

Illinois State Water Survey Division SURFACE WATER SECTION

SWS Contract Report 446

AN IMPROVED METHODOLOGY FOR ESTIMATING FUTURE RESERVOIR STORAGE CAPACITIES: APPLICATION TO SURFACE WATER SUPPLY RESERVOIRS IN ILLINOIS

by Krishan P. Singh and Ali Durgunoglu

Prepared for the Division of Water Resources Illinois Department of Transportation

> Champaign, Illinois April 1988

AN IMPROVED METHODOLOGY FOR ESTIMATING FUTURE RESERVOIR STORAGE CAPACITIES: APPLICATION TO SURFACE WATER SUPPLY RESERVOIRS IN ILLINOIS

by Krishan P. Singh, Principal Scientist Ali Durgunoglu, Research Associate

> Illinois State Water Survey 2204 Griffith Drive Champaign, Illinois 61820

CONTENTS

Page

AN IMPROVED METHODOLOGY FOR ESTIMATING FUTURE RESERVOIR STORAGE CAPACITIES: APPLICATION TO SURFACE WATER SUPPLY RESERVOIRS IN ILLINOIS

by Krishan P. Singh and Ali Durgunoglu

INTRODUCTION

Intrastate rivers arc one of the main sources of surface water supply in Illinois. With the exception of the Illinois and Fox Rivers, intrastate rivers usually have very low flows during dry years. To ensure an adequate and dependable water supply, one of the following means is used: inchannel dams, which create storage reservoirs; low-channel dams (which create enough storage to meet a few weeks' demand during very low streamflow conditions) on rivers with relatively sustained flows; side-channel reservoirs to which water is pumped from rivers during moderate or high-flow conditions; and sometimes auxiliary or standby ground-water wells.

Currently there are more than 90 public water supply systems in central and southern Illinois that have generally poor ground-water resources and that rely partially or totally on intrastate rivers for water supply. A list of these public water supply systems is given by Singh *et al.* (1988). There are more than 80 in-stream impounding reservoirs that supply water to these water supply systems. The adequacy and reliability of these water supplies are therefore largely dependent upon the ability of these reservoirs to provide sufficient water storage during the critical dry periods. However, these surface water reservoirs face many problems that may result in the decrease of their safe yield and thus in an inadequacy to supply sufficient water in the next 10-40 years. Some of these problems are: a) increases in water demand because of increases in population, industry, or per capita water use; b) gradual loss of reservoir capacity and yield because of sedimentation in the reservoirs; and c) emerging recreational demands and demands for mandatory low-flow releases from the reservoirs for maintaining streamwater quality, ecology, and aquatic habitats.

To evaluate the future reliability of public water supply systems using intrastate rivers as their main source of supply, an inventory of the systems using intrastate rivers was done by Singh *et al.* (1988). Also determined in this study were the future water demands of investigated water supply systems based on population projections, historical water use, and anticipated trends in future water needs. What is needed therefore is an evaluation of current reservoir capacities and projections of future capacities in the next 10-40 years on the basis of historical data and reservoir sedimentation modeling. Only after that can we estimate the years when each water supply system may become

inadequate under various drought scenarios. Then the systems which appear to be at high risk can be selected and further investigated to determine mitigative measures.

The purpose of this study was to develop an improved methodology for determining the future capacities of the water supply reservoirs for the next 10-40 years, based on the available data from reservoir sedimentation surveys. The results have also been used for capacity projections for the non-surveyed reservoirs that are being used for water supply.

Acknowledgments

This study was jointly supported by the Division of Water Resources (Illinois Department of Transportation) and the Illinois State Water Survey. Gary Clark of the Division of Water Resources served in a liaison capacity during the course of the study. This report was prepared under the general direction of Richard G. Semonin (Chief) and Michael L. Terstriep (Head of the Surface Water Section), Illinois State Water Survey. Abhijit Dasgupta and Wendy Knepp helped in collecting data and in preparing the computer programs and illustrations. Gail Taylor edited the report.

BACKGROUND INFORMATION

The sediment inflow rate into a particular reservoir is, in general, a function of the watershed characteristics such as drainage area, average land and channel slope, soil type, land management and use, and hydrology. The rate of storage reduction in a reservoir due to sedimentation usually depends on the rate of sediment inflow; type of sediment material (sand, silt, clay); consolidation rate of the existing sediment deposits; type of dam outlet structures; and operation of the dam.

