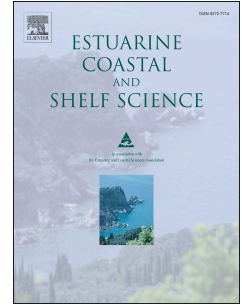


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1 Variability of depth-limited waves in coral reef surf zones

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16

17 **Abstract**

18 Wave breaking and transformation on coral reef flats is an important process protecting
19 tropical coastlines and regulating the energy regimes of coral reefs. However, the high
20 hydraulic roughness, shallow water, and steep bathymetries of coral reefs may confound
21 common surf zone assumptions, such as a depth-limited and saturated surf zone with a
22 constant wave height to water depth ratio (γ). Here, we examine wave transformation across
23 a coral reef flat, during three separate swell events, on both a time-averaged and a wave-by-
24 wave basis. We use the relationship between significant wave height and water depth (γ_s) to
25 examine the change in surf saturation across the reef flat and compare the measured wave
26 height decay to results of modelled wave energy dissipation in the surf zone. Our results
27 show that γ_s was not cross-reef constant and varied according to location on the reef flat and
28 local water depth. On average, γ_s was greatest at the outer reef flat, near the reef crest, and
29 progressively reduced towards the inner reef flat, near the reef lagoon. This was most
30 pronounced in shallow water with large γ_s values ($\gamma_s > 0.85$) at the outer reef flat and small γ_s
31 values ($\gamma_s < 0.1$) at the inner reef flat. This indicates that there is an increase in wave energy
32 dissipation in shallow water, most likely due to increased breaker and bed frictional
33 dissipation. The measured wave energy dissipation across the entire reef flat could, on
34 average, be modelled accurately; however, this required location specific calibration of the
35 free parameters, the wave friction factor (f_w) and γ , and further suggests that there is no
36 value for either parameter that is universally applicable to coral reef flats. Despite model
37 calibration inaccuracies were still observed, primarily at the outer reef flat. These
38 inaccuracies reflected the observed cross-reef variation of γ on the reef flat and potentially
39 the limitations of random wave breaker dissipation models in complex surf zones. Our
40 results have implications for the use of wave energy dissipation models in predicting
41 breaker dissipation and subsequent benthic community change on coral reef flats, and

42 suggest that careful consideration of the free parameters in such models (such as f_w and γ) is
43 required.

44 **Introduction**

45 Coastal protection and the regulation of hydrodynamic energy is one of the most important
46 ecosystem services provided by coral reef systems (e.g. Hoegh-Guldberg et al., 2007). Wave
47 breaking and transformation on coral reef flats, and the resultant wave induced flows, are
48 the dominant physical forcing mechanisms on most coral reefs (Monismith, 2007). The
49 shallow water environment and very high frictional roughness values of reef flats contribute
50 to the efficient removal of wave energy over relatively short distances (Lowe et al., 2005;
51 Monismith et al., 2015). As a result of this process, back-reef environments immediately in
52 lee of coral reef flats are typically low-energy environments with limited potential for
53 sediment transport under average conditions (Harris et al., 2015; Pomeroy et al., 2012).

54 Wave energy dissipation in the surf zone of coral reefs is similar to beach environments in
55 that it has mostly been observed to saturate (Gourlay, 1994; Nelson, 1994), with a constant
56 ratio of wave height (H) to water depth (h) over time:

$$57 \quad H = \gamma h \quad (\text{Eq. 1})$$

58 where γ is the wave height to water depth ratio . For saturated surf zones the common
59 values used for γ are: 0.78 for monochromatic waves, which was first derived from solitary
60 wave theory and observed in laboratory studies (Longuet-Higgins, 1974; McCowan, 1891);
61 and, 0.42 for root-mean square wave height (H_{rms}) which was observed in surf zones of
62 beaches (e.g. Sallenger and Holman, 1985; Thornton and Guza, 1982). However, these
63 values were derived from a limited range of conditions and beach types, and a wide range of
64 γ has been observed in more complex beach environments including observations of non-

65 constant γ in the cross-shore in unsaturated surf (Nelson, 1987; Power et al., 2010;
66 Raubenheimer et al., 1996; Ruessink et al., 2003).

67 Coral reefs have been the subject of even fewer surf-zone studies compared to siliciclastic
68 beaches and typically have much more complex bathymetries (Demirbilek and Nwogu,
69 2007). As a result γ may be poorly defined for coral reefs, despite γ being the primary
70 parameter that defines wave breaking in many wave energy dissipation models (e.g.
71 Baldock et al., 1998; Battjes and Janssen, 1978; Thornton and Guza, 1983). These models
72 and concepts have been applied to coral reefs to infer changes in coral reef geomorphology
73 and ecology (Gove et al., 2015), and to determine the coastal protection service provided by
74 coral reefs (Harris et al., 2018; Saunders et al., 2014; Storlazzi et al., 2015). They have also
75 been used to link hydraulic roughness observations to benthic ecological assemblages and to
76 determine the influence of waves on ecological zonation of coral reefs (Monismith et al.,
77 2015; Rogers et al., 2016; Storlazzi et al., 2005; Williams et al., 2013). Most of these
78 analyses are reliant on an accurate description of breaker wave energy dissipation on coral
79 reef flats despite the limited data in such systems when compared to siliciclastic beaches.
80 This study will therefore focus on wave transformation and the variation of surf zone
81 saturation and depth limited waves across the reef flat and determine what influence this
82 may have on the accuracy of breaker dissipation models.

83 The most common value of γ observed on coral reef flats is between $\gamma = 0.4 - 0.6$, which has
84 been observed in both laboratory and field studies (Table S1). However, this value is mostly
85 applicable only to the near-horizontal reef flats and may not be relevant for the steep fore-
86 reef slopes of coral reefs (Massel and Gourlay, 2000; Nelson, 1994). When the full cross-
87 reef profile is examined (including the fore-reef slope and the reef crest) a wide range of γ
88 values have been observed ($0.1 < \gamma < 1.2$) (Figure 1 and Table S1). This may be due to one

89 or a combination of: 1) the many different definitions of γ that have been reported; 2) the
90 numerous methods used to determine γ ; and/or, 3) the location of observation on the reef flat
91 (Figure 1 and Table S1). Cross-reef variability in γ has been previously observed with γ
92 values between 0.7 – 1.2 for the fore-reef slope and reef crest, and 0.2 – 0.7 for the reef flat
93 (Figure 1 and Table S1).

94 The observed variability of γ values across the surf zones of coral reefs has led to concerns
95 regarding the accuracy of commonly used wave energy dissipation models that use a
96 constant value of γ to determine the proportion of broken waves, for a given period, in the
97 surf zone (Demirbilek and Nwogu, 2007). In an attempt to mitigate this, Hearn (1999)
98 incorporated the observed cross-reef variability of γ into models of wave energy dissipation
99 and defined regions of high γ (at the reef crest, $\gamma = 0.8$) and low γ (on the reef flat, $\gamma = 0.5$).
100 These values are similar to the average results found in previous literature, when examining
101 γ values with respect to the location on the reef flat (Figure 1). However, most wave energy
102 dissipation models, including the most common models that have been used on coral reefs,
103 use a constant γ parameter to define wave breaker dissipation (e.g. Harris et al., 2018; Lowe
104 et al., 2005; Pomeroy et al., 2012). In these scenarios, wave energy dissipation models have
105 primarily been calibrated to describe the total dissipation of energy across the coral reef flats
106 and, while accurately explaining wave energy dissipation in general terms, they may
107 overlook some specific wave processes and conditions that differ between the outer reef
108 (near or at break point) and the inner reef flats (inner surf zone) (Massel and Gourlay, 2000).
109 Furthermore, the common approach of using time-averaged measurement and modelling of
110 coral reef surf zone processes may mask significant detail in the true variation of wave
111 heights in the inner surf zone. Previous studies have found that individual waves in surf
112 zones of intermediate beaches are not necessarily influenced by water depth to the same
113 extent as time-averaged wave conditions (e.g. Power et al., 2010; Power et al., 2015). To

114 date there has been no detailed wave-by-wave analysis of measured wave transformation in
115 a coral reef surf zone. Therefore, this study will: 1) examine the variability and prevalence
116 of surf zone saturation and depth limited waves by analysing wave decay (via change cross-
117 reef in γ) across a coral reef flat on time-averaged and wave-by-wave bases; and, 2) assess
118 the accuracy of a common wave energy dissipation model on a reef flat using spatially
119 variable γ in the surf zone.

