the hemp concrete, the Young's modulus is not constant because of strong nonlinear  
252 behavior even in the elastic domain. As applied in soil mechanics [24], the hemp concrete may  
253 have also four different types of moduli, which can be calculated as shown in Figure 3. The  
254 initial tangent modulus  $E_{ini}$  corresponds to the slope at the beginning of loading in the stress-  
255 strain curve. The various loading levels of the curve may be described by a “secant” modulus  
256  $E_{sec}$ , defined by the slope of the line connecting the origin at the current point and a “tangent”  
257 modulus  $E_{tan}$ , may be determined by the slope of the curve in the neighborhood of a given point.  
258 In cyclic loading, the modulus  $E_{CYC}$  may be determined by the slope of the line connecting the  
259 two points reversing the strain direction. In opposite to initial Young's modulus, which might  
260 have errors due to small strains, the tangent modulus calculated on loading phase with higher

261 strain values, which allows reducing the errors. In the current study, the modulus is calculated  
 262 according to the procedure named “floating modulus”.

263 **3.3.1.1 Young’s modulus calculation method (floating modulus).**

264 According to the frequency of data acquisition (nearly 10Hz or 10 values per second, Figure 4):  
 265 the loading steps are identified then the floating modulus is calculated in each step using:  $E = \frac{\Delta\sigma}{\Delta\varepsilon}$ ;  
 266 where: E is the modulus around a given point,  $\Delta\sigma$  and  $\Delta\varepsilon$  are strength and strain respectively  
 267 considered between -5 and +5 seconds around the considered point. The maximum of modulus is  
 268 identified for each step. The floating Young’s modulus value is therefore, the mean value of  
 269 maximum values obtained at the 2<sup>nd</sup>; 3<sup>rd</sup> and 4<sup>th</sup> loading steps.

270

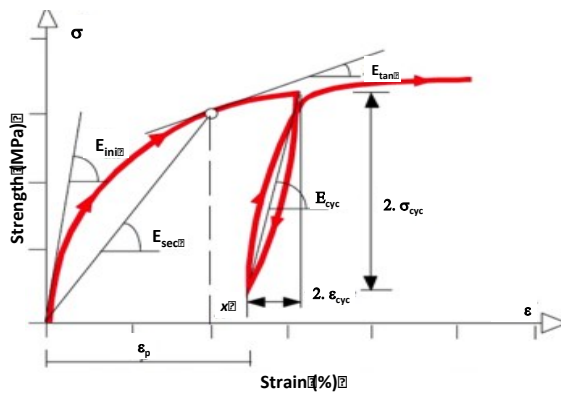


Figure 3: Modulus definitions [24]

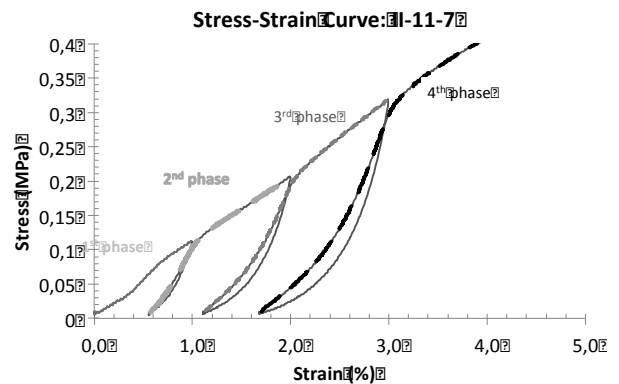


Figure 4: Identification of the loading phases to calculate the floating modulus

271 **3.3.2 Characteristic values and coefficient of variation (COV)**

272 The characteristic value of a quantity measured experimentally corresponds to the representative  
 273 value to be included in the computation procedure for a purpose of design, maintenance or  
 274 rehabilitation or any other decision process. For example, the compressive strength of concrete is  
 275 defined as the resistance below which there are only 5% of test results [25]. Under the  
 276 assumption of normality, it is proposed to calculate the characteristic value of concrete  
 277 compressive strength as follows:  $f_{ck} = f_{cm} - 1.645\sigma_{fc}$  (1); where  $f_{ck}$  is the characteristic value,

278  $f_{cm}$  is the average value of all the test results and  $\sigma_{fc}$  is the standard deviation of test results; the  
 279 coefficient 1.645 corresponds to a 5% quantile of the normal gaussian distribution. It is to note  
 280 that all experimental results were subjected to the test of normality and the test was not rejected.  
 281 Then formula in (1) is used in the current study with the probability level of 5%.

282

283 The coefficient of variation (COV) indicates the dispersion of the experimental results; it is  
 284 calculated by the ratio between the standard deviation and the mean value, in (%). Table 3 gives  
 285 accepted limits of standard deviation and coefficient of variation for concrete [15].

286

Quality control	Accepted limits for the coefficient of variation (%)
A (excellent)	10
B (average)	15
C (poor)	20

287 **Table 3: Accepted limits of variability of concrete compressive as a function of the quality control [15]**

### 288 3.3.3 Statistical tests

289 According to the samples used, they can be classified in eight populations, four batches for both  
 290 hemp shives and two specimen sizes in each batch. Using statistical tests, like Student test [26]  
 291 by comparing samples two by two or ANOVA [27], one can determine whether the samples  
 292 originate from the same population or not. ANOVA (ANalysis Of Variance) is a generalization  
 293 of the mean comparison with K subpopulations or samples. K equal to 2 corresponds to Student  
 294 test. For ANOVA test, if the null hypothesis  $H_0$  is rejected, it is not possible to know the  
 295 subpopulation that doesn't belong to the population. It is then necessary to use Student test by  
 296 comparing subpopulations two by two. The main objective of these statistical tests is to provide  
 297 the best synthetic information about the characteristics of the population. For a given population,  
 298 one can determine the probabilistic distribution that fits better the observed data. The parametric  
 299 Student test [26] has been used by comparing the mean values and a significance level  $\varepsilon=0.05$   
 300 has been considered. The tests were carried out using the software XLSTAT [28].