Most small- and medium-sized in-stream reservoirs with overflow spillways are designed to impound 5-15% of the average annual streamflow, but about 75-90% of the incoming sediment is entrapped during the process. This is because the sediment concentrations are significantly higher towards the bottom of the lake, and when the floodwater flows over an overflow spillway, cleaner water is skimmed from the top of the lake. The ratio of the volume of trapped sediment in a reservoir to the volume of incoming sediment to a reservoir is usually referred to as the reservoir's trap efficiency. Several factors may affect the trap efficiency of a reservoir, including 1) capacity-inflow (C/I) ratio = acre-feet capacity per acre-foot of annual flow, since as the capacity of the reservoir gets larger less water is released downstream and a higher percentage of incoming sediment is trapped; and 2) compaction of the sediment deposits due to different reservoir operations. Normally ponded reservoirs with sediment deposits that are always submerged will have a smaller compaction rate than desilting basins and reservoirs with periodic drawdowns. If a reservoir is periodically lowered for maintenance or other purposes, then the sediment deposits are compacted faster than the natural compaction rate.

The sedimentation process is a very complicated phenomenon governed by several hydraulic and hydrologic variables. Unfortunately there is no analytical relation that can be used directly for estimating the rate of deposition or capacity loss in a reservoir, given all the relevant parameters. Because of that, reservoir sedimentation rates are based primarily on empirical relations, which are then calibrated by using field measurements. Therefore a reservoir sediment model and a computerized methodology were needed for analyzing the available data from reservoir sediment surveys in order to calibrate the empirical relations, and for estimating the future storage capacities of the water supply reservoirs based on the empirical relations. This study has two major parts. The first part involves determining a statewide pattern of reservoir sedimentation, based on the extensive reservoir sediment survey data collected in Illinois. The second part uses the results obtained from the first part of the study to estimate the sedimentation rates of the non-surveyed water supply reservoirs, and derives the future storage capacities of these reservoirs.

The models developed here for estimating the reservoir sedimentation and future capacity projections are based upon equations for storage continuity and stream sediment yield.

RESERVOIR SEDIMENTATION MODEL

Reservoir sedimentation surveys have been conducted for more than 100 reservoirs across the state of Illinois. This extensive data base was used to establish a pattern of reservoir sedimentation in Illinois by using the methodology explained subsequently. The sedimentation pattern was then used for estimating the sedimentation rate or relevant parameters used in the method, and finally for estimating the future- storage capacities of the non-surveyed water supply rescrvoirs. In order to perform any of the above tasks we use a storage balance (or continuity) equation. The storage continuity equation used in the development of the methodology is given by

$$
C_0 = S \cdot \Delta T + C_T \tag{1}
$$

where

 C_0 = initial storage, or the design capacity of the reservoir at time T_0

 $S =$ annual reservoir capacity loss rate due to sedimentation

 $T =$ time elapsed $(T - T_0)$ in years

 C_T = available reservoir capacity at time $t = T$

For the surveyed reservoirs the C_0 value is usually available. If C_0 is not available for a surveyed reservoir, the capacity estimate from the earliest sedimentation survey can be used for C_0 , and T_O is taken as the year that survey was made. For the non-surveyed water supply reservoirs, $C₀$ values had to be estimated from several sources. S - T gives the total capacity loss in T years due to sediment deposition. S is not a constant value but changes from year to year due to fluctuations in the inflow and to changes in trap efficiency and sediment density. C_T values are usually estimated by the reservoir sediment surveys, and are used with C_0 to calculate the S values. For water supply reservoirs C_T usually indicates the projected capacity in year T, and it is estimated by using sufficiently small values of T successively in equation 1. Through this procedure, all the parameters affecting S can be updated after each T increment.