120 **One Tree Reef**

121 One Tree Reef (OTR) is a lagoonal mid-shelf reef in the Capricorn Bunker Group of the
122 Southern Great Barrier Reef (GBR) (Figure 2). OTR receives moderate wave energy with
123 Hopley (1982) reporting a 1.15 m offshore significant wave height (H_{so}) on average for the
124 southern GBR (reanalysis of the offshore wave record is shown in Figure S1). Waves are
125 predominately from the southeast and do not change significantly throughout the year
126 although OTR may be exposed to cyclone events during the summer months (November –
127 March). The reef flats of OTR are emergent during low tide with no interaction between the
128 pelagic and lagoonal environments during this time. Tides are meso-tidal with a maximum
129 tidal range of 3.5 m which result in water depths of 2.1 m over the reef flat (Harris et al.,
130 2014). This study deployed pressure transducers on the southern reef flat (P1-6, Figure 1c),
131 which is dominated by turf and crustose coralline algae, and has minimal live coral cover
132 (Thornborough and Davies, 2011). Coral boulders are randomly dispersed on the reef flat,
133 with a greater accumulation of smaller coral rubble at the lagoonward end of the reef flat
134 (P5-6) (see photos in Supplementary Material Appendix 3). The reef flat is near horizontal
135 or with a mild slope ($\tan\beta$) at near the reef crest (P1, $\tan\beta = 0.01$), near horizontal at the mid
136 reef flat (P2-3, $\tan\beta \approx 0.002$), mildly negatively sloped near the lagoon (P4-5, $\tan\beta = -0.01$),

137 and is horizontal on the live coral windrows that have formed on the back-reef sand apron
138 ($\tan\beta \approx 0$, Figure 1d).

139 **Methods**

140 **Analysis of previous literature**

141 We reviewed all the previously published values of γ – to our knowledge – to examine the
142 variation and average values of γ on coral reef flats. The definition of γ reported, the
143 observation location on the reef flat, and the method used to determine γ were assessed
144 (Table S1 and Supplementary Material). The maximum observed γ and the γ used in
145 calibrated wave models are summarized in Figure 1. These two definitions of γ were
146 selected since maximum observed values of γ in the literature have informed the values used
147 for γ in wave energy dissipation models (e.g. Gourlay, 1994; Gourlay, 1996; Lowe et al.,
148 2005; Massel and Gourlay, 2000; Nelson, 1994). We categorised values as being derived
149 from one of two main zones on the cross-reef profile: (1) the reef crest or outer surf zone,
150 and (2) the reef flat or inner surf zone (Figure 1 and 2). The average values observed in each
151 zone and in calibrated wave models are shown in Figure 1 and reported in Table S1. Table
152 S1 in the supplementary material shows the full list of publications used in the analysis.

153 **Offshore wave record**

154 The long-term offshore wave climate (H_{so} and T_p) for OTR was defined, for the first time,
155 using the Centre for Australian Weather and Climate Research (CAWCR) Wave Hindcast
156 (1979-2013), which provides hourly wave predictions at 4 arc minute resolution around the
157 Australian coastline (Durrant et al., 2014; Durrant et al., 2013) (Figure S1). The wave
158 hindcast dataset was developed using the WAVEWATCH III spectral wave model (Tolman,
159 2014) with Climate Forecast System (CFS) atmospheric forcing (Saha et al., 2010). To

160 provide offshore wave conditions at OTR (coordinates 23°30' S, 152°12' E) during the three
161 field sampling periods in 2012, 2014 and 2016, a comparable WAVEWATCH III spectral
162 wave model and CFS forcing was used, the Nearshore Coastal Ocean Wave (NCOW) model
163 (Kinsela et al., 2014), providing wave predictions in the GBR region at 0.25° resolution
164 (Figure 1).

165 To ensure consistency between the long-term CAWCR Wave Hindcast dataset and the
166 offshore wave model run for the field sampling periods (NCOW), wave height predictions
167 for the 2012 calendar year were compared to recorded waves at a Waverider Buoy station
168 located in 80 m water depth offshore of Byron Bay (500 km SSE from OTR). Both the
169 CAWCR Wave Hindcast and NCOW model performed well against waverider buoy
170 measurements until $H_{so} > 5$ m which is larger than the range of offshore wave heights
171 examined in this study (Figures S3, S4 in Supplementary Material). Extreme storm peak
172 wave heights were under-predicted by both the hindcast dataset and the wave model (Figure
173 S4). This may be attributed to the resolution of the atmospheric forcing data, which limits
174 the capacity to accurately resolve steep coastal wind gradients associated with the land-sea
175 interface (e.g. Kinsela et al., 2014; Sharp et al., 2015). This resolution effect diminishes with
176 increased distance from the coastline, and is not expected to be significant at OTR, which is
177 located 90 km from the Queensland coastline. The comparison shows that at the Byron Bay
178 location, the wave hindcast dataset and wave model provide comparable predictions for
179 significant wave heights up to 5 m (Figure S4). The probability distribution of the offshore
180 wave heights were compared to the Weibull, Gumbel, and Lognormal distributions using the
181 Wave Analysis for Fatigue and Oceanography MATLAB® toolbox (Brodtkorb et al., 2000).
182 The lognormal distribution produced the most accurate description of the offshore wave
183 heights and the mean long-term significant wave height and wave period were computed
184 from this distribution.

185 Field measurement

186 Waves were measured during non-storm conditions on the reef flat of OTR on 13-14
187 December 2012 and 15-16 March 2016 and during a storm event generated by a low to
188 moderate energy cyclone on 30-31 January 2014 (Tropical Cyclone (TC) Dylan, storm
189 conditions). In total almost 120,000 individual waves were measured and analysed. Waves
190 were measured using pressure transducers (PTs, INW Aquistar PT2X) which were deployed
191 on the southern reef flat in a cross-reef transect. PTs logged continuously at a sampling
192 frequency of 8 or 10 Hz, depending on the maximum frequency of the PT. Five PTs were
193 deployed in December 2012 (P1 and 3-6 from reef crest to lagoon) and four in January 2014
194 and March 2016 (P2-5) (Figure 2c). Repeat deployments were conducted at sites P2, 4, 5,
195 and 6 for the three measurement periods. Data were divided into 15-minute records and
196 records that were not fully submerged for the full 15 minutes were removed.

197 Data processing and wave statistics

198 The pressure records from the PTs were low-pass filtered to remove instrument noise, high-
199 pass filtered to separate infragravity effects and then split into 15-minute runs to remove the
200 tidal influence in the record (e.g. Hughes and Moseley, 2007). Pressure attenuation with
201 depth was corrected using methods outlined in Tucker and Pitt (2001). Wave height (H) and
202 wave period (T) were calculated using zero down-crossing analysis, with significant wave
203 height (H_s) calculated for each 15-minute run. Using these data, γ_s , and wave height to water
204 depth for individual waves (γ_w), were calculated using H_s/\bar{h} and H/h_w respectively, where \bar{h}
205 is the mean water depth across the 15-minute run and h_w is the average water depth for an
206 individual wave. γ_s was selected as the time-averaged value of γ due to the widespread use of
207 this term (since H_s is the most common measurement of wave height) when assessing surf
208 zone saturation in field settings (e.g. Power et al., 2010; Raubenheimer et al., 1996).

209 **Wave energy dissipation model**

210 We applied the Alsina and Baldock (2007) and Janssen and Battjes (2007) (hereafter
 211 AB07/JB07) random wave breaker dissipation model in combination with frictional
 212 dissipation equations (Jonsson, 1966; Swart, 1974) to the southern reef flat of One Tree
 213 Reef, using a similar approach to that of Lowe et al. (2005) for coral reefs. The AB07/JB07
 214 follows the approach of Battjes and Janssen (1978) and determines the mean (time-
 215 averaged) decay of wave height and energy by applying an energy flux balance across the
 216 surf zone. The AB07/JB07 models (that are identical but developed independently) include
 217 updated estimates of dissipation in the inner surf zone to remove shoreline singularities of
 218 earlier models (Baldock et al., 1998) and correct some of the errors observed by Janssen
 219 (2006) and Ruessink et al. (2003) in the inner surf zone. The model can be summarized by:

$$220 \quad \frac{dEC_g}{dx} = -D_b - D_f \quad (\text{Eq. 2})$$

221 where EC_g is the wave energy flux, D_b is the wave energy dissipation due to breaking (both
 222 initial breaking at the breakpoint and ongoing breaking due to bore processes), and D_f is the
 223 wave energy dissipation due to bed friction. The wave energy flux is defined as the wave
 224 energy multiplied by the wave group velocity:

$$225 \quad E = \frac{1}{8} \rho g H_{rms}^2 \quad (\text{Eq. 3})$$

$$226 \quad C_g = C \frac{1}{2} \left(1 + \frac{2kh}{\sinh 2kh} \right) \cos \theta \quad (\text{Eq. 4})$$

227 where H_{rms} root-mean-square wave height, C_g is the group velocity normal to the reef crest,
 228 C is the wave phase velocity, ρ is the water density, k is the wave number, g is gravitational
 229 acceleration, and θ is the incident wave angle. Wave number was determined by solving the
 230 dispersion relation for shallow water using the Newton – Raphson iteration method. Since

231 no directional wave information was available in this study, and the model was applied after
 232 wave breaking in the inner surf zone, we assume $\cos\theta = 1$. This is not strongly limiting since
 233 most of the wave refraction will have likely already occurred prior to the wave
 234 measurements on the reef flat.