301 **3.3.3.1 Identification of mean values**

302 For a Student test, two conditions are necessary: the two compared samples must satisfy the  
 303 Gaussian distribution, this condition is very often verified, and the second condition is that the  
 304 two samples must have the same variance [26]. In this study, both conditions have been satisfied  
 305 and the Student test has been performed. It leads to rejection results in some cases (Table 4).

Specimen size	Test between batches	Test results for :		
		Density	Maximum compressive strength	Young's modulus
11x22cm	I and II	not reject	reject	not reject
	III and IV	reject	reject	reject
16x32cm	I and II	not reject	not reject	not reject
	III and IV	reject	reject	not reject

306 **Table 4: Student test for batches in both specimen sizes**

307  
 308 Consequently it is not possible to combine samples into one population for statistically  
 309 meaningful size (i.e. statistical analysis requires a minimum number of samples to get acceptable  
 310 error; the required sample size depends on the statistical property or test to be applied, e.g. mean,  
 311 standard deviation, density function fitting, etc.). In order to assess the dispersion of data it is  
 312 proposed to adjust the sample mean values with respect to the reference mean; this leads to shift  
 313 the whole probability distributions, such that their mean values become centered on the same  
 314 reference point. It is important to note that this adjustment is only applied to characterize the  
 315 sample standard deviation and distribution type, but not to determine the mean values. By  
 316 applying this approach, Student's test results for all samples are not rejected. For more clarity,  
 317 the approach is detailed below.

318 Consider two given samples  $X = \{x_1; x_2; \dots; x_i\}$  and  $Y = \{y_1; y_2; \dots; y_i\}$ ; their respective mean  
 319 values  $\bar{X}$  and  $\bar{Y}$ ; if one wants to adjust the mean value of Y to the mean value of X, then one has  
 320 to proceed as following: calculate the adjusted mean value  $\bar{Y}'$  by using the formula:

$$\bar{Y}' = \frac{1}{n} \sum_{i=1}^n [y_i + (\bar{X} - \bar{Y})] \quad (3)$$

321 If the above formula in (3) is developed, it comes that:  $\overline{Y'} = \overline{Y} + \overline{X} - \overline{Y}$  and finally gives:  
322  $\overline{Y'} = \overline{X}$ , but for this time with variables of  $Y' = \{y'_1; y'_2; \dots; y'_n\}$  which are different of those  
323 of  $X = \{x_1; x_2; \dots; x_m\}$  and  $Y = \{y_1; y_2; \dots; y_n\}$  respectively. This approach leads to two different  
324 samples having the same mean value; hence it allows combining both samples for scatter and  
325 goodness-of-fit analyses.

## 326 **4 Results and Discussions**

327 By considering the testing laboratory, the batch, the hemp shiv type and the specimen size, the  
328 studied properties are: density, compressive strength and Young's modulus. In order to simplify  
329 the notations, the following abbreviations are used: MV for the Mean Value, SD for the Standard  
330 Deviation, COV for the Coefficient Of Variation and CV for the Characteristic Value.

### 331 **4.1 Repeatability of the results between testing laboratory**

332 As seen, the density, the compressive strength and the Young's modulus may vary according to  
333 many parameters such as: compaction energy [9], [10], measuring method [11] and hemp shiv  
334 type [12]. In this section, analyses for results in Table 5 and Table 6 focus on the impact of  
335 testing laboratories.

336

337

338

339

340

341

342

343



Lab name	Density (kg/m <sup>3</sup> )				Compressive strength (MPa)				Young's modulus (MPa)			
	MV	SD	COV	CV	MV	SD	COV	CV	MV	SD	COV	CV
<b>All lab</b>	471.22	28.28	6.00	424.84	0.45	0.05	10.69	0.37	36.86	7.08	19.22	25.24
<b>A</b>	496.88	31.25	6.29	445.63	0.49	0.03	5.46	0.45	33.82	4.58	13.55	26.31
<b>B</b>	476.93	17.64	3.70	448.00	0.48	0.04	8.45	0.41	40.72	5.01	12.29	32.51
<b>C</b>	471.44	29.14	6.18	423.65	0.44	0.03	6.73	0.39				
<b>D</b>	465.95	18.47	3.96	435.65	0.42	0.06	13.87	0.32	34.16	3.43	10.05	28.53
<b>E</b>	468.44	29.35	6.27	420.31	0.49	0.02	4.47	0.45	40.87	8.63	21.13	26.71
<b>F</b>	465.20	26.58	5.71	421.61	0.49	0.04	8.36	0.42	35.27	3.18	9.02	30.05
<b>G</b>	453.72	12.96	2.86	432.47	0.41	0.04	9.42	0.34	35.33	5.11	14.45	26.96
<b>H</b>	472.48	35.16	7.44	414.82	0.46	0.05	10.56	0.38	44.01	9.89	22.47	27.79
<b>I</b>	452.71	12.64	2.79	431.99	0.45	0.05	10.78	0.37	36.01	5.23	14.53	27.43
<b>J</b>	514.62	15.53	3.02	489.16	0.43	0.04	8.30	0.37	28.81	4.75	16.49	21.02

344 **Table 5: Density, maximum compressive strength and Young's modulus values per laboratory, specimens**  
345 **11x22cm**

346  
347

Lab name	Density (kg/m <sup>3</sup> )				Compressive strength (MPa)				Young's modulus (MPa)			
	MV	SD	COV	CV	MV	SD	COV	CV	MV	SD	COV	CV
<b>All lab</b>	443.53	29.70	6.70	394.81	0.38	0.06	16.77	0.28	35.58	4.46	12.54	28.26
<b>A</b>	423.41	1.96	0.46	420.20	0.32	0.02	5.07	0.29	30.97	2.28	7.35	27.24
<b>F</b>	495.45	8.56	1.73	481.42	0.48	0.02	4.47	0.44	39.13	3.04	7.77	34.14
<b>G</b>	445.47	4.10	0.92	438.74	0.39	0.00	1.17	0.39	32.77	2.95	9.02	27.93
<b>I</b>	420.38	2.10	0.50	416.93	0.32	0.01	4.15	0.29	39.06	3.42	8.76	33.45

348 **Table 6: Density, maximum compressive strength and Young's modulus values per laboratory, specimens**  
349 **16x32cm**

#### 350 **4.1.1 Density**

351 The analysis of results obtained by different labs shows small variability for a given specimen  
352 size; with a COV of 6.0% and 6.7% for all labs in both cases small and large specimens  
353 respectively, as shown in Table 5 and Table 6. The observed difference in the characteristic  
354 values of the density, while comparing both specimen sizes will be discussed in section 4.4.