Reservoir Capacity Loss **Rate**

Reservoir capacity loss rate, S, is usually derived from stream sediment yields. One method of predicting stream sediment yields is by combining intermittent sediment concentration data with continuous discharge data in the form of a rating curve. The total sediment of the stream can then be estimated by convoluting the rating curve by the flow-duration curve of the stream. This method is applicable only if sediment concentration and discharge data are available for a particular location. The method used in this study for evaluating the stream sediment yield is a modified version of the Upper Mississippi River Basin Commission (UMRBC, 1970) approach. The UMRBC approach describes the sediment yield of a stream as

$$
Y = K \cdot A^{-0.12} \tag{2}
$$

where

 $Y =$ sediment yield in tons per year per square mile of watershed area

 $K = a$ regional constant

 $A =$ watershed area in square miles

The reservoir capacity loss rate, S, in acre-feet per year can then be obtained as

$$
S = \frac{Y \cdot A \cdot TE}{2178 \cdot \delta} \tag{3}
$$

and by substituting equation 2 in equation 3, we get

$$
S = \frac{K \cdot A^{0.88} \cdot TE}{2178 \cdot \delta} \tag{4}
$$

where

 $TE = \text{trap efficiency}$ of the reservoir in percent

2178 = a conversion constant

 δ = density of sediment in pounds per cubic feet

Regional constant K: The general distribution of the regional constant K over Illinois is given by Terstriep *et al.* (1982). K values represent the degree of severity of sediment deposition in a reservoir. The land resource areas (LRA) and their K values for Illinois are shown in Figure 1. However, preliminary investigations done in this study by using the reservoir sedimentation survey data, as well as equations 1 and 4, revealed that the variation of K values within a particular LRA may be quite significant compared to the values given in the figure. Therefore, the K values of all the reservoirs for which sediment surveys had been done were calculated by using the procedure explained in the following sections, and were taken as the basis for estimating the future storage capacities of the non-surveyed water supply reservoirs.

Trap efficiency: The trap efficiency (TE), given as a percentage of the volume of stream sediment retained in the reservoir, can be estimated by using Brune's curve (Brune, 1953). Brune's curve, as shown in Figure 2, correlates the trap efficiency of a reservoir to its capacity-inflow (C/I) ratio. If the C/I ratio is high, then less water and subsequently less sediment will be released from the reservoir, and the trap efficiency will be high. Brune's curve should be used for reservoirs operated with overflow spillways under submerged conditions. The trap efficiency of a reservoir

Figure 1. Land resource areas in Illinois, and regional factors (K) (from Terstriep et al., 1982)

Figure 2. Brune's curve (from Brune, 1953)

gradually decreases during its useful life, because the C/I ratio diminishes due to sediment deposition.

Density of sediment: The density δ of the sediment deposits also varies with time due to compaction. The rate of compaction of the deposits depends on the content of the sediment material (percentage of sand, silt, and clay), and whether or not the deposits are exposed to drying due to drawdown. Lane and Koelzer (1943) presented the following empirical equation, based on the age and grain-size distribution of the sediment, for estimating the density:

$$
\delta_{\Gamma} = \delta_1 + M \cdot \log T \tag{5}
$$

where

 δ_{Γ} = density of sediment after T years of compaction

 $\delta_{\mathbf{i}}$ = density at the end of first year

 $M =$ an adjustment constant for compaction

The values of δ_1 and M for different sediment types and reservoir operation conditions are given in Table 1.

Equation 5 gives the density of the first year's deposits after T years of consolidation. The average density $\overline{\delta}_{T}$, which includes the subsequent years' deposits, can be obtained by integrating equation 5 over T years as

$$
\overline{\delta}_{\Gamma} = \delta_{i} + \frac{M}{T} \sum_{i=1}^{T} \log t
$$
 (6)

If the sediment deposits consist of a mixture of materials, then the weighted average $\overline{\delta}_{\Gamma}$ can be obtained with the following equation, by using the percent weight distribution P of the sediment materials:

$$
\overline{\delta}_{\mathrm{T}} = \frac{1}{100} \sum_{i=1}^{3} \mathrm{P}_{i} \left[\delta_{i,1} + \frac{\mathrm{M}_{i}}{\mathrm{T}} \sum_{i=1}^{\mathrm{T}} \mathrm{log} \, \mathrm{t} \right] \tag{7}
$$

where the index $i = 1, 2$, and 3 represents sand, silt and clay, respectively.

