

**THE CHARACTERIZATION OF UNSATURATED SOIL
BEHAVIOUR FROM PENETROMETER PERFORMANCE
AND THE CRITICAL STATE CONCEPT**

Volume 2 of 2

BY

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**A thesis submitted in fulfilment of the requirements
for the degree of Doctor of Philosophy
in
Agricultural Engineering**

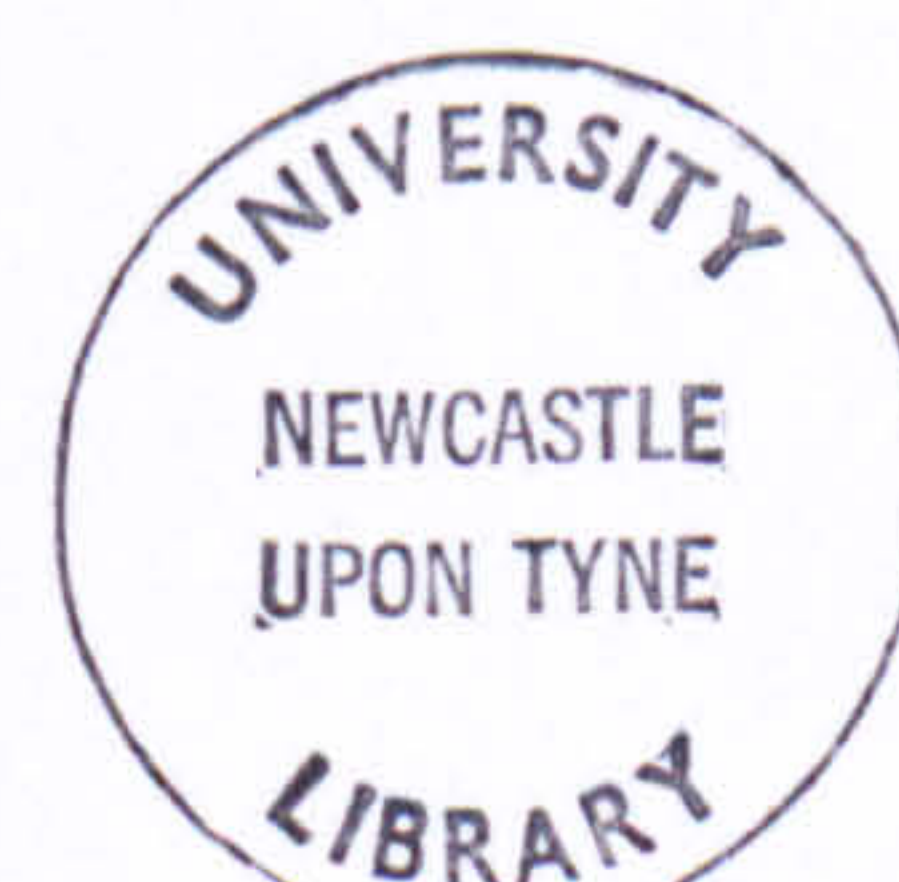
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**Department of Agricultural and Environmental Science
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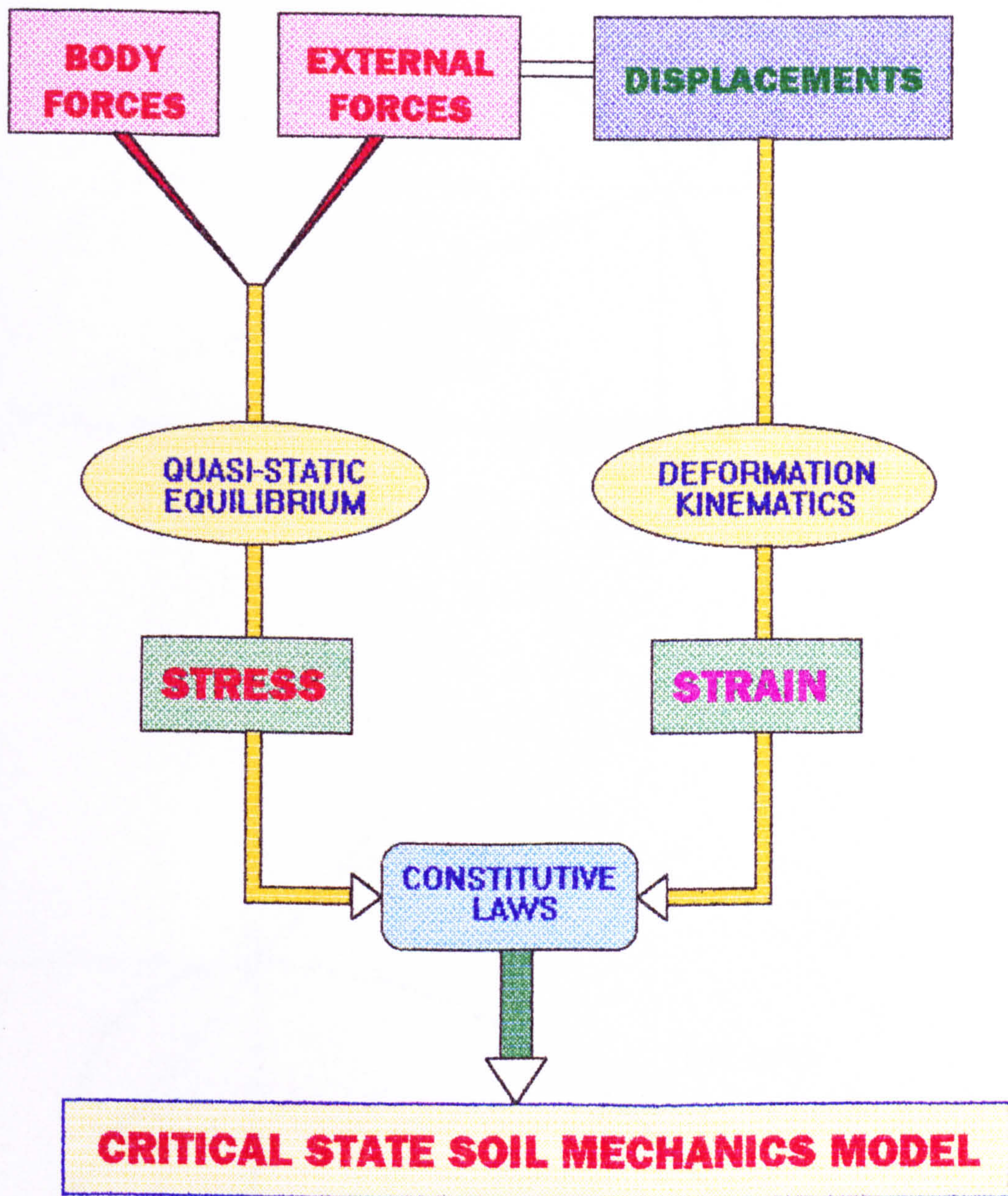


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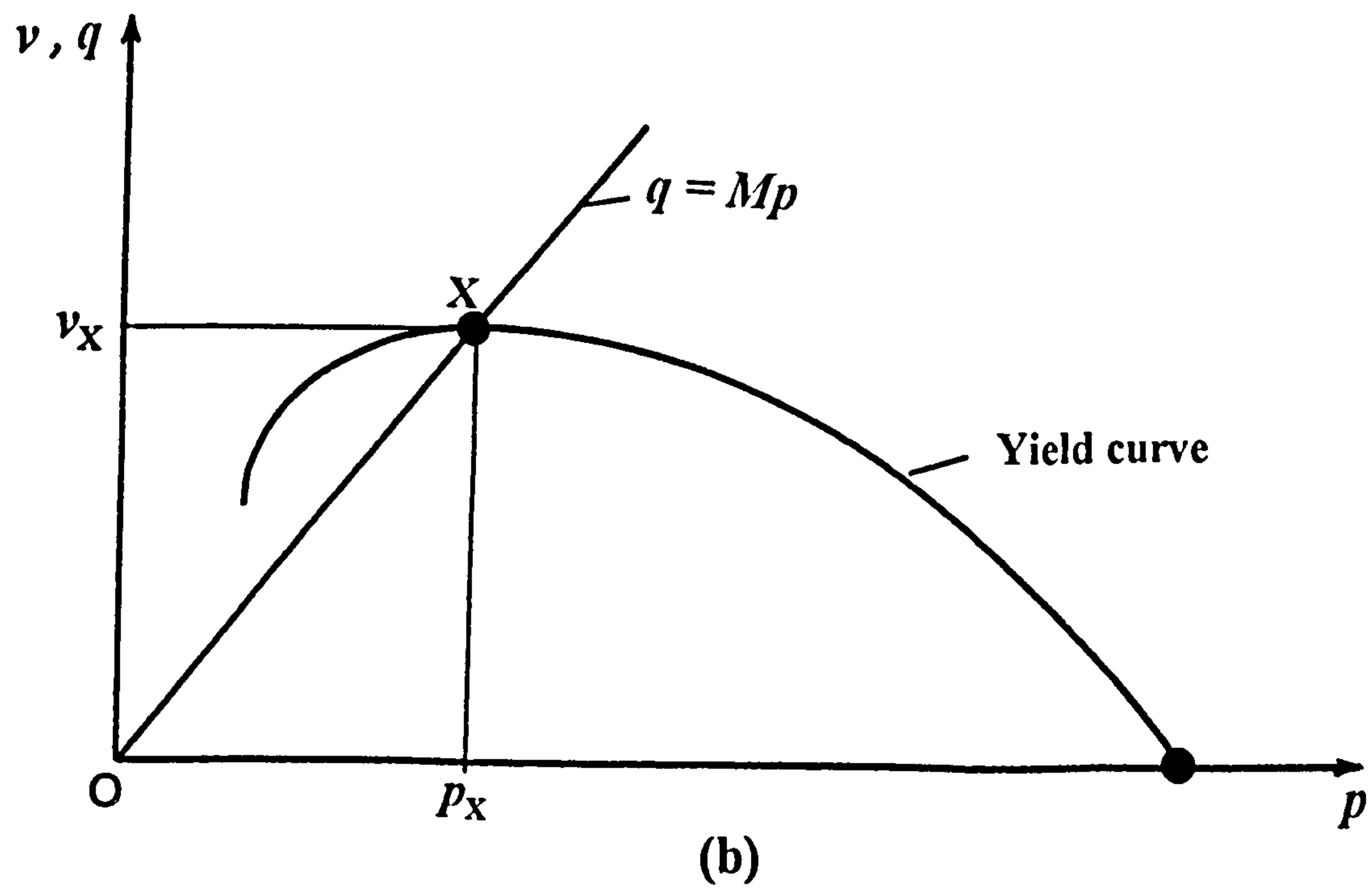
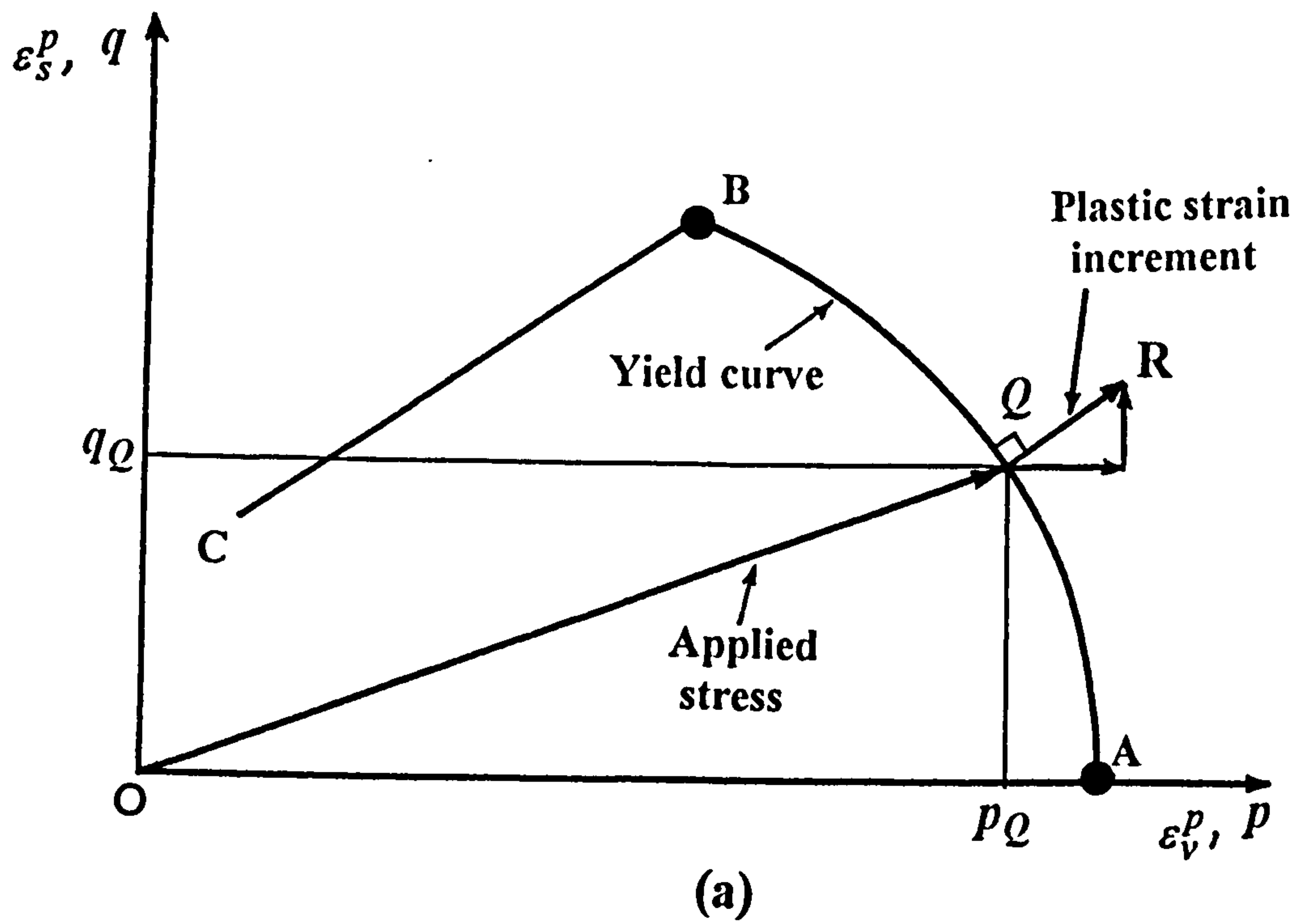


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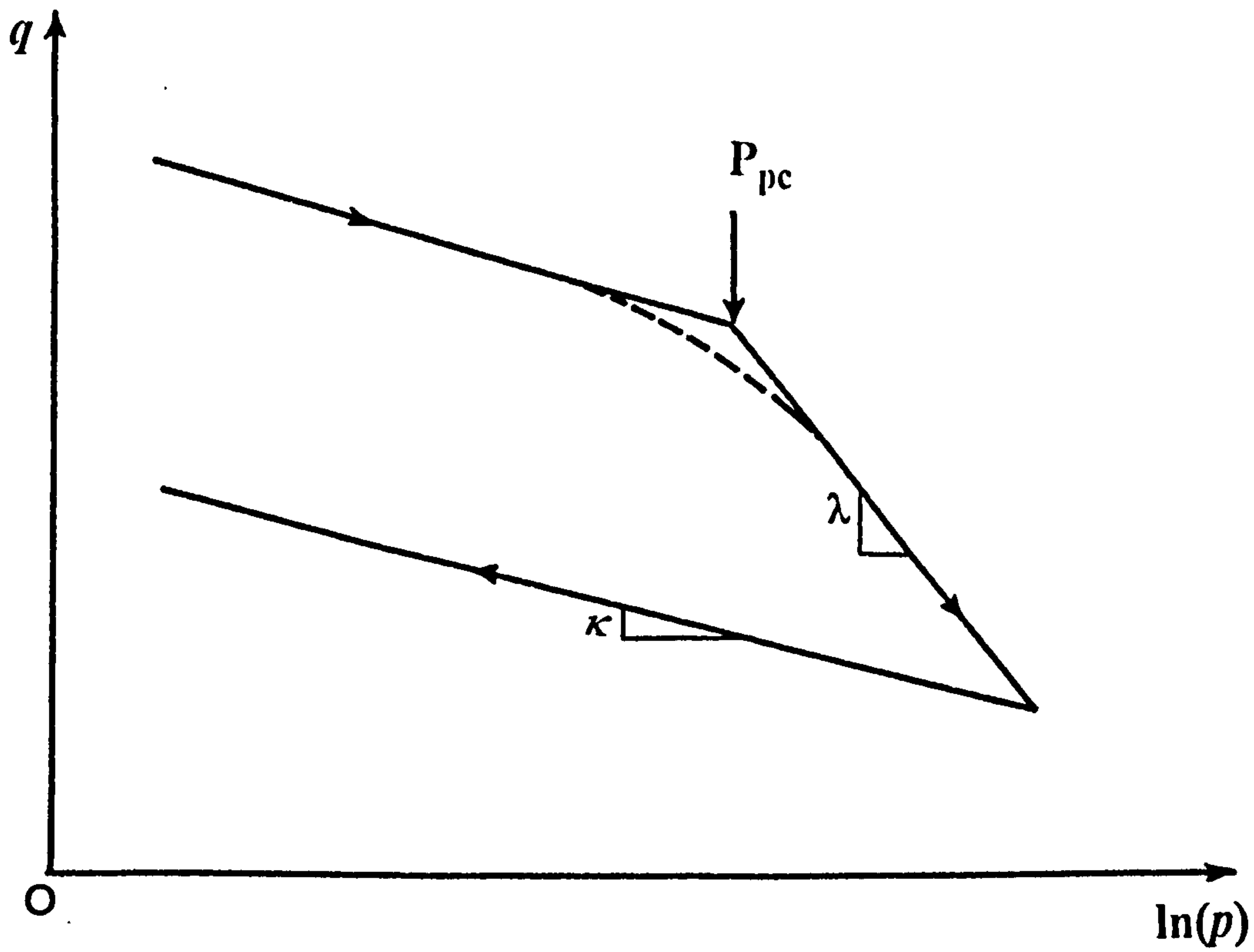


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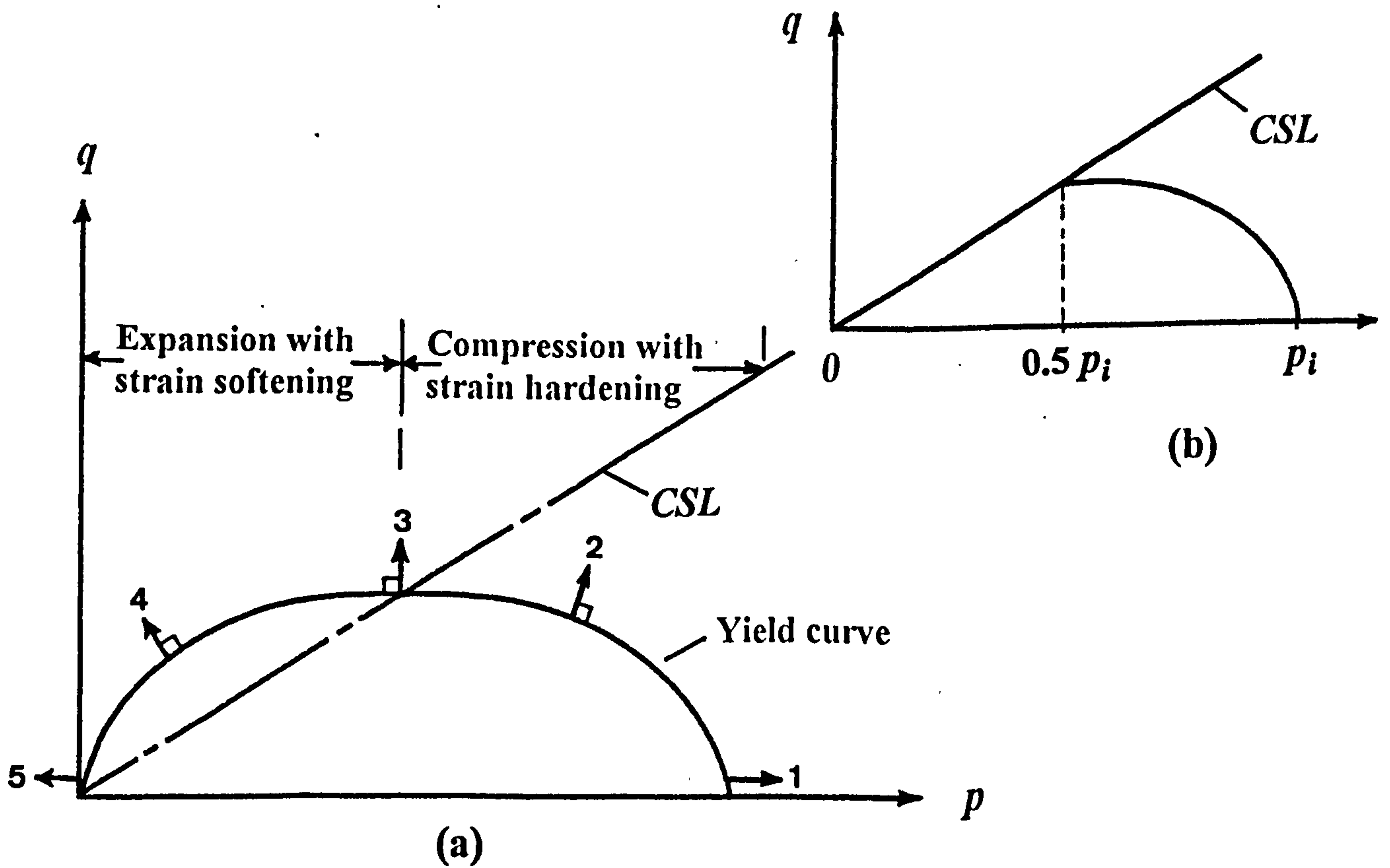


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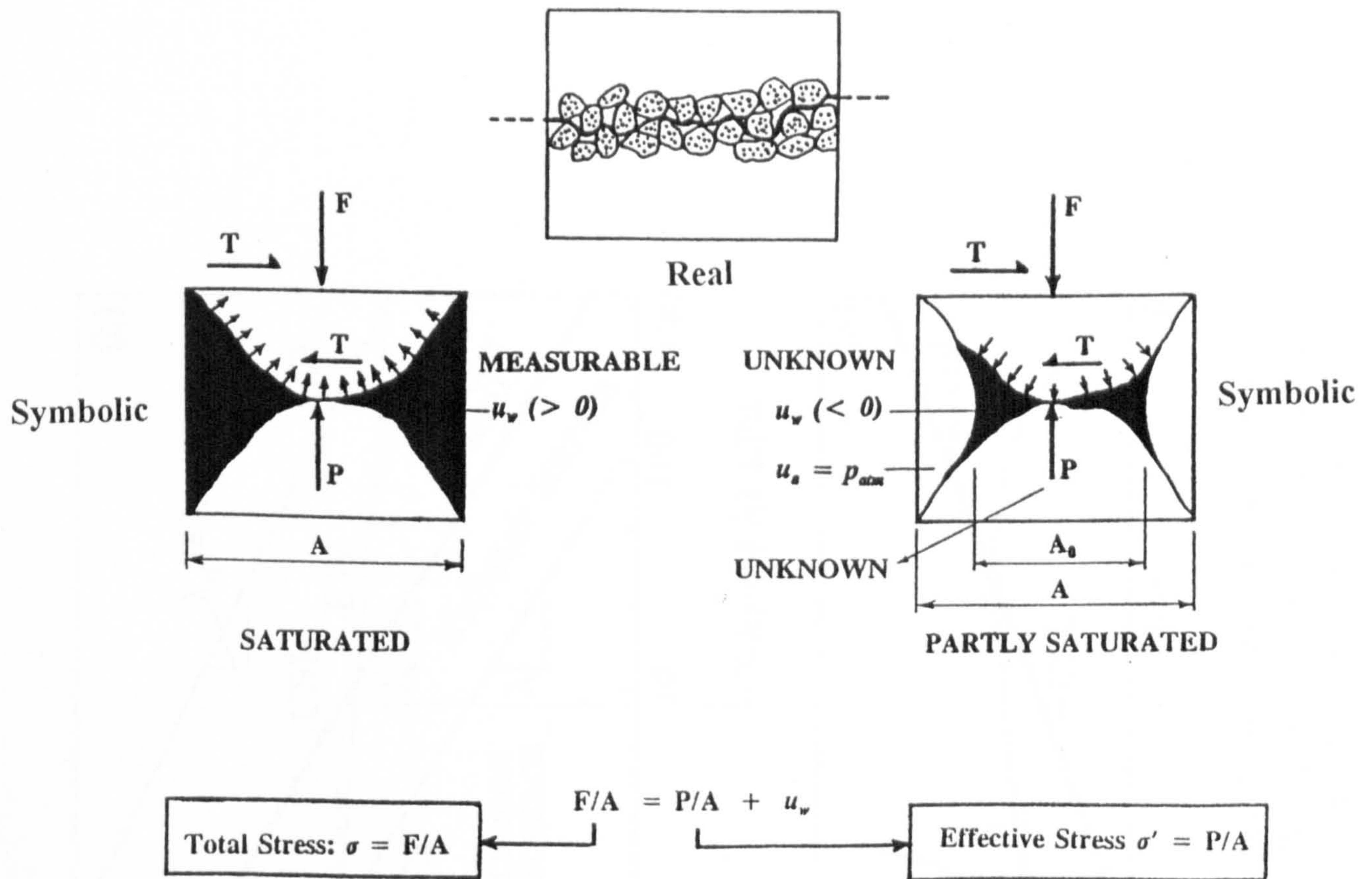


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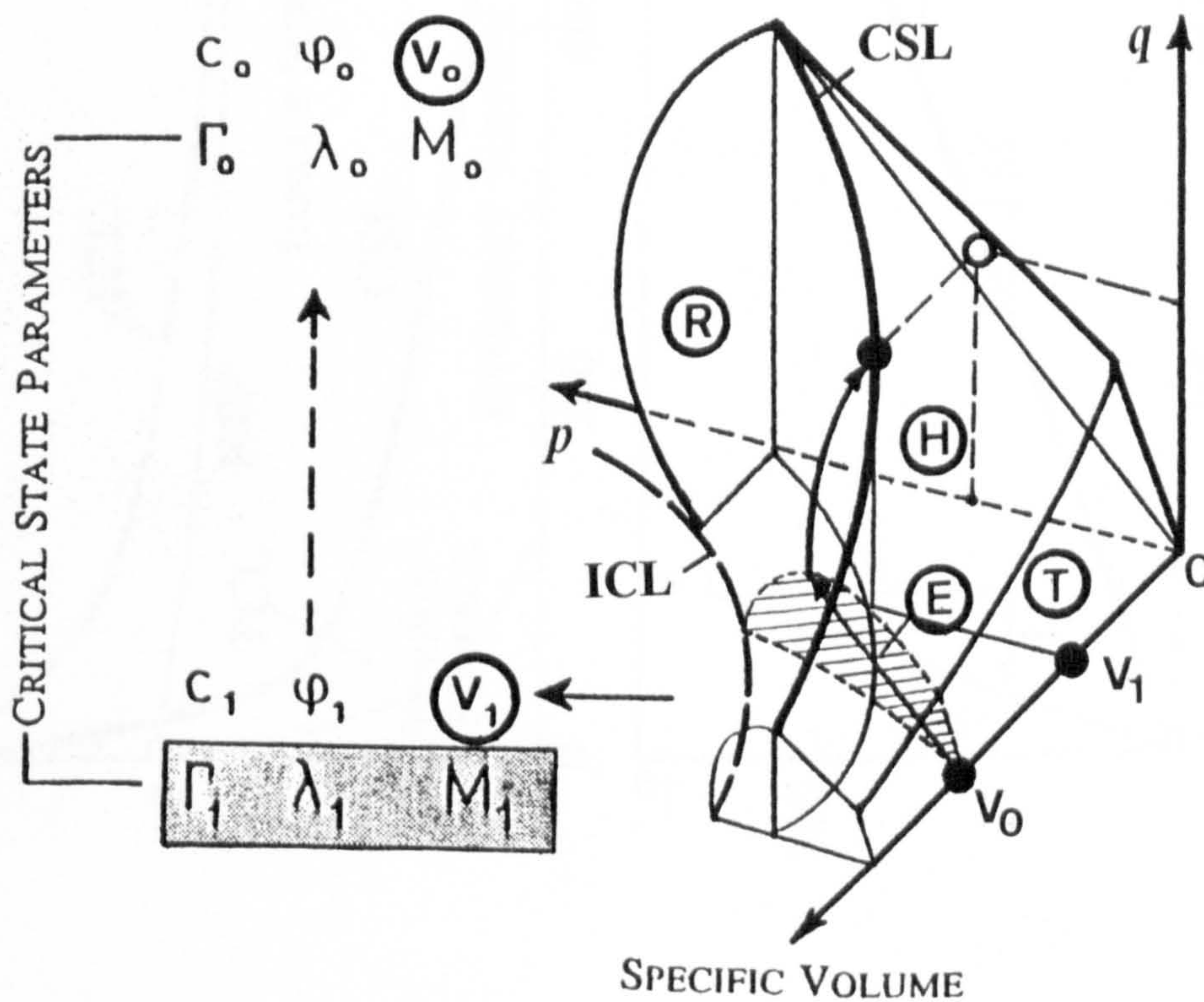


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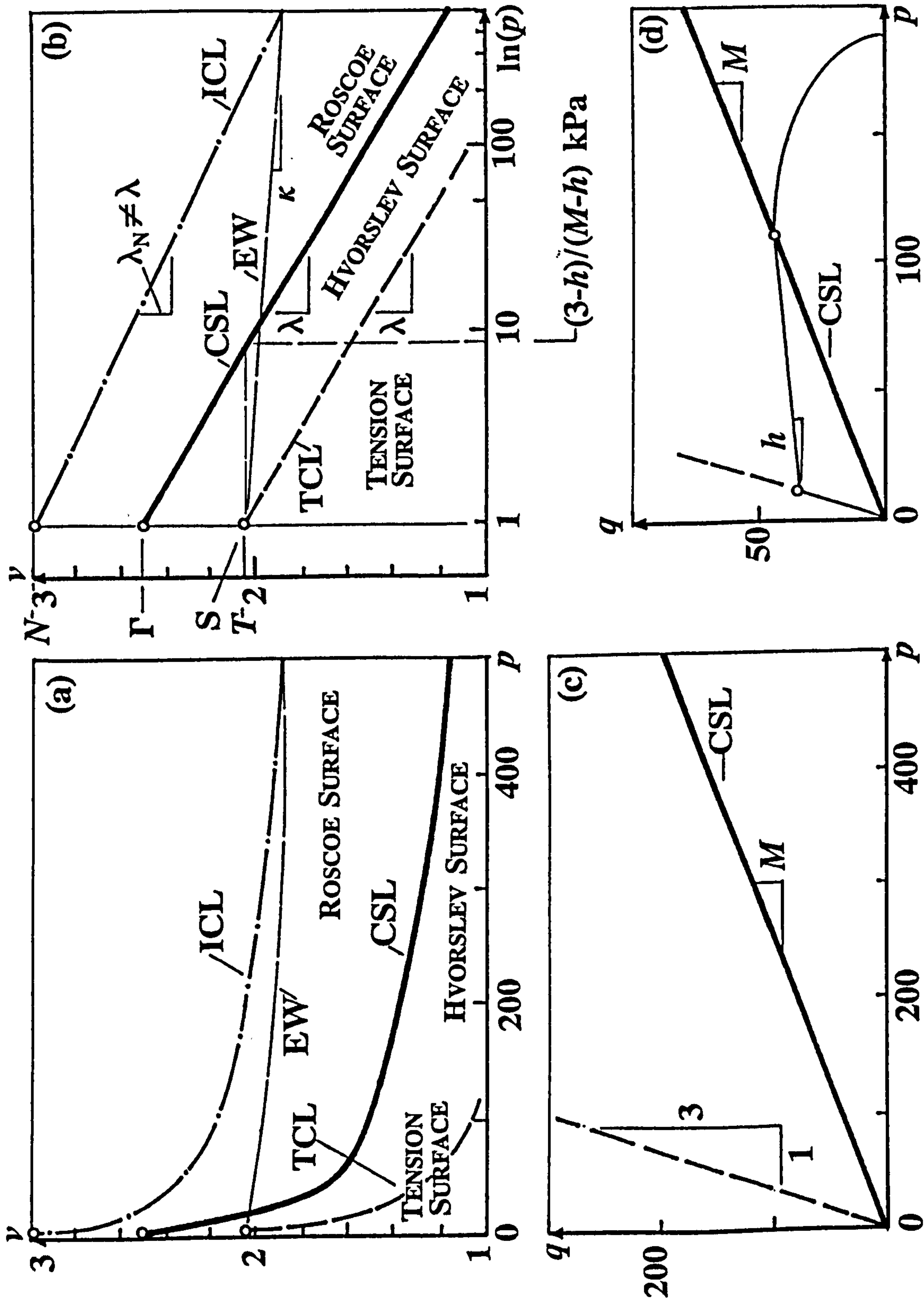


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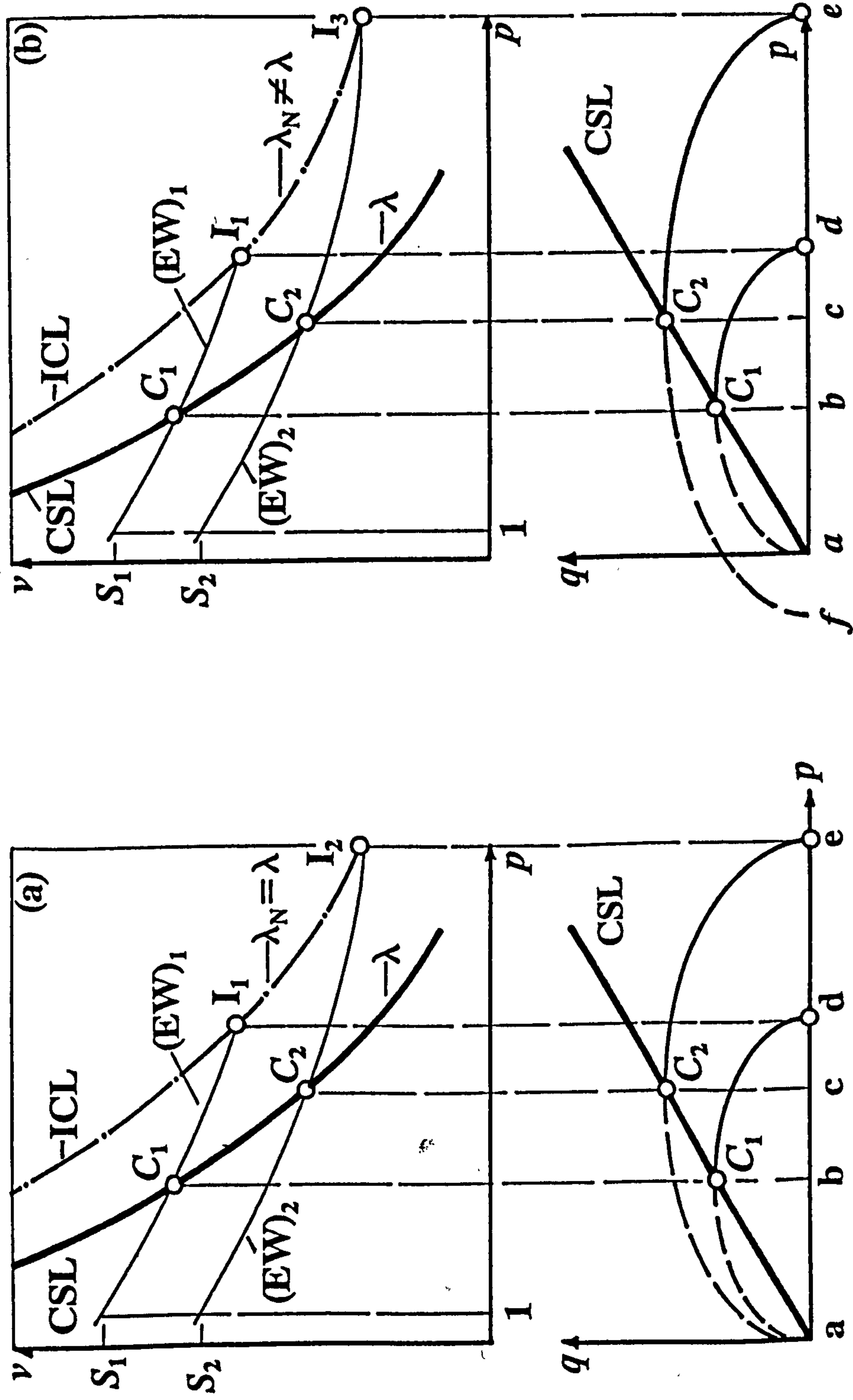