355 Within each category of a specimen size, the observed results have excellent quality with  
356 reference to the accepted limits in Table 3.

#### 357 4.1.2 Maximum compressive strength

358 For small specimens 11x22cm, the compressive strength results show values ranging from 0.32  
359 MPa for lab D to 0.45 MPa for labs A and E, as shown in Table 5. In general, there is no  
360 considerable variability in the obtained results. The method and machines used give similar  
361 results for the characteristic strength with 10.69% of COV for all labs. In case of large specimen  
362 size, the COV is 16.77% for all labs, as shown in Table 6; this high variability leading to poor  
363 quality of the strength. For small specimen size, the quality is excellent with average COV close  
364 to the accepted limits as given in Table 3.

#### 365 4.1.3 Young's modulus

366 Results taking into account the impact of testing laboratory on the evaluation of Young's  
367 modulus show mean values ranging from 28.81 MPa to 44.01 MPa. In fact, there are two classes  
368 of values, one in the interval from 33 MPa to 38 MPa, and the other in the interval from 40 MPa  
369 to 45 MPa, the value of lab J looks like an isolated case. For larger specimen size, the results  
370 seem to be homogeneous with a maximum COV equal to 9.02%. These results must be analysed  
371 carefully as the number of specimens are not statistically large. Two laboratories have high COV  
372 values of 22.47% and 21.13%, leading to a COV for all laboratories equal to 19.22 %, (Table 5).  
373 With such COV, the results are of poor quality compared to the limits in Table 3. There is a  
374 significant impact of the testing laboratory on the Young's modulus where the obtained results  
375 have poor quality, although the obtained results have excellent quality for the compressive  
376 strength. This has to be considered carefully, since it is known that there is a strong correlation  
377 between the Young's modulus and the compressive strength. The main explanation to this  
378 observation is the nonlinear behavior of strength-strain curve, because the maximum strength  
379 was calculated beyond the linear phase of the curve, as detailed in section 4.5.

380 **4.2 Repeatability of the results between batches**

381 Although the batch type is not yet studied in the literature to our knowledge, but this parameter  
 382 may influence the results as shown in Table 7.

Batch type	Compressive strength (MPa)				Young's modulus (MPa)			
	MV	SD	COV	CV	MV	SD	COV	CV
<b>I. (11x22cm)</b>	0.41	0.04	9.78	0.35	30.46	3.81	12.52	24.21
<b>II. (11x22cm)</b>	0.47	0.04	8.18	0.41	33.35	3.85	11.53	27.04
<b>III. (11x22cm)</b>	0.44	0.05	10.93	0.36	38.04	4.54	11.94	30.59
<b>IV. (11x22cm)</b>	0.48	0.04	8.16	0.42	44.07	7.46	16.93	31.84
<b>I. (16x32cm)</b>	0.32	0.01	4.15	0.29	39.06	3.42	8.76	33.45
<b>II. (16x32cm)</b>	0.32	0.02	5.07	0.29	30.97	2.28	7.35	27.24
<b>III. (16x32cm)</b>	0.39	0.00	1.17	0.39	32.77	2.95	9.02	27.93
<b>IV. (16x32cm)</b>	0.47	0.02	4.51	0.44	39.13	3.04	7.77	34.14

383 **Table 7: Maximum compressive strength and Young's modulus values per batch, specimens 11x22cm and**  
 384 **16x32cm**

385

386 **4.2.1 Maximum compressive strength**

387 The results for compressive strength show that the values for batch IV are higher for both  
 388 specimen sizes than is the case for Young's modulus. Batches from S2 seem to have high values  
 389 as shown in Table 7. This trend is analyzed in section 4.3 where the impact for both shives is  
 390 studied. As it will be discussed in the next section for the Young's modulus, the compressive  
 391 strength shows also some variability for different batches, therefore the mixture in different  
 392 batches must be carefully performed.

393 **4.2.2 Young's modulus**

394 Mean values for Young's modulus increase from Batch I with 30.46 MPa to Batch IV with  
 395 44.07 MPa as given, Table 7. There is no explanation for this observed trend. However, even  
 396 with this trend, it is clear that batches from the same shiv have comparable results. In batch IV,  
 397 the COV equal to 16.93% is greater than other batches, as this one had been manufactured the

398 latest, maybe the operators did not maintain the same conditions (e.g. compaction energy...)  
 399 since the beginning up to the end. As this trend is not the same case for large specimens, the  
 400 justification given above is not necessarily true. For both cases (small and large specimen sizes),  
 401 an average quality is observed, with respect to limits in Table 3. This means that the batch does  
 402 not have a great impact on the results, but sometime it may cause variability, as it is the case of  
 403 batch IV. Therefore, it is necessary to be careful when mixture is done in different batches.

#### 404 4.3 Repeatability of the results for different hemp shiv types

405 Arnaud and Gourlay [12] studied the impact of hemp shiv; they concluded that the use of smaller  
 406 shiv results in concretes whose higher mechanical properties at long term. Nguyen [10]  
 407 compared two shives one pure another containing fibers; as conclusion to its study there was no  
 408 big difference on their mechanical properties. In our study, some differences have been  
 409 observed, according to the results given in Table 8.