Another form of the density function can be obtained by integrating the capacity loss rate S, given by equation 4, to obtain an average capacity loss rate \overline{S} for a period of T years.

$$
\overline{S} = \frac{K \cdot A^{0.88} \cdot TE}{2178 \cdot \overline{\delta}_{T}} = \frac{1}{T} \sum_{\nu=1}^{T} \left[\frac{K \cdot A^{0.88} \cdot TE}{2178 \cdot \delta_{t}} \right]
$$
(8)

By substituting equation 5 in equation 8, and cancelling identical terms (assuming that TE values do not change significantly), we get

$$
\frac{1}{\overline{\delta}_{\mathrm{T}}} = \frac{1}{\mathrm{T}} \sum_{t=1}^{\mathrm{T}} \left[\frac{1}{\delta_{\mathrm{I}} + \mathrm{M} \cdot \log t} \right]
$$
(9)

and, similarly, for sediment deposits composed of sand, silt, and clay equation 9 becomes

$$
\frac{1}{\tilde{\delta}_{\mathrm{T}}} = \frac{1}{\mathrm{T}} \sum_{\mathrm{rel}}^{\mathrm{T}} \frac{1}{100} \sum_{i=1}^{3} \left[\frac{P_i}{\delta_{i,1} + M_i \cdot \log t} \right]
$$
(10)

Equation 7 may be more desirable for simple hand calculations; however, more accurate results may be obtained by using equation 10 in a computer program.

Combining equation 1 with equation 4, replacing δ with $\overline{\delta}_{T}$, and dividing both sides by the annual inflow I, we get a new form of the continuity equation:

$$
\frac{C_0}{I} = \frac{C_T}{I} + \frac{K \cdot A^{0.88} \cdot TE}{2178 \cdot I \cdot \overline{\delta}_T} \Delta T
$$
 (11)

Annual flow I is incorporated into the continuity equation, so that it is easier to use Brune's curve for calculating TE in the algorithm. If the initial conditions and all other parameters are determined (or estimated), then the future reservoir capacity C_T can be estimated by using equation 11 successively with any selected T value over the period T_0 to T .

DATA USED IN THE STUDY

The main volume of data used in this study came from the reservoir sedimentation surveys in Illinois. More than 100 reservoirs have been surveyed, covering most of the state with the exception of some northeastern counties. The names and code numbers of the counties where surveys have been conducted and water supply reservoirs are located are given in Table 2. The reservoir sedimentation surveys provide valuable information about the drainage area, initial storage, construction year, and capacities of the reservoirs during years in which subsequent surveys were conducted. Any changes regarding the storage capacities of the reservoirs are also available in the State Water Survey files. Average annual inflow values are taken from the Upper Mississippi River Basin Commission report (UMRBC, 1970) (see Figure 3).

More than 20 of the reservoir surveys in Illinois included particle size analysis for determining the granulometric distribution of sediment deposits. On the basis of these data, sediment materials were classified under 3 groups with respect to their average particle diameter D, as follows:

Figure 3. Average annual runoff for Illinois in inches per square mile (from UMRBC, 1970)

D 0.004 mm Clay 0.004 mm $\lt D$ 0.062 mm Silt 0.062 mm $< D$ 2.0 mm Sand

The locations of the reservoirs for which particle size distribution analyses were done, and their percentages of sand, silt, and clay, are shown in Figures 4, 5, and 6. From these data, contour maps were generated to determine a pattern of particle size distribution of sediment deposits in Illinois. These maps were then used for estimating the sand, silt, and clay percentages of the sediment deposits of the reservoirs for which particle size distribution analysis was not done. The particle size data for sand, silt, and clay for the surveyed reservoirs were generated from these maps and are given in Table 3.

All the available and estimated data for the surveyed reservoirs, including the annual inflow and the dam coordinates, are also given in Table 3. The storage capacities listed in Table 3 in most cases show decreases with time. However, if the reservoir was dredged or the spillway crest was raised at any time, this condition is indicated by an increase in the storage. For example, the spillway of Mt. Sterling Reservoir was raised by 1 foot in 1954, resulting in a storage increase of 62 acre-feet (295.2 minus 233.3). The K values given in Table 3 for the surveyed reservoirs were calculated by the algorithm that is explained in detail in the next section. The locations of the surveyed reservoirs are shown in Figure 7.

The data for the non-surveyed water supply reservoirs were collected from personal communication with the municipalities and water treatment plants, from Corps of Engineers dam safety reports, and from Illinois Environmental Protection Agency records and publications (Illinois Environmental Protection Agency, 1978a, 1978b). Drainage areas were usually verified from topographical maps. Particle size distributions were estimated from Figures 4, 5, and 6, as mentioned earlier. The data used in the analysis of the non-surveyed water supply reservoirs are listed in Table 4. The locations of the water supply reservoirs are shown in Figure 8.