235 The most common formulation of D_b is to approximate energy dissipation as propagating
 236 water bores, first proposed by Lamb (1932), which is then multiplied by the number of
 237 broken waves over the total number of waves per unit of the cross-shore distance. The
 238 fraction of broken waves is determined by assuming a Rayleigh distribution of waves at
 239 each location in the surf zone. This approach includes all processes that lead to energy
 240 dissipation such as, boundary shear, friction between the wave roller and wave surface, and
 241 turbulence due to breaking. In the AB07/JB07 model, D_b is given by:

$$242 \quad D_b = \frac{3\sqrt{\pi} f \rho g H_{rms}^3}{16 h} \left[1 + \frac{4}{3\sqrt{\pi}} \left(\frac{H_b}{H_{rms}} \right)^3 + \frac{3}{2} \frac{H_b}{H_{rms}} \right] \exp\left(-\frac{H_b^2}{H_{rms}^2}\right) - \operatorname{erf}\left(\frac{H_b}{H_{rms}}\right) \quad (\text{Eq. 5})$$

243 where f is the dominant frequency of the wave spectrum, H_b breaker wave height ($H_b = \gamma h$),
 244 erf is the error function (implemented in MATLAB® here). The loss of wave energy due to
 245 bed friction may be large in coral reef environments (Lowe et al., 2005; Monismith et al.,
 246 2015) so an additional factor that determines frictional energy loss is required (D_f). The
 247 Swart (1974) frictional dissipation equation was used incorporating the wave friction factor
 248 (f_w , Jonsson 1966):

$$249 \quad D_f = \frac{2}{3\pi} \rho f_w U^3 \quad (\text{Eq. 6})$$

250 where U is the near bed orbital velocity determined from linear wave theory.

251 The two free parameters in the full model are γ (used to define H_b) and f_w . f_w was varied
 252 spatially during model calibration between the locations of wave measurement, similar to

253 Lowe et al. (2005) and Péquignet et al. (2011), resulting in four different zones of roughness
254 on the reef flat. We also calibrated the wave model with a spatially constant f_w value since
255 this method has been used previously on coral reefs (e.g. Harris et al., 2018; Monismith et
256 al., 2013; Pomeroy et al., 2012). Calibration of both parameters was performed by applying
257 the Generalised Likelihood Uncertainty Estimation (GLUE, Beven and Binley (1992))
258 method for 100,000 model runs, see Simmons et al. (2015) for an example of applying the
259 GLUE method for coastal models. The best fitting value of γ was 0.57 and the values for f_w
260 are shown in Table 1 for the model with spatially varying f_w . For the wave energy
261 dissipation model with a constant f_w , $\gamma = 0.48$ and $f_w = 0.2$.

262 Results

263 Reef flat and offshore wave conditions

264 The long-term offshore wave data indicate that the mean significant offshore wave height
265 ($\overline{H_{so}}$) and offshore wave period ($\overline{T_p}$) for the southern GBR are $\overline{H_{so}} = 1.5$ m and $\overline{T_p} = 6.7$ s,
266 which are larger than values reported by Hopley (1982) (Figure S1). The offshore wave
267 height from the NCOW wave model is shown in Figure 3; the average H_s for the 2012,
268 2014, and 2016 during the measurement periods were 2.6 m, 4.9m, and 1.21 respectively.
269 The conditions during 2012 and 2016 are considered non-storm conditions since $H_s < 3$ m
270 and 2014 considered storm conditions since $H_s > 3$ m (as defined by Lord and Kulmar
271 (2001)).

272 Waves on the reef flats did not vary with changes in the offshore wave conditions (Figure
273 3). The time-averaged wave heights on the reef flat during all deployments were depth
274 limited and could mostly be described by local water depth where $H_s = 0.31\bar{h}$ ($\overline{\gamma_s} = 0.31$, R^2
275 $= 0.96$, $n = 114726$, Figure 4a). However, when examined for each 15-minute run, γ_s varied
276 considerably (Figure 4c). γ_s values had greater variance in shallow water conditions with a

277 maximum value of 0.9 and minimum of 0.05 when $\bar{h} \approx 0.1$ (Figure 4c). These differences
 278 were dependent on the measurement location on the reef flat. For measurements closer to
 279 the reef crest at the outer reef flat (P1-4) γ_s increased as depth decreased resulting in large γ_s
 280 values in shallow water conditions ($\bar{h} < 1$ m) (Figure 5a). Measurements that were furthest
 281 from the reef crest at the inner reef flat (P5-6) showed the opposite trend with γ_s decreasing
 282 as depth decreased (Figure 5a). The γ_s values at the outer and inner reef flats for changing
 283 depth could be explained by Eq. 7 ($R^2 = 0.66$, $n=495$) and Eq. 8 ($R^2 = 0.66$, $n=232$)
 284 respectively which were derived from exponential regression for $\bar{h} \geq 0.1$ where: $\gamma_{s,outer} =$

$$0.61 \exp(-4.87\bar{h}) + 0.33 \quad (\text{Eq. 7})$$

$$\gamma_{s,inner} = -0.87 \exp(-0.07\bar{h}) + 1.06 \quad (\text{Eq. 8})$$

287 Eq. 7 and 8 are shown in Figure 5a. The values of γ_s in greater water depths tended towards
 288 the mean of $\gamma_s = 0.31$ at both the outer and inner reef flat (Figure 4c and 5a). The γ_s values in
 289 general decreased as waves propagated across the reef flat and at greater depths (Figure 5a
 290 and b). The wave shape and wave deformation also changes during propagation over the
 291 reef flat particularly during shallow water conditions (Figure 6 and 7 and Figure S7 in the
 292 Supplementary Material).

293 **Wave-by-wave analysis**

294 Large variation in wave height to water depth ratios was also observed on a wave-by-wave
 295 basis (Figure 8). The distribution of γ_w showed larger values of up to $\gamma_w = 3$, over three times
 296 greater than that of the maximum observed values in the time-averaged analysis of $\gamma_s = 0.9$
 297 (Figure 8). The trends in γ_s and γ_w values with changing water depths were similar whereby
 298 γ_w increased on the outer reef flat (P1-4) and decreased on the inner reef flat (P5-6) as water
 299 depth decreased (Figure 8c and d). As such, a greater proportion of individual waves were

300 dissipated at the outer reef flat under shallow water conditions resulting in smaller waves
301 near the lagoon (Figure 8c and d). Maximum γ_w values (γ_{w_max}) also changed with water
302 depth: all recorded waves for the inner reef flat were below a maximum γ_w of 0.5.
303 $\gamma_{w_max} = 0.5$ also adequately explained most of the waves for the outer reef flat when $h > 1$ m
304 (Figure 8d). A value of 0.8 was more appropriate when $h < 1$ m for the outer reef flats,
305 although γ_{w_max} could also be much higher when $h < 0.5$ m (Figure 8). The maximum value
306 of γ_w changed throughout the tidal cycle with no one value adequately explaining the limit of
307 wave height to water depth for all water depths and locations (Figure 8).

308 Discussion

309 Wave transformation on coral reef flats

310 Wave conditions on the reef flat were saturated and independent of offshore incident wave
311 height even under cyclone generated swell conditions (Figure 3). Consequently, the
312 significant wave height, averaged for the combined period of all deployments, could be
313 accurately predicted using local water depth ($H_s = 0.31\bar{h}$) (Figure 4a). This is consistent with
314 previous studies that have noted strong correlations between wave heights and water depths
315 on coral reef flats (e.g. Gourlay, 1994; Hardy and Young, 1996; Monismith et al., 2013;
316 Nelson, 1994). Despite the correlation between wave height and water depth observed here
317 (Figure 4a), γ_s was not cross-reef constant and showed considerable variation depending on
318 the location of measurement on the reef flat (Figure 4c and 5a). This clearly shows that there
319 is no single value for γ_s that is applicable for an entire tidal cycle nor for all locations on the
320 coral reef flat. The γ_s values, when averaged for each location from the three measurement
321 periods, were greater on the outer reef flat and decreased as waves propagated across the
322 reef flats towards the lagoon (Figure 5b). The average γ_s values for each location were 0.2 –
323 0.5 (Figure 5b) and are similar to previous observations of reef flat waves (Hardy et al.,

324 1990, Table S1) particularly at the inner reef flat where the maximum wave-by-wave γ
325 (γ_{w_max}) is 0.5 (Figure 8b and d), i.e., the same value observed by Nelson (1994), Gourlay
326 (1994) and many subsequent studies (Figure 1 and Table S1).