Hemp shiv	Density (kg/m <sup>3</sup> )				Compressive strength (MPa)				Young's modulus (MPa)			
	MV	SD	COV	CV	MV	SD	COV	CV	MV	SD	COV	CV
S2 (11x22cm)	488.88	23.11	4.73	450.9	0.46	0.05	10.38	0.38	43.45	6.72	15.47	32.43
S1 (11x22cm)	451.61	19.17	4.25	420.1	0.44	0.05	10.79	0.37	31.86	3.14	9.87	26.71
S2 (16x32cm)	469.78	27.08	5.76	425.3	0.43	0.05	10.58	0.36	32.77	2.95	9.02	27.93
S1 (16x32cm)	422.10	2.51	0.60	417.9	0.32	0.01	4.00	0.30	30.97	2.28	7.35	27.24

410 **Table 8: Density, maximum compressive strength and Young modulus values per hemp shiv**

##### 411 4.3.1 Density

412 When comparing both hemp shives in terms of density, slight differences are observed between  
 413 the obtained densities, even with the specimen size. 420.16kg/m<sup>3</sup> and 450.97kg/m<sup>3</sup> with 417.98  
 414 kg/m<sup>3</sup> and 425.38kg/m<sup>3</sup> characteristic values for S1 and S2 in both small and large specimen  
 415 sizes, respectively are obtained as shown in Table 8. Large values have been observed for S2,  
 416 which is consistent with the drying kinetics.

417

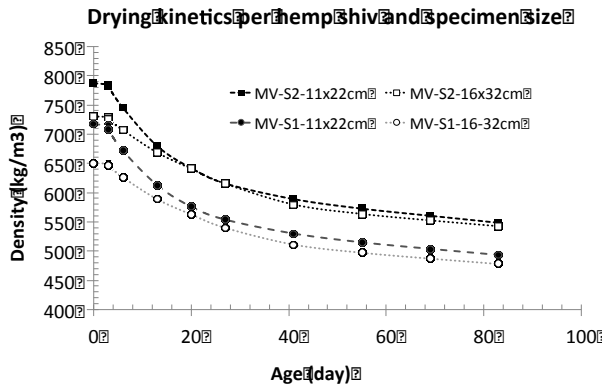


Figure 5: Drying kinetics per hemp shiv and specimen size

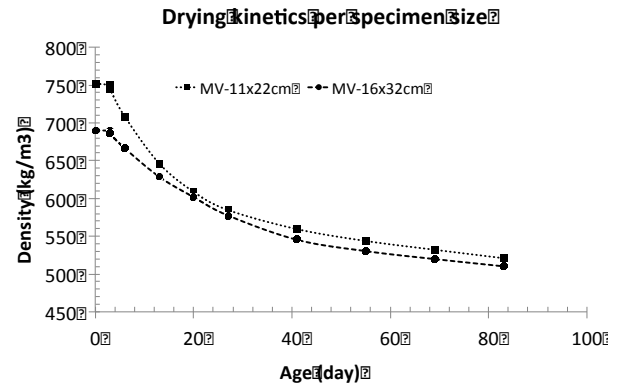


Figure 6: Drying kinetics per specimen size

418 According to the drying kinetics in Figure 5, it appears that the drying is only affected by the  
 419 specimen size, which seems normal, because they dry faster since they have a greater specific  
 420 area than larger specimens. On the other hand, a difference in fresh density is also observed  
 421 depending on both specimen sizes and hemp shiv types. Small specimens have a higher fresh  
 422 density than the large specimen, which could be explained by a greater compaction (same  
 423 "compaction energy" applied by the operator on a smaller area). The specimens made from S2  
 424 have a higher fresh density than those from S1, which means that, they were more compacted.  
 425 The initial water contents measured are 10,18% and 11,12% for S1 and S2 respectively. This  
 426 difference in initial water content between S1 and S2 confirm our results. As the water content of  
 427 the S2 was more important than in S1, the initial absorption of water was reduced (which is the  
 428 case according to results in Figure 1) and S2 was more easily compacted which explains the  
 429 high value for fresh density.

#### 430 4.3.2 Maximum compressive strength

431 The observed compressive strength results are 0.38 MPa and 0.37 MPa for small specimen size;  
 432 with 0.36 MPa and 0.30 MPa for large specimen size both for S2 and S1, respectively. The  
 433 maximum strength values for S2 are greater than for S1; this trend is the same for Young's  
 434 modulus.

### 435 **4.3.3 Young's modulus**

436 For both specimen sizes, results show that, Young's modulus values for S2 are greater than for  
437 S1 values. Observed results show also a high variability for S2 with a COV equal to 15.47%.  
438 This is probably due to the fact that batch IV is for S2 and as shown in the previous section,  
439 there is a high variability within this batch.

440 With respect to the type of shiv, in both cases of Young's modulus and compressive strength:  
441 these differences can be explained by the fact that, since S2 has a small specific area  $13187\text{mm}^2$ ,  
442 versus  $13913\text{mm}^2$  for S1, the hemp particles are better coated by the binder during the mixing  
443 process of the concrete, which may explain this better mechanical properties of the hemp  
444 concretes made from S2. This remark is similar to the results obtained by Arnaud [12] where he  
445 remarked that after 4 months, the finer hemp particles gave better mechanical properties than  
446 longer hemp particles. This difference may be also justified by the fact that the initial water  
447 absorption of S2 is 146% and for S1 is 212%. This means that S1 absorbs a lot of mixing water  
448 and this results in a dry mixture, leading to poor mechanical properties. To avoid this problem,  
449 shiv particles may be wetted before the mixing process.

## 450 **4.4 Repeatability of the results with respect to specimen sizes**

### 451 **4.4.1 Density**

452 Although the results for each specimen size are not varying too much, density characteristic  
453 values obtained for both sizes are  $424.84\text{ kg/m}^3$  and  $394.81\text{ kg/m}^3$  for small and big size,  
454 respectively (Table 5 and Table 6). Unlike to what is observed in the case of the maximum  
455 compressive strength, there is no difference for the COV values, as discussed in 4.3.1, there are  
456 always great values for small specimen size.