Figure 4. Locations of reservoirs for which sediment particle size analysis has been conducted, and contour map for percent sand distribution

Figure 5. Locations of reservoirs for which sediment particle size analysis has been conducted, and contour map for percent silt distribution

Figure 6. Locations of reservoirs for which sediment particle size analysis has been conducted, and contour map for percent clay distribution

K is the regional constant.

Ш

Figure 7. Locations of surveyed reservoirs

 $\mathcal{L}^{\mathcal{L}}$

Figure 8. Locations of water supply reservoirs

CALCULATION OF FUTURE RESERVOIR STORAGE CAPACITIES

The future reservoir storage capacities C_T can be estimated by using equation 11 if all the required parameters are known. Some of the parameters, such as TE and $\bar{\delta}_{r}$, are time dependent and need to be changed at certain time intervals. Other parameters such as C_0 , inflow, drainage area, and K are assumed to be constants, and may be estimated easily from physiographic properties of the reservoir. However, selection of the value for K may require additional care, since a preliminary investigation of the results of reservoir sedimentation surveys indicated that the K values given by UMRBC (see Figure 1) differ significantly from the K values calculated by using equation 11. This indicated a need to calculate the K values of the surveyed reservoirs by using the reservoir sedimentation surveys and equation 11. Therefore one of the purposes of developing this methodology was to estimate K values of the surveyed reservoirs by using the data from the reservoir sedimentation surveys.

The distribution of the calculated K values could then be used for estimating the unknown K values of the non-surveyed water supply reservoirs. If the surveyed reservoir is also used for a water supply reservoir, its future capacity could be projected by using the calculated K value. If a water supply reservoir had not had a sediment survey performed for it, then its K value was estimated by using the distribution of K values of the surveyed reservoirs.

The following algorithm was developed to perform the tasks required to estimate the future reservoir storage capacities of the water supply reservoirs. It summarizes the step-by-step procedure used in the methodology and can be used for the following purposes: 1) calculating the average K values by using the data from the reservoir sedimentation surveys, and 2) estimating the future capacity by inputting C_0 , T_0 , and K values. If a surveyed reservoir is also used as a water survey reservoir, both steps can be performed at once by the algorithm.

Algorithm

1. Input:

 $I = inflow (inches/year)$

- $A = \text{drainage area}$ (square miles)
- P_i = percent sand, silt, or clay; $i = 1, 2,$ or 3
- $t(j)$ and C_j = years and capacity estimates of each survey $(j = 0, ..., N)$

 $(j = 0$ indicates initial conditions and N is the actual number of surveys)

2. Set $j = 0$.

If $N = 0$, then input K (no surveys), and go to step 9 (do not estimate K).

If $j = N$, go to step 8.

Otherwise $\Delta t_j = t(j+1) - t(j)$.

3. Estimate an average capacity-inflow ratio CIR and trap efficiency by using the surveys j and **j+1.**

$$
CIR = \frac{C_j + C_{j+1}}{2I}
$$

TE (CIR) = f (CIR)

where TE(CIR) is a function of CIR and is estimated from Brune's curve.

- 4. Calculate average sediment density, $\overline{\delta}$, for Atj years (equation 7 or 10).
- 5. Calculate an initial average estimate of K from equation 11:

$$
\overrightarrow{\mathbf{K}}_{j} = \frac{(\mathbf{C}_{j} - \mathbf{C}_{j+1}) \cdot \overline{\delta} \cdot 2178}{\mathbf{A}^{0.88} \cdot \mathbf{TE(CIR)} \cdot \Delta t_{j}}
$$

6. Calculate an estimate of capacity \mathbf{C}_{j+1}^* by usin $\overline{\mathbf{K}}_j$ nd equation 11, with t^* year increments $(At^* - t_i)$:

$$
C_n^* = C_{n-1}^* - \frac{\overline{K}_j \cdot A^{0.88} \cdot TE(C_{n-1}^*) \cdot \Delta t^*}{2178 \cdot \overline{\delta}_n} \quad \text{for} \quad n = t_j + \Delta t^*, \dots, t_{j+1}
$$

where \mathbf{C}_n^* is a capacity estimate at the intermediate year, n, between two successive surveys, and TE(C_{n-1}^*) is the trap efficiency of the intermediate storage C_{n-1}^* , obtained from Brune's curve. The estimated capacity C_{j+1}^* should match the surveyed capacity C_{j+1} . In this study, At* was taken as 1 year.