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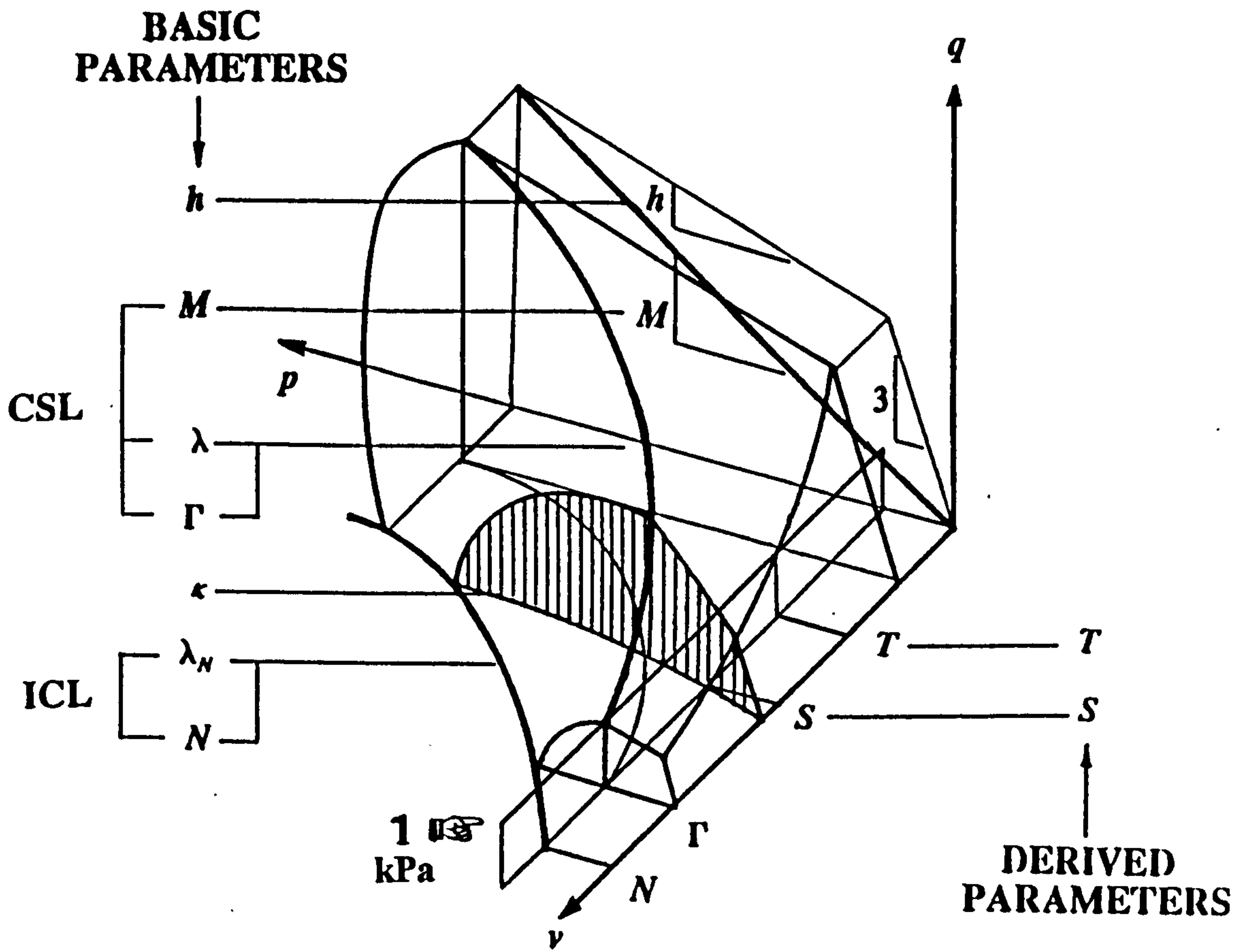


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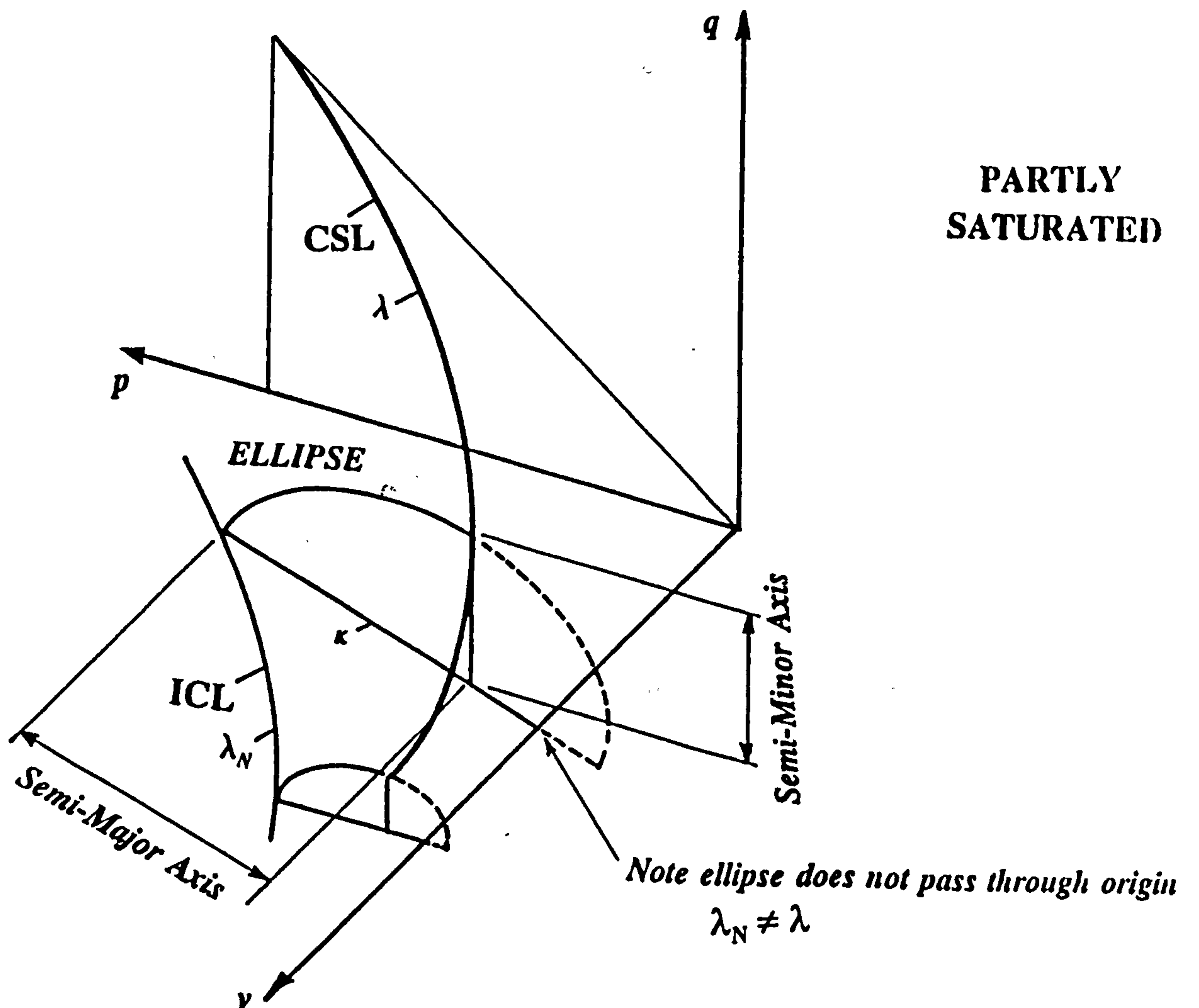


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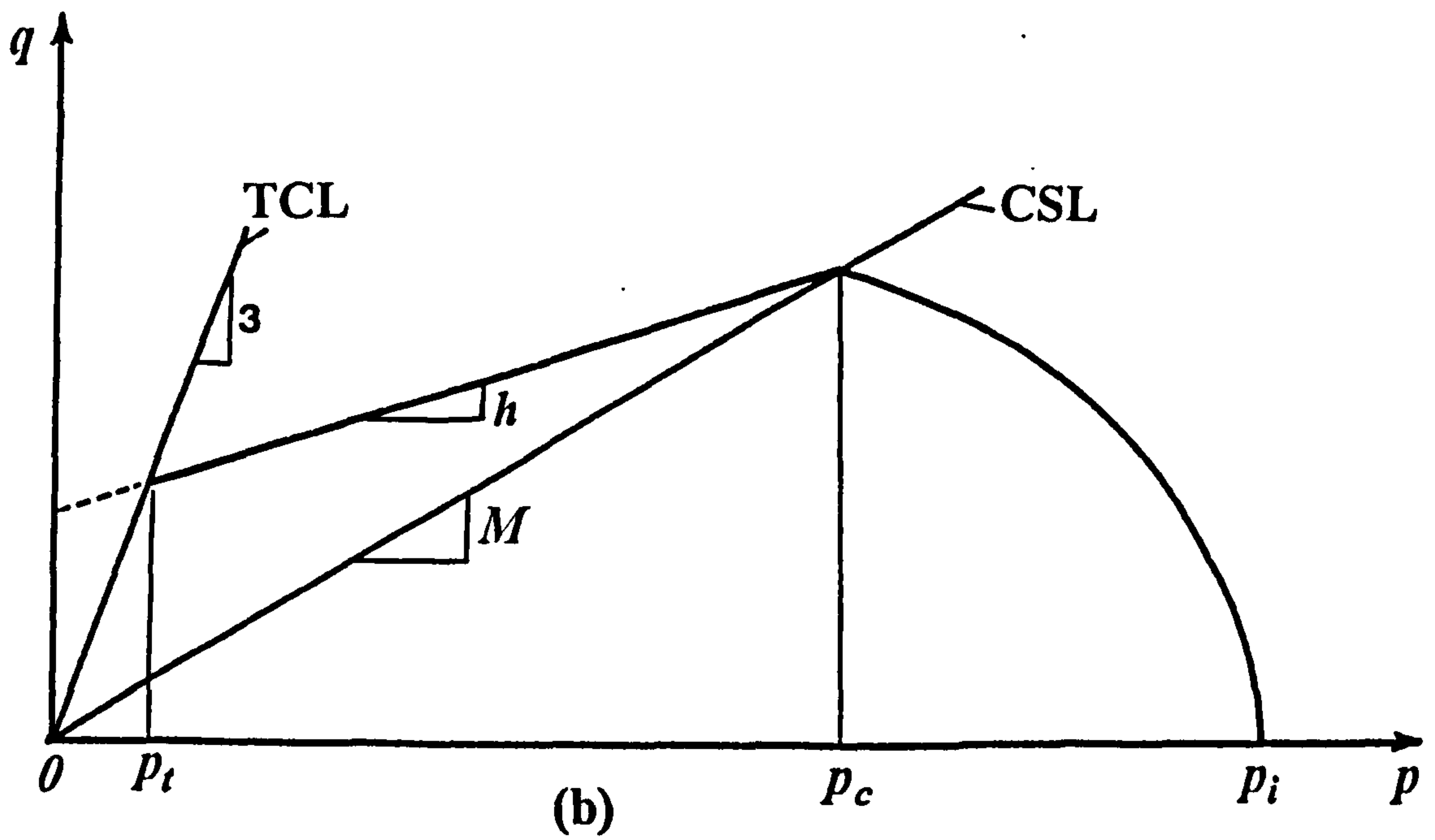
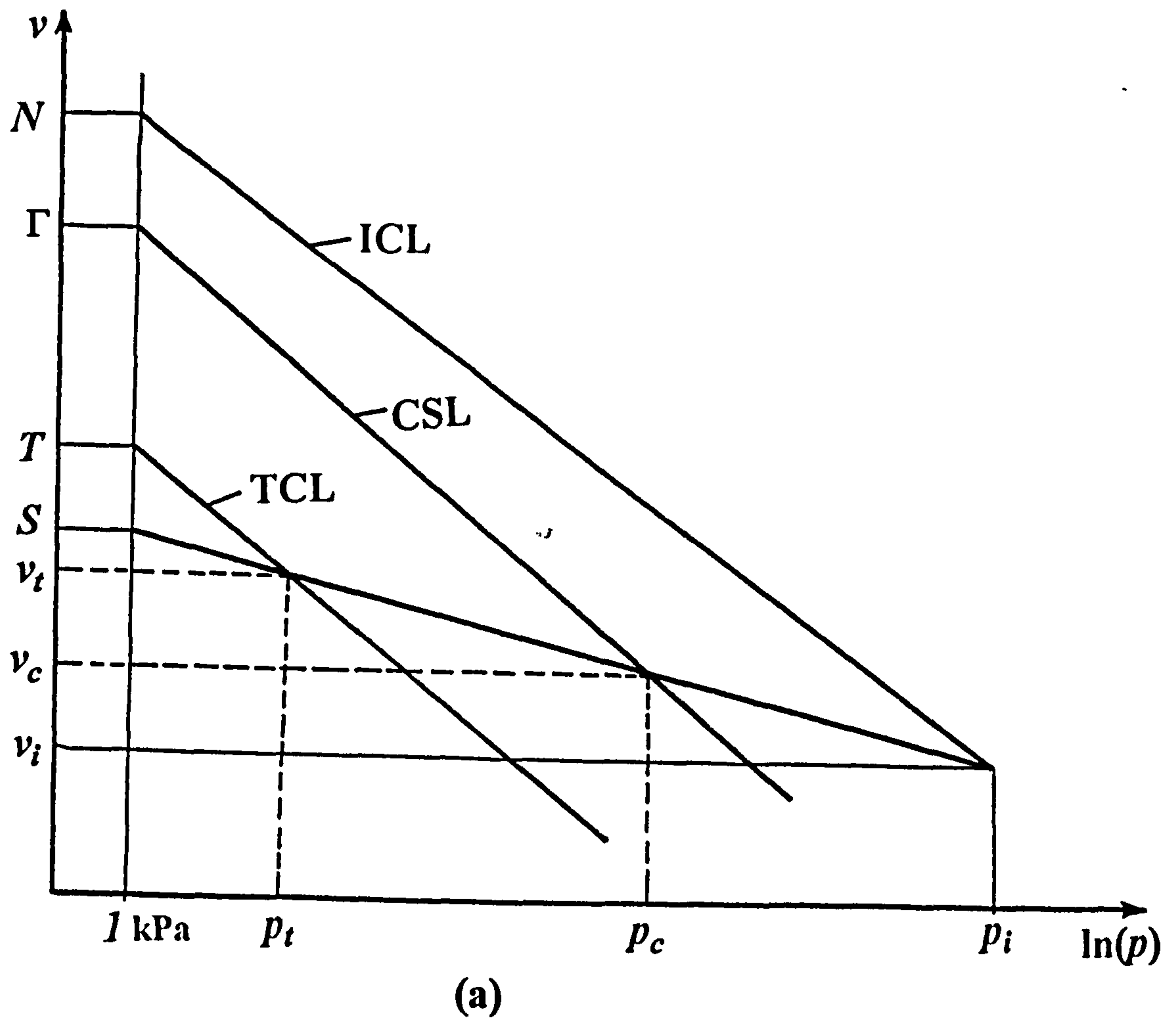


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Evesham clay
 m.c. = 24.5 %

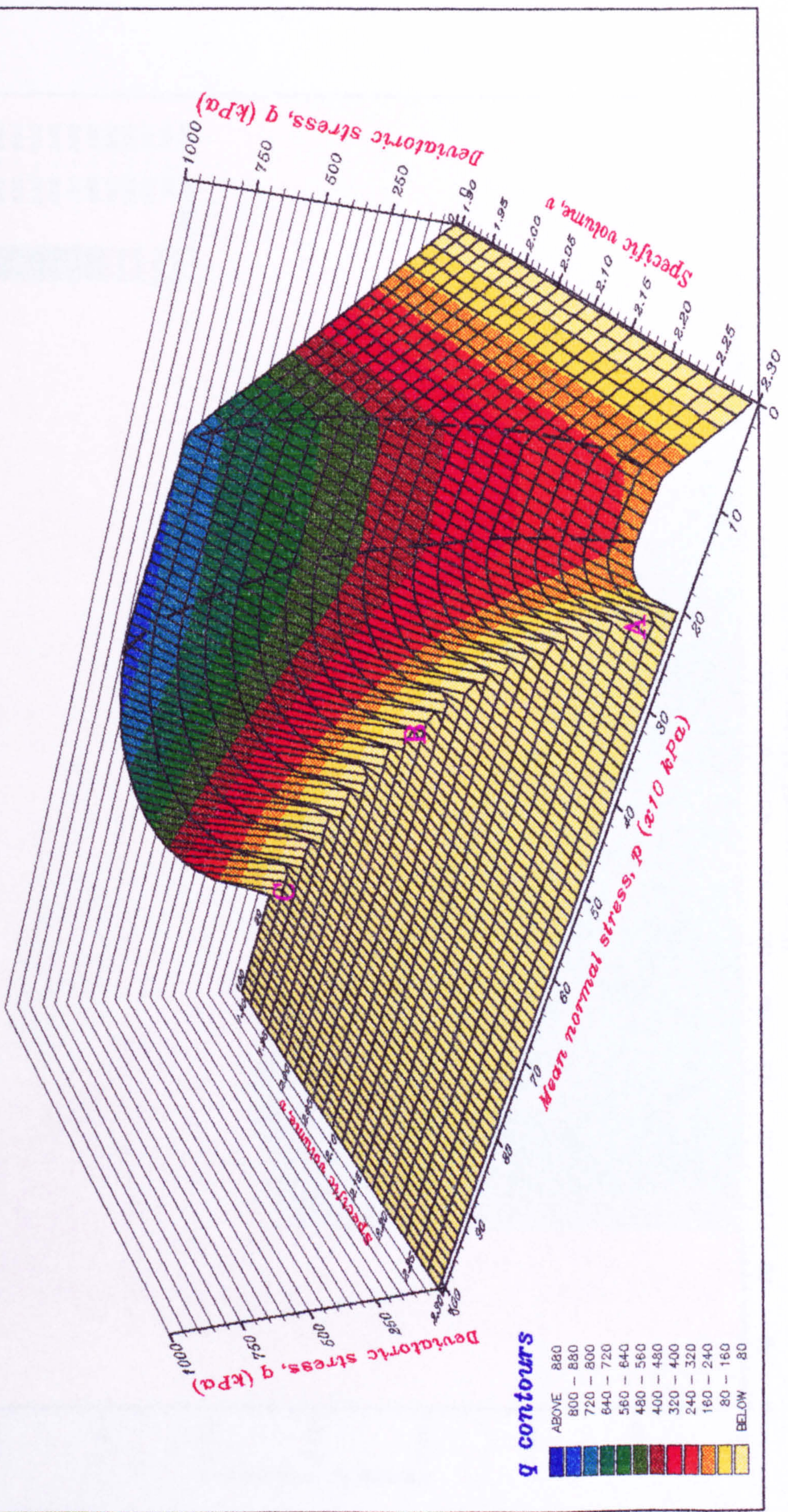


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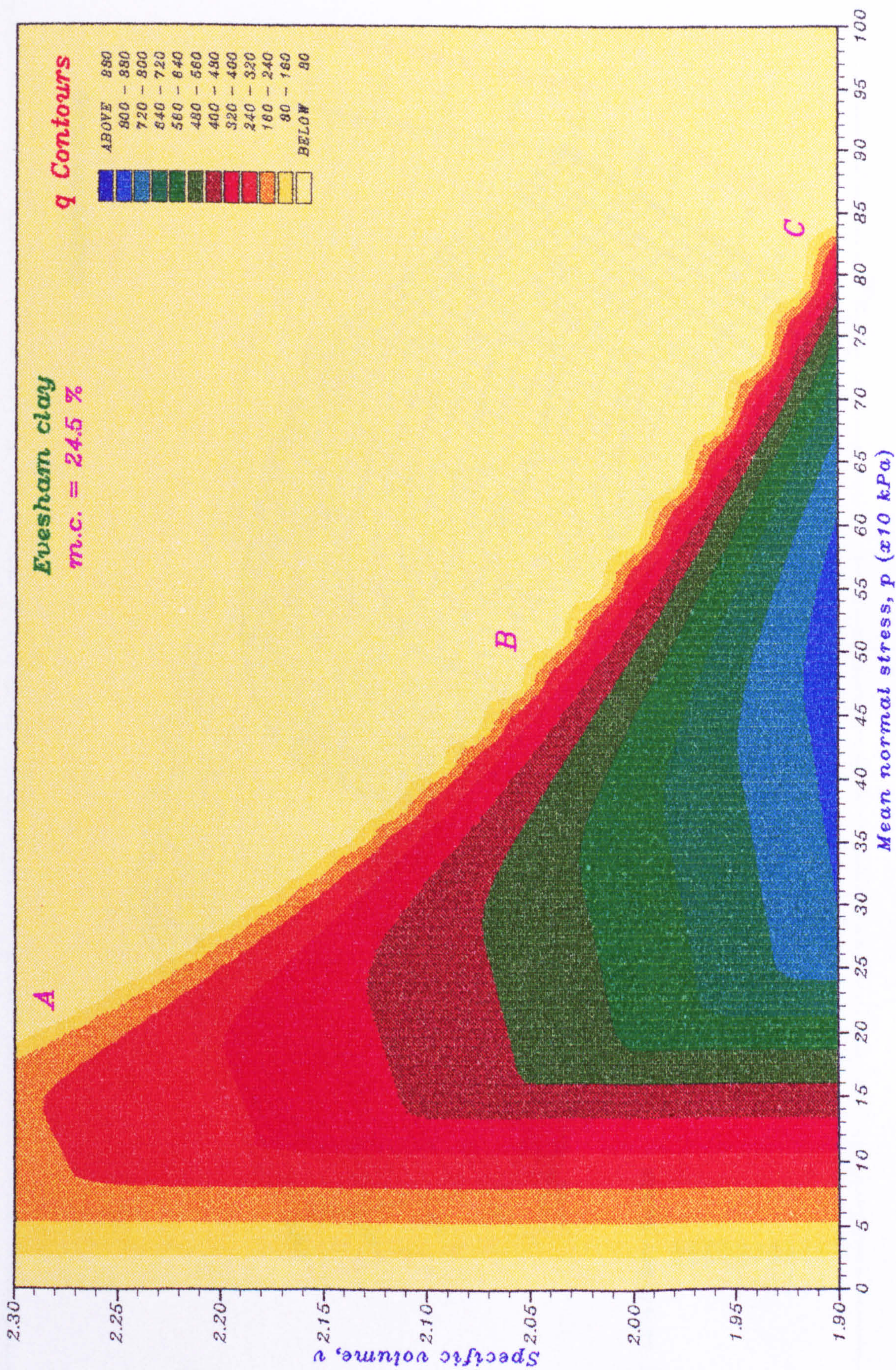


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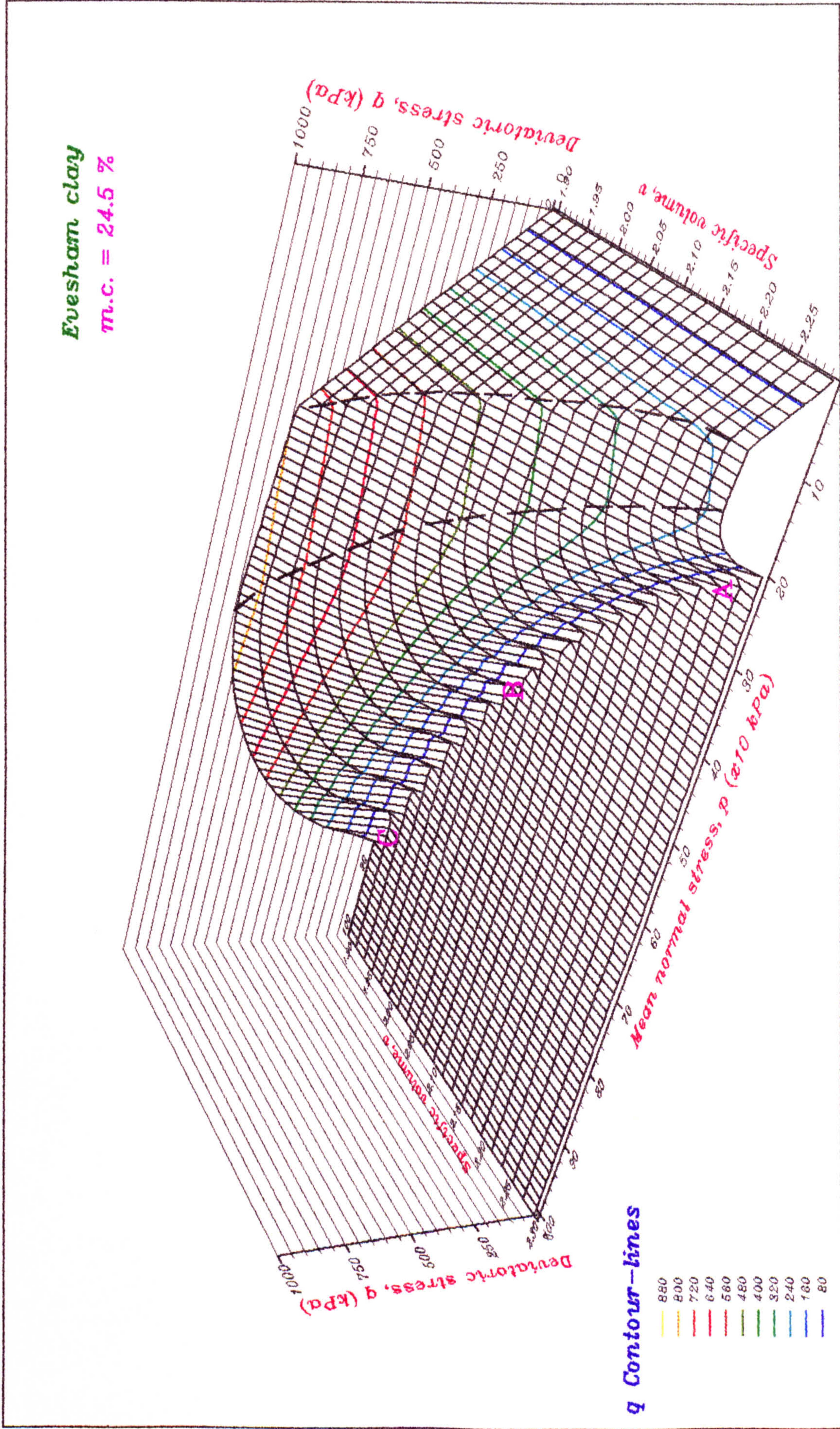


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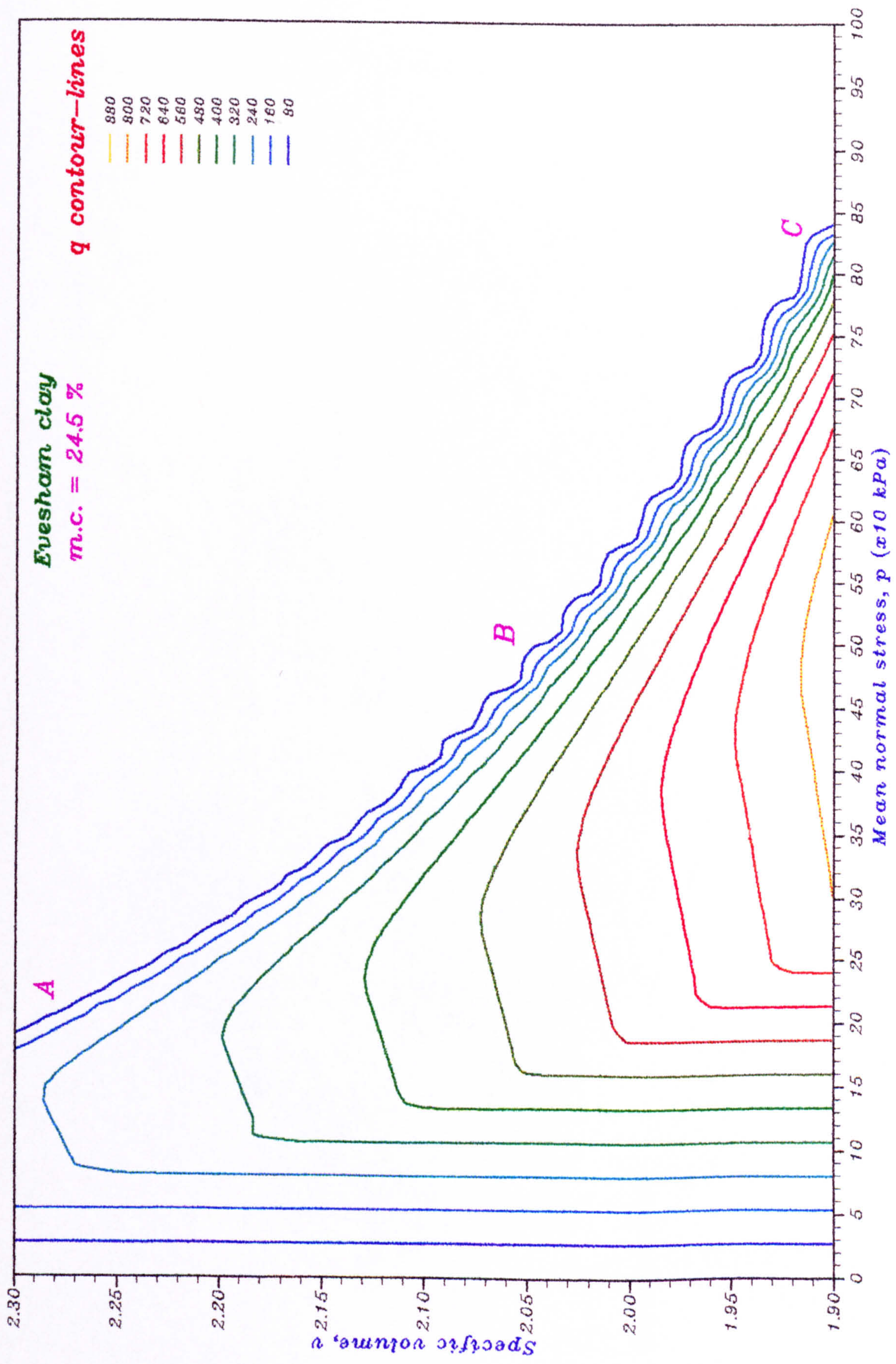


Fig. 3.12 (d) Typical output (2D-lines) of the model in v - p plane for Evesham clay soil

Winton clay loam

m.c. = 13.0 %

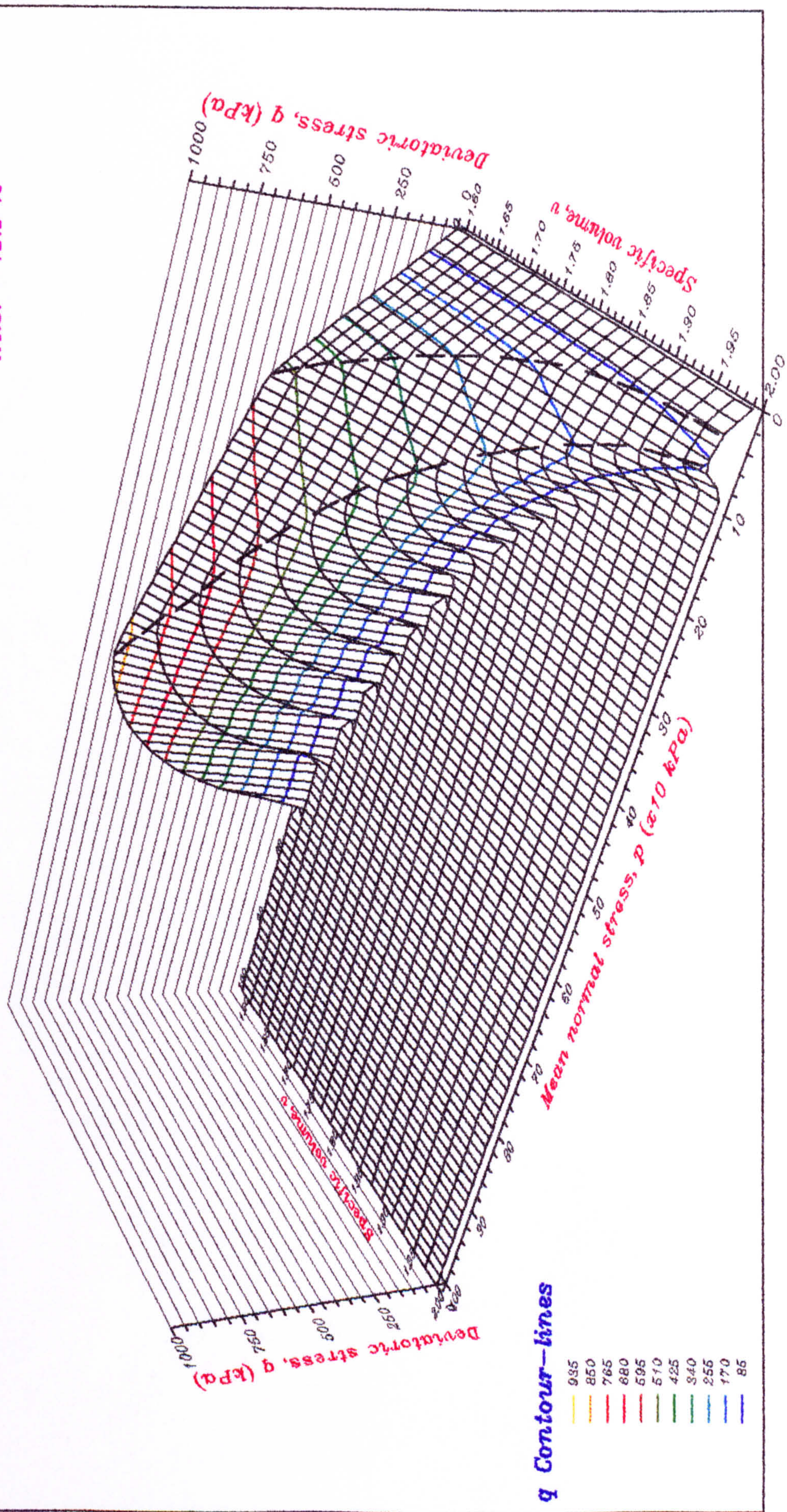


Fig. 3.13 Typical output (3D-lines) of the model in p - q - v space for Winton clay loam soil