327 However, the average γ_s values from the entire measurement period mask significant detail
328 in the variation of γ_s with changing depth that has not been observed in previous studies
329 (Figure 4c and 5a). At the inner reef flat, on near horizontal or mild negative slopes, $\gamma_s = 0.5$
330 and therefore $H_s = 0.5h$ effectively defined the upper limit of wave heights and therefore
331 showed that waves were saturated. The outer reef flat maximum wave heights could also be
332 predicted by $H_s = 0.5h$ when $h > 1 - 1.5$ m. However, during shallow water at the outer reef
333 flat when h approached zero, H_s did not decrease at the same rate, leading to high values of
334 γ_s . A similar increase in γ_s was observed by Raubenheimer et al. (1996) and Baldock et al.
335 (1998) in the shallow nearshore zones of steep beaches due to increased breaker dissipation
336 as “shore breaks”. This may also explain the increase in γ_s during shallower water near the
337 reef crest and the break-down of wave height predictions based on assumptions of depth-
338 limited waves heights (e.g. Equation 1). The disparity between the high γ_s values on the
339 outer reef flat ($\gamma_s > 0.8$) and the low γ_s values on the inner reef flat ($\gamma_s < 0.1$) in shallow water
340 indicated that most of the wave energy was dissipated on the outer reef flat and reef crest
341 (Figure 8d). This may be due to both increased breaker dissipation but also greater frictional
342 dissipation at the outer reef flat leading smaller than expected waves in the inner reef flat
343 when assuming depth limited wave heights. The much higher calibrated values of f_w in the
344 wave energy dissipation model at the outer reef flat when compared to the inner reef flat
345 may support this conclusions (Table 1). The high f_w values would also suggest that the outer
346 reef flat has higher structural complexity and, while higher coral cover was observed on the
347 reef flats in this location, in absence of any direct quantitative measurement it is not possible
348 to define the benthic ecological assemblages that led to the high f_w values. The low γ_s values

349 at the inner reef flat during shallow water is most likely also due to the slight increase in
350 local water depth due to the mild negative slope in this region that does not correspond with
351 an increase in H . Despite the inaccuracy of depth limited assumptions during shallow water
352 at both the inner and outer reef flat, the surf zone was still characterized by saturated
353 conditions as local wave heights were not influenced by changes in offshore wave
354 conditions.

355 In order to assess the effects of the observed changes in γ_s on wave energy dissipation
356 models, we applied the modified AB07/JB07 wave energy dissipation model described
357 above to the data recorded in 2012 (Figure 9a and b). The deployment record from 2012 was
358 selected since it covers most of the full width of the reef flat from P1-P6 (without P2 due to
359 equipment failure). We find that the model shows good overall agreement with regards to
360 the average rate of wave energy dissipation across the reef flat (Figure 9) suggesting that
361 small γ_s values at the inner reef flat are indeed due to the enhanced breaker dissipation in
362 shallow water at the outer reef flat. However, there were significant differences between the
363 measured and modelled wave heights for outer and inner reef flat (Figure 9c). The model
364 tended to under predict wave height in shallow water and over predict wave height in deeper
365 water at the outer reef flat (Figure 9c). For the inner reef flat, wave heights were generally
366 over-predicted. Despite these inaccuracies in the model, most discrepancies for the inner
367 reef flat region could be corrected by spatially varying the values of reef roughness (f_w)
368 across the reef flat; an approach that is not available for models that use a spatially constant
369 f_w (Table 1, Figure 9 and Supplementary Material). In contrast, allowing for spatially
370 varying roughness coefficients only marginally improved the accuracy of breaker
371 dissipation at the outer reef flat, particularly during shallow water (Figure 9b and
372 Supplementary Material). This indicates that, while random wave energy dissipation models

373 can be successfully tuned to calculate wave energy dissipation on the reef flat, they may
 374 inaccurately represent breaker dissipation near the reef crest.

375 The errors observed in the models are most likely linked to the observed cross-reef and
 376 depth variable values of γ observed in this study (Figure 5) and may highlight the difficulties
 377 in applying wave energy dissipation models to coral reef environments with steep or
 378 complex bathymetries (Salmon and Holthuijsen, 2015). We also note that the choice of
 379 spatially varying or constant f_w within the wave model resulted in different values of γ and f_w
 380 being assigned during model calibration. This is not unexpected but it indicates that γ and f_w
 381 produced from model calibrations are dependent on the mechanics of the wave energy
 382 dissipation model selected and potentially on each other. Due to the many varied approaches
 383 taken to determine γ and f_w in the literature, including a wide range of f_w values applied to the
 384 same coral reef (e.g. $f_w = 0.3-7$, Gove et al. (2015), Monismith et al. (2015), Rogers et al.
 385 (2016)) it is clear that there is not yet a consistent method for determining the free
 386 parameters in wave energy dissipation models of γ and f_w (or equivalent roughness
 387 coefficient), nor are there universal values that can be assigned to coral reefs (Rosman and
 388 Hench, 2011).

389 In order to further explain the variability in γ_s , a number of additional wave parameters were
 390 examined similar to the approach taken by Ruessink et al. (2003) (Supplementary Material).
 391 We found a correlation was between wave deformation and γ_s (for $\gamma_s < 0.6$) with the least
 392 square linear regression fit given by:

$$393 \quad \gamma_s = -0.2def + 0.53 \quad \text{for } \gamma_s < 0.6 \quad (\text{Eq. 9})$$

394 where def is the time-averaged wave deformation, which is the ratio of the time between the
 395 zero up-crossing and wave crest (a) and the time between the wave crest and zero down-
 396 crossing (b) obtained from the pressure time series ($def = a/b$) (Cowell, 1982) (Figure 6 and

397 Figure S6 in Supplementary Material). This result suggests that the larger γ_s values are due
398 to waves with higher deformation values that Cowell (1982) defined as ‘hyper crested’
399 waves and that more closely resembled surf zone bores (Figure 7). Conversely, as waves
400 propagate into the inner reef flat they reduce in deformation potentially indicating the
401 reforming of wave shape or that the rate of energy dissipation due to breaking decreases
402 through a reduction in bore strength (Figure 6 and 7). However, Eq. 9 was still unable to
403 describe the maximum values of γ_s (e.g. $\gamma_s > 0.6$) in the outer reef flat during shallow water.
404 As such, there are most likely additional mechanisms driving the largest values of γ_s such as
405 the rate of energy dissipation in the surf zone when compared to rate of change in water
406 depth and offshore wave steepness that is beyond the current data set of this study (Battjes
407 and Stive, 1985; Raubenheimer et al., 1996).

408 The form of γ_s presented in Eq. 13 can only be calculated through wave-by-wave
409 measurements, which are shown here to provide insights into the controls of wave decay for
410 individual waves during propagation across reef flats. Depth was the primary control in
411 defining the maximum height of individual waves, despite the considerable variation in
412 wave height. This has not always been observed in previous studies, for example
413 unsaturated surf has been observed for individual waves on siliciclastic beaches (Power et
414 al., 2010). However, in a similar trend to those observed in the time-averaged results, the
415 wave-by-wave maximum wave height to water depth ratio (γ_{w_max}) was not cross-reef
416 constant under shallow water conditions. Greater wave decay across the surf zone was
417 observed during shallow water with much larger waves observed in the outer reef flat when
418 compared to the inner reef flat. A limiting γ_w value of $\gamma_{w_max} = 0.5$ explained most of the
419 wave record for the inner reef flat, particularly at higher tidal stages (Figure 8). When $h > 1$
420 – 1.5 m, $\gamma_{w_max} = 0.5$ adequately described wave conditions on the outer reef flat (Figure 8).
421 However, under shallow water a limiting γ_{w_max} value of 1.5 explained most of the waves

422 with some instances of $\gamma_{w_max} \approx 3$ also observed under shallow water (Figure 8 and Figure S7
423 in Supplementary Material). This indicates that there can be individual waves three times
424 larger than what is suggested by time-averaged results. Waves such as this are likely to be
425 important non-linear mechanisms of change in coral reefs, and have the potential to dislodge
426 of coral colonies from reef substrate, however, the effects of these individual waves have
427 not been examined in detail to date.

428 **Conclusions**

429 The results here show that surf zones are saturated on the southern reef flat of One Tree
430 Reef but not necessarily depth limited. This result is consistent with most reef flats based on
431 our literature review. Wave height to water depth ratios were shown to vary most during
432 shallow water; increasing on the outer reef flat ($\gamma_s > 0.85$) and decreasing on the inner reef
433 flat ($\gamma_s < 0.1$). In contrast, wave height to water depth ratios were consistent across the entire
434 reef flat ($\gamma_s = 0.31$, $\gamma_{w_max} = 0.5$) under greater water depths and were close to previously
435 reported values. The difference in γ_s between the inner and outer reef flats was also observed
436 in the wave-by-wave analysis and is indicative of increased wave energy dissipation under
437 shallow water due to wave breaking and bed friction. The effects of cross-reef and depth
438 variable γ on a random wave energy dissipation model was also examined. The model
439 results were accurate, but showed errors that were most likely due to the cross-reef
440 variability of γ observed in the wave data. This led to inaccurate descriptions of wave
441 conditions, primarily for the outer reef flat. Our results highlight the challenges when using
442 wave energy dissipation models in environments with steep and complex bathymetry such
443 as coral reefs we show the importance of accurate model calibration with site-specific field
444 data in such systems.