7. If $|C_{j+1}^* - C_{j+1}| \le \varepsilon C_{j+1}$, then

$$
K_j = \overline{K}_j
$$

$$
j = j + 1
$$
 Go to step 2.

Otherwise, change $\overline{\mathbf{k}}_j$ by

$$
\Delta K = C_{j+1}^{*} - C_{j+1}
$$

$$
\overline{K}_{j} = \overline{K}_{j} + \Delta K
$$
 Go to step 6.

In this study, e has been taken as 0.001.

8. Compute the weighted average K for the entire survey period, or just input K if the algorithm is to be used for estimating capacity projections (for $N = 0$):

$$
K = \frac{\sum_{j=0}^{N-1} K_j \cdot \Delta t_j}{\sum_{j=0}^{N-1} \Delta t_j}
$$

9. Estimate capacity projections by using equation 11, K, $\overline{\delta}_i$, and t(j) for t(j) > t(N):

$$
\frac{C_{j+1}}{I} = \frac{C_j}{I} + \frac{K \cdot A^{0.88} \cdot TE(C_j) \cdot (t_{j+1} - t_j)}{2178 \cdot \overline{\delta}_{j+1}}
$$

A computer program was written to execute the algorithm explained above. For calculating K and the capacity projections, all the steps in the algorithm must be performed. However, if a water supply reservoir has had no surveys and its K value is estimated from the results of reservoir sedimentation surveys, then only steps 1, 2, and 9 need to be performed.

Brune's curve was used for calculating TE, by expressing it in an analytical form of piecewise equations. The TE value used in step 6 was recalculated for each C* value. It has been found that, for reliable results, the time increment At* used in step 6 should be less than 5 years or Atj, whichever is smaller. The reliability of K_j depends highly on the accuracy of the survey results and Atj. Another factor that may affect the weighted average K values is the time difference between two successive surveys. If At is very large C_j - C_{j+1} will be large, and then the average TE value calculated in step 3 will be very rough. In such a case t^* , used in step 6, should be taken in as small an increment as 1 year to compensate for the error introduced in step 5.

The algorithm can handle situations where the reservoir capacity is increased by dredging or construction. Additional data needed to incorporate these situations are the capacity estimates for just before and after any changes were made, and the corresponding years of these changes. This is a very useful feature of the algorithm since some of the reservoirs in Illinois have their capacities changed either by increases in the spillway elevation or by periodic dredging. Another major feature of this methodology is the use of time-varying sediment density and trap efficiency. The average density of the sediment deposits containing an average of 50% clay and 50% silt may increase by 10 pounds per cubic foot during the first 20 years. Another 5-pound increase will take place in the next 40 years. Therefore, the difference between using a general average density or a time-varying density can be very significant over the early life of a reservoir. If the C/I ratio of a reservoir reduces from 0.10 to 0.01 over its useful life, then the trap efficiency will reduce by about 40%.

RESULTS

The results of this study are presented under two main subsections. The first section includes the analysis of surveyed reservoirs to establish statewide patterns of reservoir sedimentation and the distribution of sediment materials. These results may also serve as a basis for determining the causes of apparent local deviations in the sedimentation patterns in Illinois. The second section is basically an extension and application of the results of the sedimentation surveys to the water supply reservoirs for the estimation of future reservoir storage capacities. This information can be used to determine the safe yields of the water supply reservoirs in the future for various drought recurrence intervals, and can be combined with the results of a parallel study (Singh *et al.,* 1988), which is oriented toward estimating the future demands of the water supply systems. Then it will be possible to determine when a reservoir will become inadequate, and the type of mitigative measures that can extend the adequacy of the reservoirs for a number of years into the future.

Analysis of Surveyed Reservoirs

Reservoirs for which sedimentation surveys had been conducted were analyzed in order to develop more reliable sedimentation patterns in Illinois and to update the K values to be used in the sediment deposition model that was developed. K values represent the degree of severity of sediment deposition in a reservoir, and using inaccurate values may yield serious errors in storage capacity projections. The K values given by the UMRBC (see Figure 1) were not used in this methodology since they did not reflect the large regional deviations indicated by the reservoir sedimentation surveys. Rather than using the K values given in Figure 1, a new pattern of K values was developed by calculating the K values from the surveyed reservoirs (by using the developed model). This pattern was then used for projections of future storage capacity of the water supply reservoirs.