Darvel sandy loam

m.c. = 8.5 %

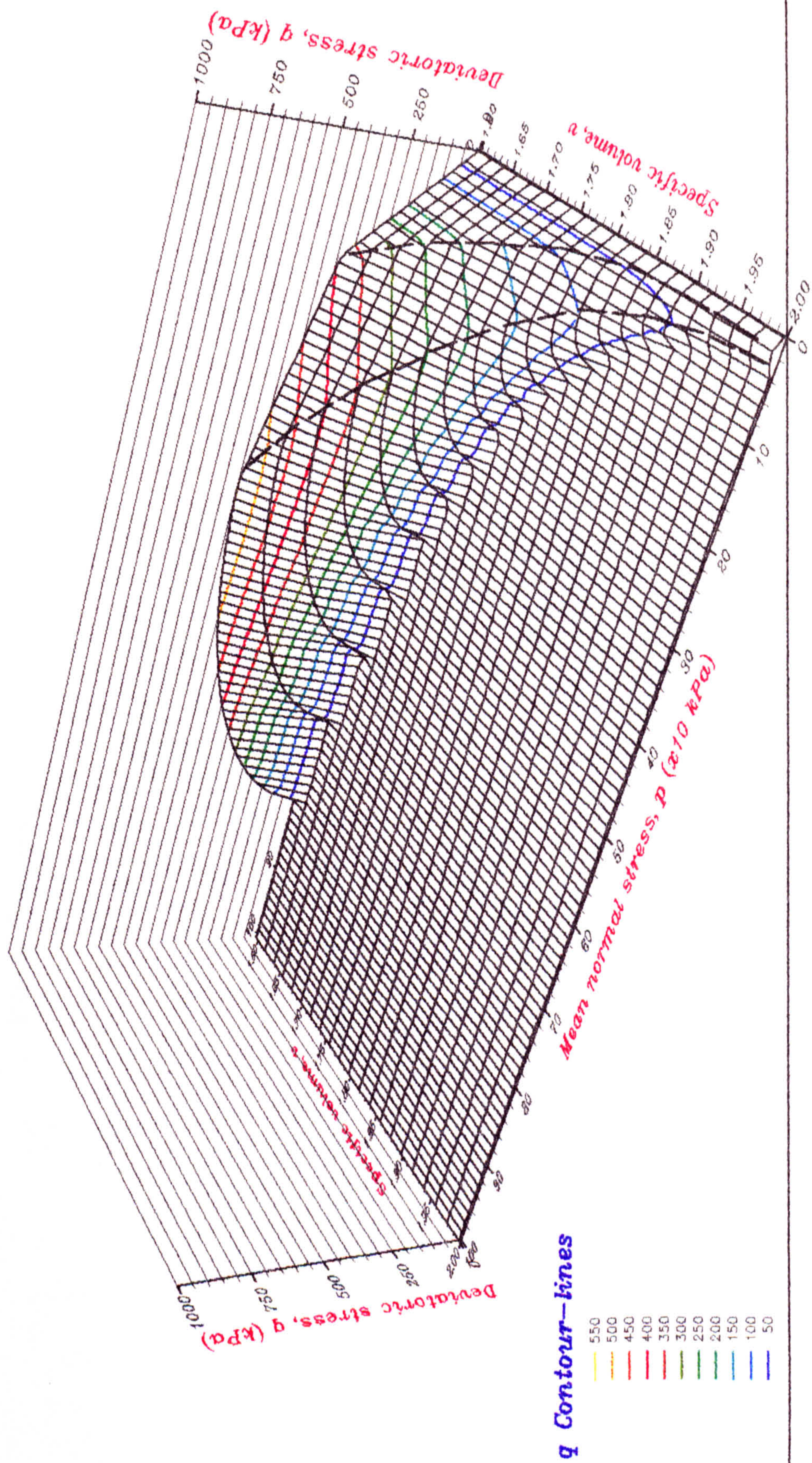


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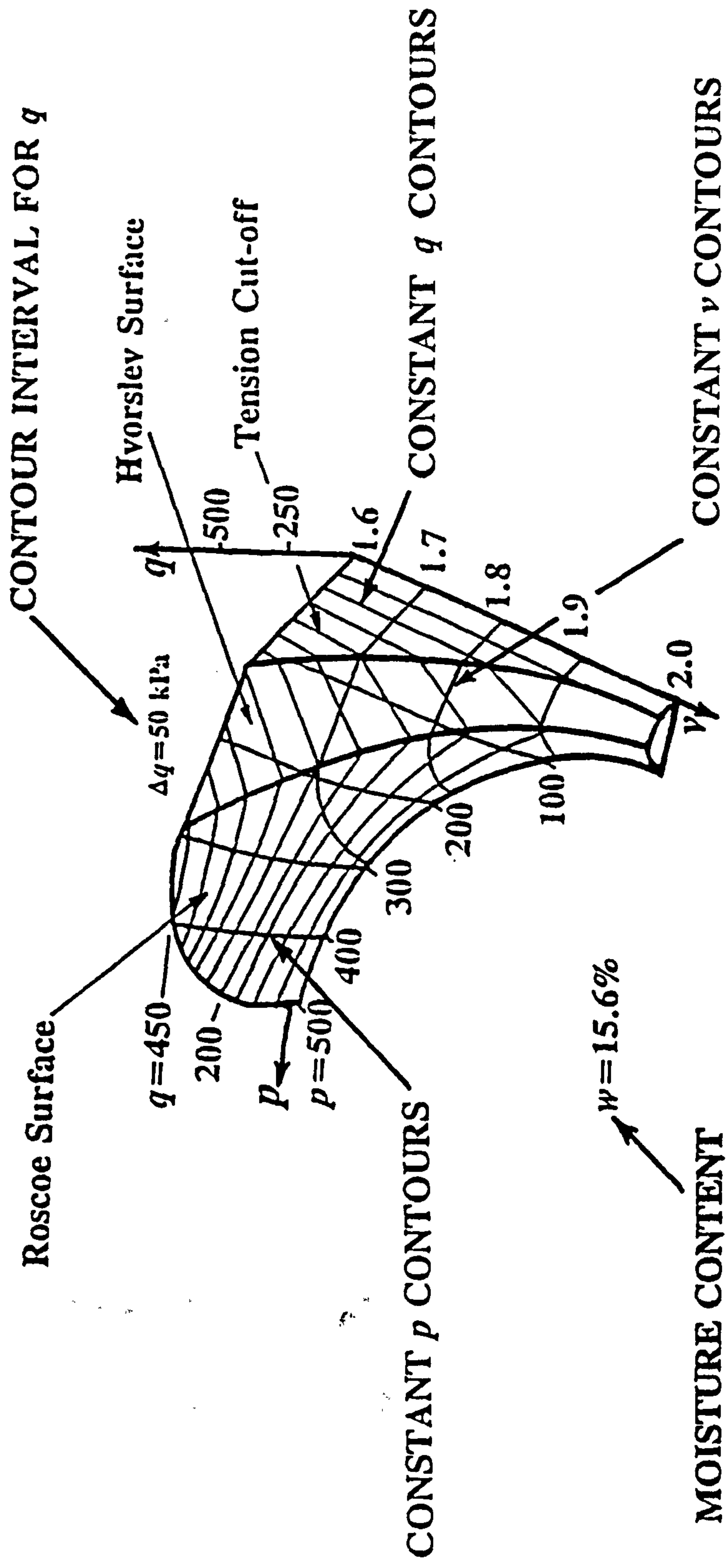


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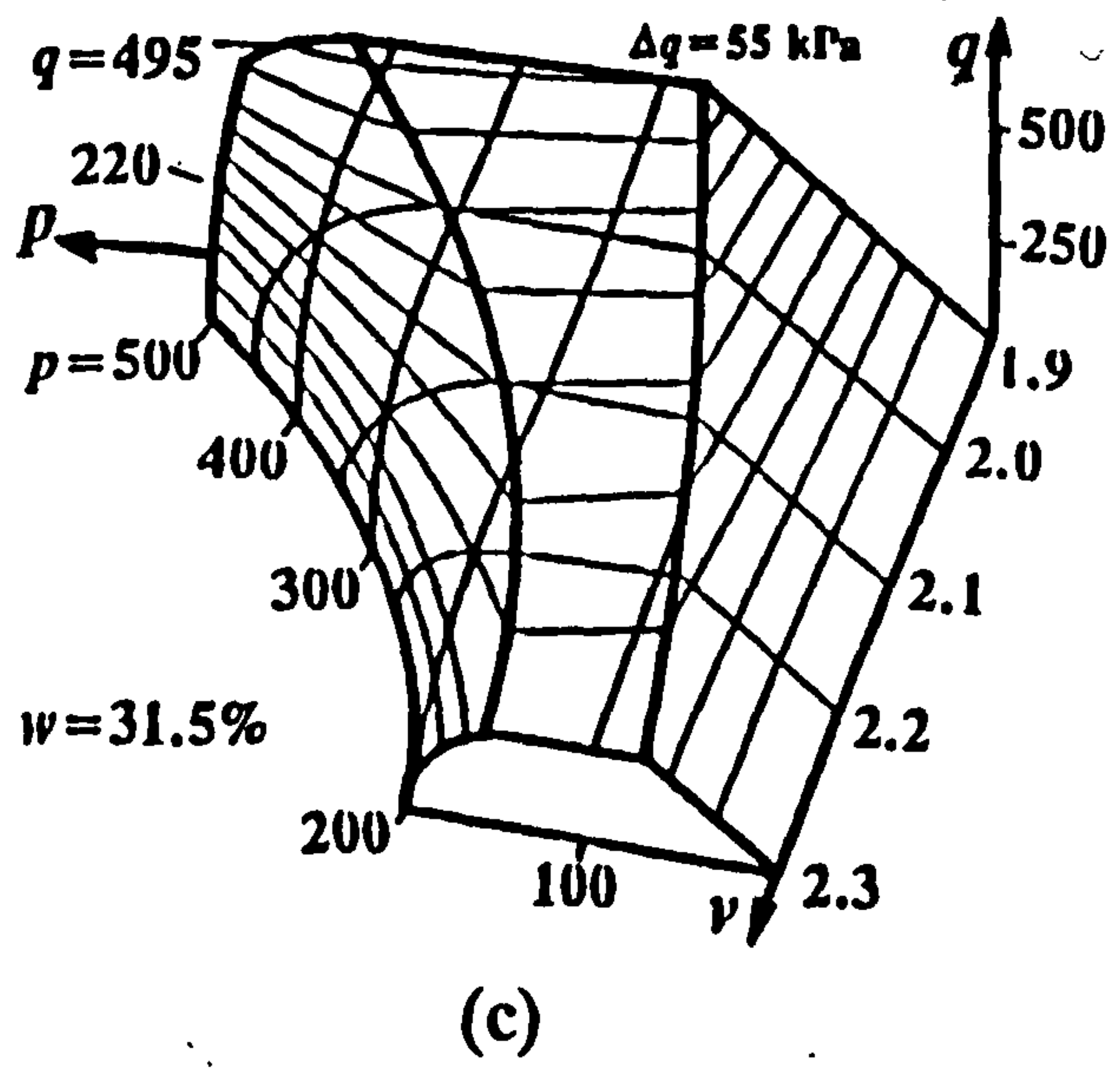
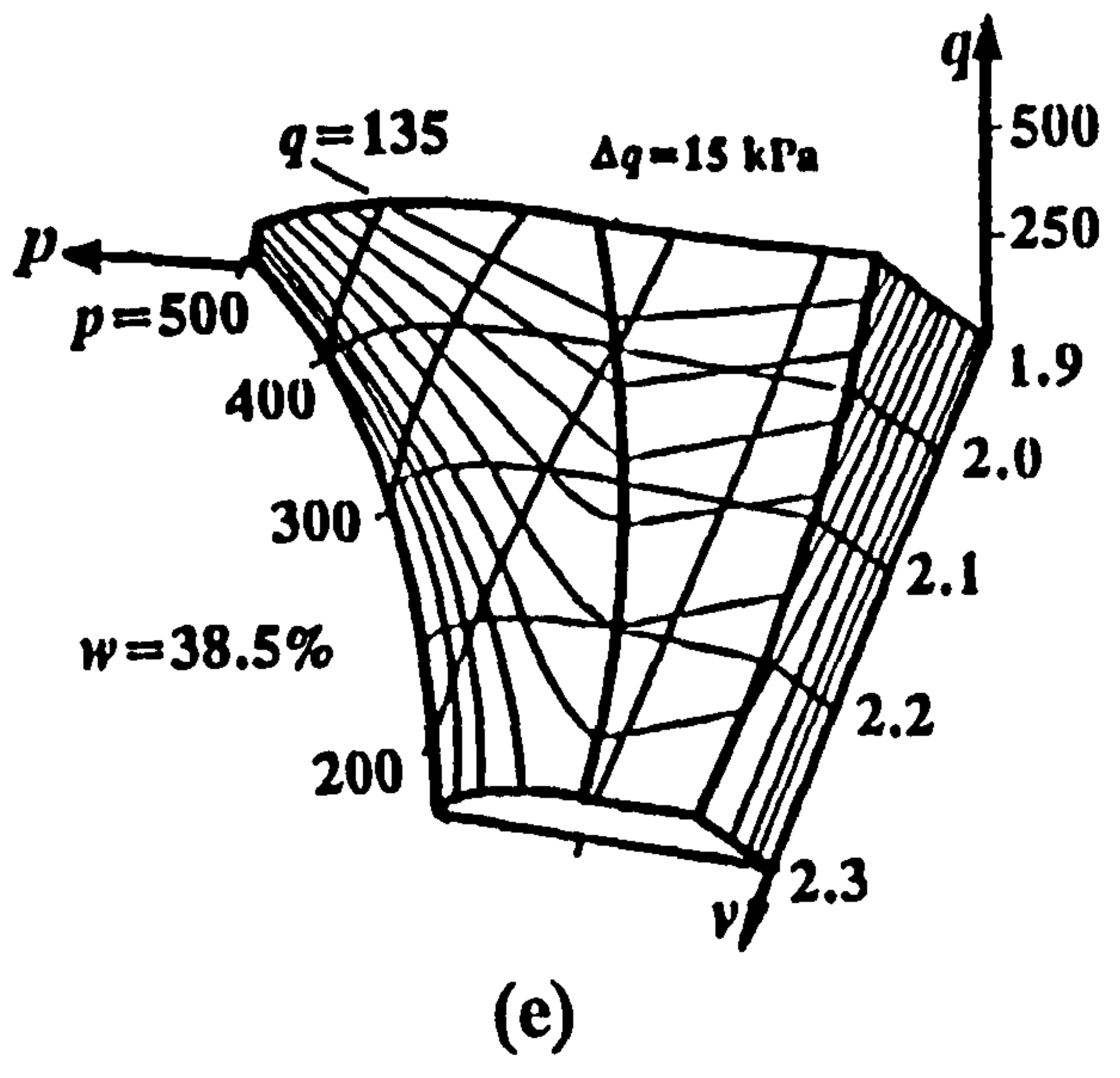
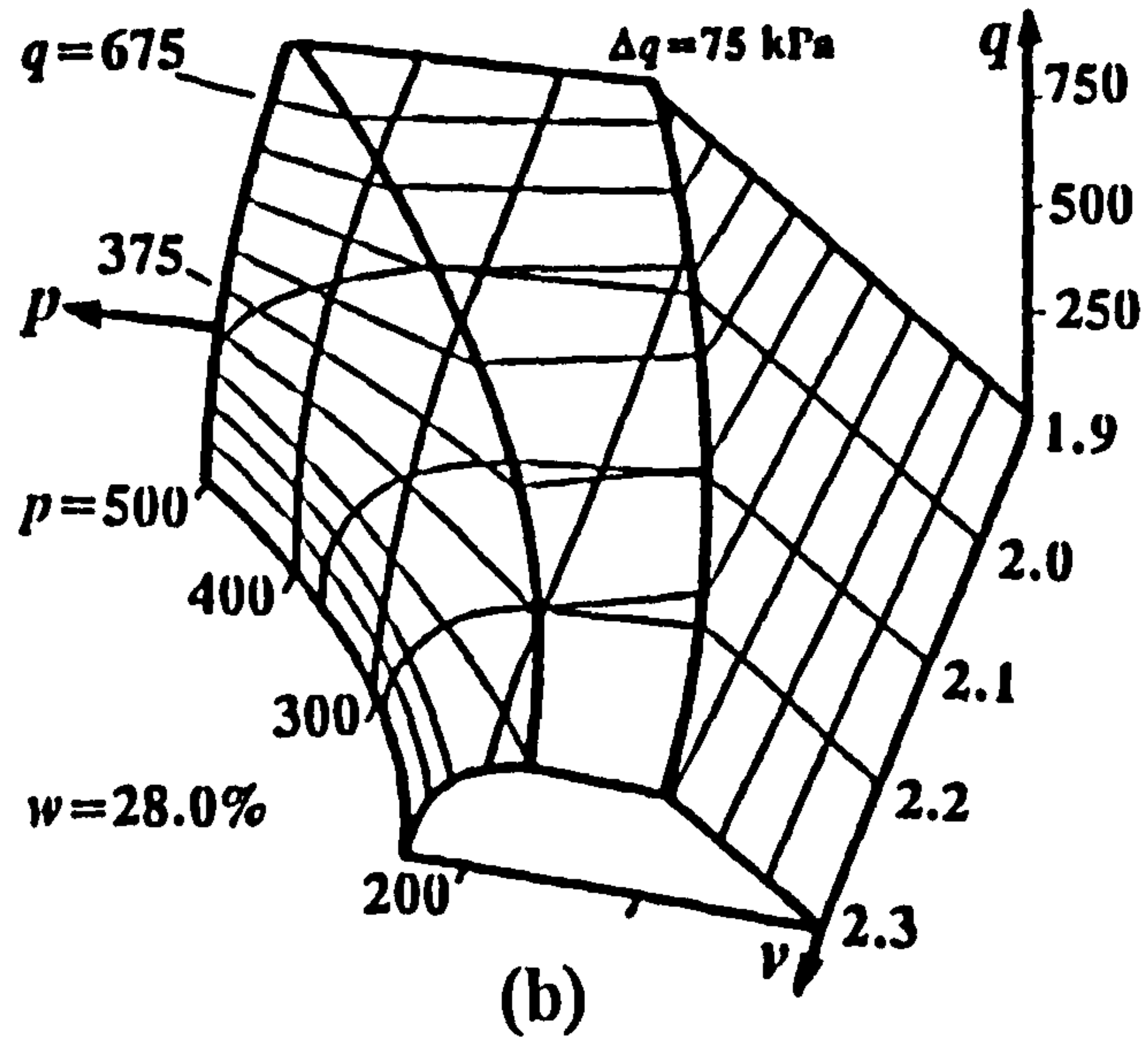
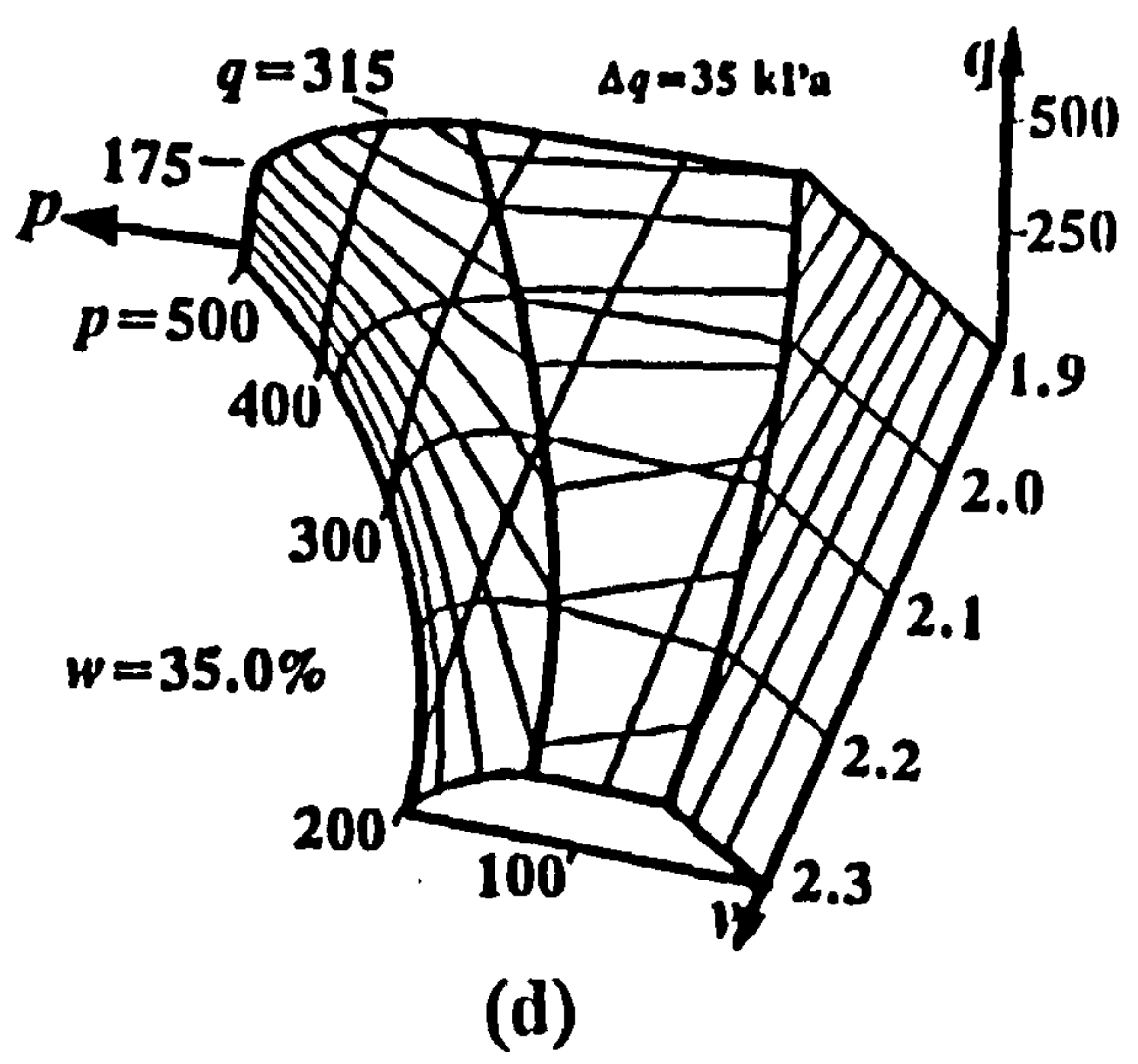
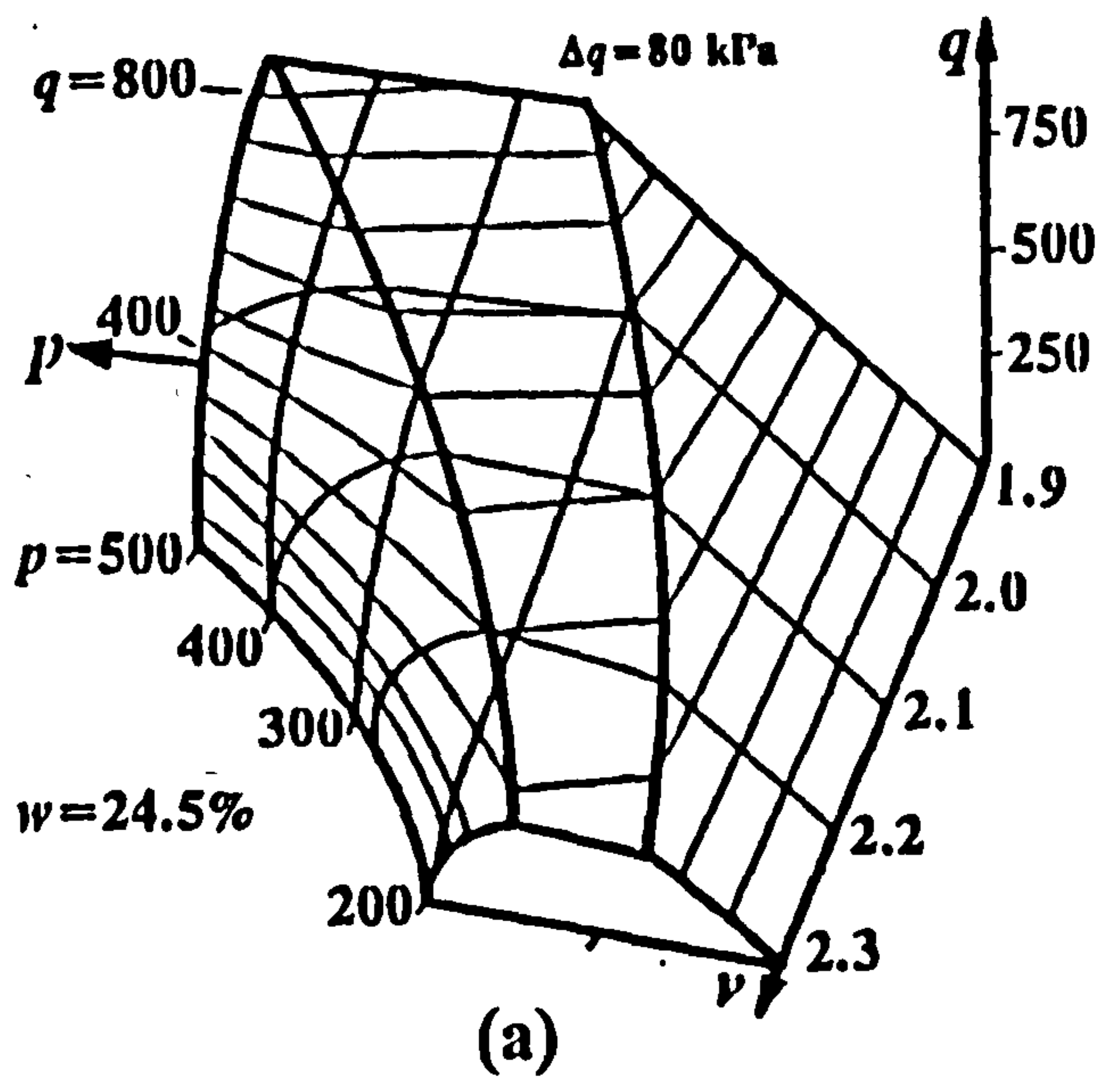


Fig. 3.16 Critical State Space for Evesham clay:
 (a) $w = 24.5\%$
 (b) $w = 28.0\%$
 (c) $w = 31.5\%$
 (d) $w = 35.0\%$
 (e) $w = 38.5\%$

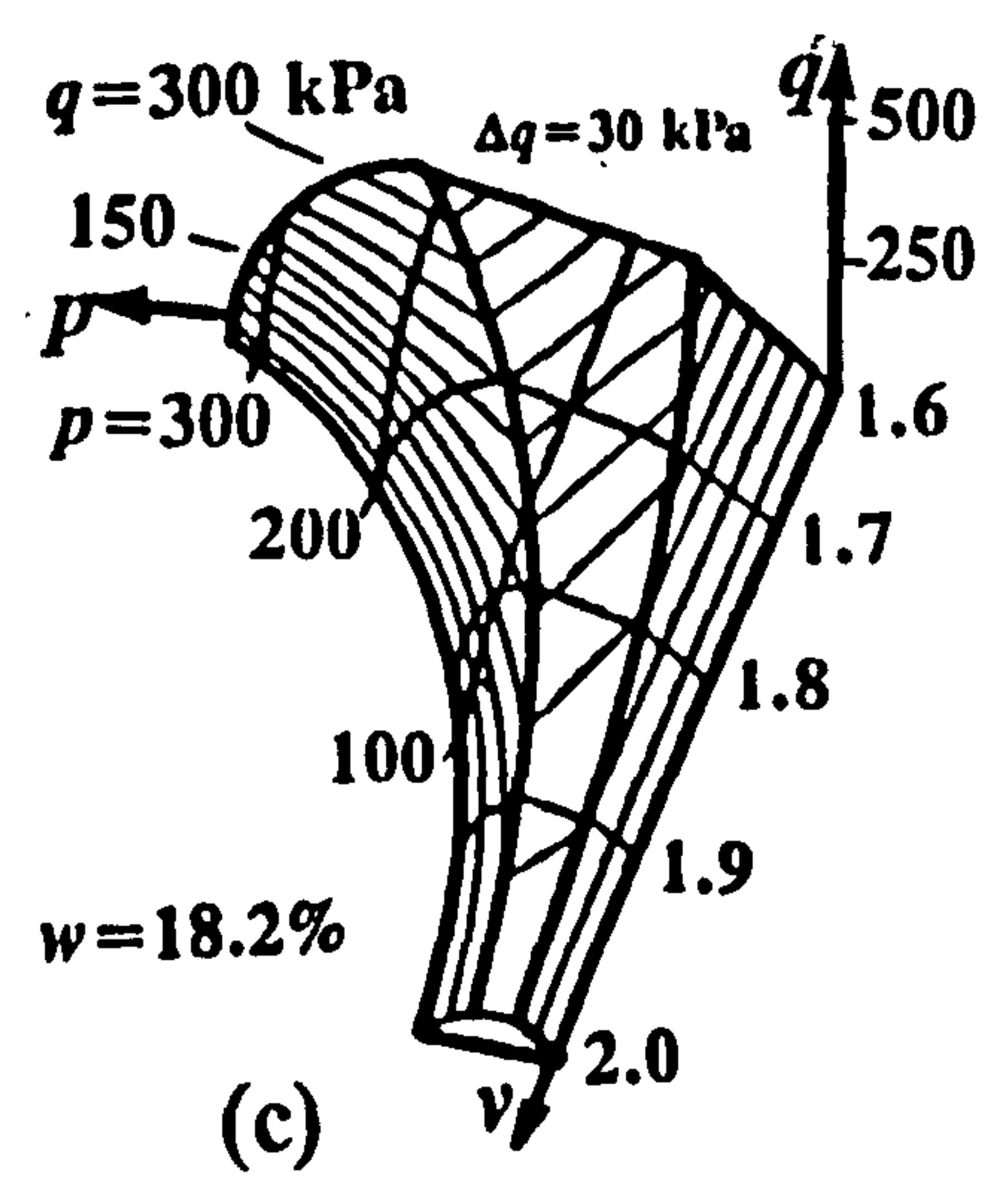
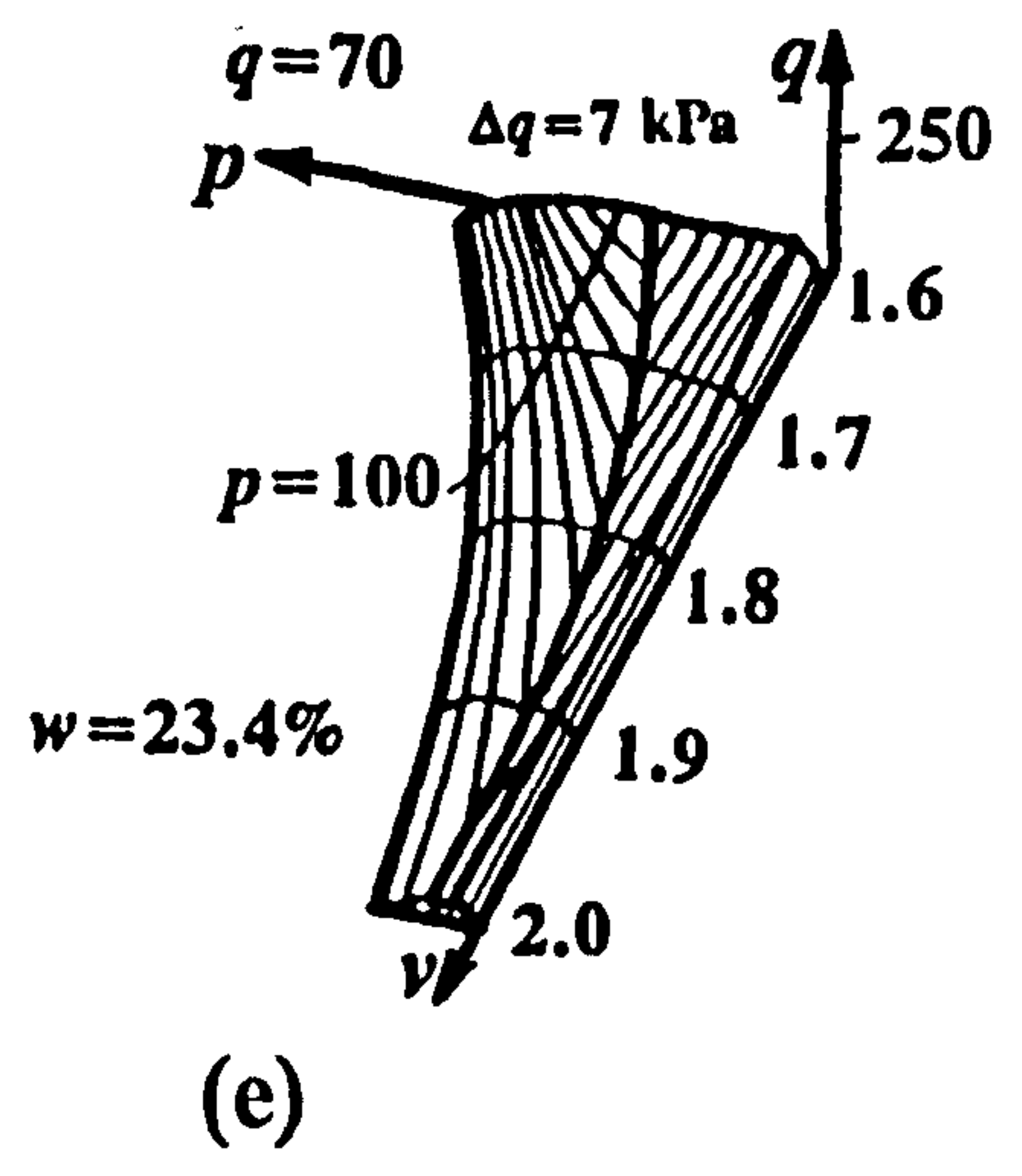
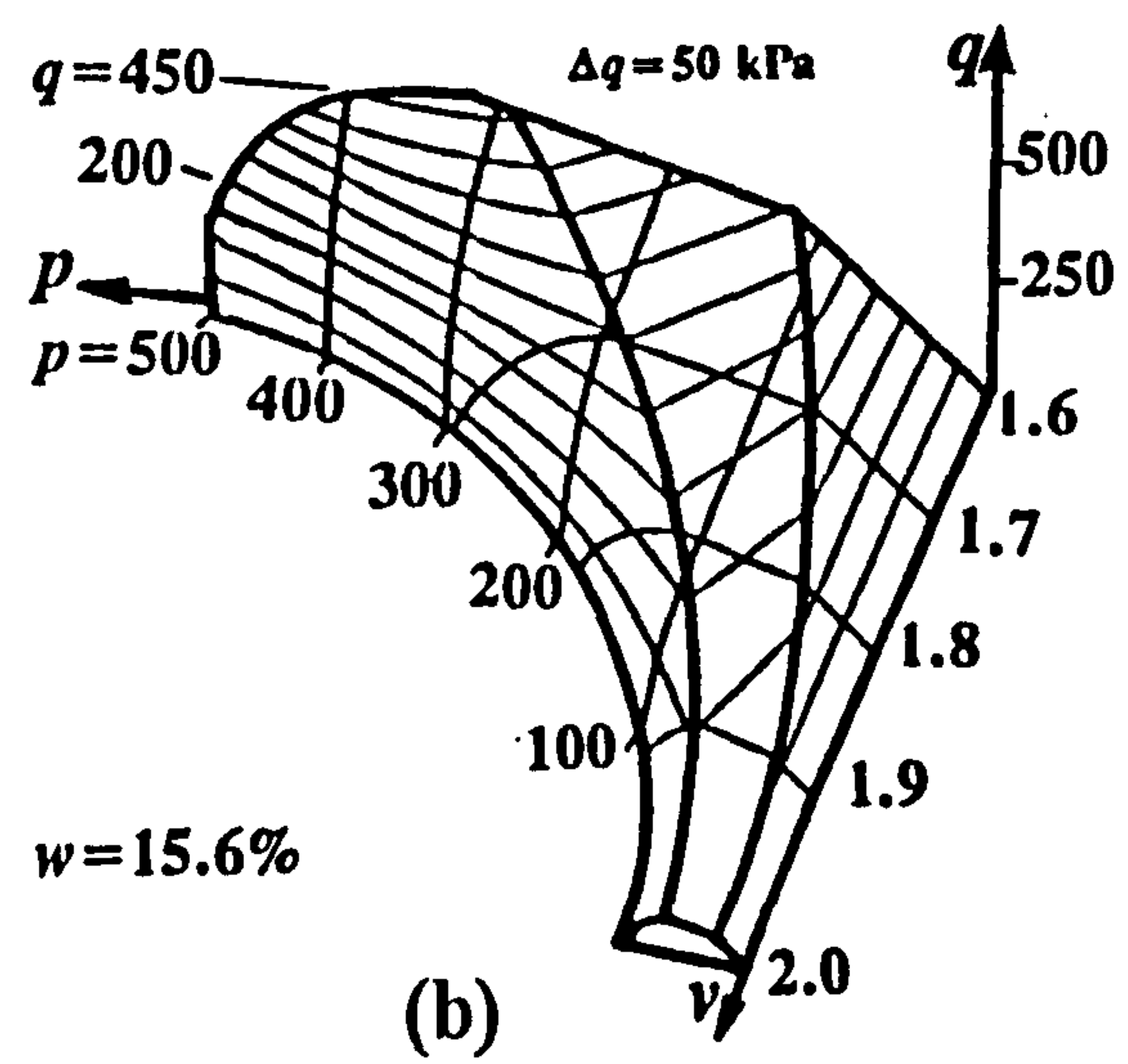
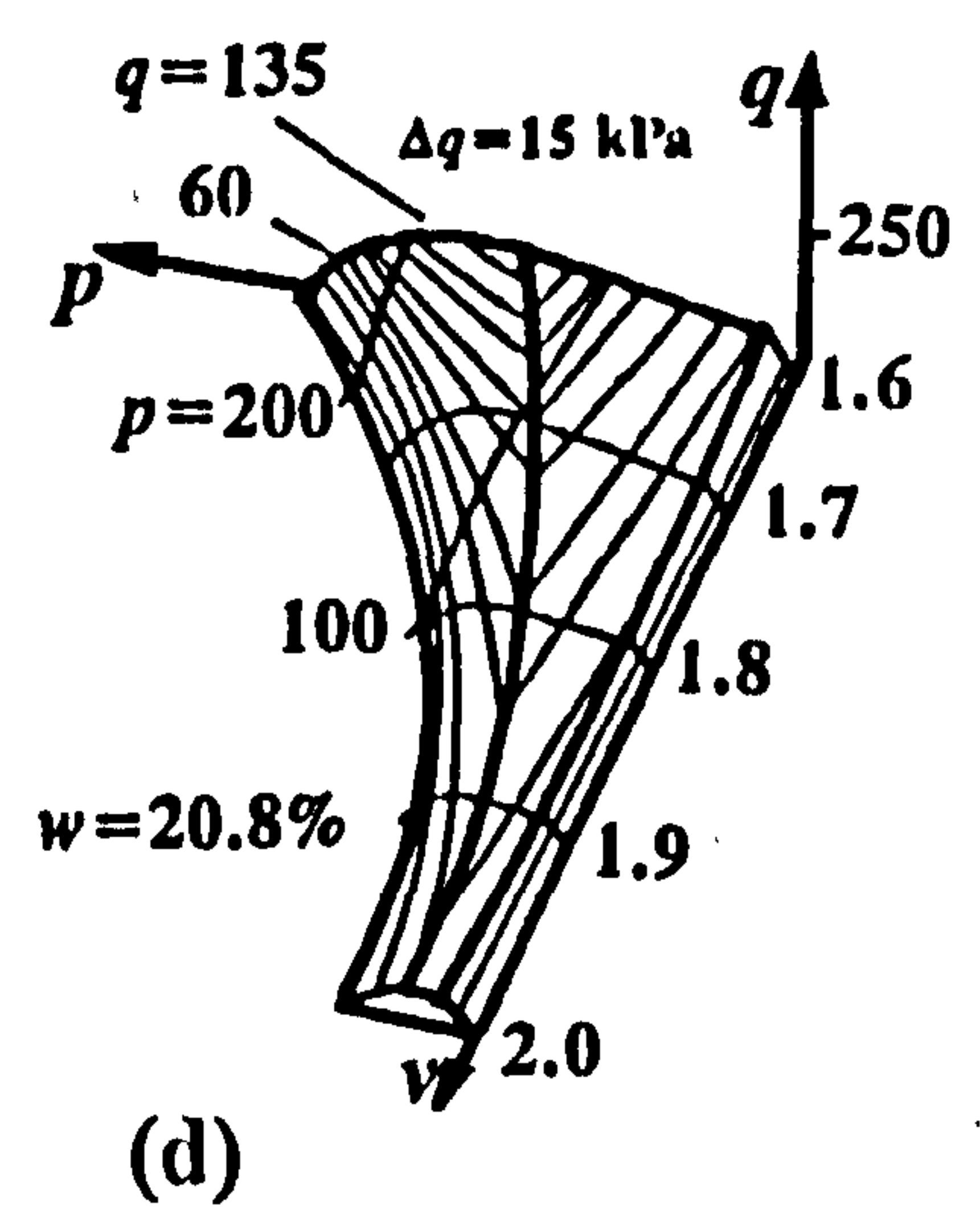
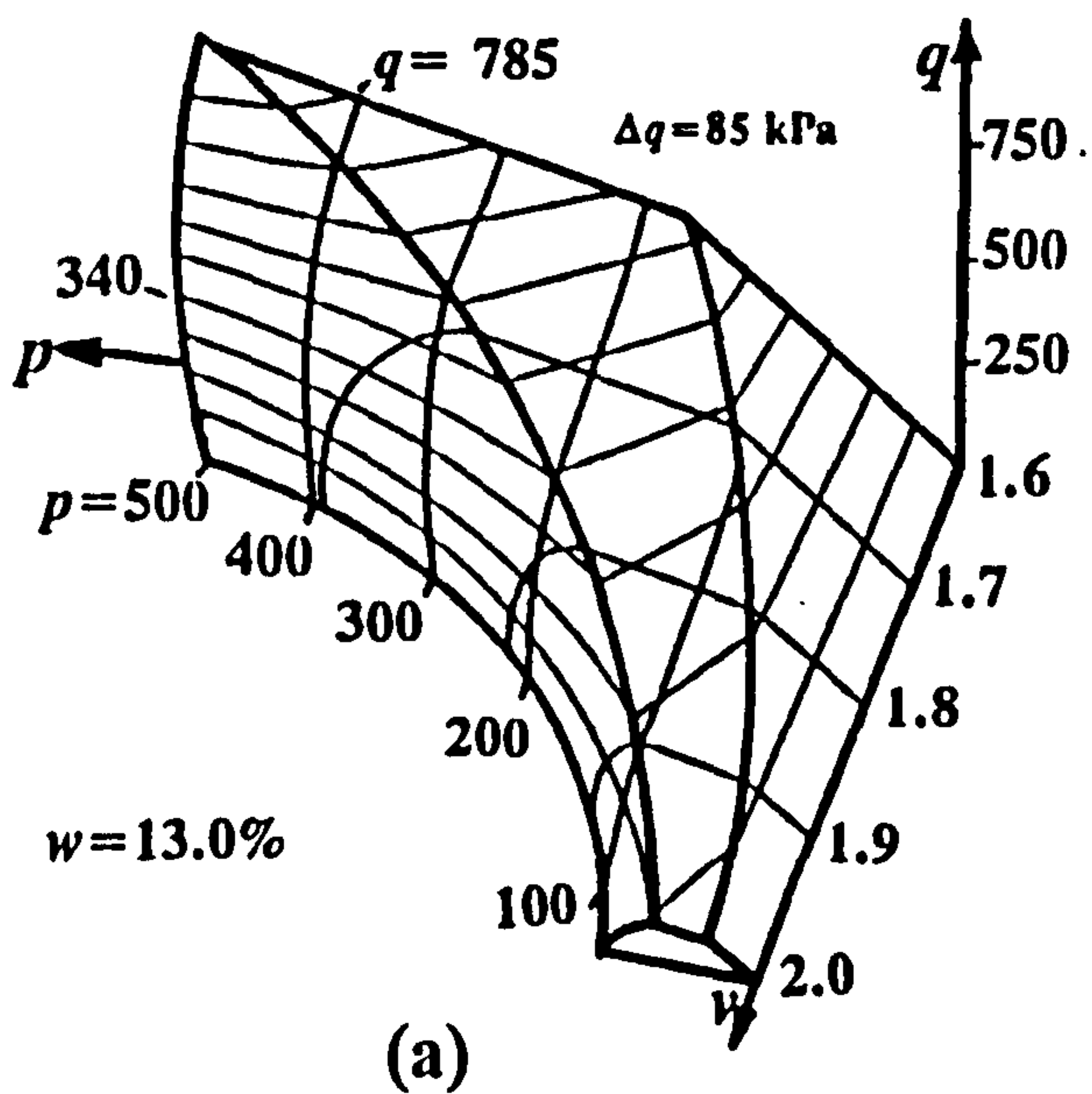