445

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456

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609 **Figure Captions**

610 **Figure 1.** Average wave height to water depth ratios (γ) reported in previous literature based
611 on location of measurement on coral reefs and those derived from wave model calibration.
612 The reported values for both the maximum observed γ and the γ used in calibrated wave
613 models are shown here. The number of observations is shown inside the bars. Errors bars
614 represent one standard deviation. Full data are shown in full in Table S1 and S2 in the
615 supplementary material.

616 **Figure 2.** Location of field site and deployment of pressure transducers (PTs). (A) Location
617 of One Tree Reef (in green) off the southeast coast of Queensland (QLD), Australia; (B)
618 WorldView-2 satellite image combined digital elevation model from Harris et al. (2014) and
619 Beaman (2010) with the location of pressure transducer (PT) deployments shown in the red
620 box; (C) cross-reef schematic of PT deployment locations with mean sea level (MSL)
621 shown in blue; and, (D) aerial view of PT locations on the reef flat with red line showing
622 location of transect in (C).

623 **Figure 3.** (A) Offshore wave height (H_o , from NCOW (Kinsela et al. 2014)) during each of
624 the measurement periods compared to the recorded reef flat significant wave height (H_s);
625 (B) significant wave height to water depth ratios (γ_s) for all measurement locations during
626 each of the measurement periods compared to H_o . Note the non-continuous nature of the x-
627 axis.

628 **Figure 4.** Wave characteristics compared to water depth at each location for the three
629 measurement periods where faded colours are averages for the 15-minute data records for
630 all measurement locations and dark colours are averages for depth bins of 0.2 m: (A)
631 significant wave height with the mean ratio of significant wave height (H_s) to water depth

632 $(\bar{h}, \gamma_s = H_s/\bar{h} = 0.31)$ shown by the black line; (B) zero down-crossing wave period (T); and,
 633 (C) γ_s . The dashed line in (C) corresponds to the slope of the line of best fit in (A).

634 **Figure 5.** (A) Ratio of significant wave height to water depth (γ_s) compared to water depth
 635 at each location. Faded colours are averages for 15-minute data records and dark colours are
 636 averages for depth bins of 0.2 m. Error bars show standard deviation for the binned data.
 637 The equations for the magenta and green lines are Eq. 7 and 8 respectively. (B) Average γ_s
 638 from all measurements for P1-P6 and the reef flat topography. Shaded area shows the
 639 standard deviation.

640 **Figure 6.** The relationship between γ_s and wave deformation (def) for the outer (P1-4, in
 641 magenta) and inner reef flat (P5-6, in green). The linear regression for $\gamma_s < 0.6$ is shown by
 642 the black line (Eq. 9). The circle markers are the values excluded from the linear regression.

643 **Figure 7.** Example time series of the water surface (η) from the pressure record during the
 644 2014 deployment from P2-5. Note the different scales on the y-axis.

645 **Figure 8.** Wave by wave analysis for the 2012 (blue), 2014 (red), and 2016 (black)
 646 measurement periods (A and B). Wave by wave analysis for the six measurement locations
 647 (P1-6) on the reef flat recorded during all three measurement periods (C and D). (A)
 648 Individual wave heights (H) compared to the still water level for individual waves (h_w) (see
 649 panel (B) for legend for dashed lines); (B) The ratio of H to h_w (γ_w) compared to h_w . Faded
 650 colours are averages for 15-minute data records and dark colours are averages for depth bins
 651 of 0.2 m. Error bars show standard deviation for the binned data (see panel (A) for legend
 652 for points); (C) Individual wave heights (H) compared to the mean water depth of the wave
 653 (h_w) (see panel (D) for legend for points and panel (B) for legend for dashed lines); and (D)
 654 the ratio of H to h_w (γ_w) compared to h_w (see panel (B) for legend for dashed lines).

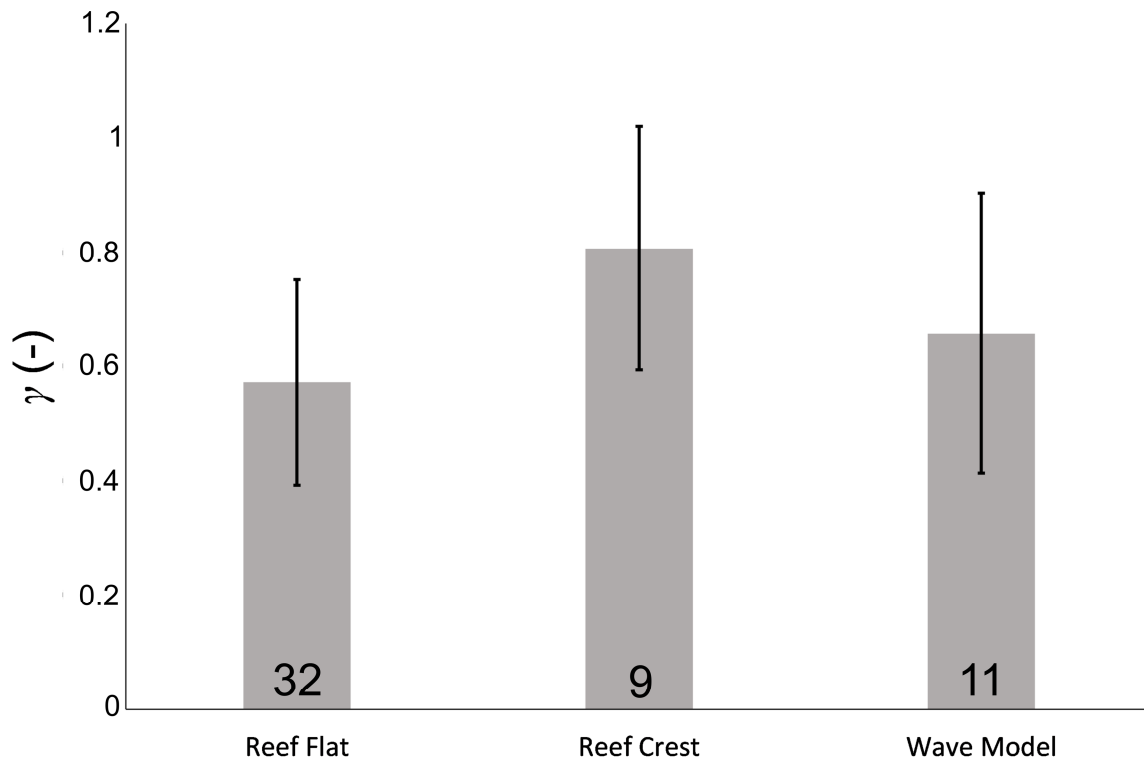
655 **Figure 9.** (A) Average cross-reef modelled and measured root-mean-square wave height
656 (H_{rms}); (B) Comparison of measured and modelled waves for the entire deployment record
657 of 2012 with a spatially varying wave friction factor (f_w) shown in Table 1 (see panel (C) for
658 legend); and, (C) a constant f_w value. The inner reef flat is in green and outer reef flat in
659 magenta. The values used for γ and f_w are shown in panels (B) and (C).