This task was achieved by analyzing 118 reservoirs for which sedimentation surveys had been conducted. A list of these reservoirs is given in Table 3, together with the data needed for calculating the K values. These surveyed reservoirs cover most of the state (except for the northeastern part) as shown in Figure 7. Forty-one of the surveyed reservoirs are also being used as water supply reservoirs; they are identified by asterisks following the reservoir codes in Table 3 and by underlines below the reservoir codes in Figure 7. The K values of the surveyed reservoirs were obtained by using a computer program developed to perform the procedure given by the algorithm. Changes in trap efficiency and in the density of sediments due to compaction over time have been incorporated in the model. The K values calculated from the reservoir sedimentation surveys are listed in Table 3, and these values are shown in Figure 9 to illustrate the statewide variation of K. Underlined K values in Figure 9 indicate surveyed water supply reservoirs.

Every attempt was made to simulate the actual reservoir conditions. For example, any increases in the storage capacities of the reservoirs due to construction or dredging were considered in the method. Streams that had multiple reservoirs in series (such as Lake Mattoon and Lake Paradise on the Little Wabash River, and Carlyle Lake and Lake Shelbyville on the Kaskaskia River) were treated differently, because the upstream reservoirs trap part of the sediment coming from the drainage areas upstream of the reservoirs. In such cases, the effective drainage areas of the downstream reservoirs were estimated by considering the drainage area below the upstream reservoirs and the trap efficiency of the upstream reservoirs.

For some large or multi-purpose reservoirs like Carlyle Lake and Rend Lake, storage capacities were taken at normal pool elevations, although the levels may rise above these levels during flood conditions. For Carlyle Lake normal pool elevation was taken as 445 feet (summer pool), and for Rend Lake the main spillway crest elevation at 405 feet was considered as the normal pool.

Variation in K values: The distribution of K values in Figure 9 exhibits a great deal of spatial variability. Although the general trend follows the pattern given by Figure 1, in some cases the variation of K values in a particular land resource area may be more than 100%. For example, in Franklin County (Code No. 28) the K values range from $K = 901$ for Valier Outing Club Reservoir (Reservoir Code No. 3) to $K = 4270$ for Rend Lake (Reservoir Code No. 6). The K value for Franklin County is given as $K = 1500$ in Figure 1. Also, in Sangamon County (Code No. 84) K = 653 for Schmidt Pond (Reservoir Code No. 4), and K = 1655 for Aschauer Pond (Reservoir Code No. 1). The K value for Sangamon County is given as $K = 1200$ in Figure 1.

Several factors may affect the variation of K. Analysis of the surveyed reservoir sites on topographic maps indicated that most of the variation in K can be explained by variation of land slope and watershed size. If the watershed is small, there is a greater chance that the average land slope of the reservoir drainage area will not be representative of the overall slope of the land resource area used for defining K values.

Duration of the survey records is also important. Reservoirs with relatively short records (less than 10 years, for example) may also show significant deviations since the hydrologic variables, like inflow, used in the calculations represent long-term averages and may not reflect the conditions that occur in a relatively short period. Therefore, it is suggested that careful consideration should be given to estimating K values, especially in regions where there is considerable variation in sedimentation patterns.

Figure 9. Calculated K values for the surveyed reservoirs

Analysis of Water Supply Reservoirs

In this study future storage capacities of 82 water supply reservoirs were estimated by using the developed methodology. Forty-two of the water supply reservoirs investigated here had also had reservoir sedimentation surveys conducted, and thus their K values were calculated by using data from the sediment surveys. The average K values of the remaining non-surveyed water supply reservoirs were estimated on the basis of the distribution of the K values calculated from the sediment surveys (Figure 9).

Several factors that were found to contribute to the regional variability of the K values were also considered in the estimation of K. For example, land slope, watershed size, and land use of the surrounding surveyed reservoirs were examined before selecting the K values of the non-surveyed water supply reservoirs. The estimated and calculated K values of all the water supply reservoirs are given in Table 5 and are also shown in Figure 10. The surveyed water supply reservoirs are indicated by asterisks in Table 5, and by underlined K values in Figure 10.