Fig. 3.17 Critical State Space for Winton clay loam:
 (a) $w = 13.0\%$
 (b) $w = 15.6\%$
 (c) $w = 18.2\%$
 (d) $w = 20.8\%$
 (e) $w = 23.4\%$

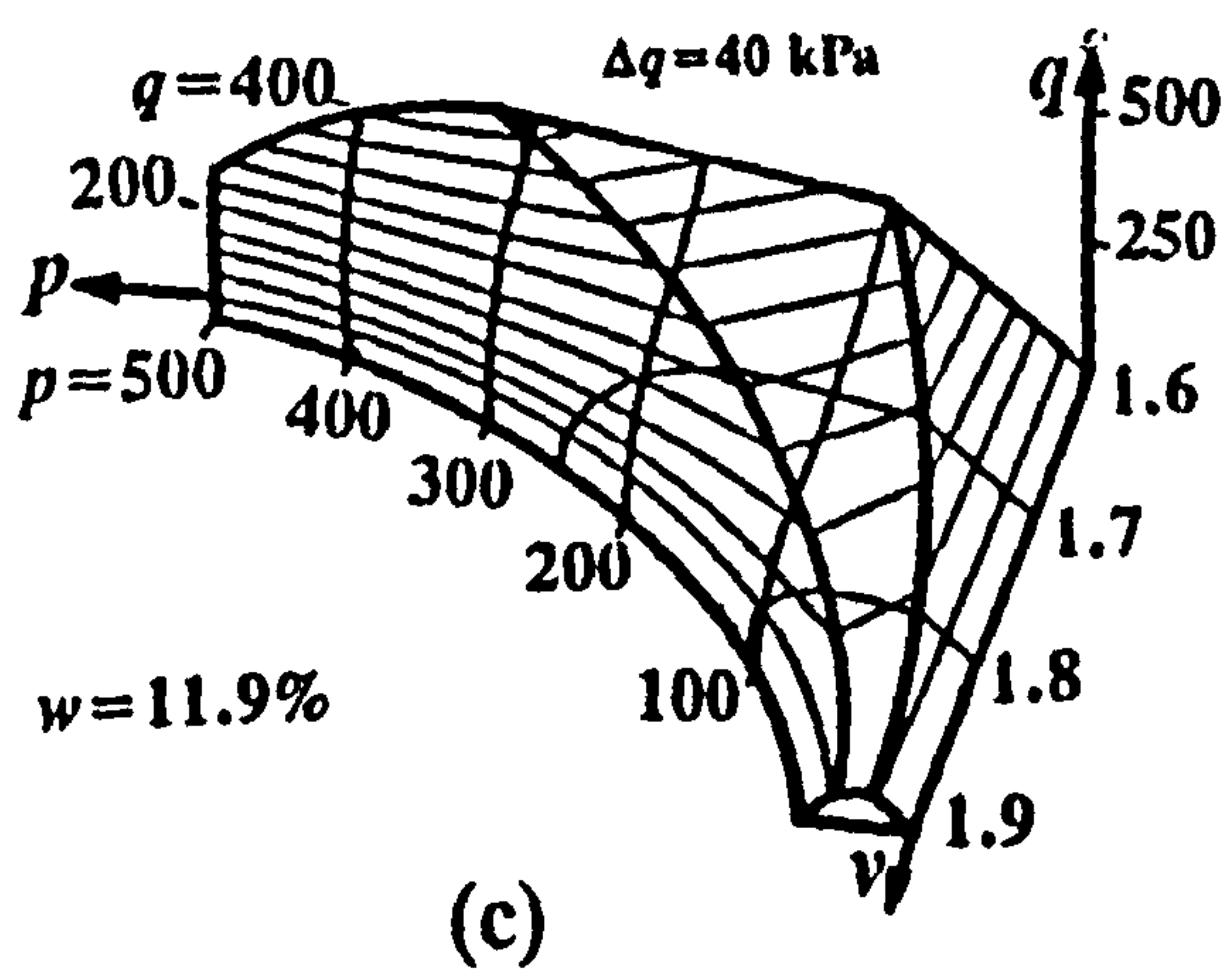
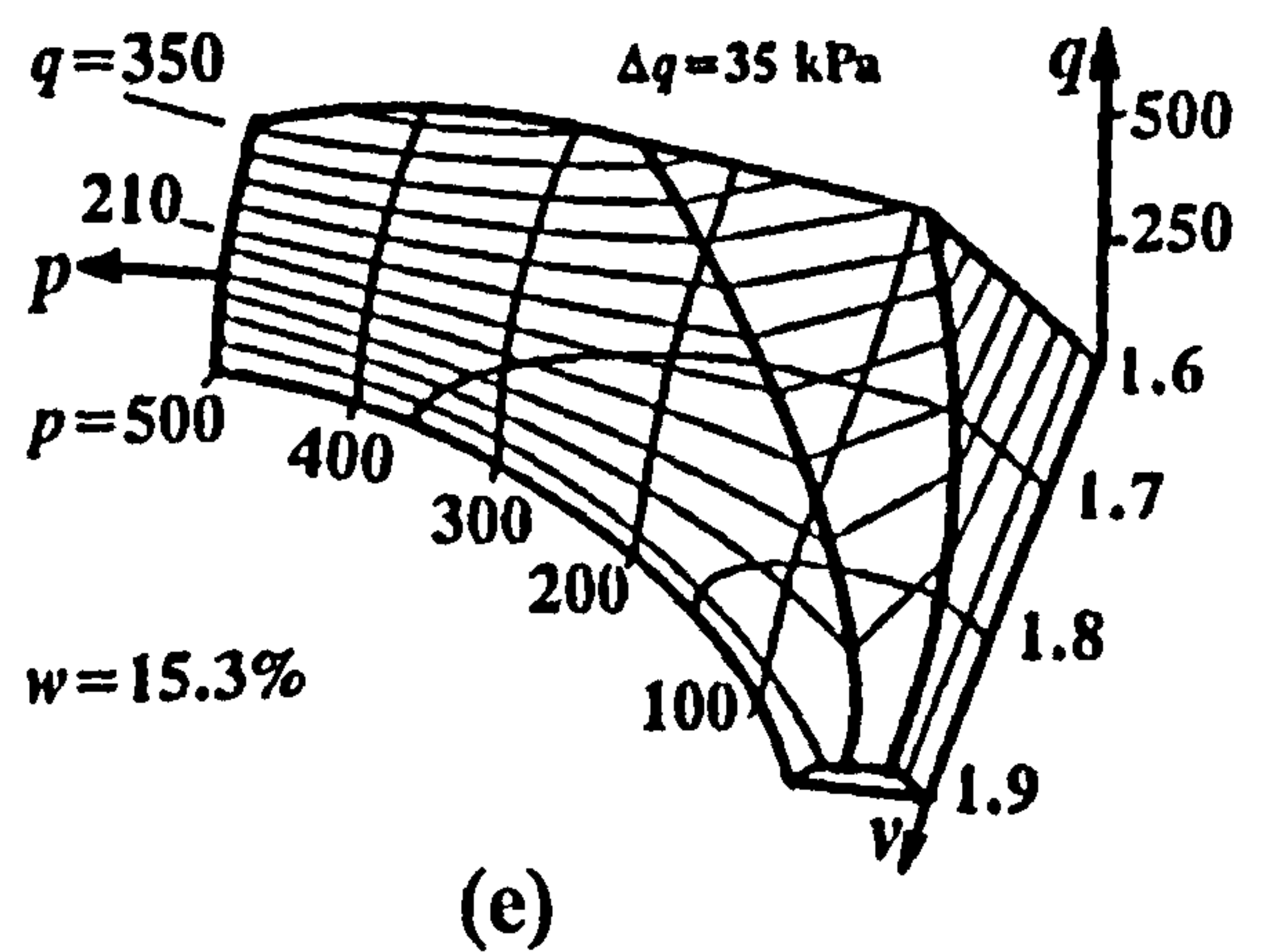
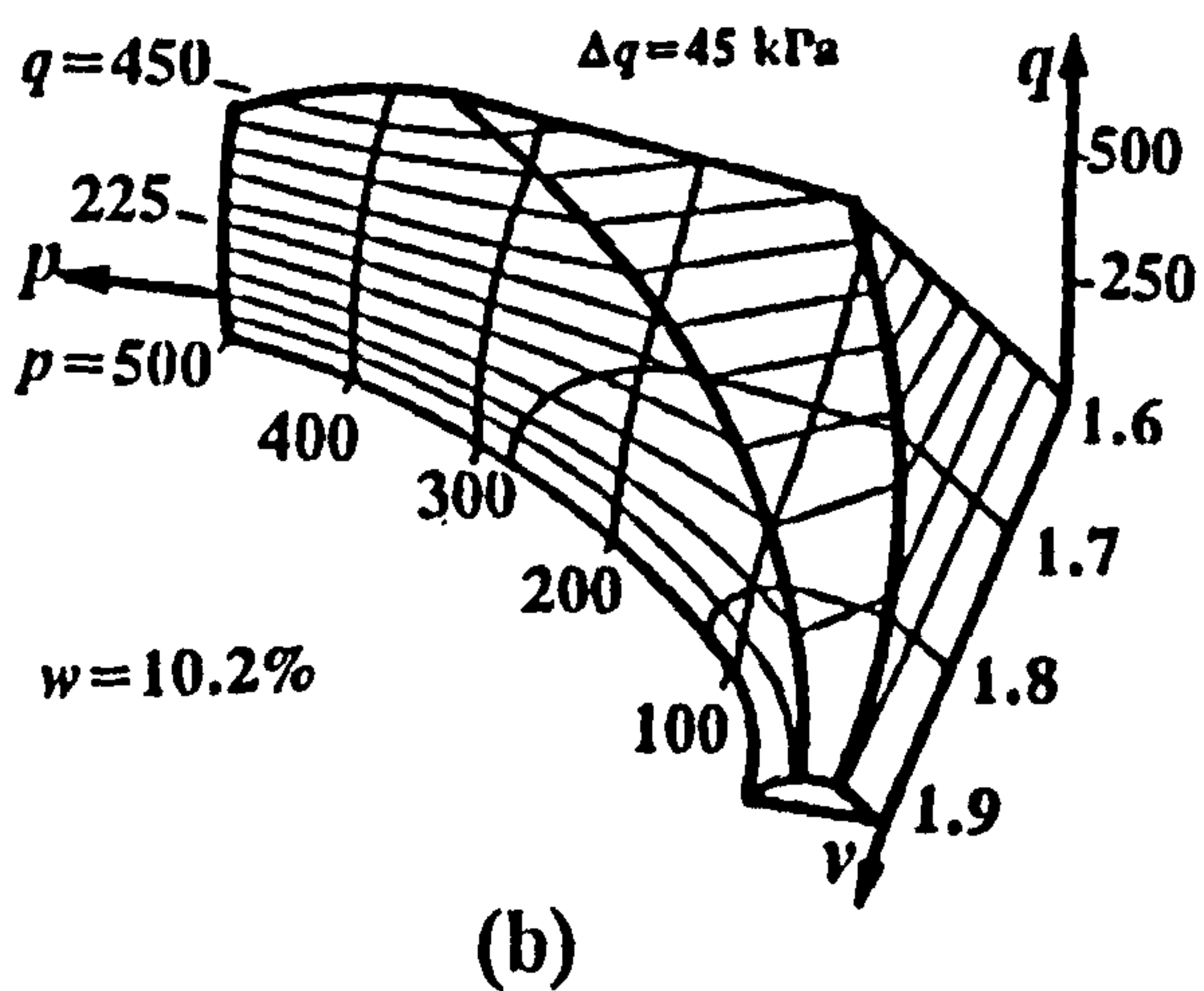
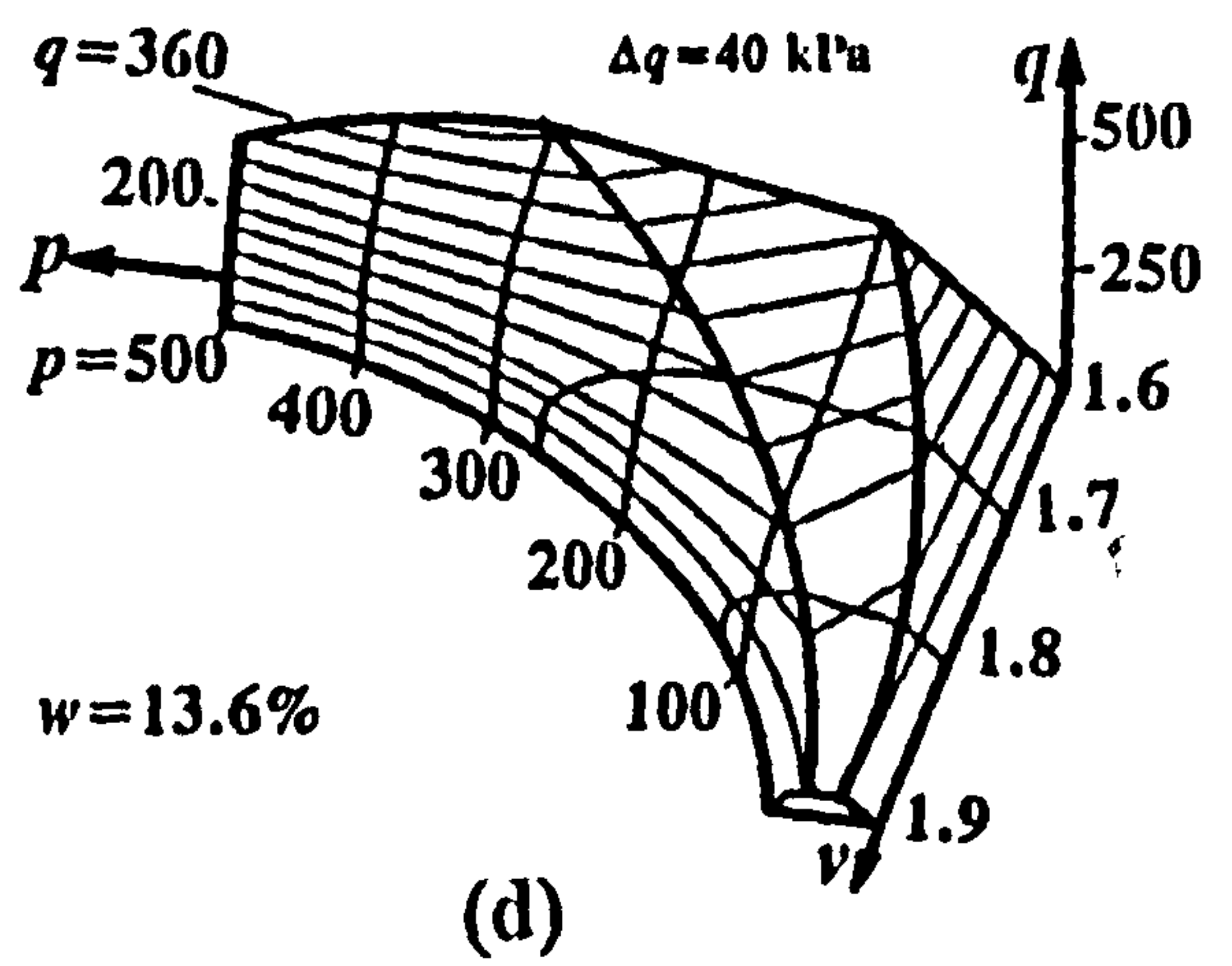
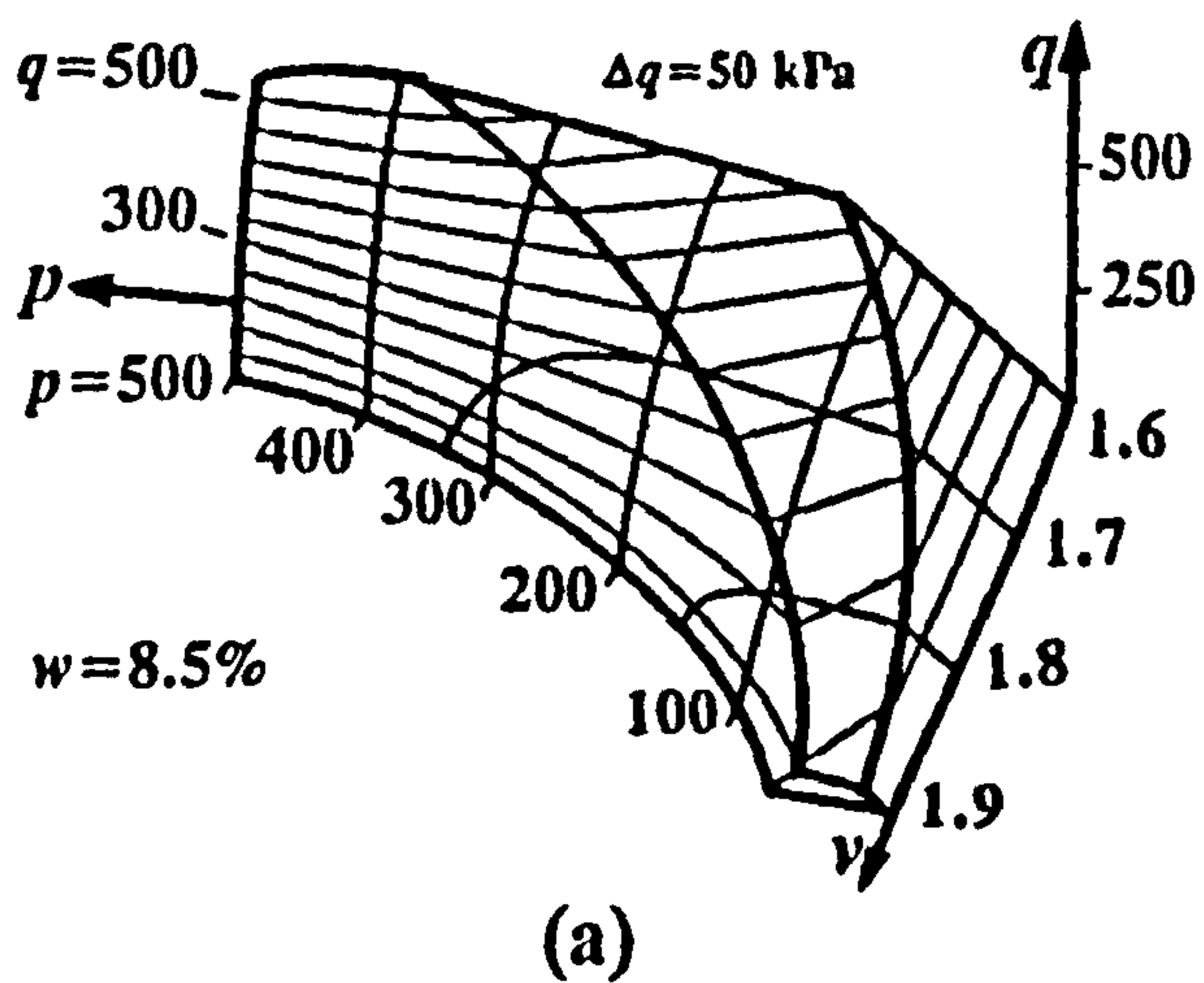


Fig. 3.18 Critical State Space for Darvel sandy loam:
 (a) $w = 8.5\%$
 (b) $w = 10.2\%$
 (c) $w = 11.9\%$
 (d) $w = 13.6\%$
 (e) $w = 15.3\%$

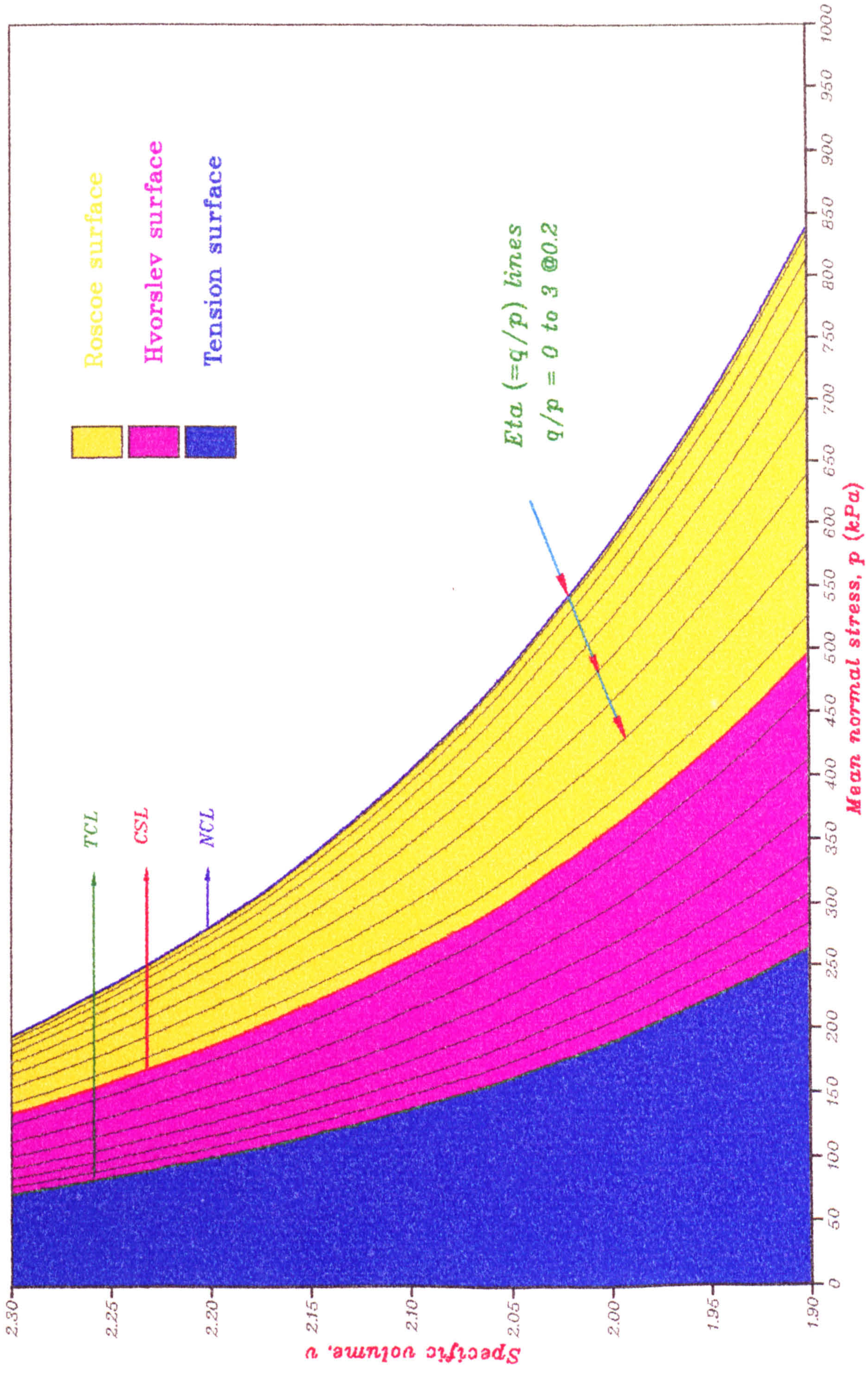


Fig. 3.19 Typical output of State boundaries and Eta lines in $p-v$ plane
(Evesham clay, $m.c.= 24.5\%$)

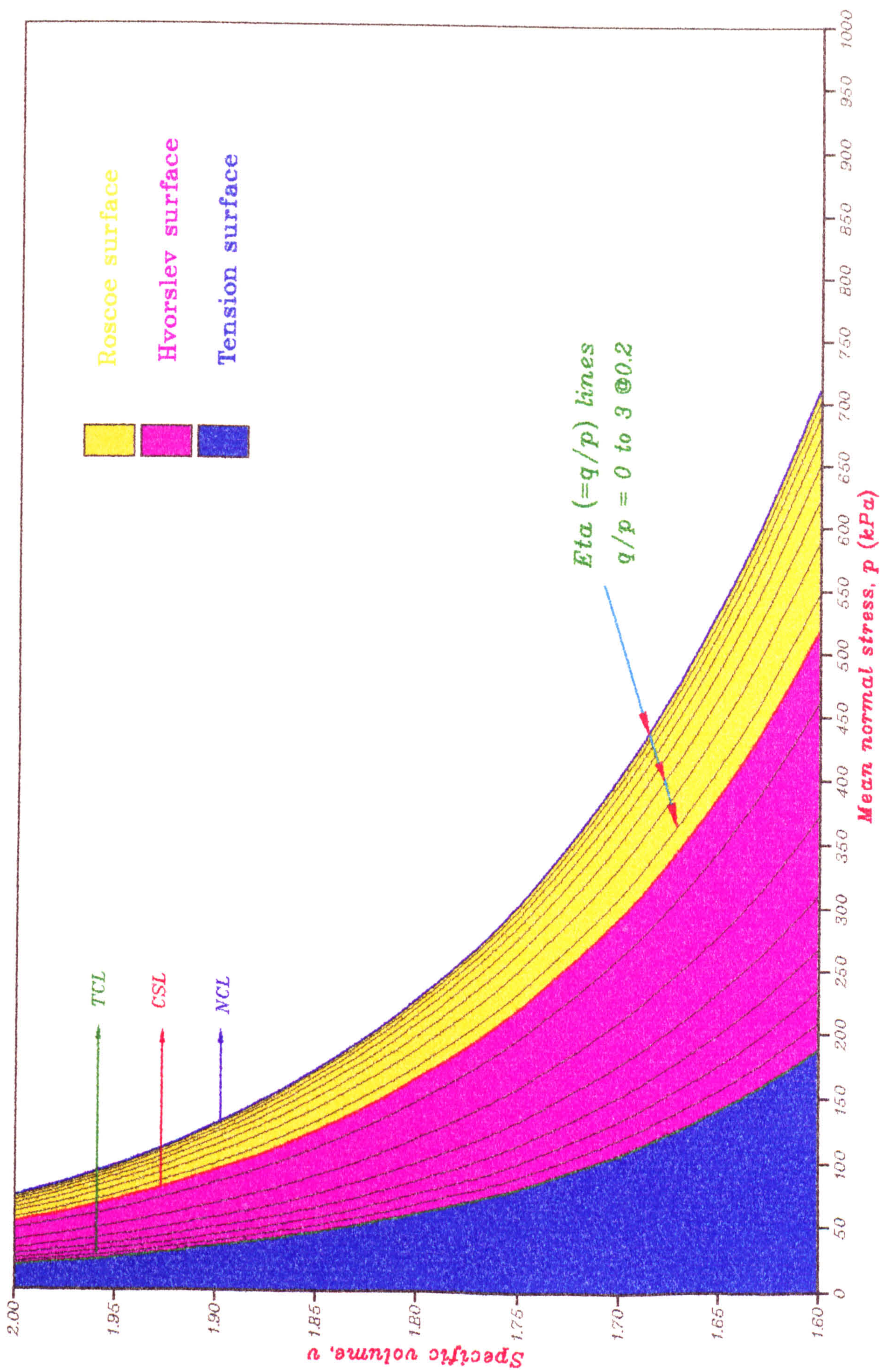


Fig. 3.20 Typical output of State boundaries and Eta lines in $p-v$ plane
(Winton clay loam, $m.c. = 13.0\%$)