660 **Table Captions**

661 **Table 1.** The calibrated wave friction factor (f_w) used in the wave energy dissipation model
662 (Equation 2) for the regions between the five measurement location in 2012 (Equation 2)

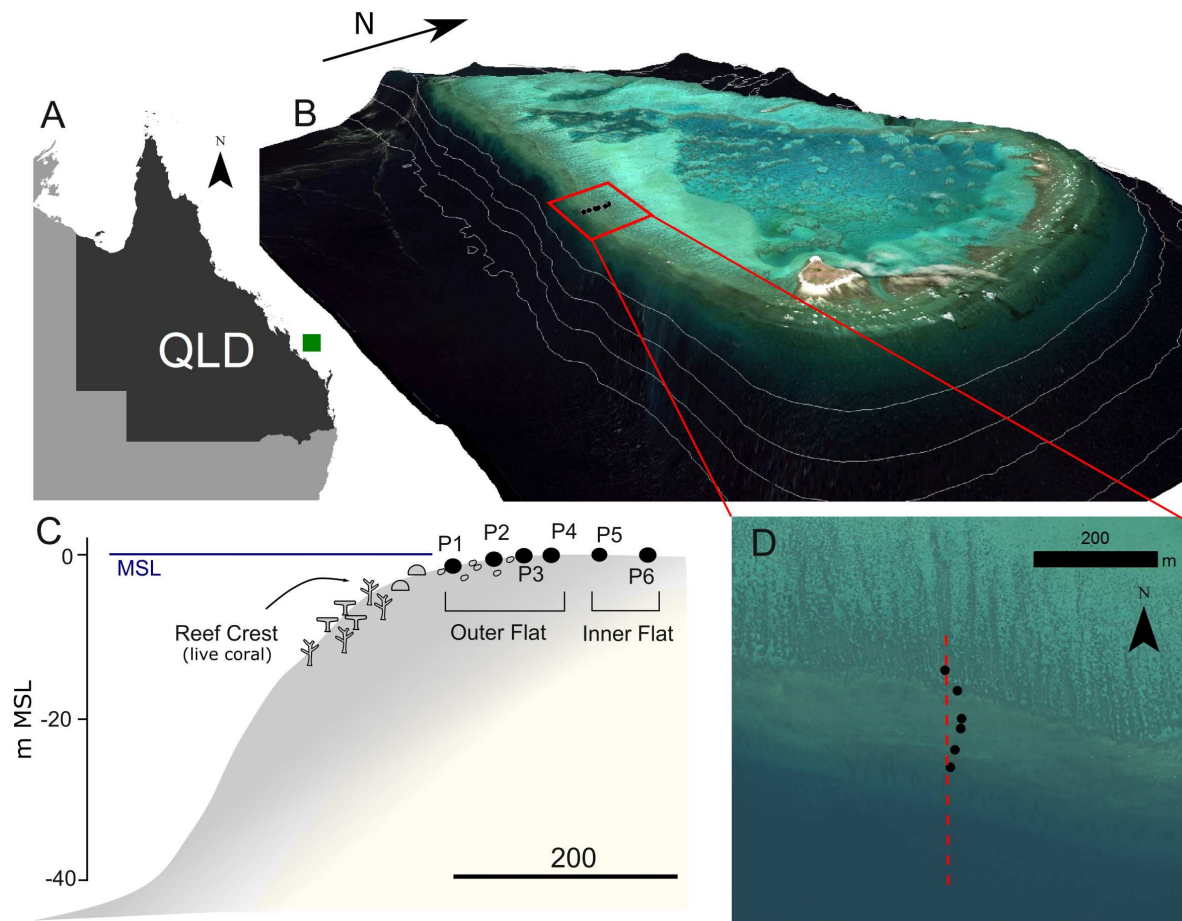
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664

665 **Figures**

666

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673

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 677 Beaman (2010) with the location of pressure transducer (PT) deployments shown in the red
 678 box; (C) cross-reef schematic of PT deployment locations with mean sea level (MSL)
 679 shown in blue; and, (D) aerial view of PT locations on the reef flat with red line showing
 680 location of transect in (C).

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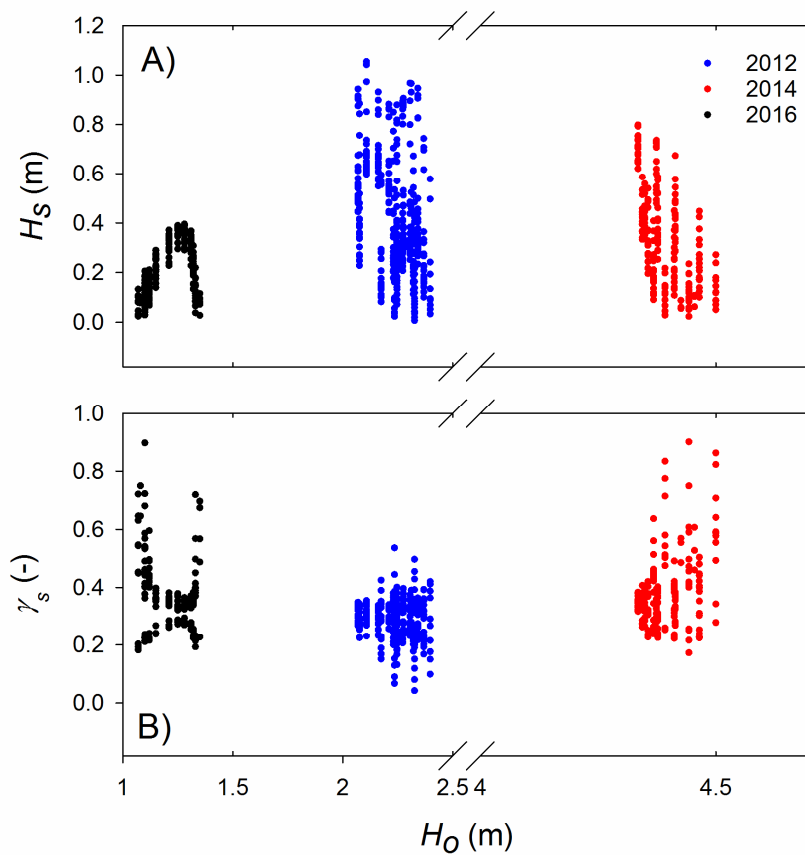
684 **Table 1.** The calibrated wave friction factor (f_w) used in the wave energy dissipation model
685 for the regions between the five measurement location in 2012.

| | f_w |
|------|-------|
| P1-3 | 0.29 |
| P3-4 | 0.18 |
| P4-5 | 0.05 |
| P5-6 | 0.11 |

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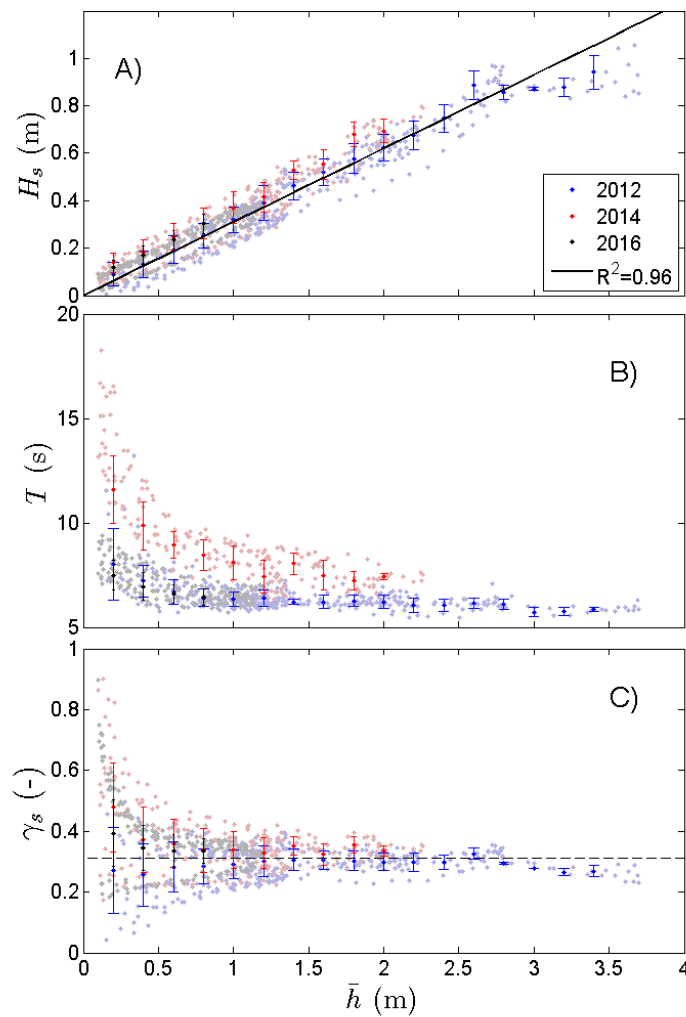
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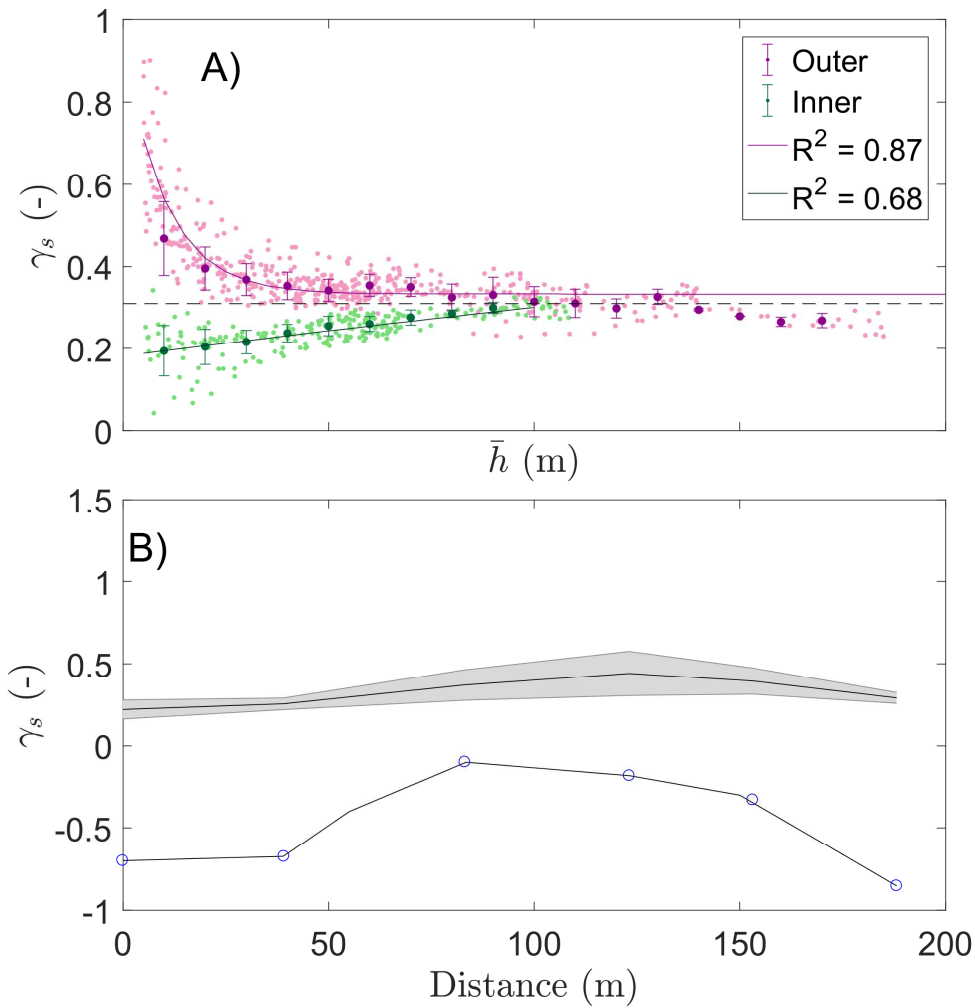
688