The projected future storage capacities of the water supply reservoirs up to the year 2030 are given in Table 5 for 10-year increments. These storage projections reflect an extension of the past sedimentation patterns of the reservoirs. Utmost care has been given in estimating the K values used in the capacity projections, by trying to use the local variations of the parameters believed to affect the sedimentation process in reservoirs. It should be kept in mind that all the storage capacity projections made here are based on the normal reservoir operations, and on hydrological conditions based on data for fairly long durations. Persistent deviations from normal conditions, such as changes in the operation policy of the reservoirs, or long periods of very wet or dry spells, would obviously affect the physiographic and hydrologic parameters used in the model.

K is the regional constant.

Figure 10. K values used for storage capacity projections for the water supply reservoirs

REFERENCES

- Brune, G.M. 1953. Trap Efficiency of Reservoirs. Transactions of American Geophysical Union 34(3):407-418.
- Illinois Environmental Protection Agency. 1978a. Assessment and Classification of Illinois Lakes: Volume I. Illinois Environmental Protection Agency, 208 Water Quality Management Planning Program Staff Report. Planning and Standards Section, Division of Water Pollution Control, Springfield, Illinois.
- Illinois Environmental Protection Agency. 1978b. Assessment and Classification of Illinois Lakes: Volume II. Illinois Environmental Protection Agency, 208 Water Quality Management Planning Program Staff Report Planning and Standards Section, Division of Water Pollution Control, Springfield, Illinois.
- Lane, E.W., and V.A. Koelzer. 1943. Density of Sediments Deposited in Reservoirs, Report 9. *In* A Study of Methods Used in Measurement and Analysis of Sediment Loads in Streams. U.S. Corps of Engineers, St. Paul District Sub-Office, Hydraulic Laboratory, University of Iowa, Iowa City, Iowa.
- Singh, K.P., S.M. Broeren, R. B. King, and M. L. Pubentz. 1988. Future Water Demands of Public Surface Water Supply Systems in Illinois. Illinois State Water Survey Contract Report 442, 51 P.
- Terstriep, M.L., M. Demissie, D.C. Noel, and H.V. Knapp. 1982. Hydrologic Design of Impounding Reservoirs in Illinois. Illinois State Water Survey Bulletin 67.
- Upper Mississippi River Basin Commission. 1970. Comprehensive Basin Study, Vol. 3.

Note: Various State Water Survey (SWS) personnel have been conducting lake sediment surveys for the last 50 years. The formal and informal reports, relevant files, contract reports, and reprints of technical papers were reviewed to compile and verify the data used in this report. The SWS professionals who contributed largely to the sediment surveys are John B. Stall, William C. Bogner, Nani G. Bhowmik, and William P. Fitzpatrick. Others include A. A. Klingebeil, S. W. Melsted, E. L. Sauer, L. C. Gottschalk, J. R. Adams, M. Demissie, M. T. Lee, and P. B. Makowski. However, this is not an exhaustive list.

Robin B. King compiled the information on percent sediment constituents from the Water Survey files and reports. His contribution to the project is greatly appreciated.

Some Recent Reports on Volumetric and Particle Size Analyses for Illinois Lakes

- Fltzpatrick, W. P., W. C. Bogner, and N. G. Bhowmlk, 1987. "Sedimentation and Hydrologic Processes in Lake Decatur and Its Watershed" Illinois State Water Survey Report of Investigation 107.
- Fltzpatrick, W. P. 1987. "Sedimentation Survey of Dawson Lake, Moraine Yiew State Park, Mc Lean County, Illinois." Illinois State Water Survey Contract Report 413, prepared for the Illinois Department of Conservation.
- Fitzpatrick, W. P., W. C. Bogner, and N. G. Bhowmik, 1985. "Sedimentation Investigation of Lake Springfield, Springfield, Illinois." Illinois Slate Water Survey Contract Report 363 prepared for the City of Springfield.
- Bogner, W. C. W. P. Fitzpatrick, and D. S. Blakley, 1985. "Sedimentation Rates In Horseshoe Lake, Alexander County, Illinois." Illinois State Water Survey Contract Report 364 prepared for the Illinois Department of Conservation and the Horseshoe Lake Task Force.
- Bogner, W. C, W. P. Fitzpetrick, and N. G. Bhowmik, 1984. "Sedimentation Survey of Lake Decatur, Decatur, Illinois." Illinois State Water Survey Contract Report 342 prepared for the City of Decatur.