#### 457 **4.4.2 Maximum compressive strength**

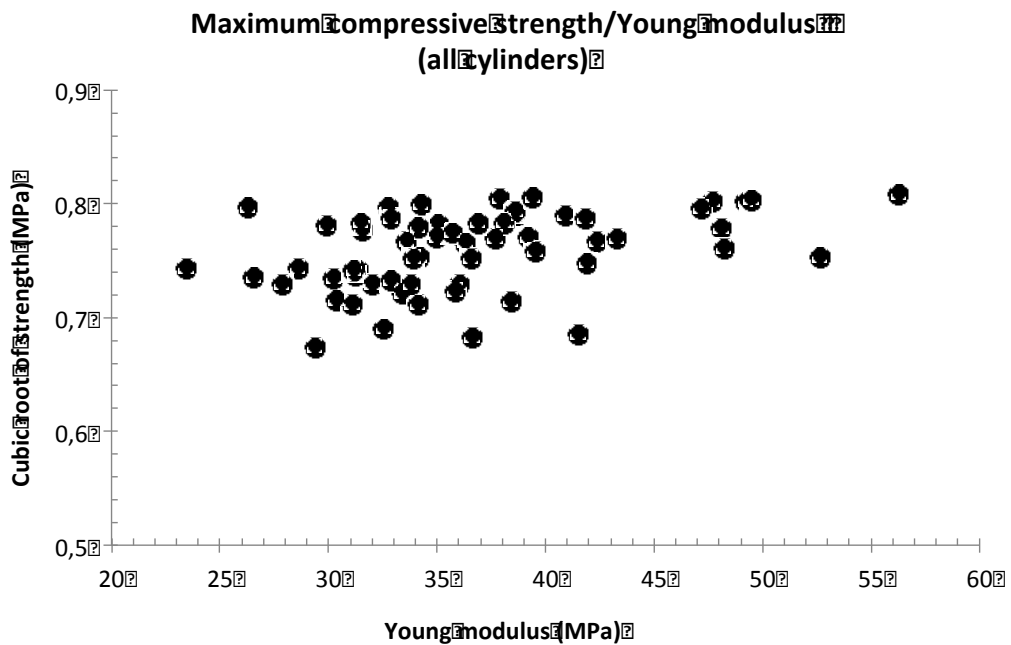
458 Considering the results obtained for the characteristic values 0.37 MPa and 0.28 MPa for small  
459 and big specimens, respectively (Table 5 and Table 6); the specimen size does not have exactly  
460 the same trend for the compressive strength as for Young's modulus. Since there is no big  
461 difference for minimum, maximum and mean compressive strength values, then the observed  
462 difference for characteristic values is related to the COV values.

#### 463 **4.4.3 Young's modulus**

464 Results on the impact of specimen size in the case of Young's modulus show comparable values  
465 for the mean and characteristic values, (Table 5 and Table 6). A significant difference is  
466 observed for the maximum values with a factor equal to 1.32.

#### 467 **4.5 Correlation between mechanical characteristics**

468 The representation given in (Figure 7) between cubic root of compressive strength and Young's  
469 modulus shows that these two mechanical properties are correlated. According to Hooke's law,  
470 the Young's modulus corresponds to the slope calculated in linear stage on the curve strength/  
471 strain. For a given homogeneous material; different samples should give almost the same values  
472 of stress and Young's modulus for a given strain. Hence the graph strength/Young's modulus  
473 may correspond to a concentrated cloud of points. As in the current study, the maximum  
474 strengths are obtained around 5% of strain, the Figure 7 was expected to be a concentrated cloud  
475 of points but it is not the case.



476

477

**Figure 7: Correlation for cubic root of maximum strength and Young modulus for all specimens**

478

The trend observed in Figure 7 shows to what extent the mechanical properties of hemp

479

concrete are sensitive to studied parameters. As explained in section 4.1.3, this may also due to

480

the fact that the maximum compressive strength is calculated beyond the linear phase of the

481

curve. Compressive strength varies from 0.3 MPa to 0.52 MPa. A great amount of values is

482

located between 30 MPa and 40 MPa for Young's modulus. Nevertheless, the cubic root of

483

compressive strength is in general increasing with Young's modulus.

#### 484 **4.6 Summary of observations**

485

At one hand, there are more or less considerable variabilities for hemp concrete properties

486

related to the type of parameters considered. The results, seen so far and performed analyses,

487

have significant scatter that is difficult to estimate or to measure. The main source of these

488

dispersions is the interference of different parameters on the observed results.

489

Moreover, according to the literature review, it has been shown the inadequacy of the accuracy

490

and the confidence level to be given to the results in literature. Regarding the characteristic

491

values of this study, they are up to now given with respect to different parameters. A study

492

taking into account all parameters for a unique characteristic value is necessary. This study is



493 proposed in the next section, with the goal of computing the characteristic values for the three  
494 properties.

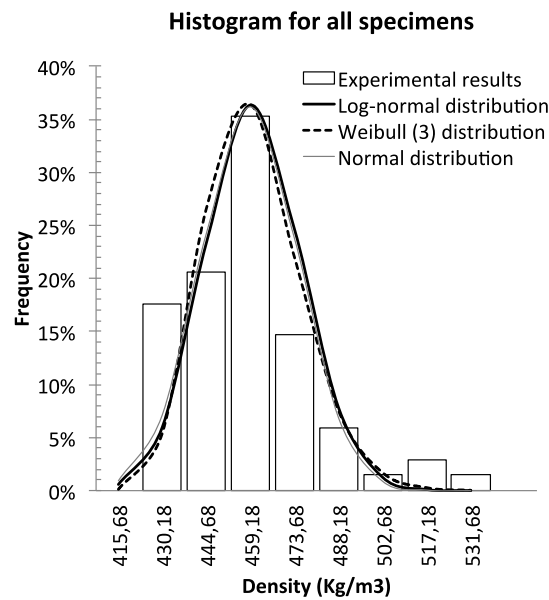
#### 495 **4.7 Probability distributions**

496 In addition to statistical values (mean, standard deviation...) computed in the previous sections  
497 of this paper, it is now required to specify the probability distribution that fits properly the  
498 experimental data. As a first step, a preliminary statistical analysis has been carried out to test a  
499 large number of probability density functions, in order to select the most appropriate candidates.  
500 For each batch, and also for grouped batches, the goodness-of-fit tests have been performed with  
501 various distributions, in order to determine which distribution fits better the data. Although  
502 various types of probability distributions have been considered to fit the experimental results,  
503 three distributions are recommended for practical engineering, namely: normal, log-normal and  
504 Weibull distributions, which are commonly used in reliability analysis of civil engineering  
505 structures [29]. For Weibull distribution it is commonly used in mechanical engineering to  
506 describe statistical variation of failure strength of a material [30]; maybe it is not suitable for  
507 hemp concrete, in the current study, it is proposed for information, others studies are required to  
508 confirm its use. This goodness-of-fit test is conducted separately for the density, the compressive  
509 stress and the Young's modulus. However, before performing these tests, it is mandatory to  
510 check whether the dispersion is due to the scatter of the population, or due to mixing different  
511 populations with different mean values. For this reason, a test has been conducted to verify that  
512 the batches belong to only one consistent population, as the underneath populations have similar  
513 mean values; otherwise the goodness-of-fit test results will be insignificant and the batches  
514 should be splitted into two or more populations.