689 **Figure 3.** (A) Offshore wave height (H_o , from NCOW (Kinsela et al. 2014)) during each of
 690 the measurement periods compared to the recorded reef flat significant wave height (H_s) ;
 691 (B) significant wave height to water depth ratios (γ_s) for all measurement locations during
 692 each of the measurement periods compared to H_o . Note the non-continuous nature of the x-
 693 axis.



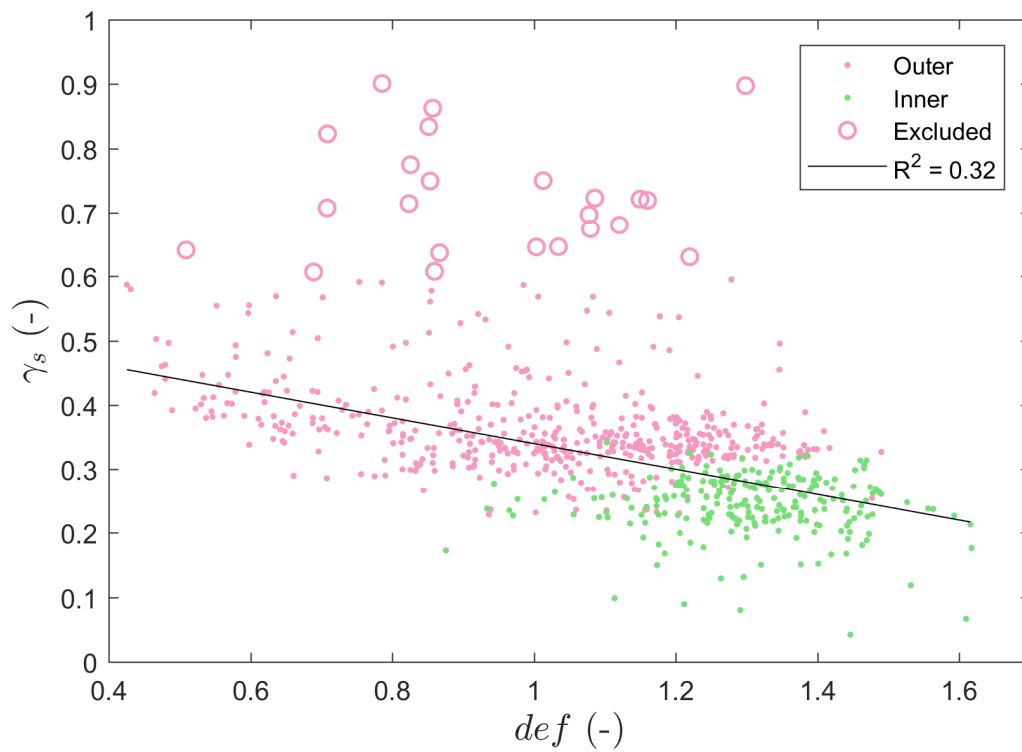
694

695 **Figure 4.** Wave characteristics compared to water depth at each location for the three
 696 measurement periods where faded colours are averages for the 15-minute data records for
 697 all measurement locations and dark colours are averages for depth bins of 0.2 m: (A)
 698 significant wave height with the mean ratio of significant wave height (H_s) to water depth
 699 (\bar{h} , $\gamma_s = H_s/\bar{h} = 0.31$) shown by the black line; (B) zero down-crossing wave period (T); and,
 700 (C) γ_s . The dashed line in (C) corresponds to the slope of the line of best fit in (A).



701

702 **Figure 5.** (A) Ratio of significant wave height to water depth (γ_s) compared to water depth
 703 at each location. Faded colours are averages for 15-minute data records and dark colours are
 704 averages for depth bins of 0.2 m. Error bars show standard deviation for the binned data.
 705 The equations for the magenta and green lines are Eq. 7 and 8 respectively. (B) Average γ_s
 706 from all measurements for P1-P6 and the reef flat topography. Shaded area shows the
 707 standard deviation.



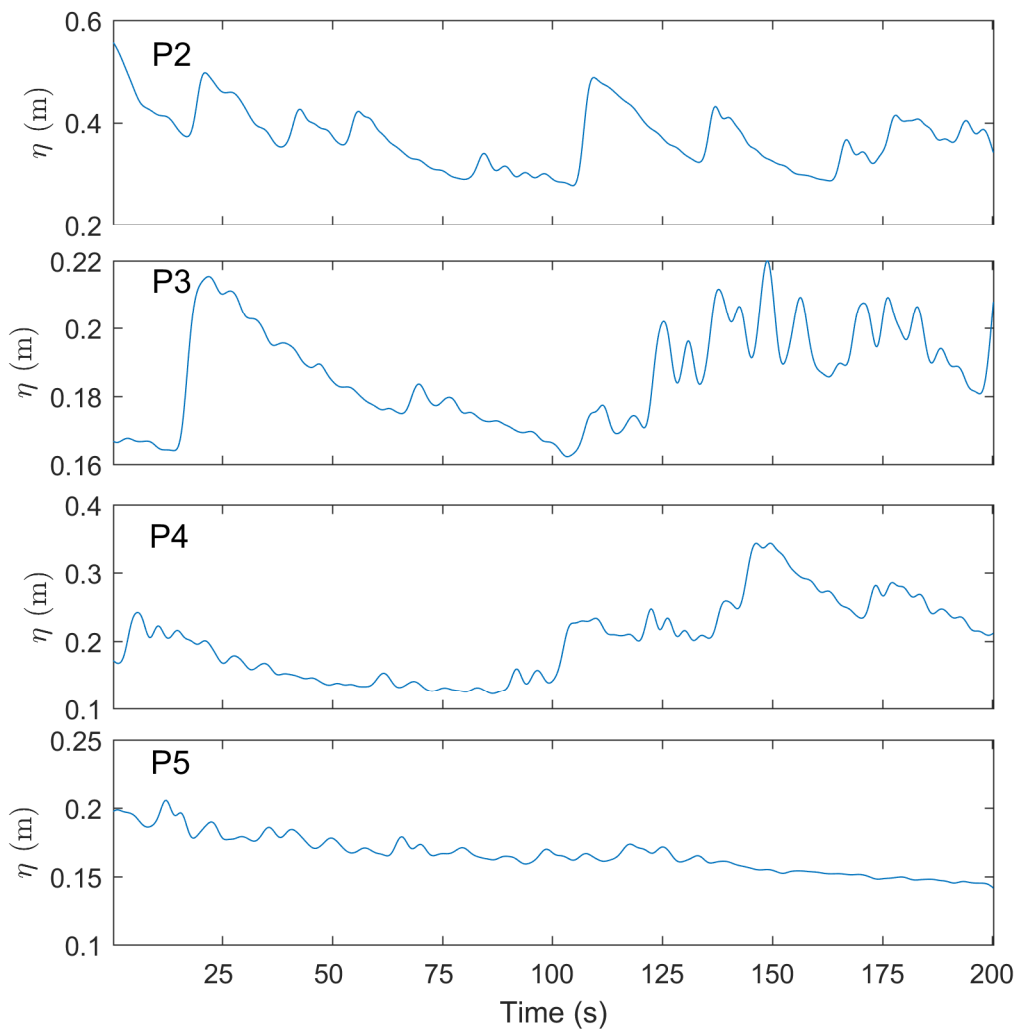
708

709 **Figure 6.** The relationship between γ_s and wave deformation (def) for the outer (P1-4, in
 710 magenta) and inner reef flat (P5-6, in green). The linear regression for $\gamma_s < 0.6$ is shown by
 711 the black line (Eq. 9). The circle markers are the values excluded from the linear regression.