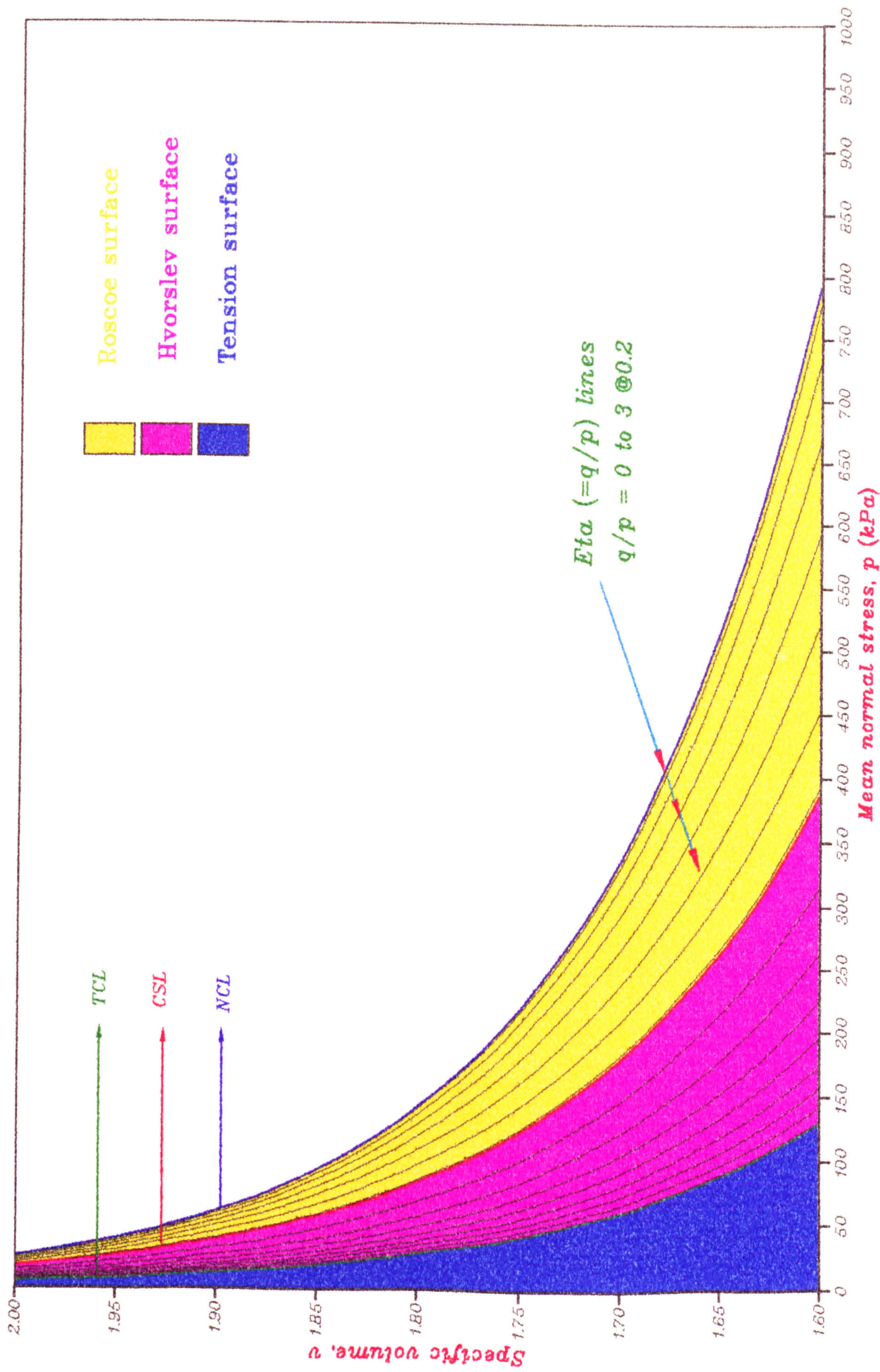


Fig. 3.21 Typical output of State boundaries and Eta lines in $p-v$ plane
(Darvel sandy loam, $m.c. = 8.5\%$)

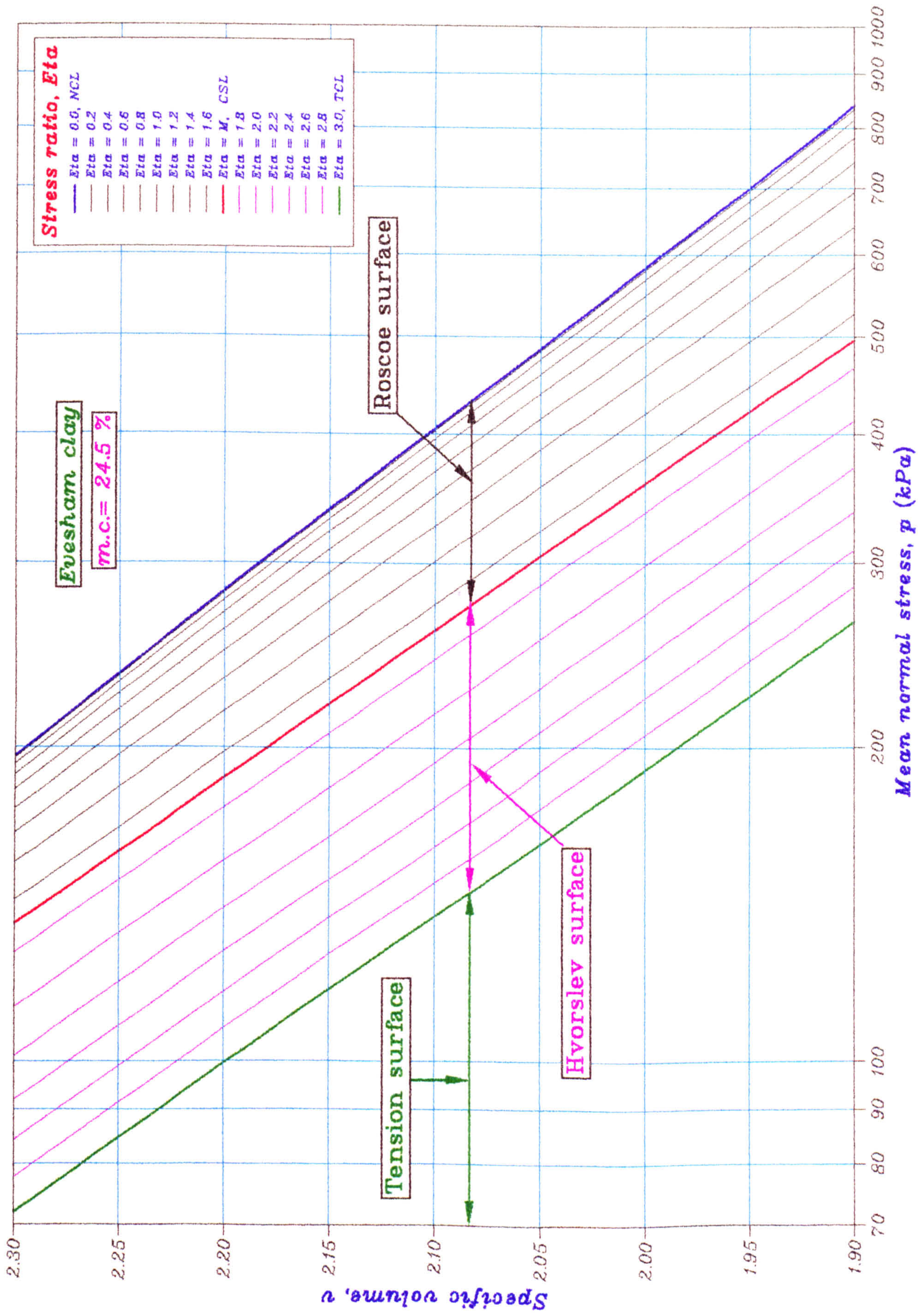


Fig. 3.22 Typical output for state boundaries and Eta-lines in v - $\ln(p)$ plane