#### 515 4.7.1 Density

516 As explained in section 3.3.3, the statistical tests have been done on different batches. The  
517 results given in Table 4 show that it is only possible to combine batches II and I. This leads to  
518 have three populations: batches (I&II), batch III and batch IV.

519 As a matter of fact, although batches III and IV come from the same type of hemp shiv S2, the  
520 statistical tests showed that they do not belong to the same population, as this hypothesis has  
521 been rejected! In order to explore the reasons for this statistical test result, it has been assumed  
522 that there could be a laboratory, which disturbs the results in one or both batches. To detect this  
523 laboratory, ANOVA test may have been used, but specimens in each lab are not enough (Table  
524 1) to perform a significant test. As result, comparison tests have been conducted for both batches  
525 by eliminating lab results, one after another. Unfortunately, the test results remained negative.  
526 Furthermore, it has been noticed that by eliminating the batch III results for lab G, the test  
527 showed that both batches III and IV belong to the same population, with the risk of rejecting the  
528 null hypothesis  $H_0$ , equal to 9.05%. This result let believe that the impact of lab G is related to  
529 the fact that it is the only laboratory which tested 6 specimens of batch III, while the others had  
530 tested 2 or 3 specimens each. For this reason, the statistical test has been conducted by keeping  
531 only 5, 4, 3 and 2 specimens among those of lab G, but in vain. As a conclusion both batches are  
532 considered as two separate populations.



533

534

**Figure 8: Probabilistic law for density**

Distribution	Normal		Log-normal		Weibull (3)		
	$\mu$	$\sigma$	$\mu$	$\Sigma$	$\kappa$	$\lambda$	$\eta$
Parameters	450.992	22.348	6.11	0.049	1.917	39.806	415.679
Statistical moments ( $\mu$ ; $\sigma$ )	(450.992; 22.348)		(450.995; 21.99)		(451.207; 22.236)		

535

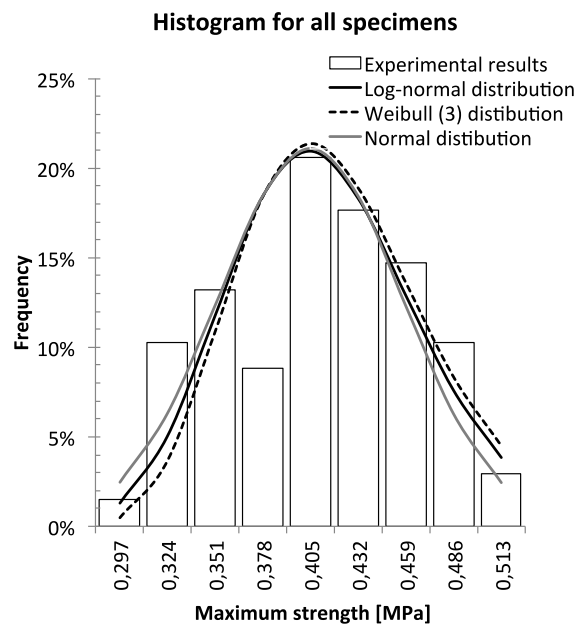
**Table 9: Distribution parameters and statistical moments for material density**

536

537 As there are not a large number of specimens in each one of the three populations, the  
 538 characterization of the coefficient of variation can be carried out by scaling the experimental  
 539 result of each specimen using the mean value of the population corresponding to batch I. The  
 540 approach described in section 3.3.3.1 is therefore applied to scale the mean values, in order to get  
 541 appropriate representation of the dispersion. The obtained population is thus shown to follow  
 542 properly normal, log-normal and three-parameter Weibull distributions, while logistic and GEV  
 543 provide also good fitting. For the considered distributions, **Figure 8** shows how the density  
 544 functions fit the experimental data and Table 9 indicates their statistical moments.

545 **4.7.2 Maximum compressive strength**

546 Regarding the maximum compressive strength, the statistical tests have been performed in order  
 547 to determine whether it can be considered only one population or not. All test results were  
 548 negative for both specimen sizes and batch parameters, except batch I and II with large specimen  
 549 dimension, as shown in Table 4. In other words, the Student's tests have led to 7 different  
 550 populations, which should then be fitted by normal, log-normal and logistic distributions.



551

552

**Figure 9: Probabilistic law distributions for maximum strength**

Distributions	Normal		Log-normal		Weibull (3)		
Parameters	$\mu$	$\sigma$	$\mu$	$\Sigma$	$\kappa$	$\lambda$	$\eta$
	0.397	0.052	-0.93	0.135	2.3	0.115	0.295
Statistical moments ( $\mu$ ; $\sigma$ )	(0.397; 0.055)		(0.397; 0.055)		(0.397; 0.045)		