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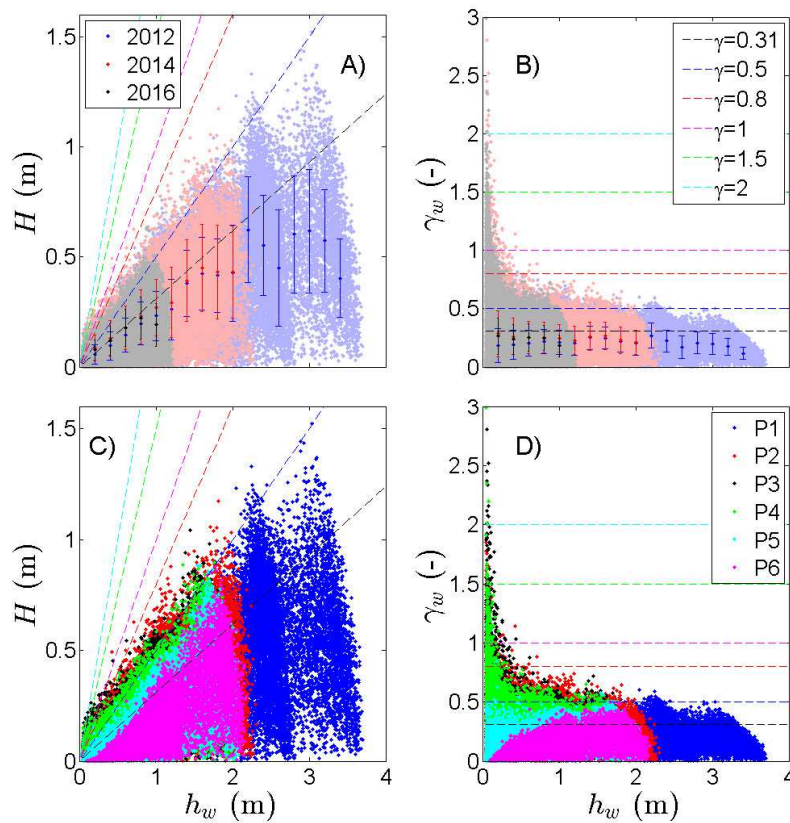


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716 **Figure 7.** Example time series of the water surface (η) from the pressure record during the

717 2014 deployment from P2-5. Note the different scales on the y-axis.

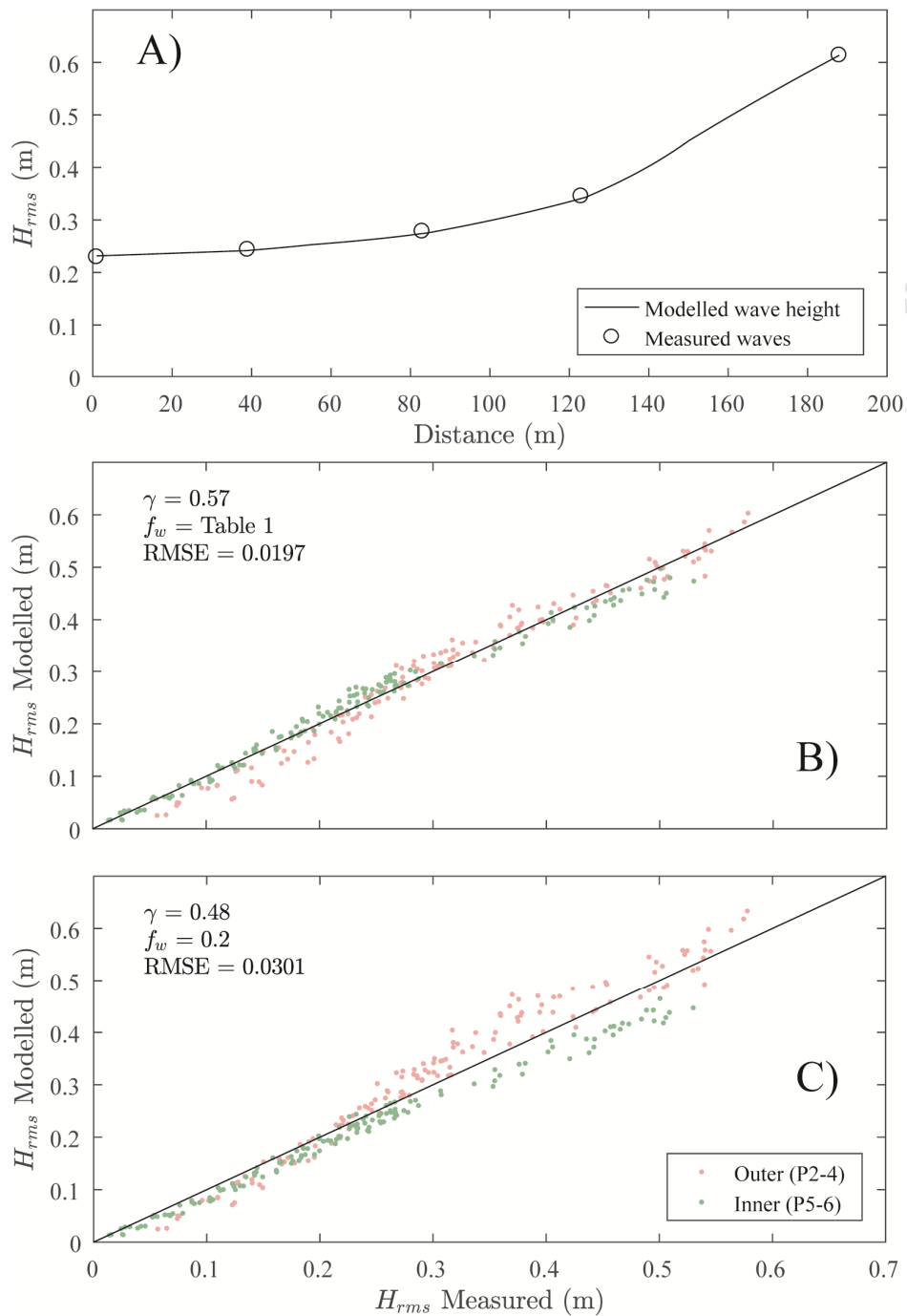
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720 **Figure 8.** Wave by wave analysis for the 2012 (blue), 2014 (red), and 2016 (black)
 721 measurement periods (A and B). Wave by wave analysis for the six measurement locations
 722 (P1-6) on the reef flat recorded during all three measurement periods (C and D). (A)
 723 Individual wave heights (H) compared to the still water level for individual waves (h_w) (see
 724 panel (B) for legend for dashed lines); (B) The ratio of H to h_w (γ_w) compared to h_w . Faded
 725 colours are averages for 15-minute data records and dark colours are averages for depth bins
 726 of 0.2 m. Error bars show standard deviation for the binned data (see panel (A) for legend
 727 for points); (C) Individual wave heights (H) compared to the mean water depth of the wave
 728 (h_w) (see panel (D) for legend for points and panel (B) for legend for dashed lines); and (D)
 729 the ratio of H to h_w (γ_w) compared to h_w (see panel (B) for legend for dashed lines).

730



731

732 **Figure 9.** (A) Average cross-reef modelled and measured root-mean-square wave height
 733 (H_{rms}); (B) Comparison of measured and modelled waves for the entire deployment record
 734 of 2012 with a spatially varying wave friction factor (f_w) shown in Table 1 (see panel (C) for
 735 legend); and, (C) a constant f_w value. The inner reef flat is in green and outer reef flat in
 736 magenta. The values used for γ and f_w are shown in panels (B) and (C).

Saturated surf zone waves were measured and compared to a common wave energy dissipation model

Surf zone saturation and depth limited wave conditions were observed to vary depending on water depth and location on coral reef flats

Errors were observed in the dissipation model correlated with variation in wave height to water depth ratios in the surf zone

Some errors were corrected by spatially varying bed frictional dissipation on the reef flat

Model calibration from surf zone data is required to accurately describe wave height decay on coral reef flats