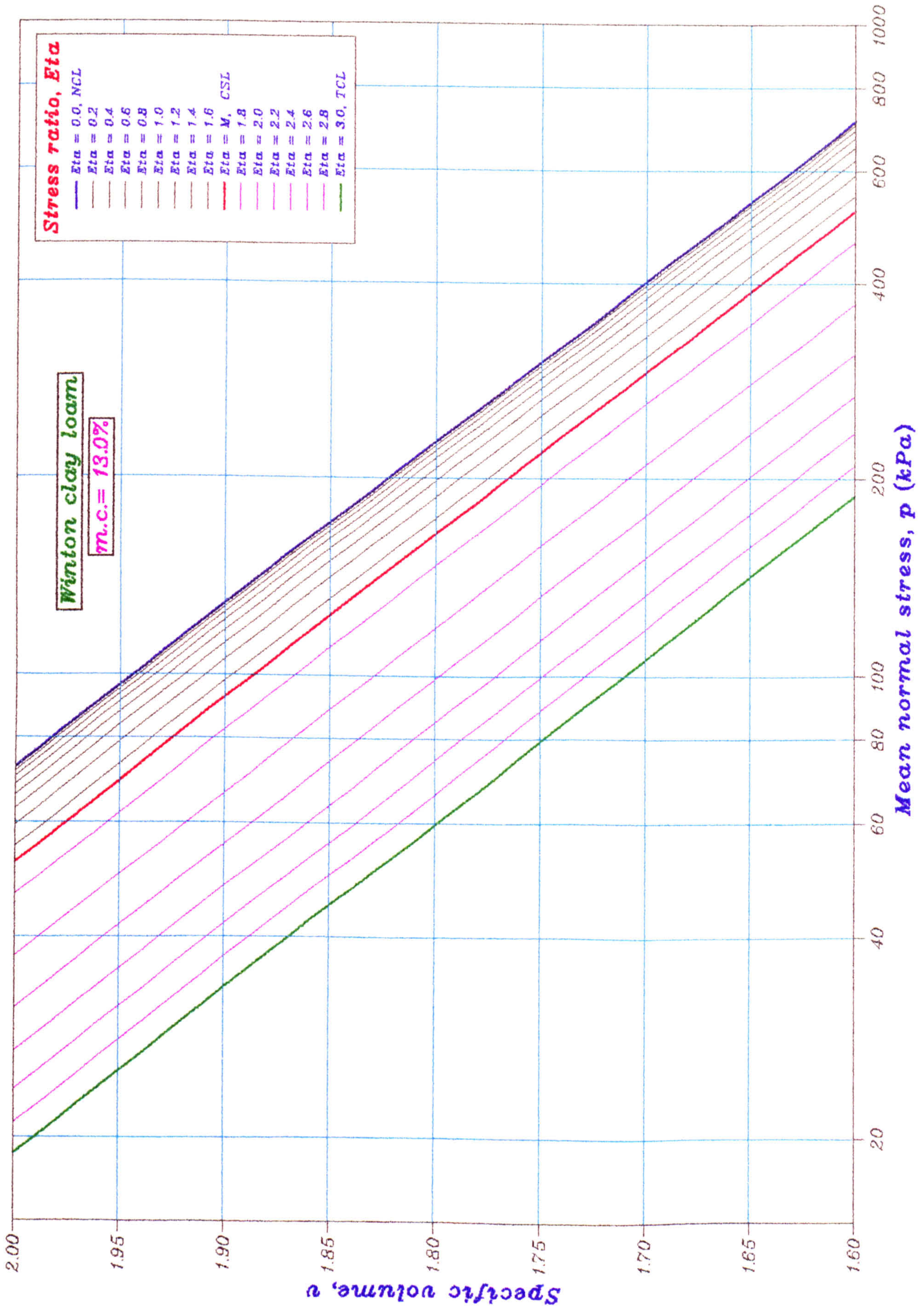


Fig. 3.23 Typical output for state boundaries and η -lines in v - $\ln(p)$ plane

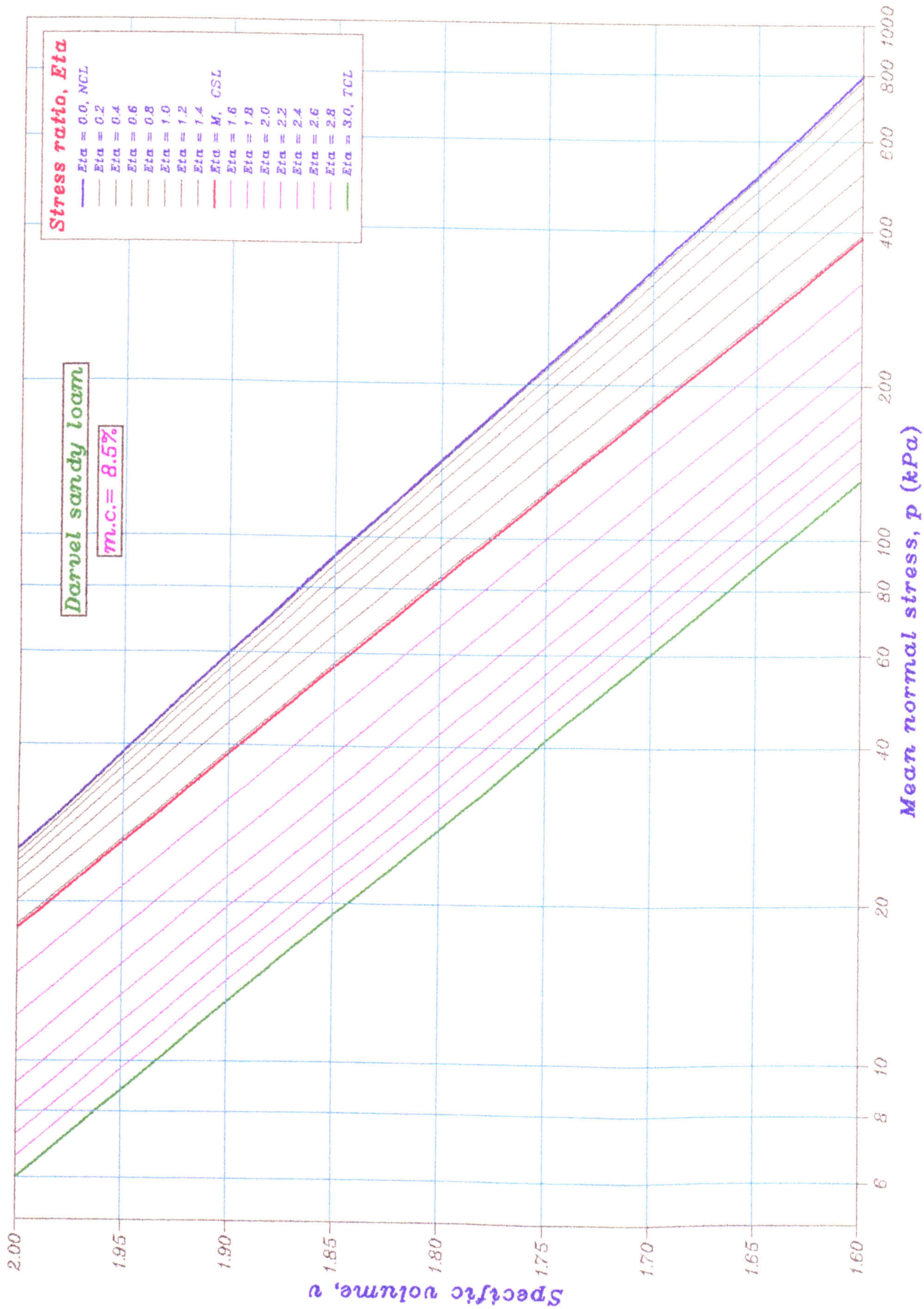


Fig. 3.24 Typical output for state boundaries and η -lines in $v-\ln(p)$ plane

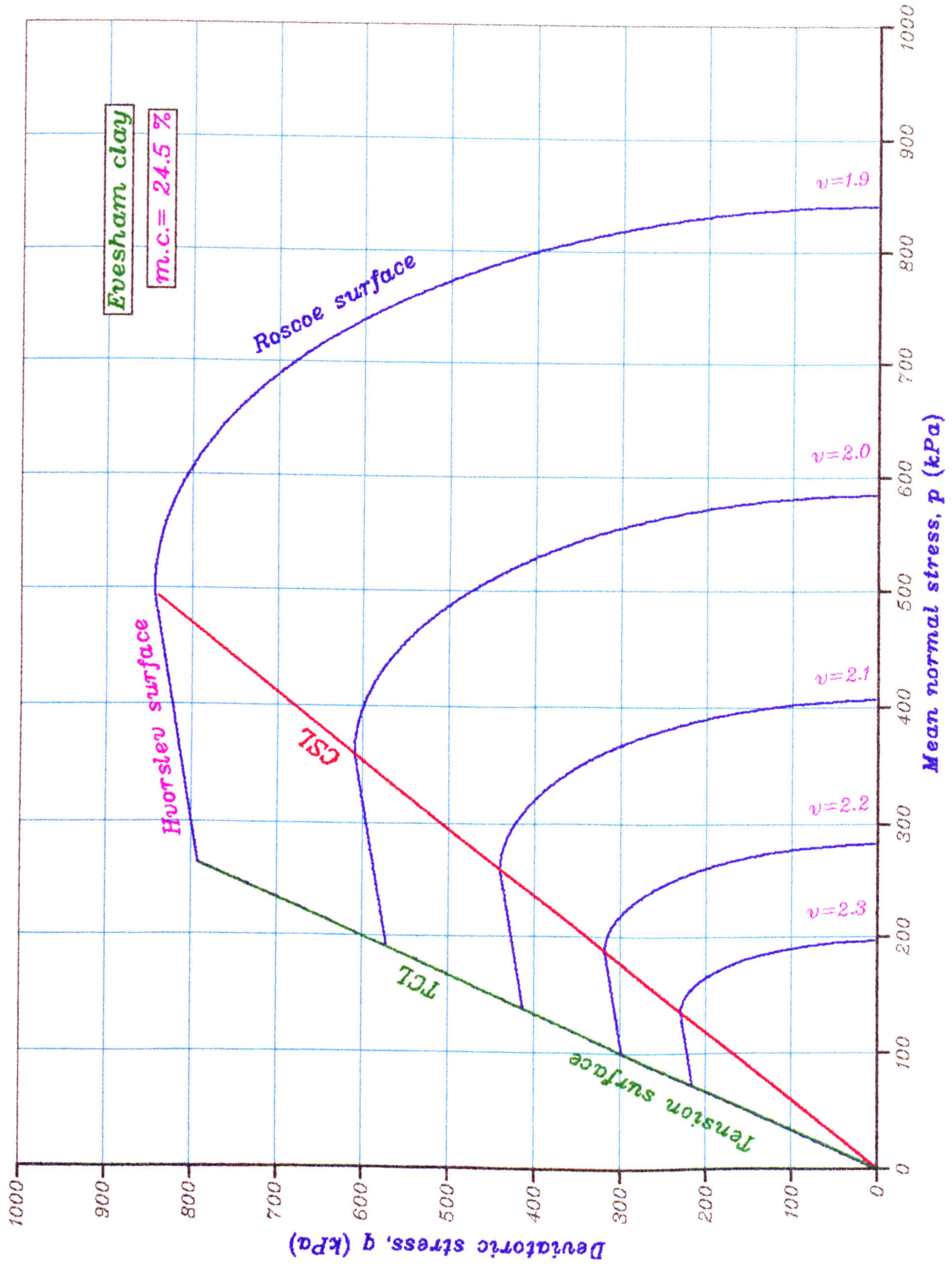


Fig. 3.25 Typical plot for constant v -lines in q - p plane

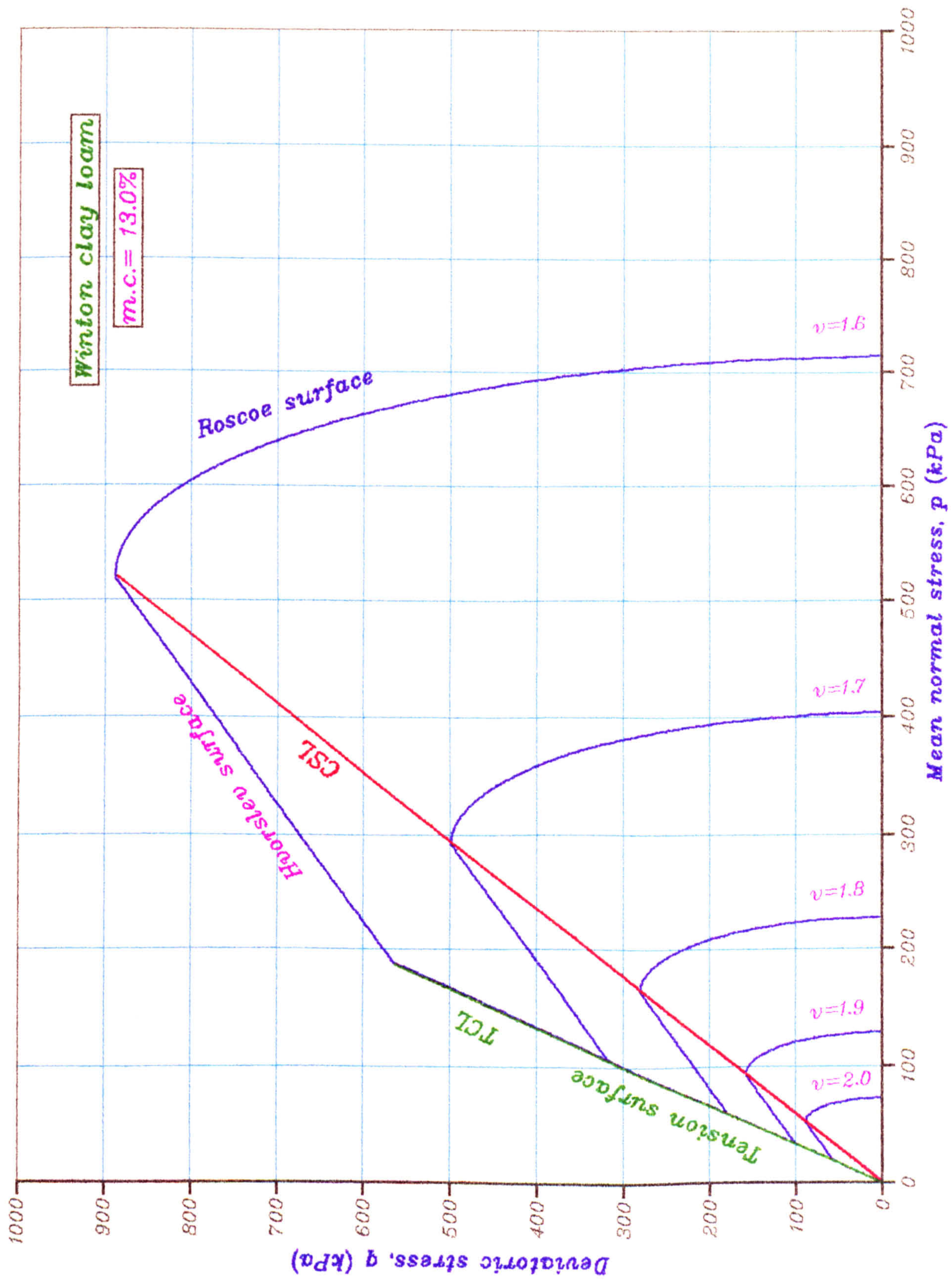


Fig. 3.26 Typical plot for constant v -lines in q - p plane

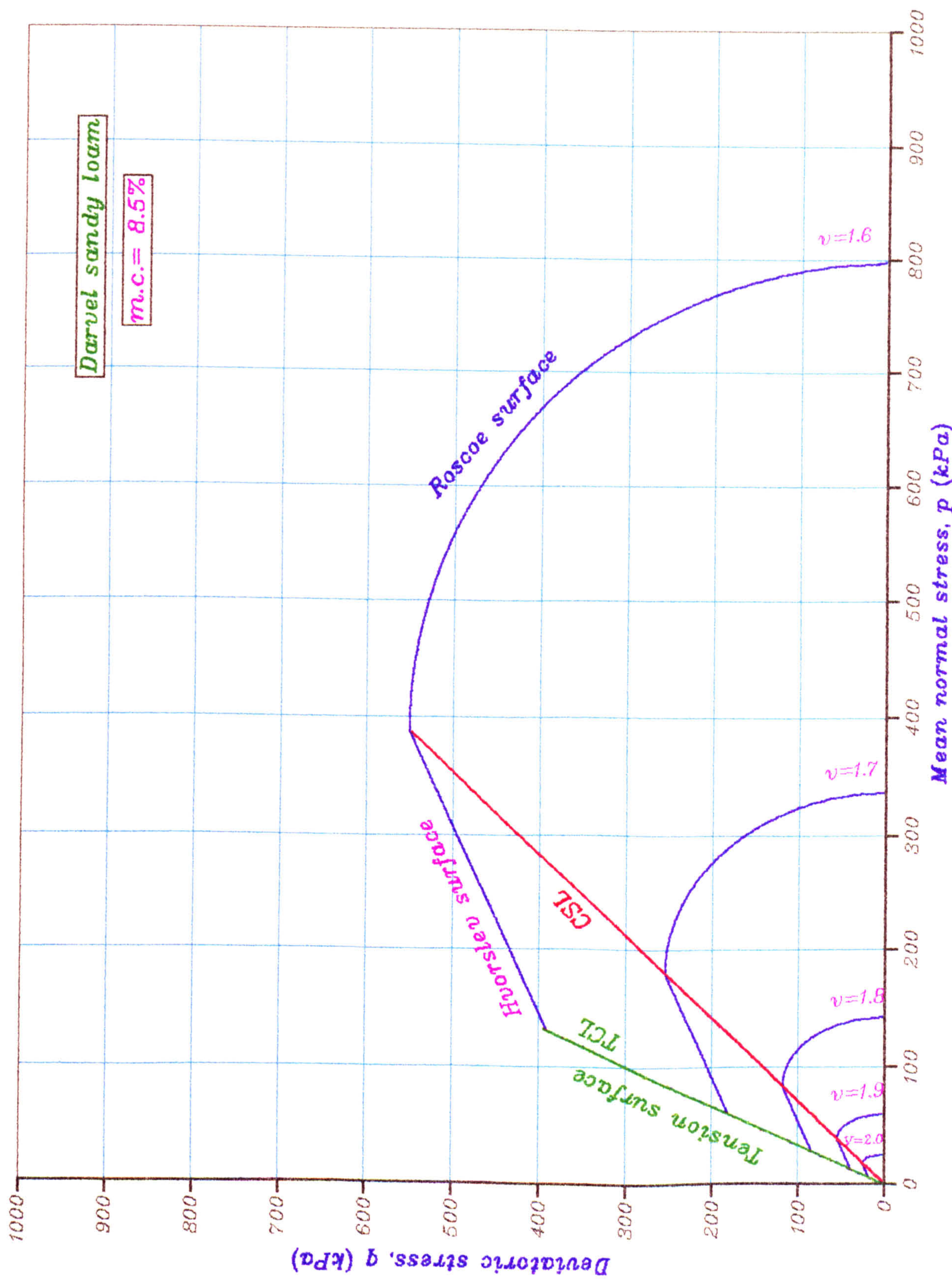


Fig. 3.27 Typical output for constant v -lines in q - p plane

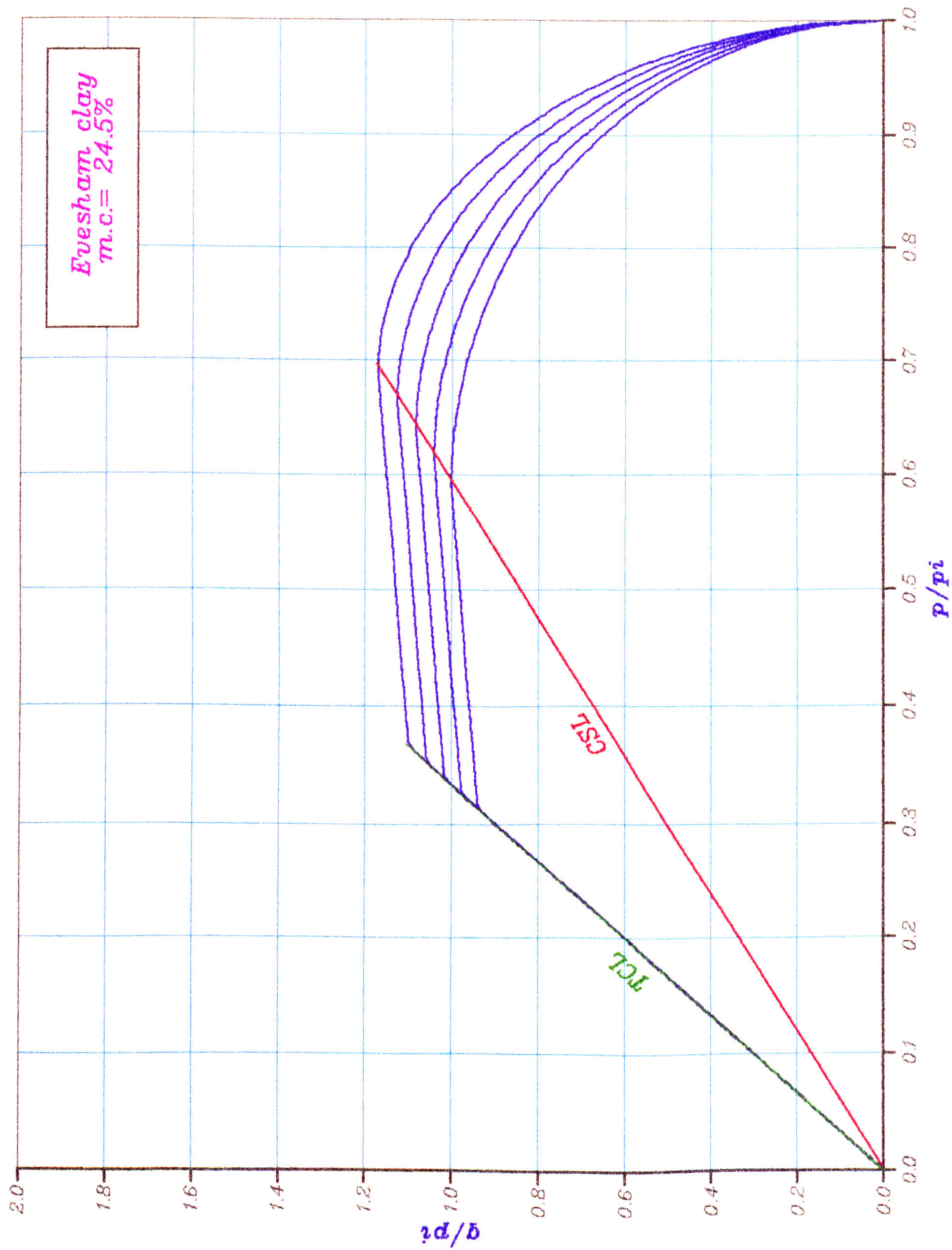


Fig. 3.28 Typical normalized plot on $q/\pi - p/\pi$ plane for Evesham clay

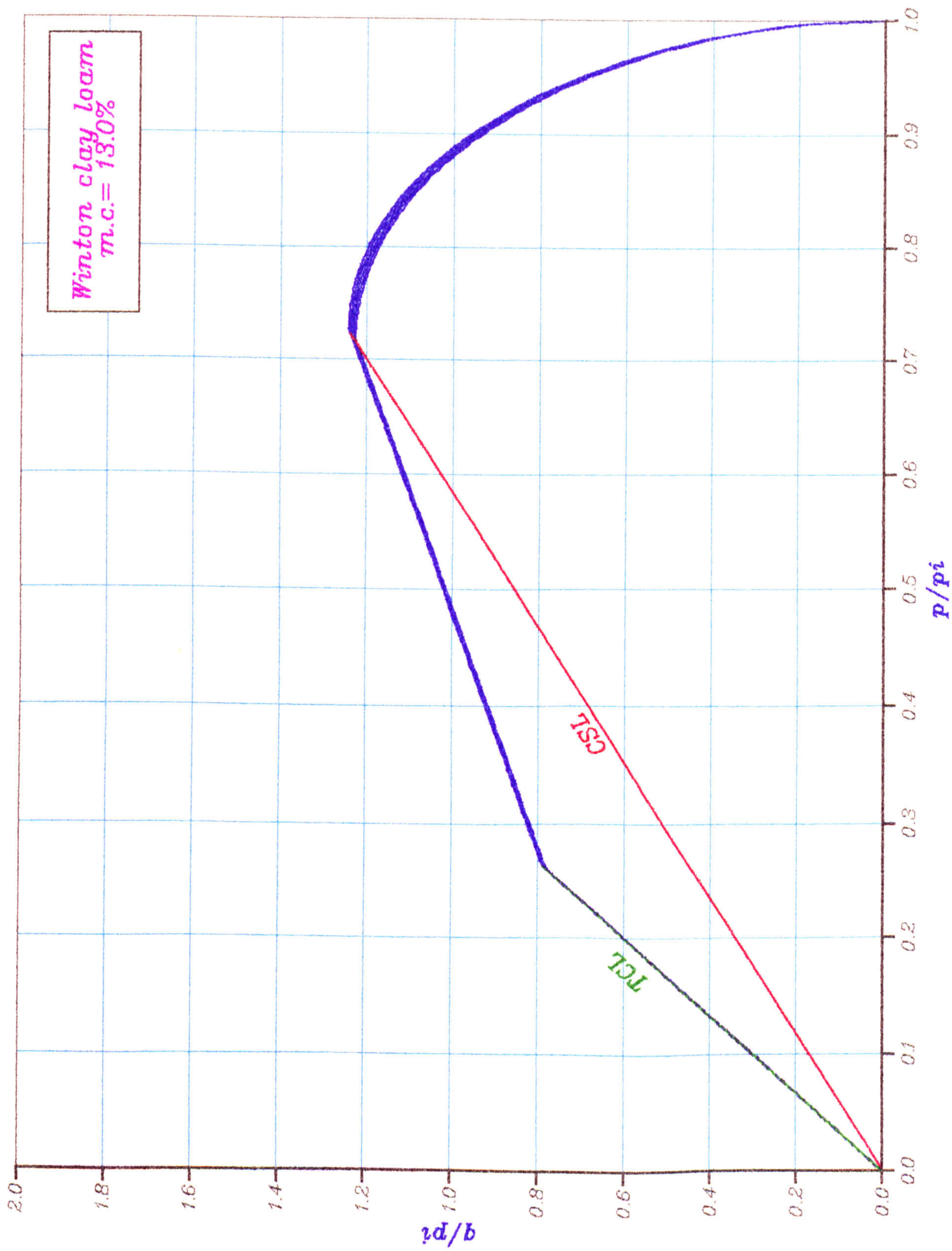


Fig. 3.29 Typical normalized plot on $q/\pi - p/\pi$ plane for Winton clay loam

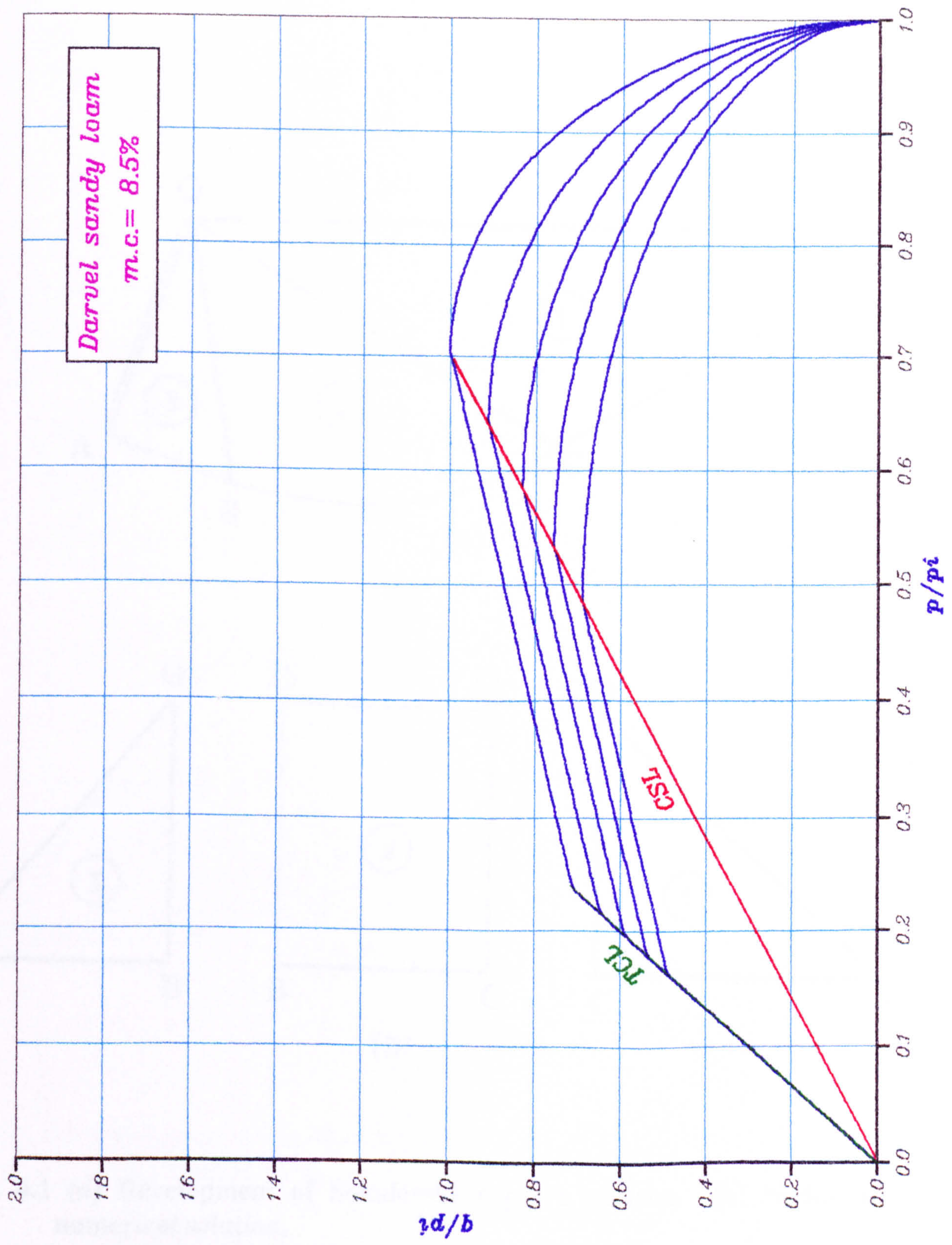


Fig. 3.30 Typical normalized plot on q/π - p/π plane for Darvel sandy loam

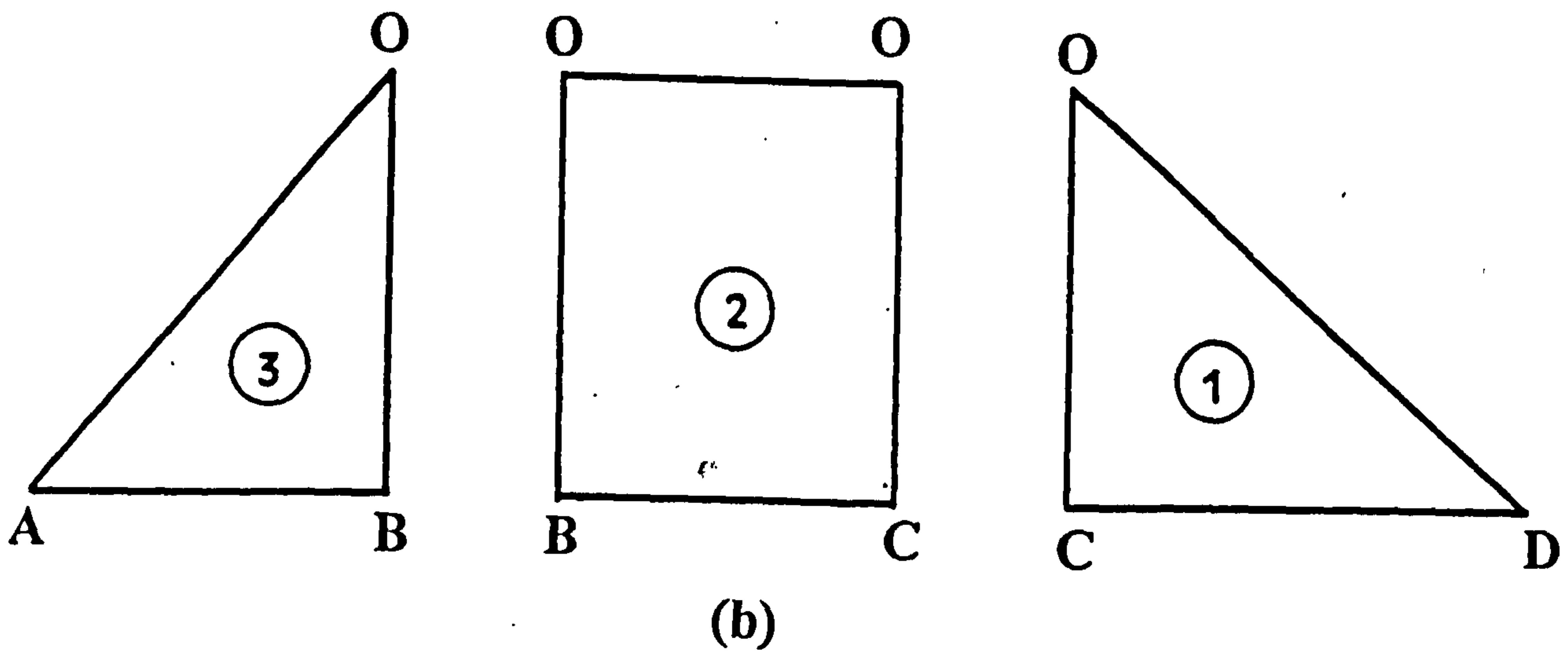
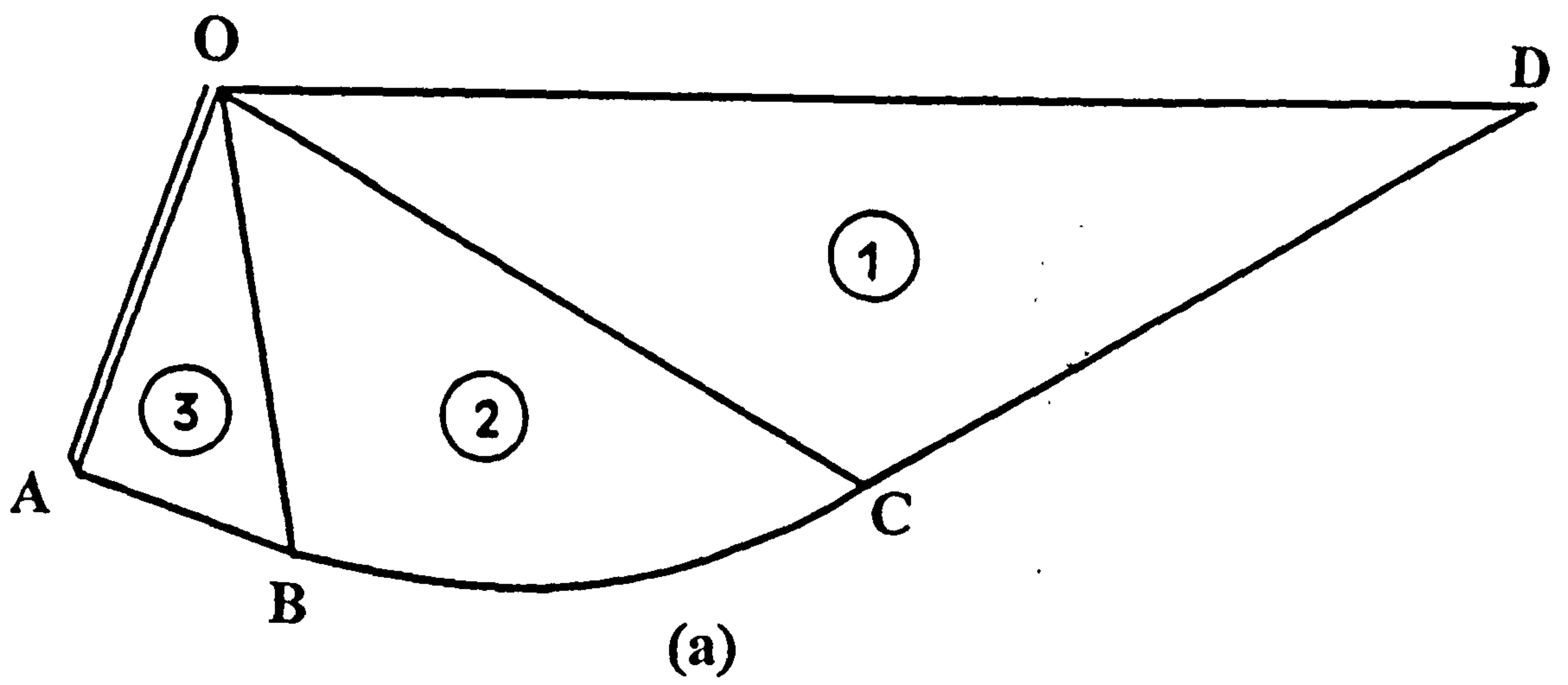


Fig. 4.1 (a) Development of Sokolovski rupture surface. (b) Nodes in numerical solution.

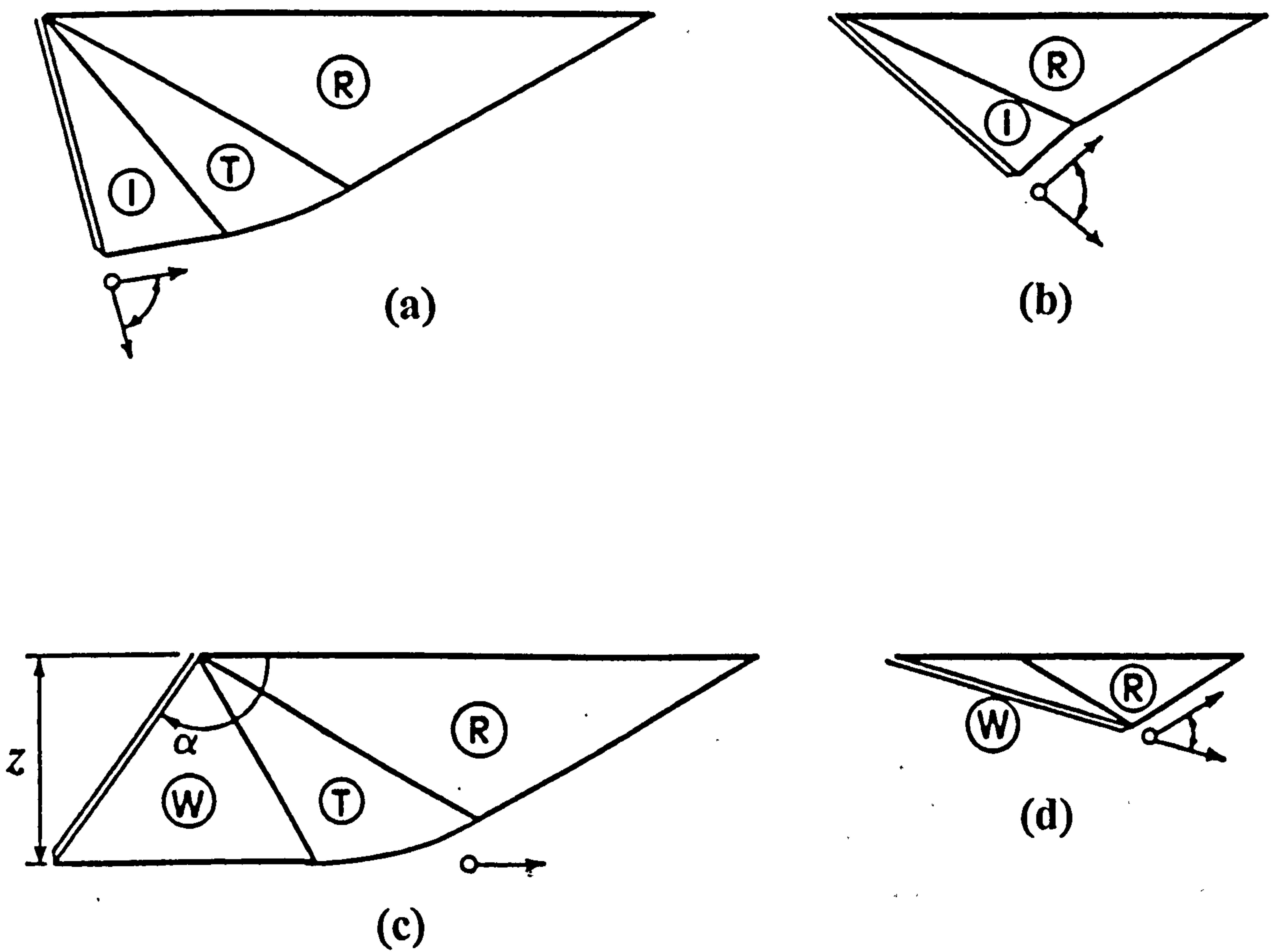


Fig. 4.2 Main types of slip-line fields; (a) Basic field comprising the Interface (I), Transition (T) and Rankine (R) zones. (b) Small rake angles inducing a stress discontinuity between (I) and (R). (c) Large rake angles with soil boundary wedge (W) fixed to interface. (d) Small rake angles with wedge or discontinuity for a fully rough interface.

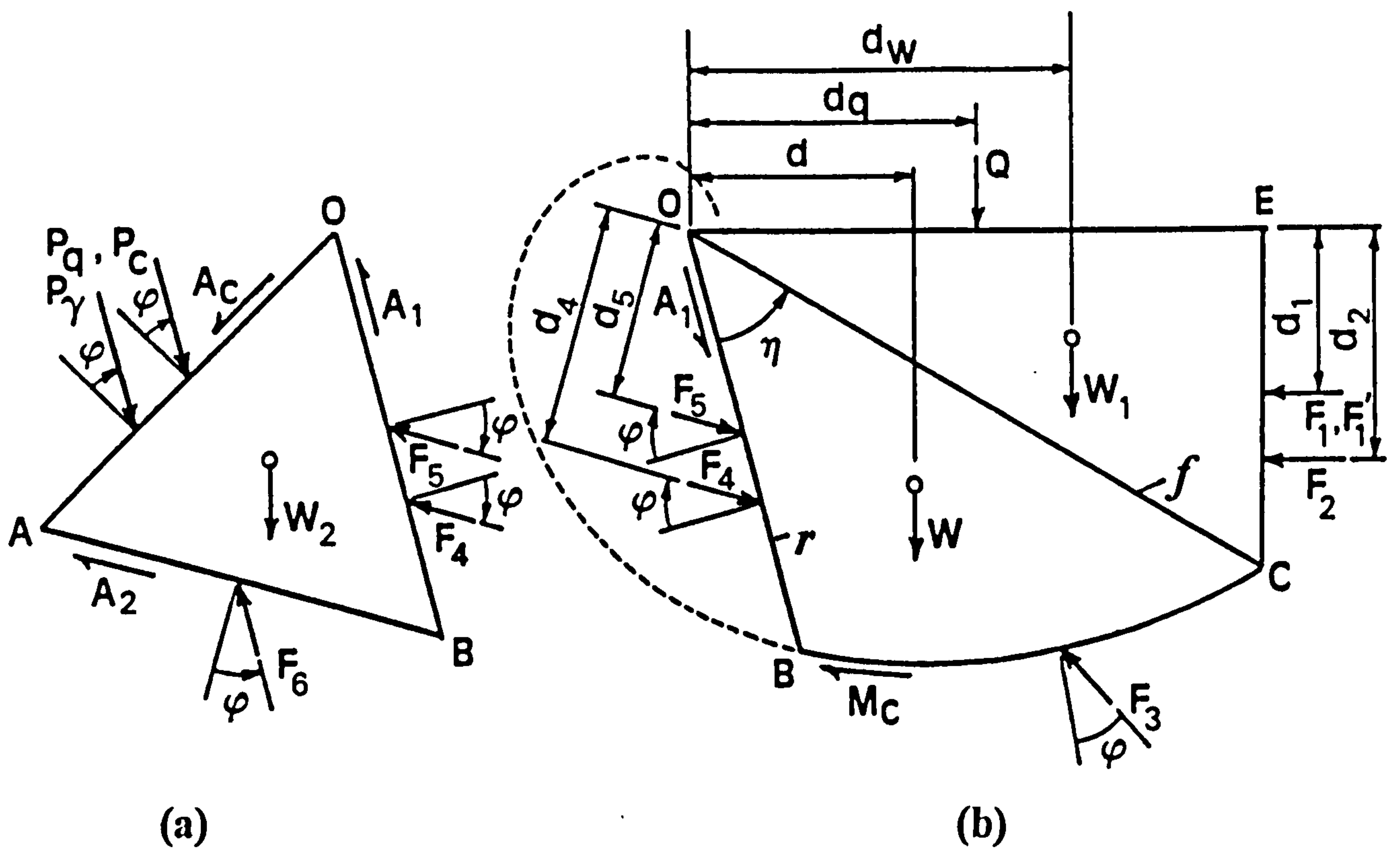


Fig. 4.4 Forces on the soil rupture block in basic passive failure; (a) Interface zone (b) Transition zone and half the passive Rankine zone.

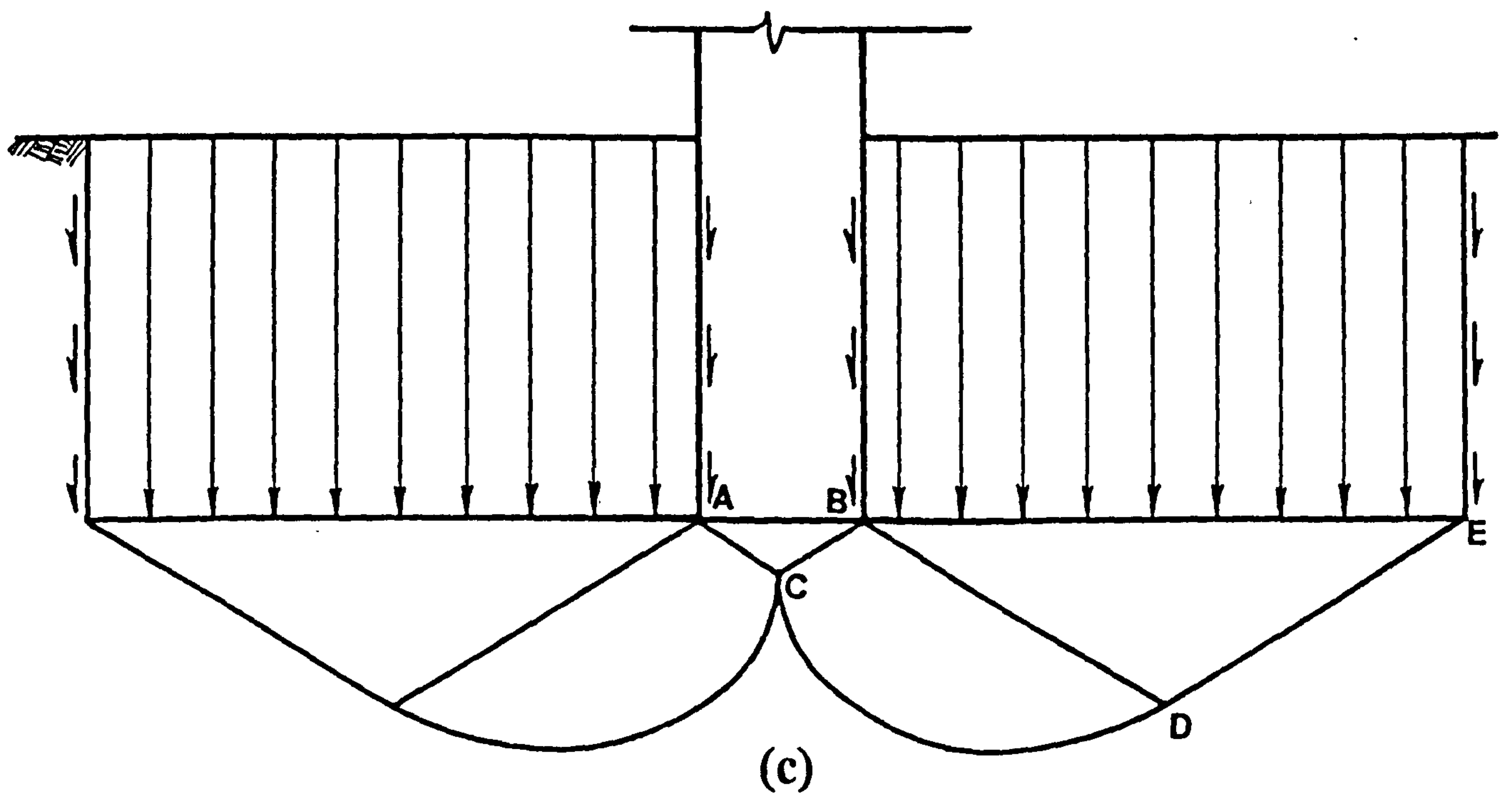
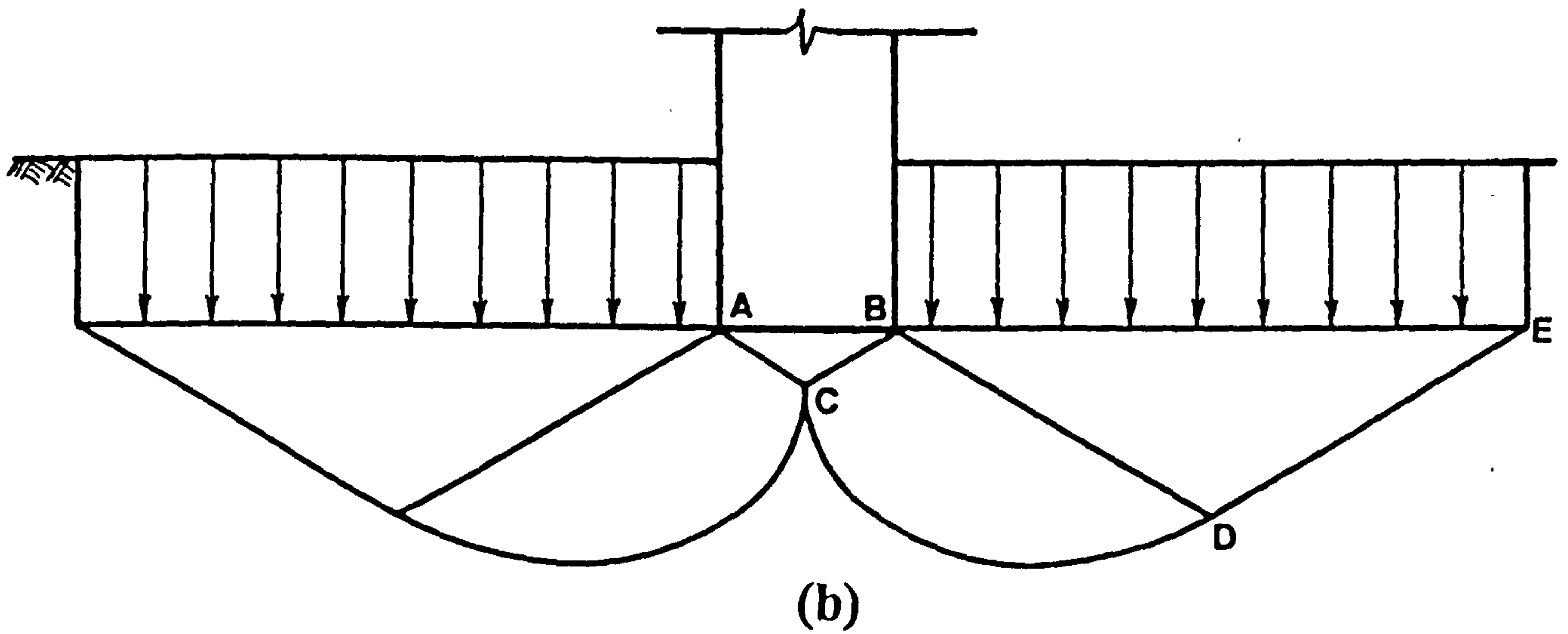
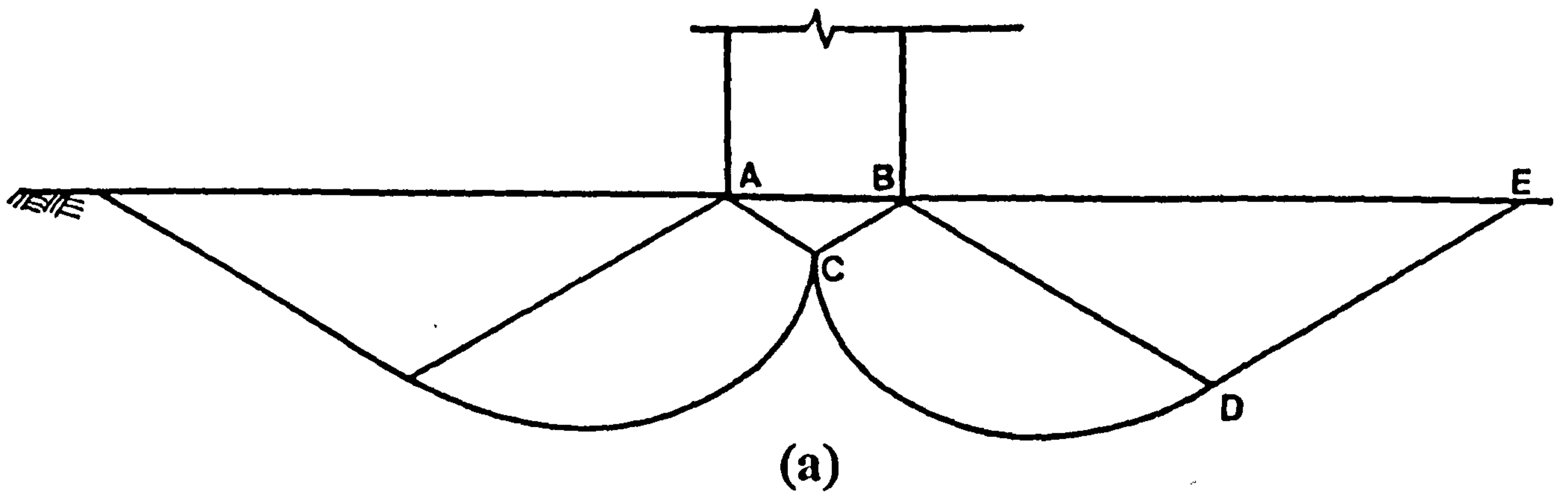
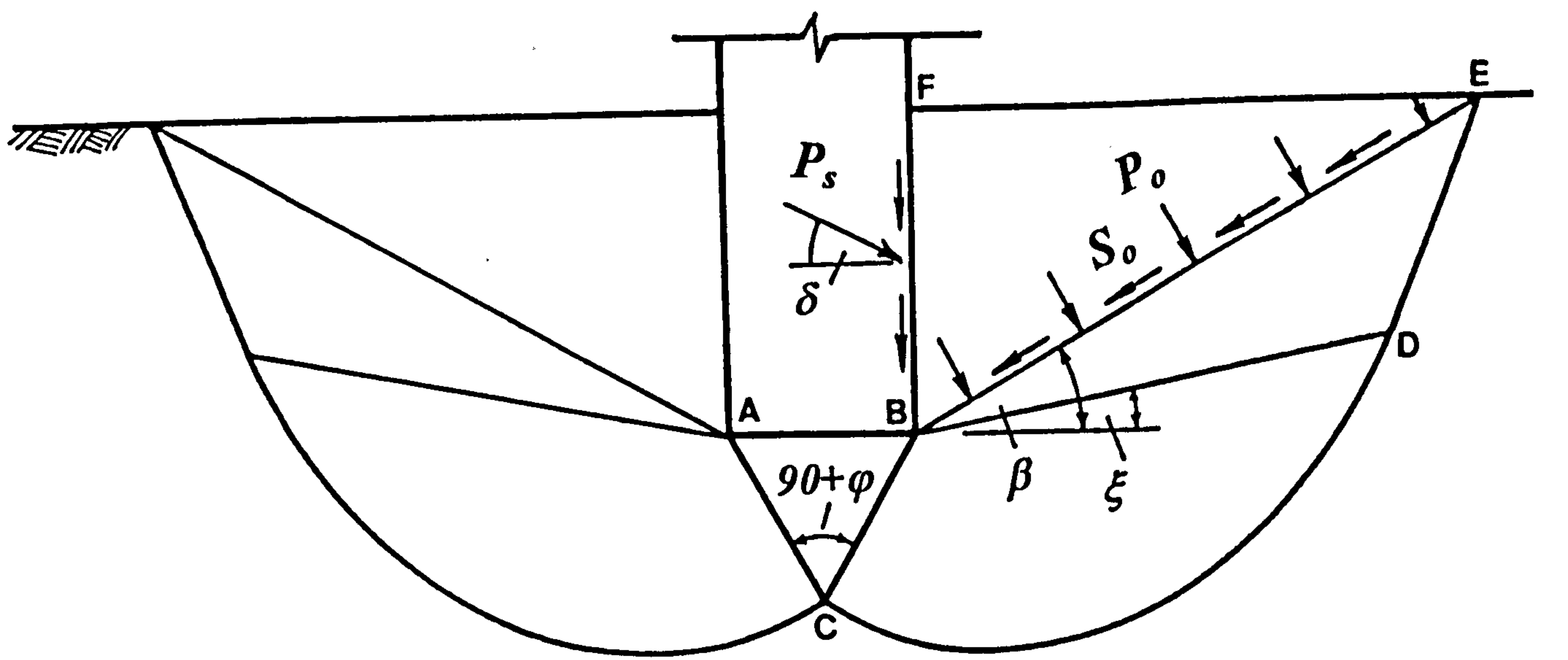
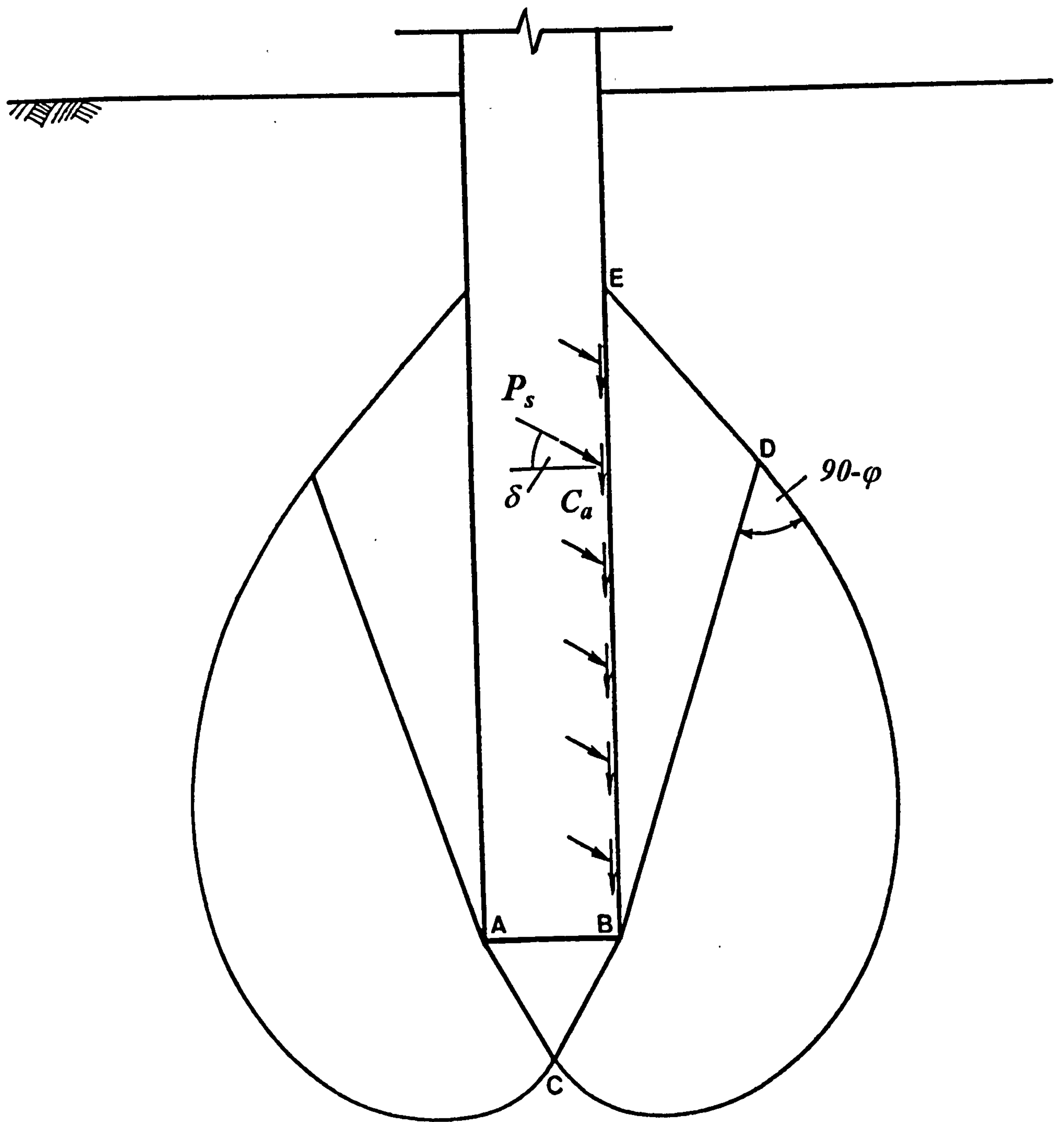


Fig. 4.5 Terzaghi's foundation failure; (a) Surface foundation (b) Shallow foundation (c) Deep foundation.