553

**Table 10: Distribution parameters and statistical moments for the maximum compressive strength**

554

555 In order to analyze the scatter of experimental results, the scaling procedure described in section

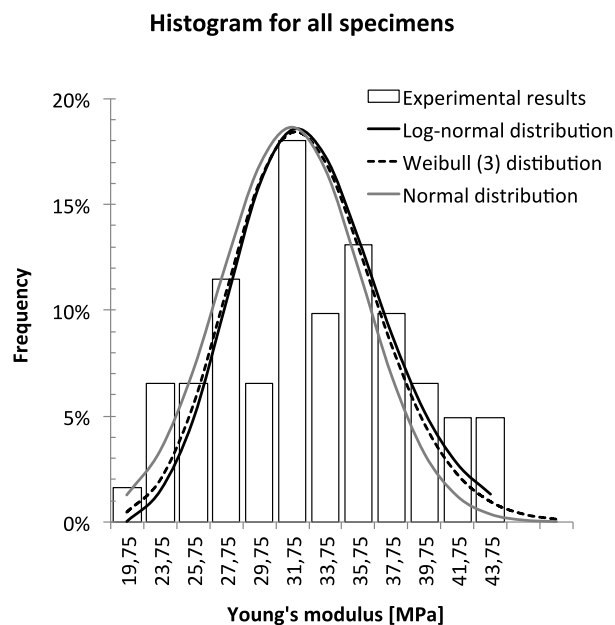
556 3.3.3.1 is applied to get a unique scaled population. The experimental results can then be fitted to

557 normal, lognormal and Weibull distributions, **Figure 9**, with the statistical moments given in Table  
558 10.

### 559 **4.7.3 Young's modulus**

560 The Young's modulus is calculated by the method explained in section 3.3.1. The experimental  
561 results obtained by this method are subjected to Student's test, and the results are given in Table  
562 4. As the tests are rejected in the case of small specimen for batches III and IV, we used the  
563 approach described in section 3.3.3.1 to scale the results. The fitting of probability distributions  
564 is shown in **Figure 10** with their statistical moments in Table 11.

565 The trend of the experimental results indicates the existence of two sub-populations: a first  
566 subpopulation is located at the mean value of 27.75MPa and a second subpopulation has a mean  
567 of 35.75MPa, as shown in **Figure 10**. This trend may be due to the fact that we have two specimen  
568 sizes. The same trend was also observed in 4.4.3 where a significant difference is observed for  
569 the maximum values with a factor equal to 1.32.



570

571

**Figure 10: Probabilistic law distributions for Young's modulus**

572

573

Distributions	Normal		Log-normal		Weibull (3)		
parameters	$\mu$	$\sigma$	$\mu$	$\Sigma$	$\kappa$	$\lambda$	$\eta$
	31.874	5.669	3.445	0.185	2.76	15.868	17.751
Statistical moments ( $\mu ; \sigma$ )	(31.874; 5.669)		(31.898; 5.943)		(31.865; 5.694)		

Table 11: Distribution parameters and statistical moments for Young's modulus

574

575

#### 576 4.8 Proposed characteristic values for studied properties

577 As discussed above, it is not possible to consider each parameter separately. Further probabilistic  
578 studies could be required to take into account the interaction of all parameters, but this is beyond  
579 the scope of the present work. As the marginal probability distribution for each parameter is  
580 determined, one can compute the characteristic values with the formula given in eq (1) with the  
581 probability level equal to 5%. Using the obtained normal distributions, the characteristic values  
582 of the three parameters are computed as: **22.5 MPa** for the Young's modulus, **0.30 MPa** for the  
583 compressive strength and **415 kg/m<sup>3</sup>** for the density.

#### 584 5 Conclusion

585 The statistical analysis has been performed for three material properties, namely the density, the  
586 compressive strength and the Young's modulus, by taking into account four parameters: testing  
587 laboratory, batch type, hemp shiv type and specimen size. The results obtained by different  
588 laboratories show that there is an accurate repeatability for compressive strength and dry density.  
589 However, the results for Young's modulus are of a large variability, with results varying from  
590 excellent to poor quality. The results also showed that there is some variability between different  
591 batches, and therefore the mixing procedure must be done with an utmost care. The impact of  
592 initial water content on the density has been also highlighted. More initial water content is, less

593 will be the density of the corresponding hemp concrete. It has also been noticed that the hemp  
594 with small particle sizes leads to better mechanical properties of hemp concrete.

595 According to the obtained results, plausible evidence for specimen size effect was observed.  
596 However, further investigations should be undertaken in the future on larger number of  
597 specimens with different sizes, in order to provide full understanding of the effect of specimen  
598 size.

599 Regarding the statistical analysis, the mean values and standard deviations of the considered  
600 batches have been computed and provide consistent results. A statistical procedure has been  
601 proposed to assess the scatter and the distribution type of the combined batches. The goodness-  
602 of-fit test has shown that the experimental results are in good agreement with the probability  
603 distributions: normal, log-normal and Weibull. According to usual recommendations in civil  
604 engineering, especially in Eurocodes, the log-normal distribution may be suggested to model the  
605 considered properties.

606 This study will be enhanced by ongoing works on separating the statistical contributions of each  
607 basic parameter (batches, hemp shiv...), through the development of Bayesian network  
608 approaches. This Bayesian network study may bring useful informations to answer to the  
609 remaining questions.

610 For future works, on one hand, the acoustical and thermal properties for hemp concrete material  
611 should be also analysed. On the other hand the impact of fabrication method such as vibration  
612 damping on the properties performance of hemp concrete material should also be investigated.

## 613 **6 References**

614 [1] V. Nozahic, S. Amziane, G. Torrent, K. Saïdi, and H. De Baynast, “Design of green  
615 concrete made of plant-derived aggregates and a pumice–lime binder,” *Cem. Concr.*  
616 *Compos.*, vol. 34, no. 2, pp. 231–241, 2012.