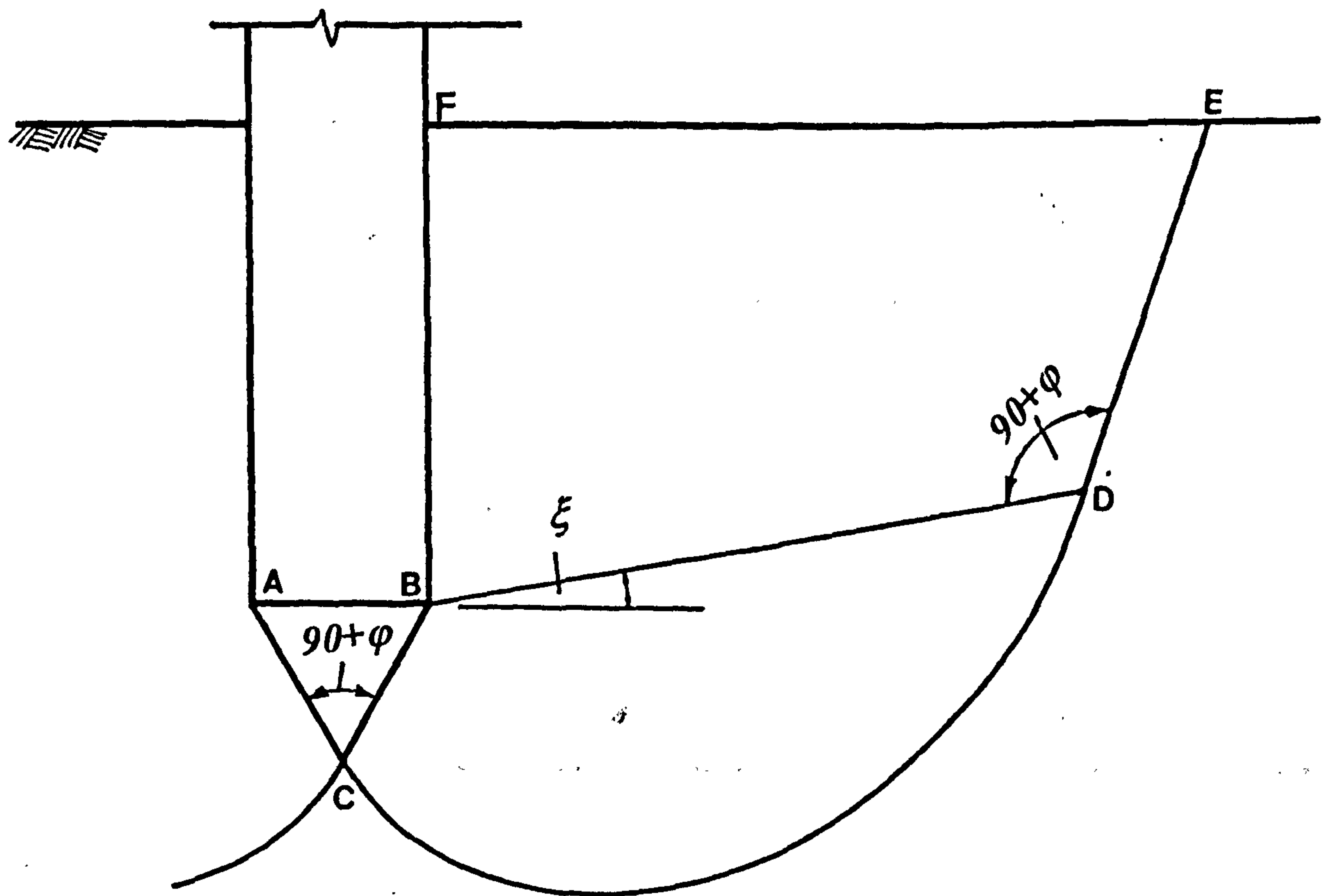


(a)

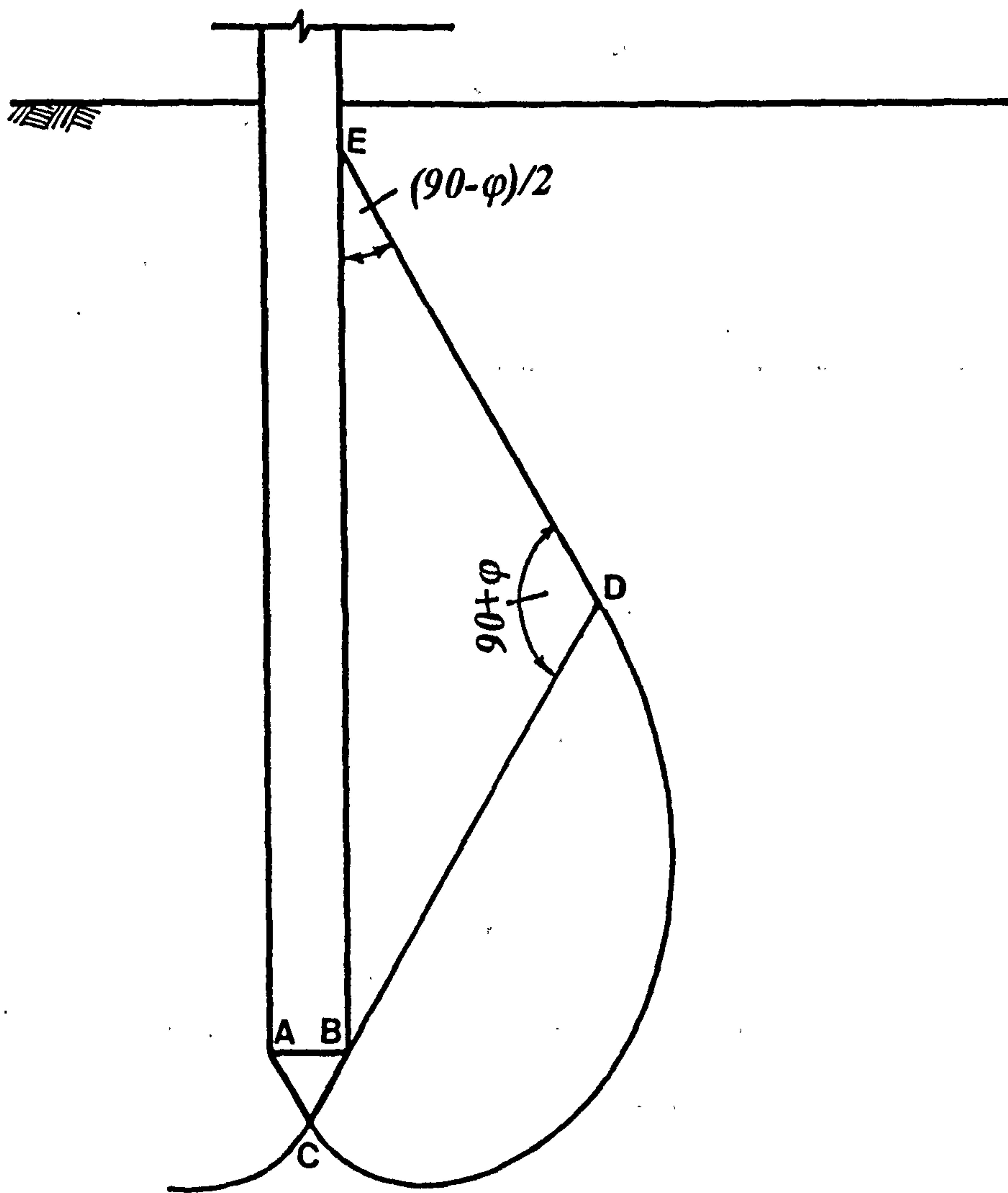


(b)

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(a)



(b)

Fig. 4.7 Witney's foundation failure geometry; (a) Shallow Sinkage (b) Deep sinkage.

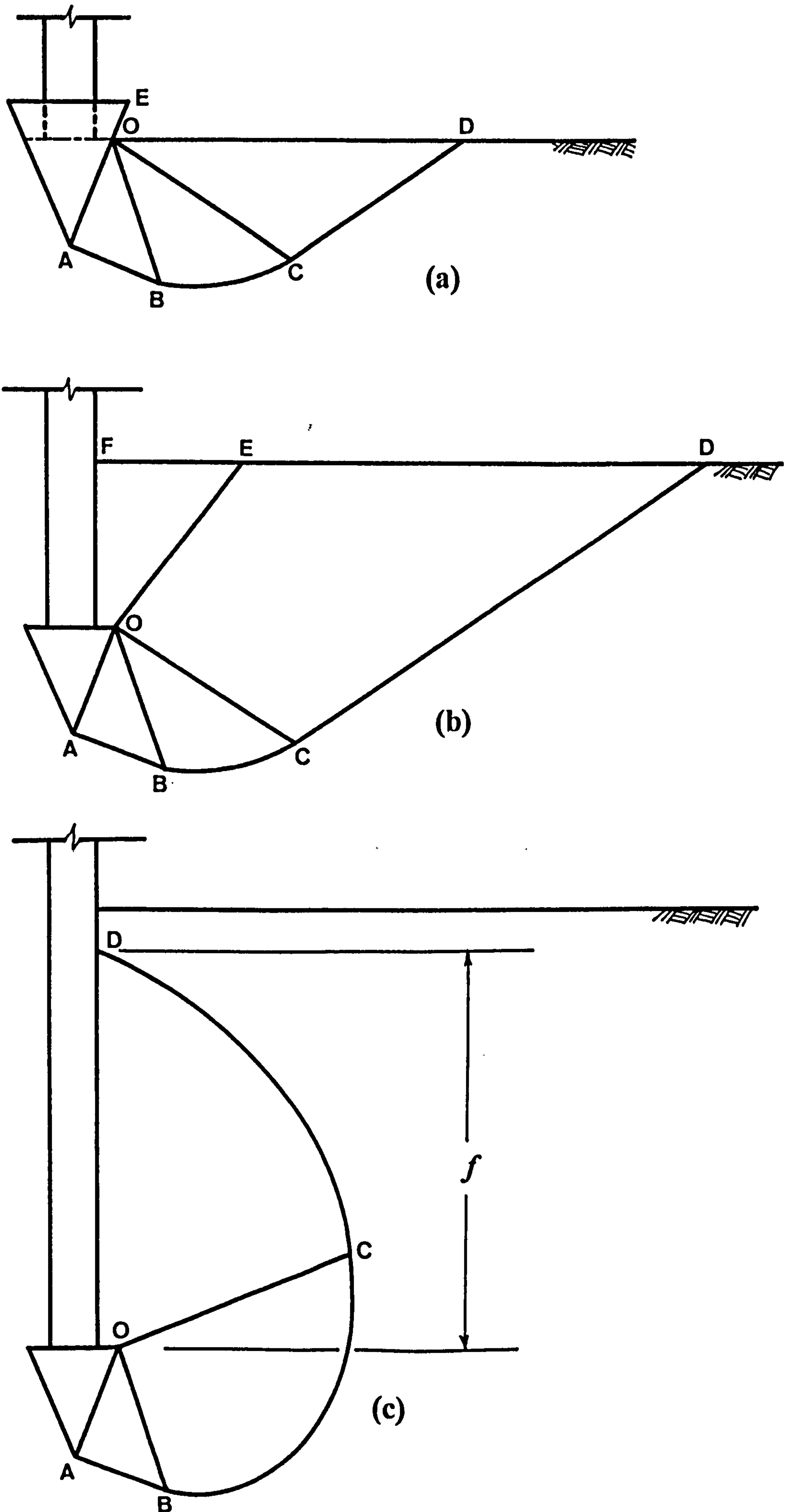


Fig. 4.8 The geometrical phases in the indentation process; (a) Initial phase of indentation. (b) Shallow penetration. (c) Deep penetration.

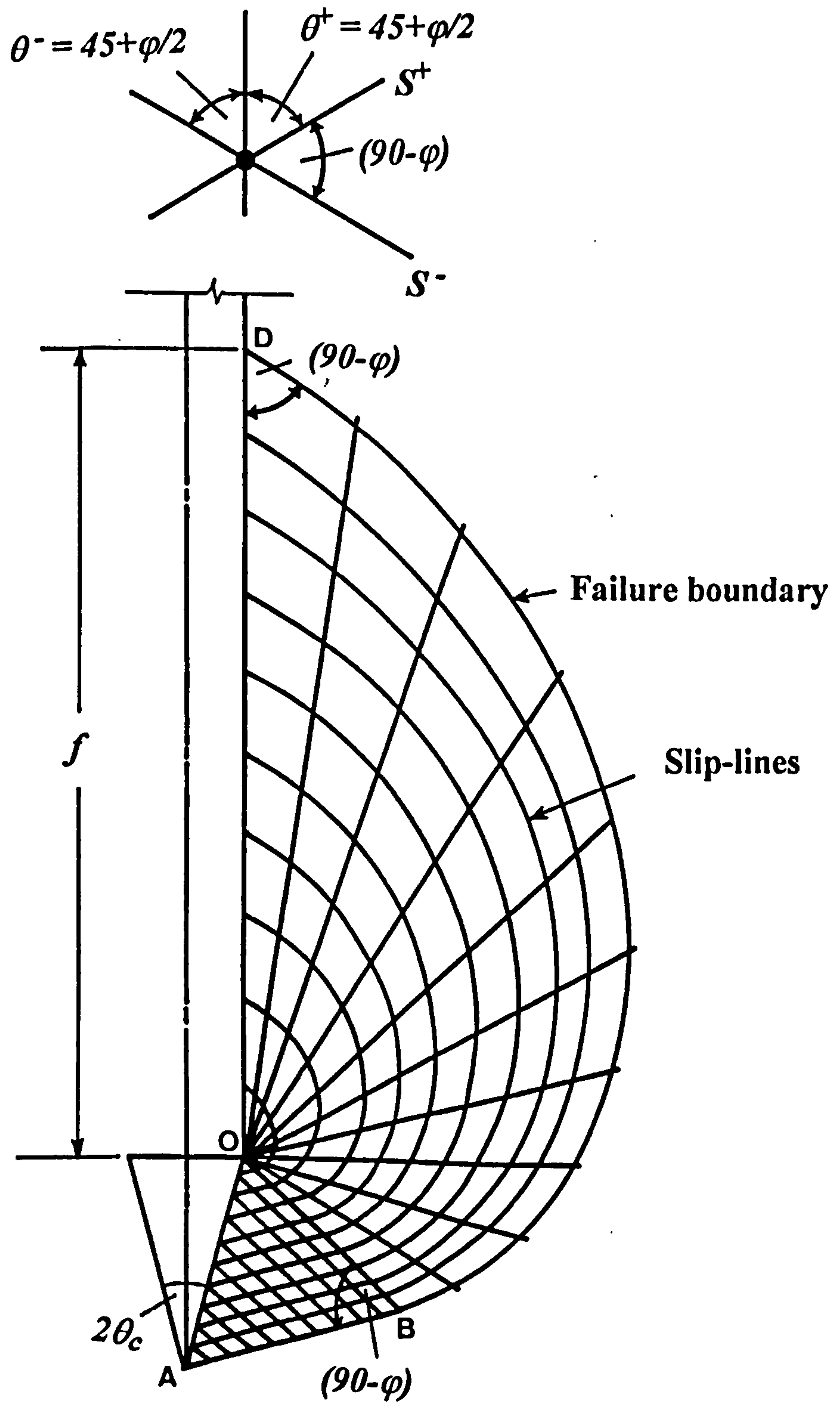


Fig. 4.9 (a) Development of Slip-line field for cone penetrometer analysis
($\varphi = 30^\circ$ and $\delta = 0^\circ$).

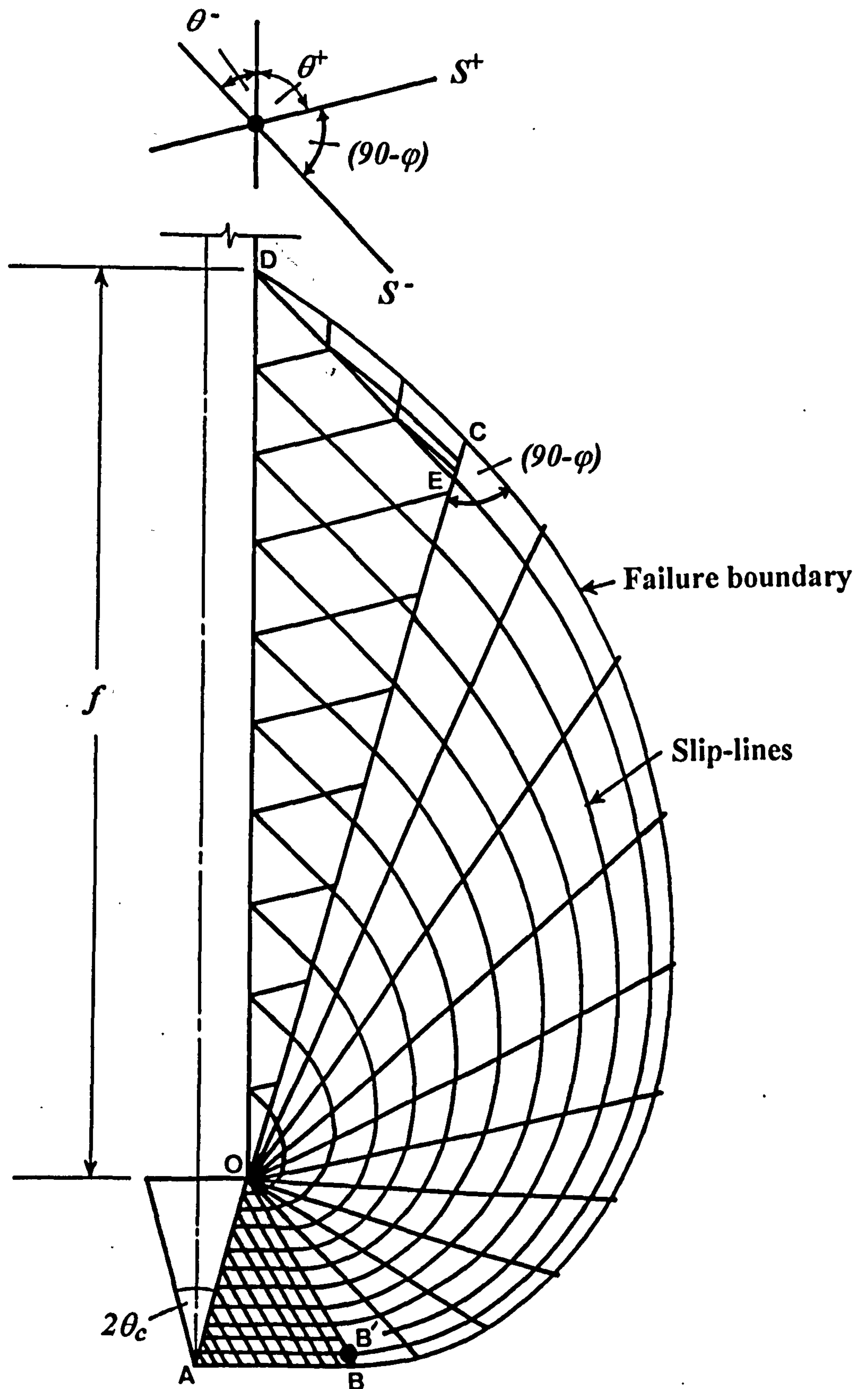


Fig. 4.9 (b) Development of Slip-line field for cone penetrometer analysis ($\phi = 30^\circ$ and $\delta = 10^\circ$).

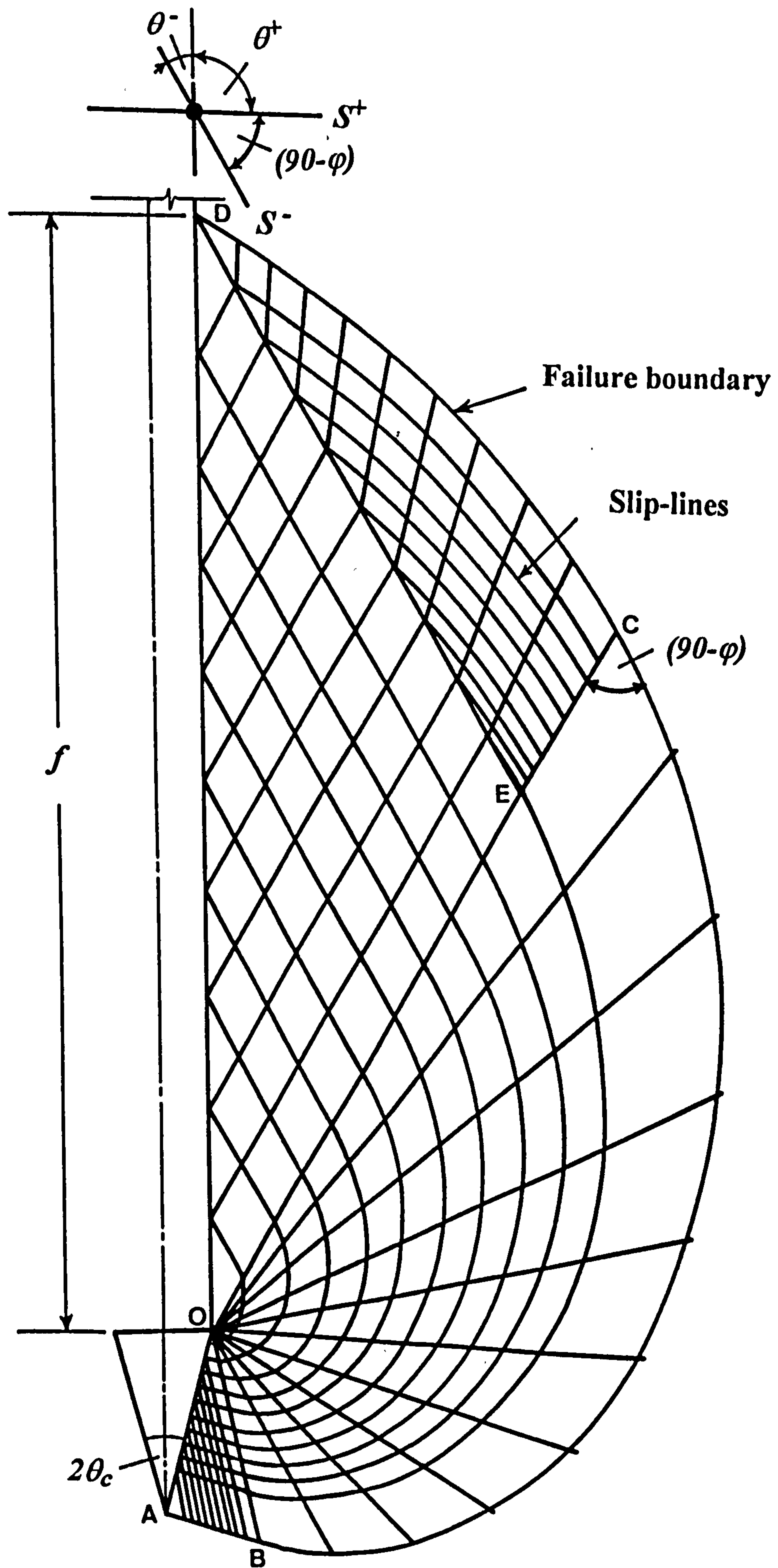


Fig. 4.9 (c) Development of Slip-line field for cone penetrometer analysis ($\phi = 30^\circ$ and $\delta = 20^\circ$).

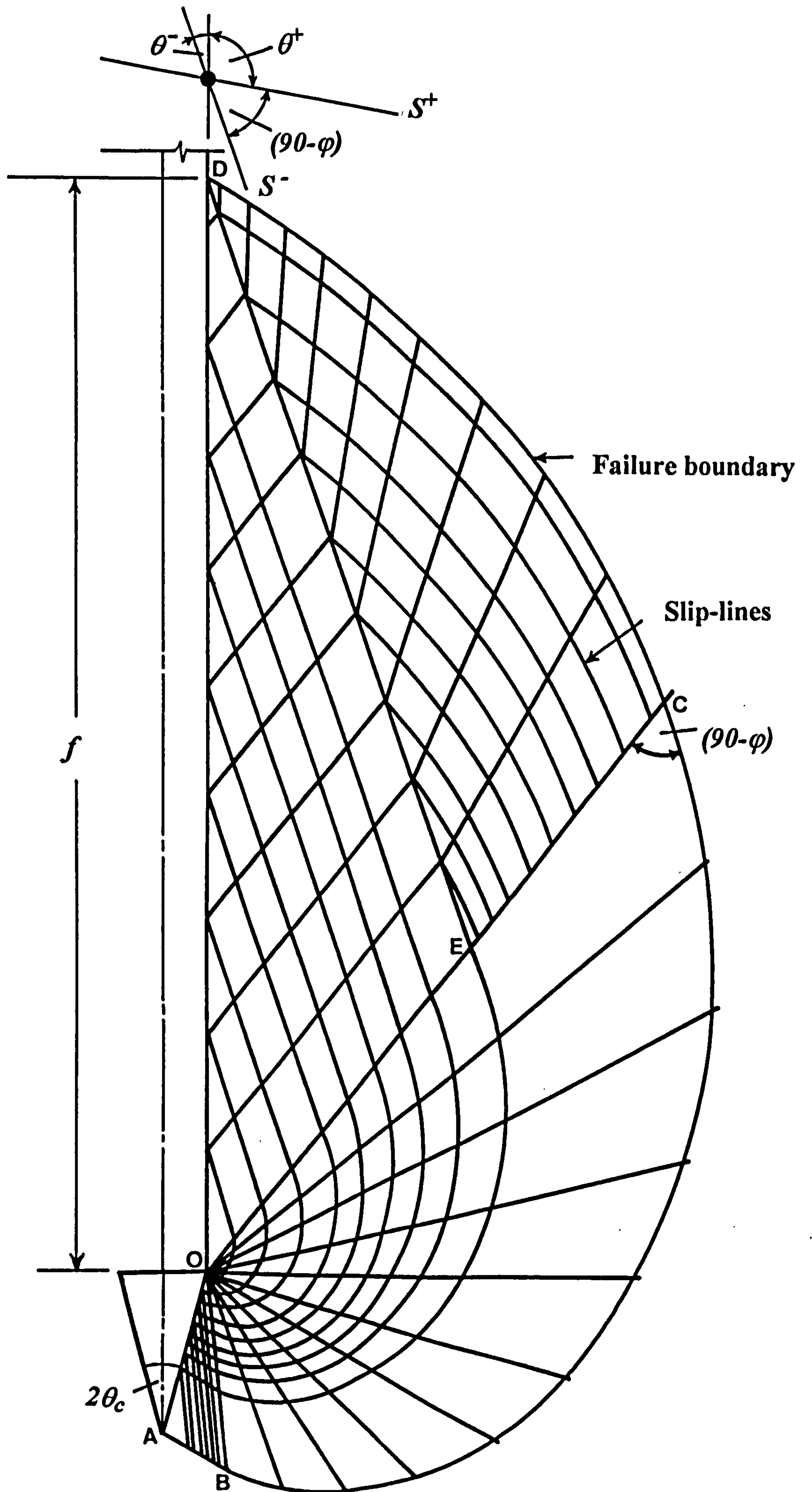


Fig. 4.9 (d) Development of Slip-line field for cone penetrometer analysis ($\varphi = 30^\circ$ and $\delta = 25^\circ$).

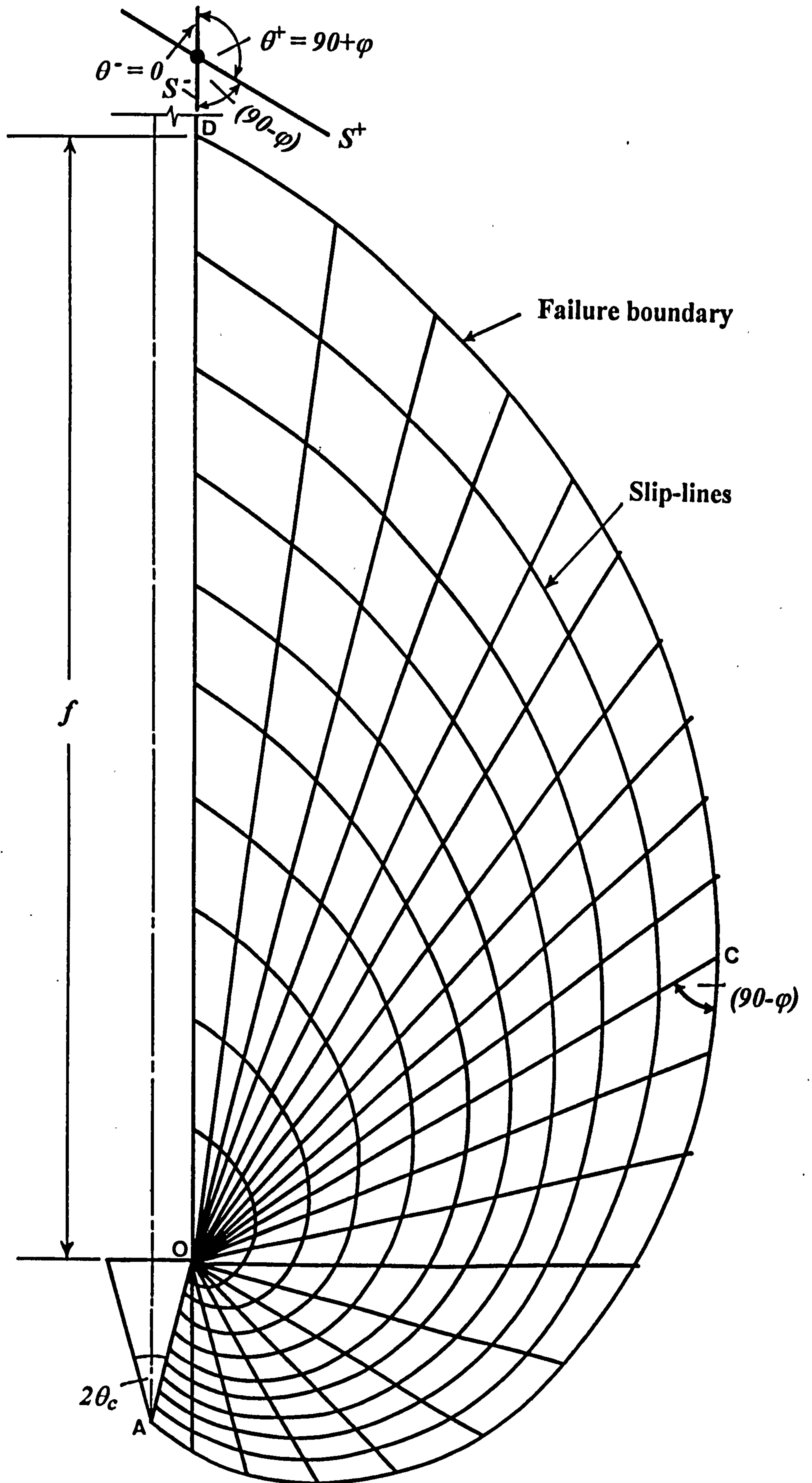


Fig. 4.9 (e) Development of Slip-line field for cone penetrometer analysis ($\varphi = \delta = 30^\circ$).

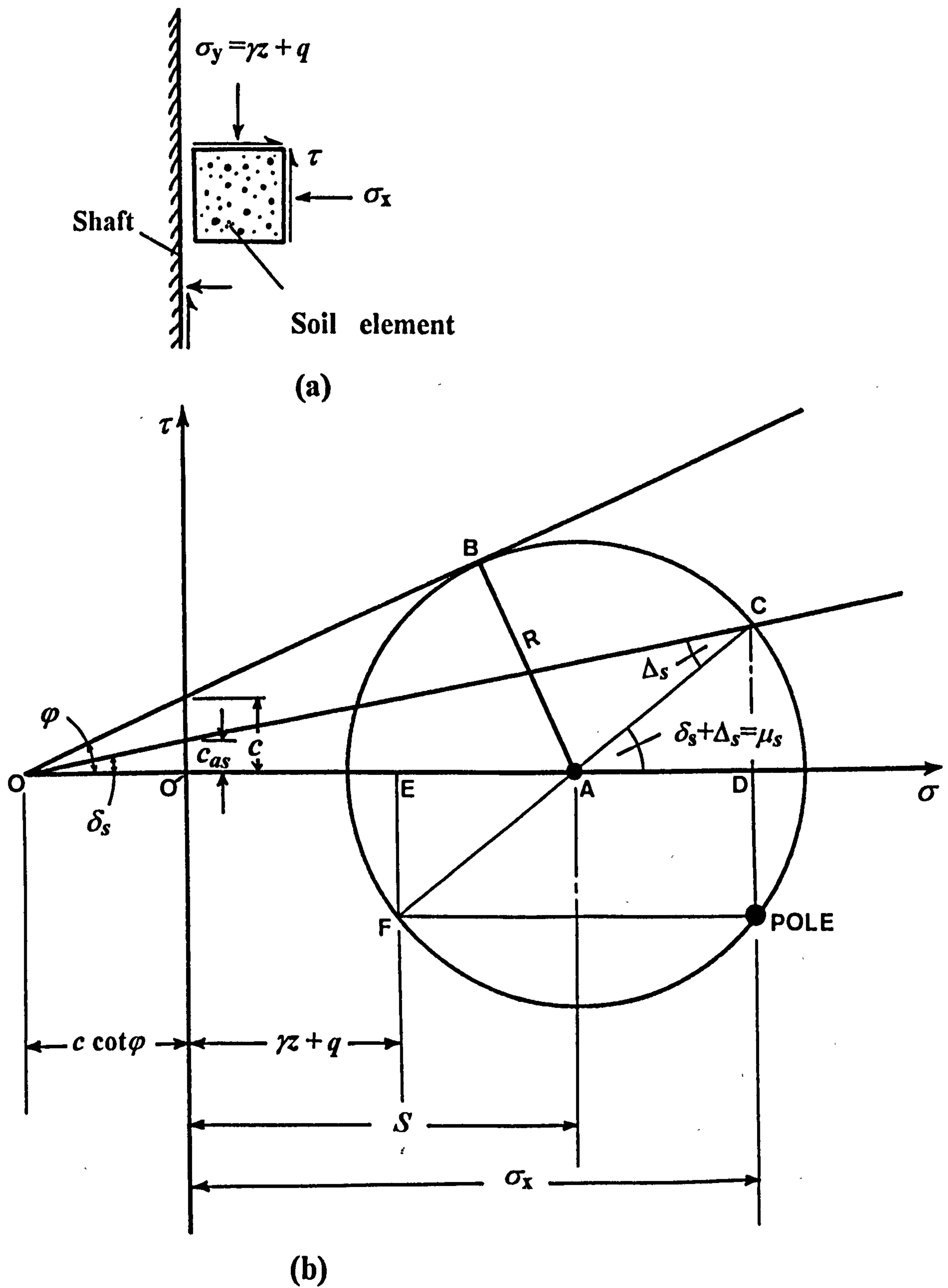


Fig. 4.11 (a) Equilibrium of soil element. (b) Mohr's diagram for shaft boundary.

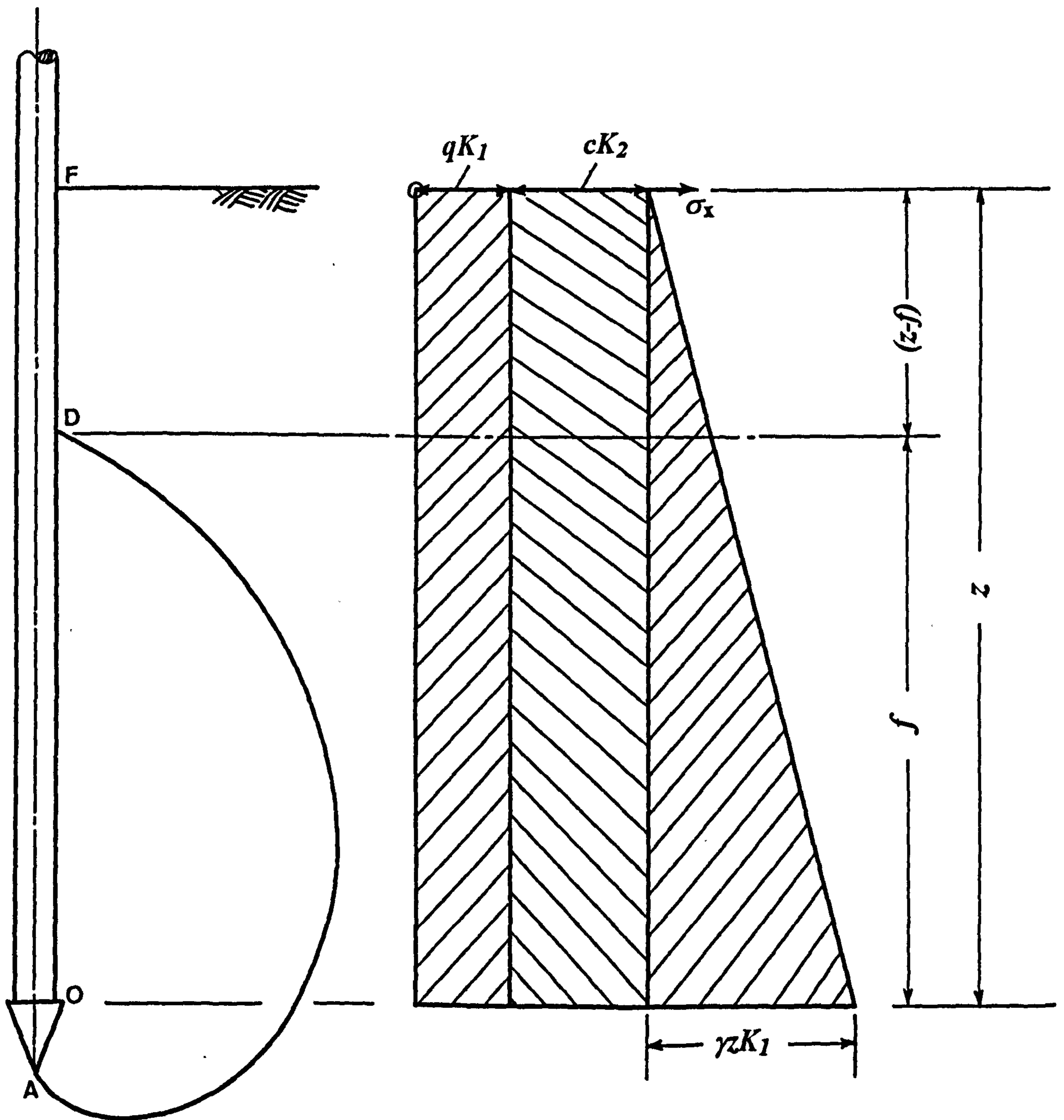


Fig. 4.12 Variation of three stress components with depth of penetration.