- 617 [2] V. Nozahic and S. Amziane, “Influence of sunflower aggregates surface treatments on  
618 physical properties and adhesion with a mineral binder,” *Compos. Part A Appl. Sci.*  
619 *Manuf.*, vol. 43, no. 11, pp. 1837–1849, 2012.
- 620 [3] P. de Bruijn and P. Johansson, “Moisture fixation and thermal properties of lime–hemp  
621 concrete,” *Constr. Build. Mater.*, vol. 47, pp. 1235–1242, 2013.
- 622 [4] P. B. de Bruijn, K.-H. Jeppsson, K. Sandin, and C. Nilsson, “Mechanical properties of  
623 lime–hemp concrete containing shives and fibres,” *Biosyst. Eng.*, vol. 103, no. 4, pp. 474–  
624 479, 2009.
- 625 [5] S. Elfordy, F. Lucas, F. Tancret, Y. Scudeller, and L. Goudet, “Mechanical and thermal  
626 properties of lime and hemp concrete (‘hempcrete’) manufactured by a projection  
627 process,” *Constr. Build. Mater.*, vol. 22, no. 10, pp. 2116–2123, 2008.
- 628 [6] A. Sellami, M. Merzoud, and S. Amziane, “Improvement of mechanical properties of  
629 green concrete by treatment of the vegetals fibers,” *Constr. Build. Mater.*, vol. 47, pp.  
630 1117–1124, 2013.
- 631 [7] H. Binici, M. Eken, M. Dolaz, O. Aksogan, and M. Kara, “An environmentally friendly  
632 thermal insulation material from sunflower stalk, textile waste and stubble fibres,” *Constr.*  
633 *Build. Mater.*, vol. 51, pp. 24–33, 2014.
- 634 [8] F. Collet, J. Chamoin, S. Pretot, and C. Lanos, “Comparison of the hygric behaviour of  
635 three hemp concretes,” *Energy Build.*, vol. 62, pp. 294–303, 2013.
- 636 [9] V. Cérézo, “Propriétés mécaniques, thermiques et acoustiques d’un matériau à base de  
637 particules végétales: approche expérimentale et modélisation théorique.” Ecole Nationale  
638 des Travaux Publics de l’Etat, 2005.
- 639 [10] T. T. Nguyen, “Contribution à l’étude de la formulation et du procédé de fabrication  
640 d’éléments de construction en béton de chanvre.” Université de Bretagne Sud, 2010.
- 641 [11] F. Collet-Foucault, “Caractérisation hydrique et thermique de matériaux de génie civil à  
642 faibles impacts environnementaux.” 2004.



- 643 [12] L. Arnaud and E. Gourlay, “Experimental study of parameters influencing mechanical  
644 properties of hemp concretes,” *Constr. Build. Mater.*, vol. 28, no. 1, pp. 50–56, 2012.
- 645 [13] T.-T. Nguyen, V. Picandet, S. Amziane, and C. Baley, “Influence of compactness and  
646 hemp hurd characteristics on the mechanical properties of lime and hemp concrete,” *Eur.  
647 J. Environ. Civ. Eng.*, vol. 13, no. 9, pp. 1039–1050, 2009.
- 648 [14] P. Mounanga, P. Poullain, G. Bastian, P. Glouannec, and H. Khelifi, “Influence de la  
649 composition et du mode de mise en œuvre sur le développement des propriétés  
650 mécaniques du béton de chanvre,” *Rencontres de l’AUGC*, 2009.
- 651 [15] F. M. Almeida Filho, B. E. Barragán, J. R. Casas, and A. L. H. C. El Debs, “Hardened  
652 properties of self-compacting concrete—a statistical approach,” *Constr. Build. Mater.*,  
653 vol. 24, no. 9, pp. 1608–1615, 2010.
- 654 [16] INRA, “National website, research center of Jouy-en-Jonas,” 2014. [Online]. Available:  
655 <http://w3.jouy.inra.fr/unites/miaj/public/matrixq/jbdenis/notes/notions/notions.html#stat>.  
656 [Accessed: 03-Apr-2014].
- 657 [17] V. Nozahic, “Vers une nouvelle démarche de conception des bétons de végétaux  
658 lignocellulosiques basée sur la compréhension et l’amélioration de l’interface  
659 liant/végétal: application à des granulats de chenevotte et de tige de tournesol associés à  
660 un liant ponce/chaux.” Université Blaise Pascal-Clermont-Ferrand II, 2012.
- 661 [18] L. Arnaud, *Bio-aggregate-based Building Materials: Applications to Hemp Concretes*.  
662 John Wiley & Sons, 2013.
- 663 [19] AFNOR, *Essais pour déterminer les caractéristiques géométriques des granulats – Partie*  
664 *2: détermination de la granularité – Tamis de contrôle, dimensions nominales des*  
665 *ouvertures- Norme NF EN 933-2*. 1996.
- 666 [20] C. Igathinathane, L. O. Pordesimo, E. P. Columbus, W. D. Batchelor, and S. Sokhansanj,  
667 “Sieveless particle size distribution analysis of particulate materials through computer  
668 vision,” *Comput. Electron. Agric.*, vol. 66, no. 2, pp. 147–158, 2009.

- 669 [21] A. M. Nazar, F. A. Silva, and J. J. Ammann, “Image processing for particle  
670 characterization,” *Mater. Charact.*, vol. 36, no. 4, pp. 165–173, 1996.
- 671 [22] T. Ferreira and W. Rasband, “The ImageJ user guide,” *USA Natl. Institutes Heal.*, 2011.
- 672 [23] S. d’édition du bâtiment et des travaux publics, *Construire en chanvre: règles*  
673 *professionnelles d’exécution*. SEBTP, 2012.
- 674 [24] S. Borel and P. Reiffsteck, “Caractérisation de la déformabilité des sols au moyen d’essais  
675 en place,” *Etudes Rech. des Lab. des ponts chaussées. Série Géotechnique*, 2006.
- 676 [25] D. Ricotier and D. Vié, *Dimensionnement des structures en béton selon l’Eurocode 2 De*  
677 *la descente de charge aux plans de ferrailage*. Le Moniteur, 2012.
- 678 [26] P. Jaffard, *Initiation aux méthodes de la statistique et du calcul des probabilités*. Masson,  
679 1986.
- 680 [27] R. Journeaux, *Traitement des mesures : interprétation, modélisation, outil statistique*,  
681 Ellipses. Paris, 2009.
- 682 [28] S. Adinsoft, “XLSTAT-software, version 10,” *Addinsoft, Paris, Fr.*, 2010.
- 683 [29] R. Atadero, L. Lee, and V. M. Karbhari, “Consideration of material variability in  
684 reliability analysis of FRP strengthened bridge decks,” *Compos. Struct.*, vol. 70, no. 4, pp.  
685 430–443, 2005.
- 686 [30] T. L. Anderson and T. L. Anderson, *Fracture mechanics: fundamentals and applications*.  
687 CRC press, 2005.
- 688
- 689