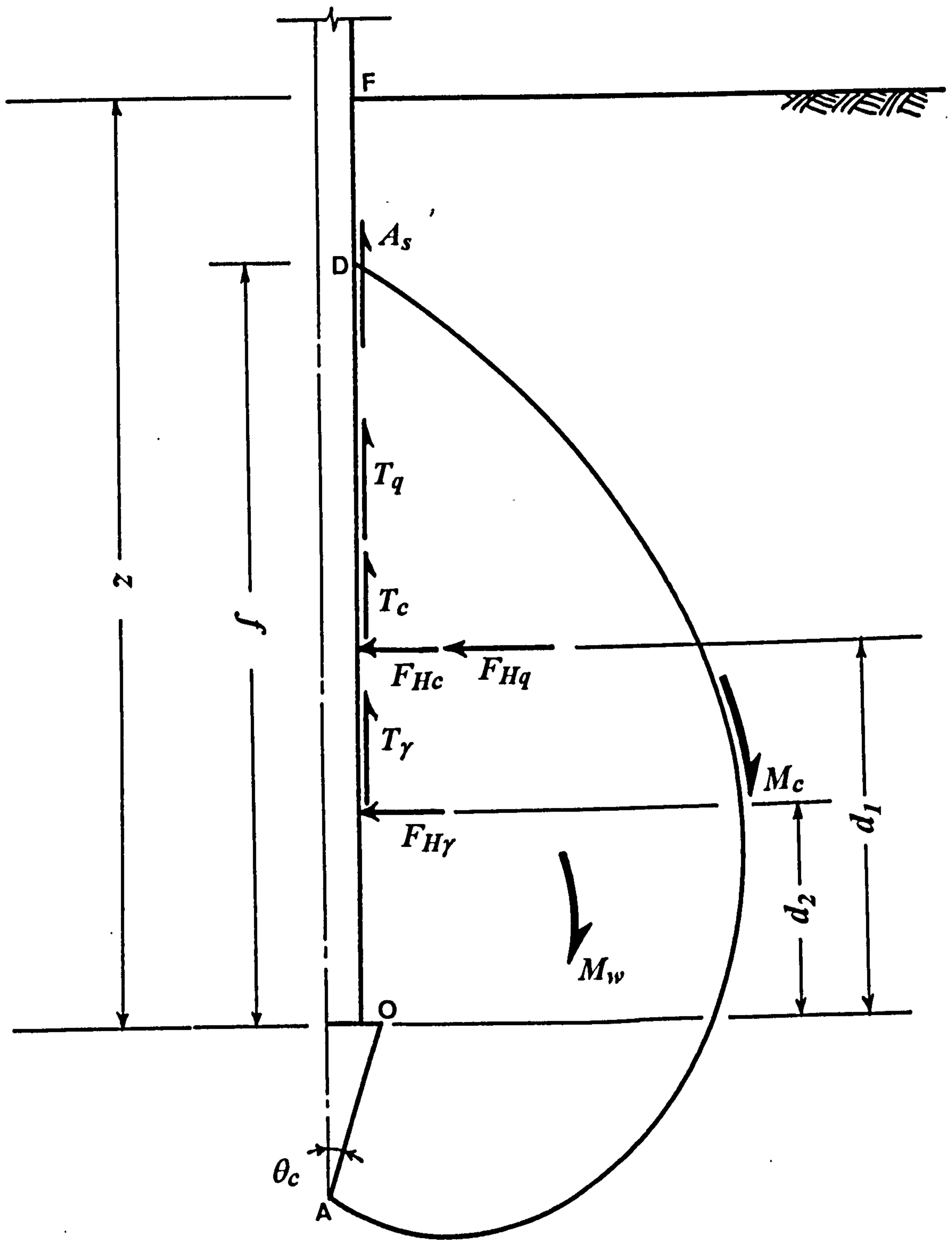


Fig. 4.13 Forces acting on shaft surface.

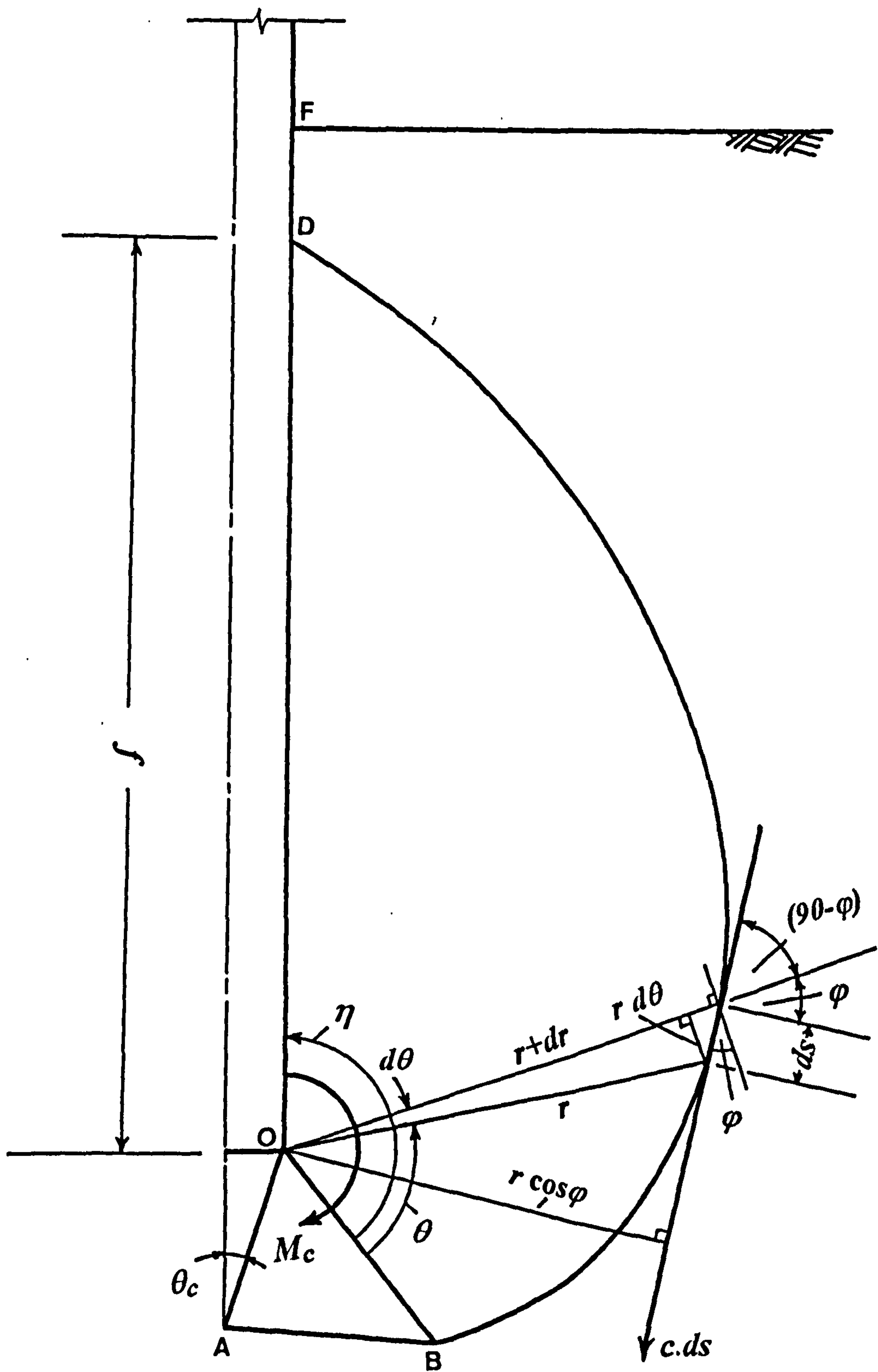


Fig. 4.14 Determination of Cohesive moment (M_c) along failure boundary.

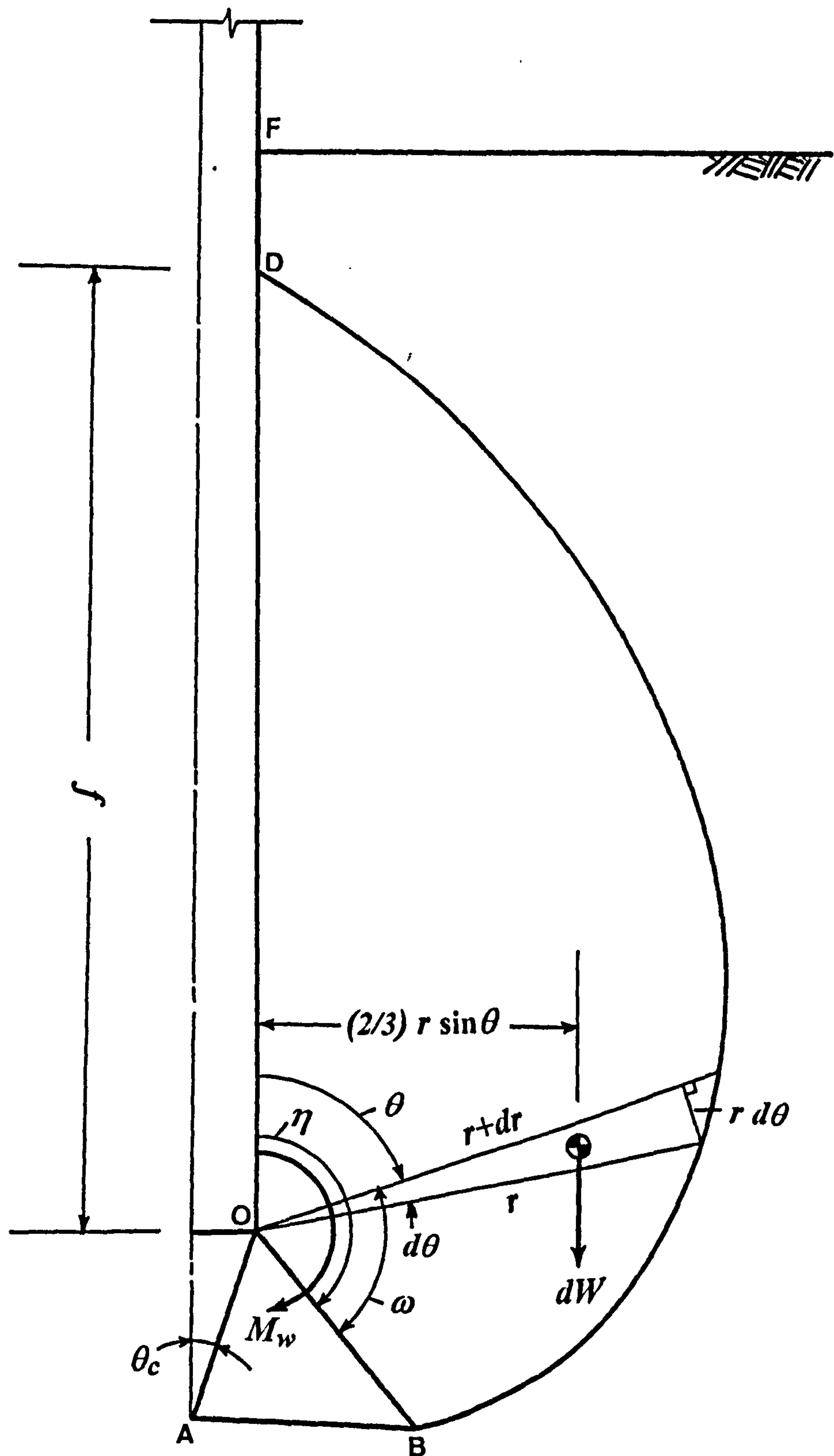


Fig. 4.15 Determination of Gravitational moment (M_w) for rupture surface.

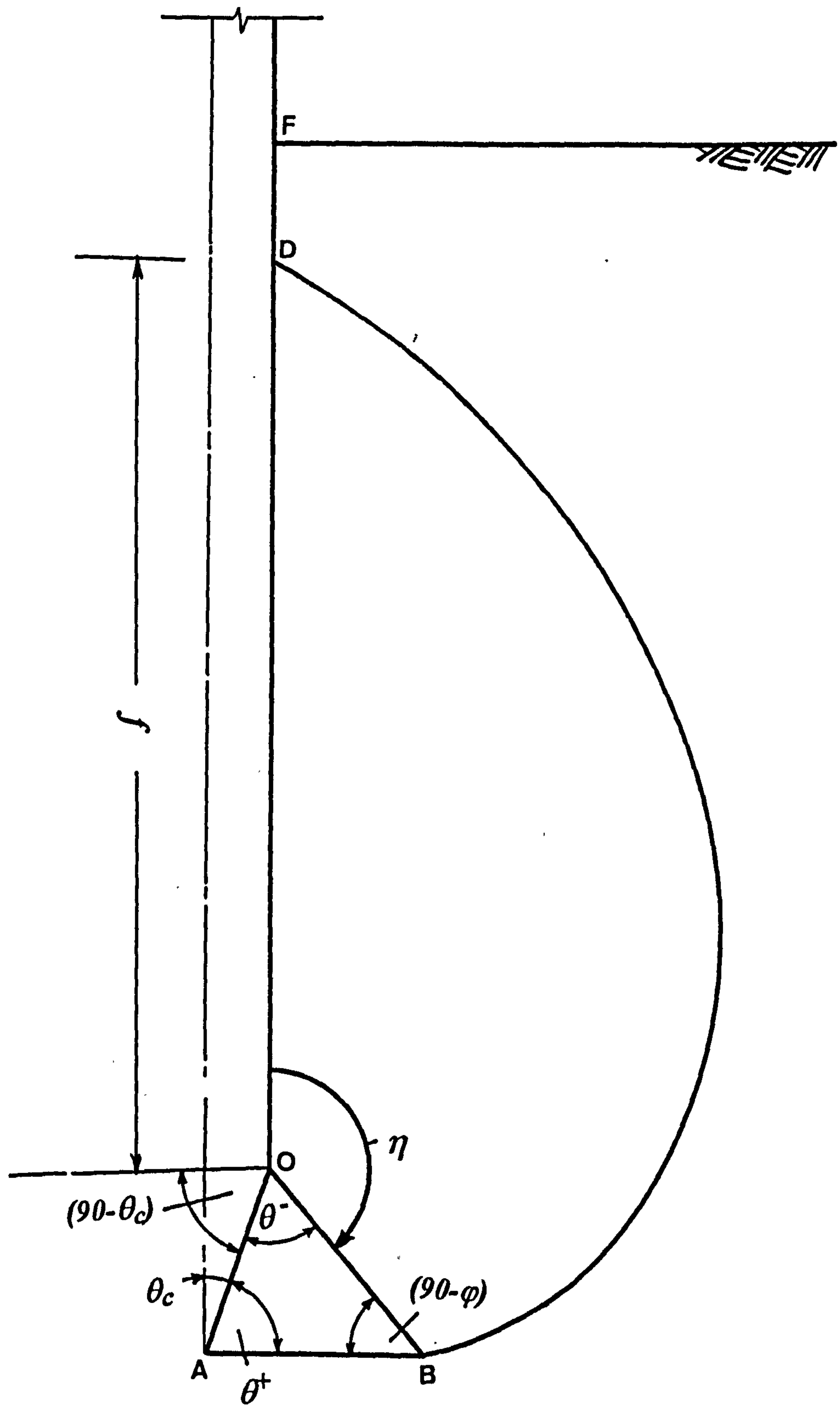


Fig. 4.16 Determination of angle (η) and the critical depth limit (f).

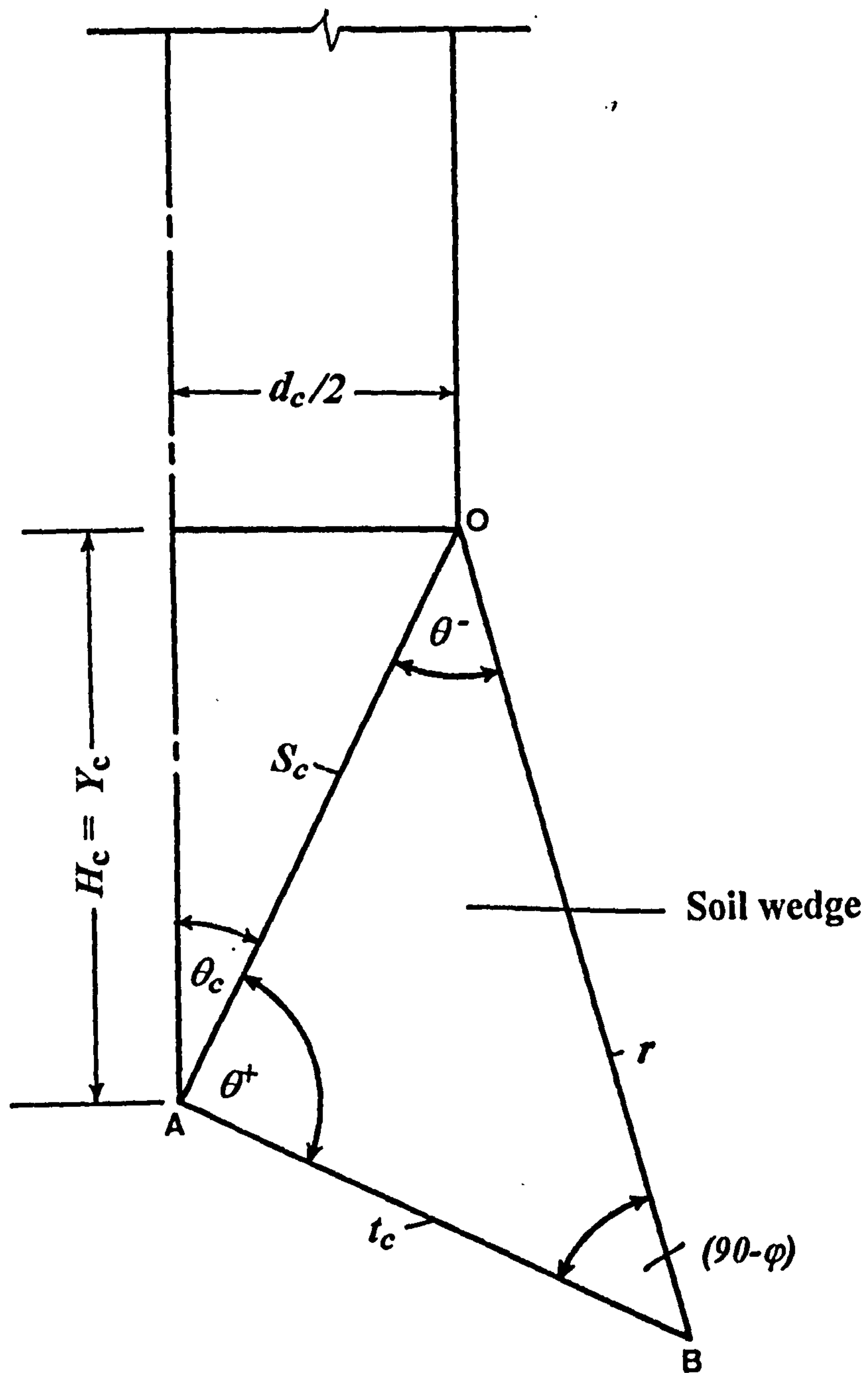


Fig. 4.17 Calculation of interface length (r), S_c and t_c .

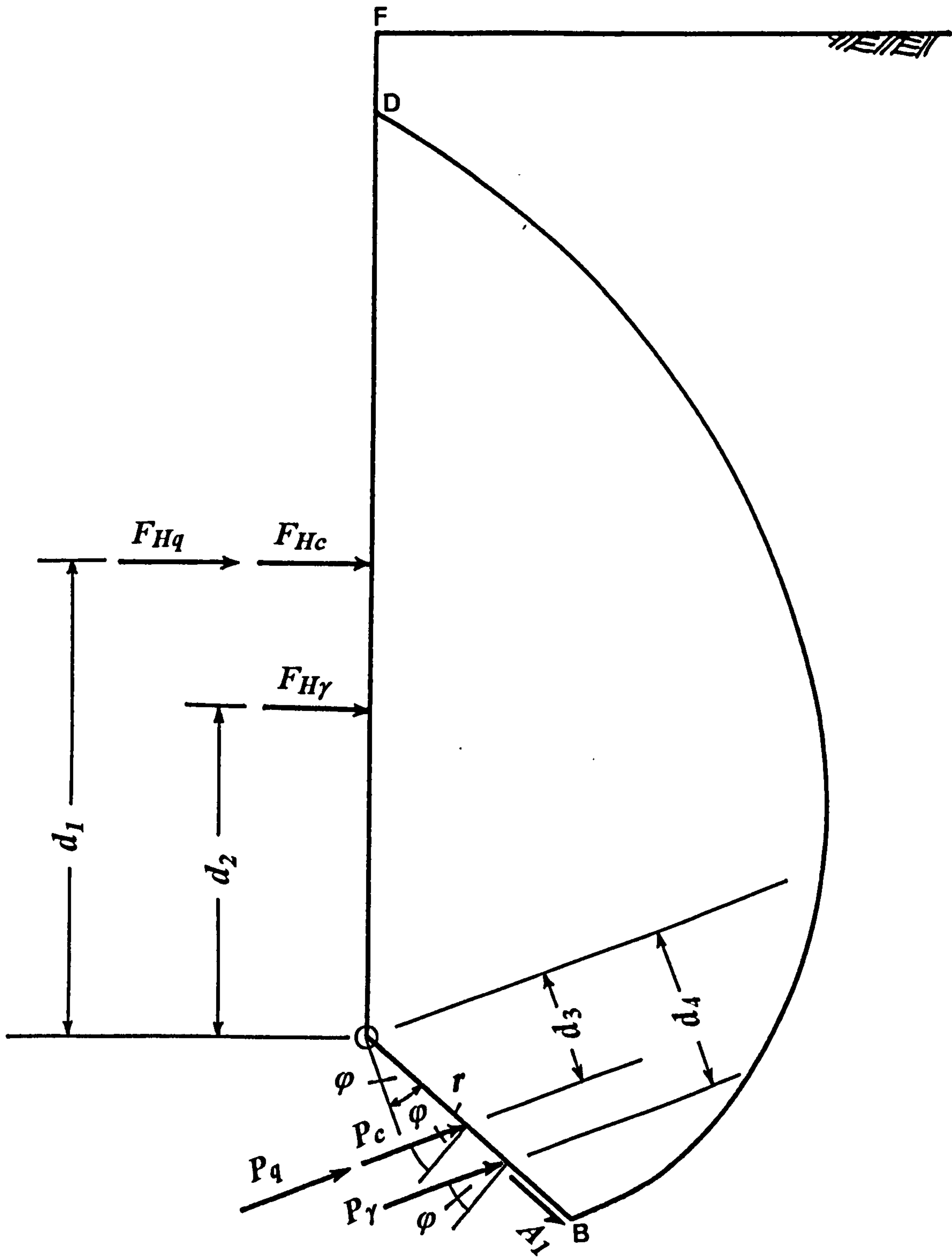


Fig. 4.18 Determination of moment arms.

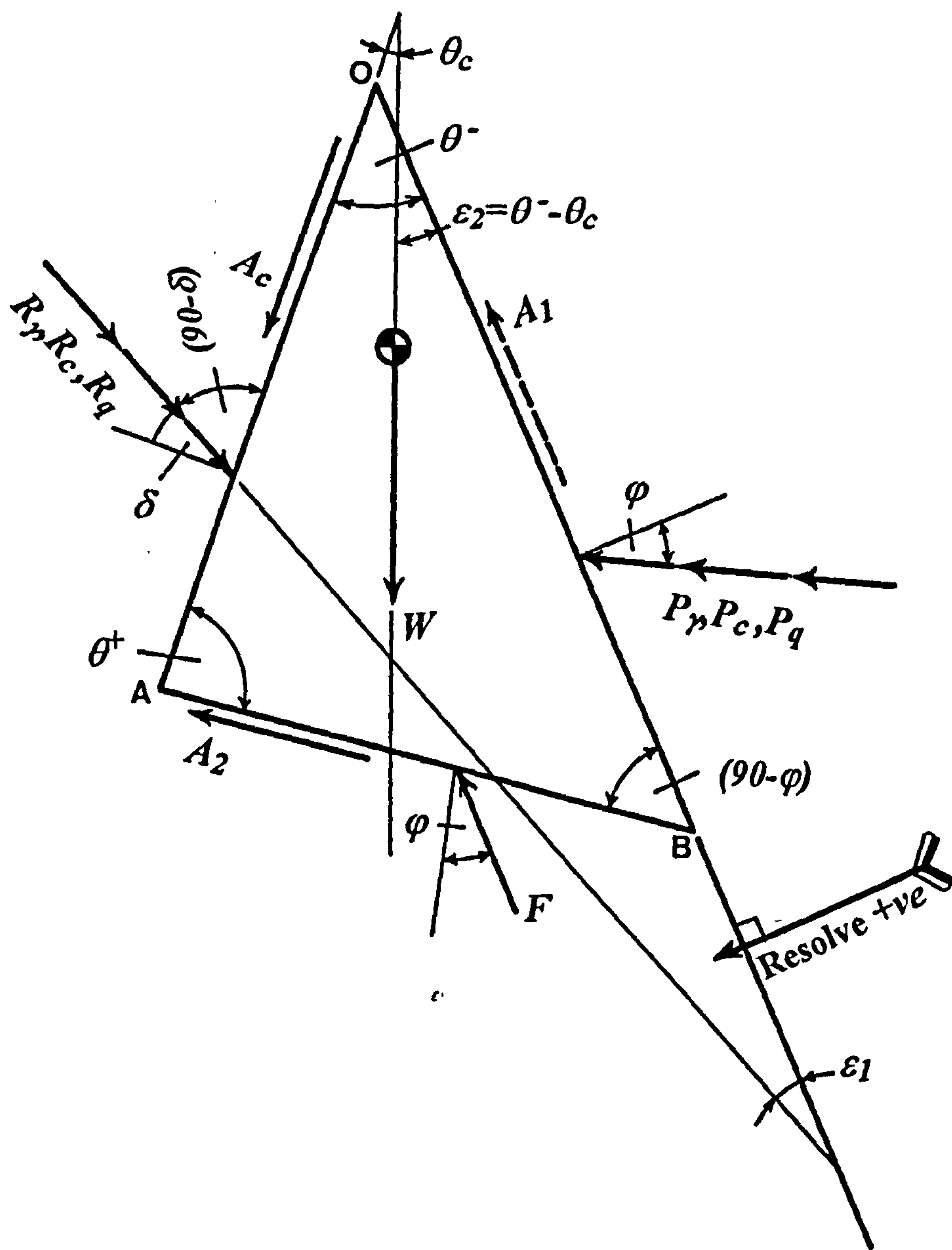


Fig. 4.20 Equilibrium of forces acting on the soil wedge.

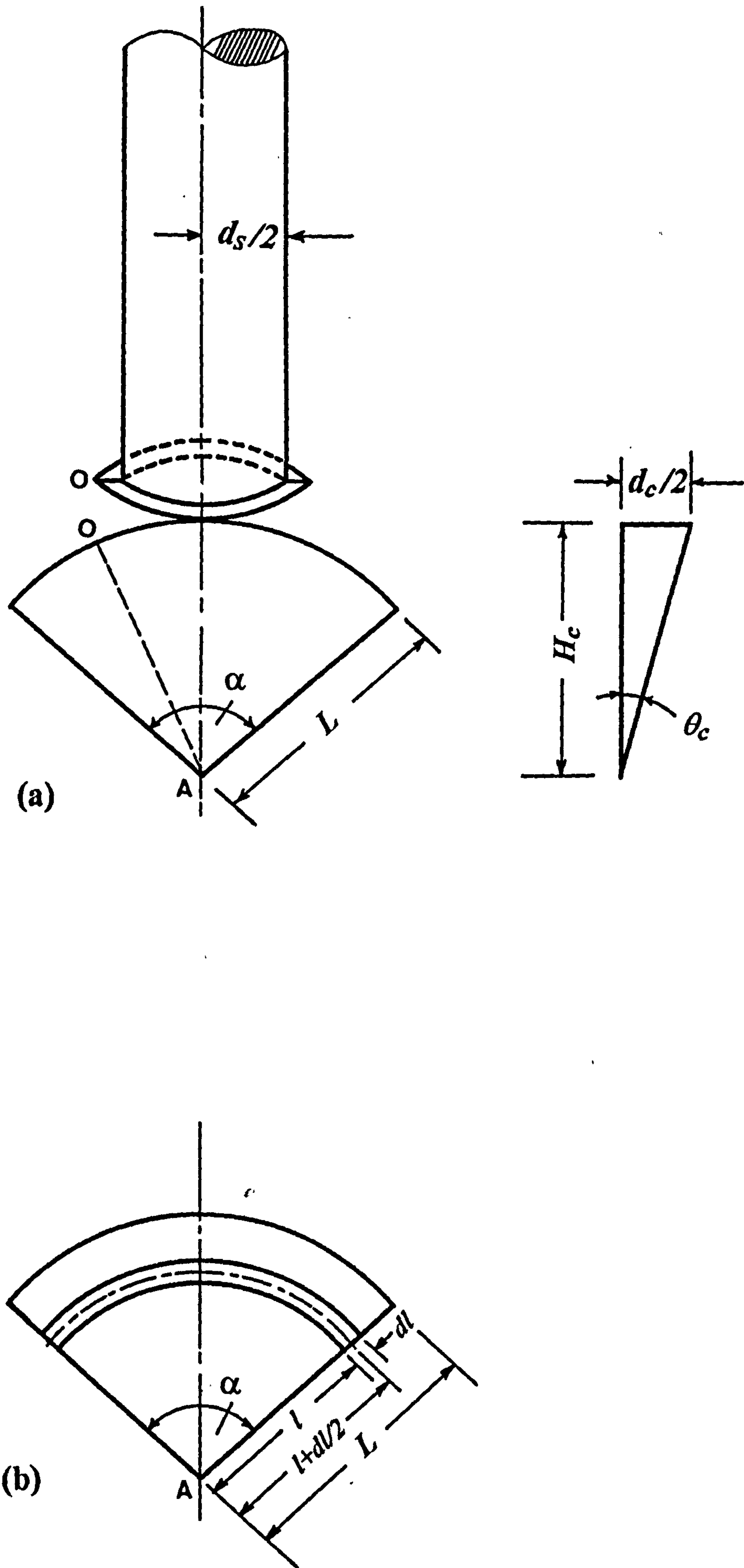
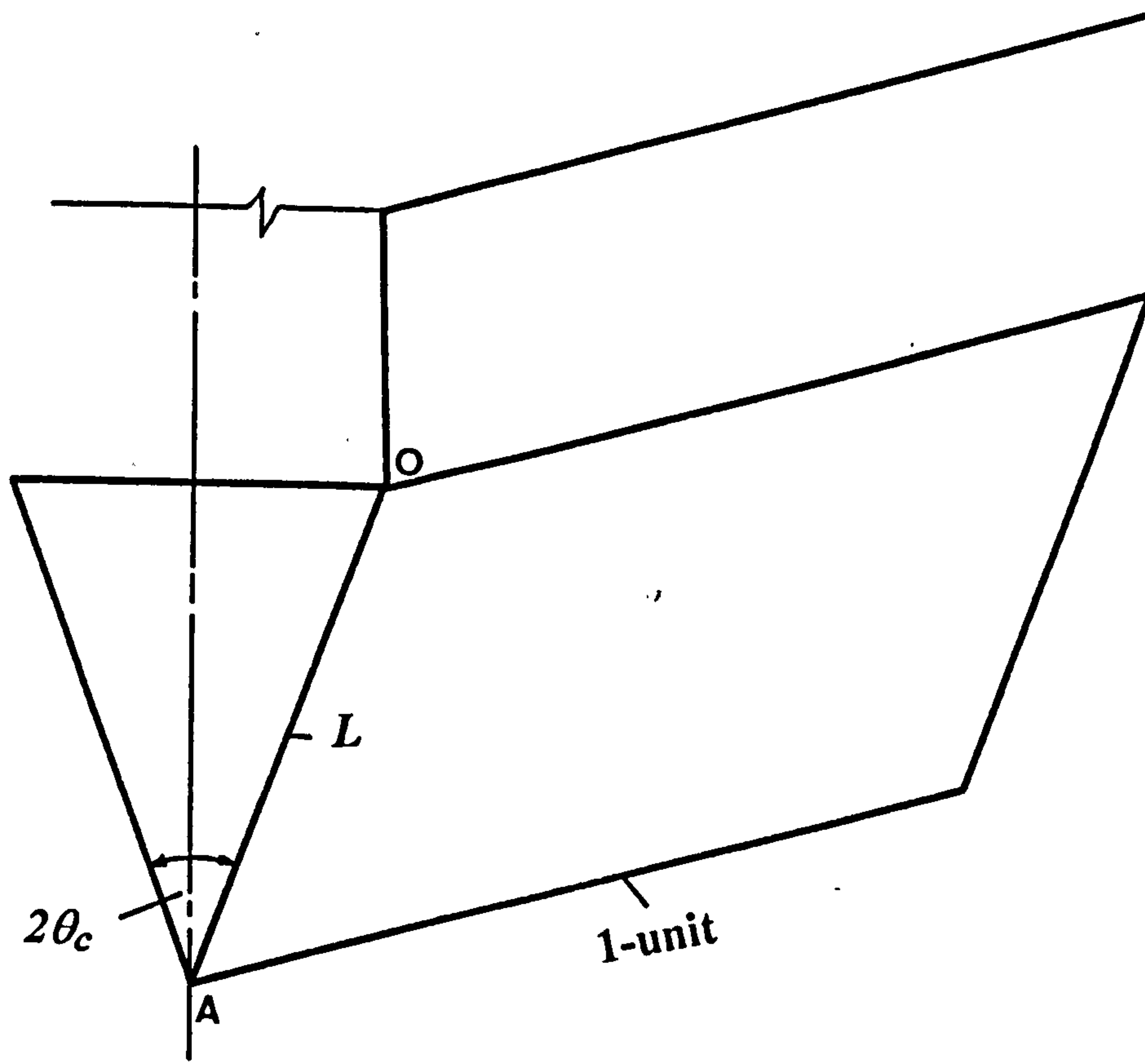
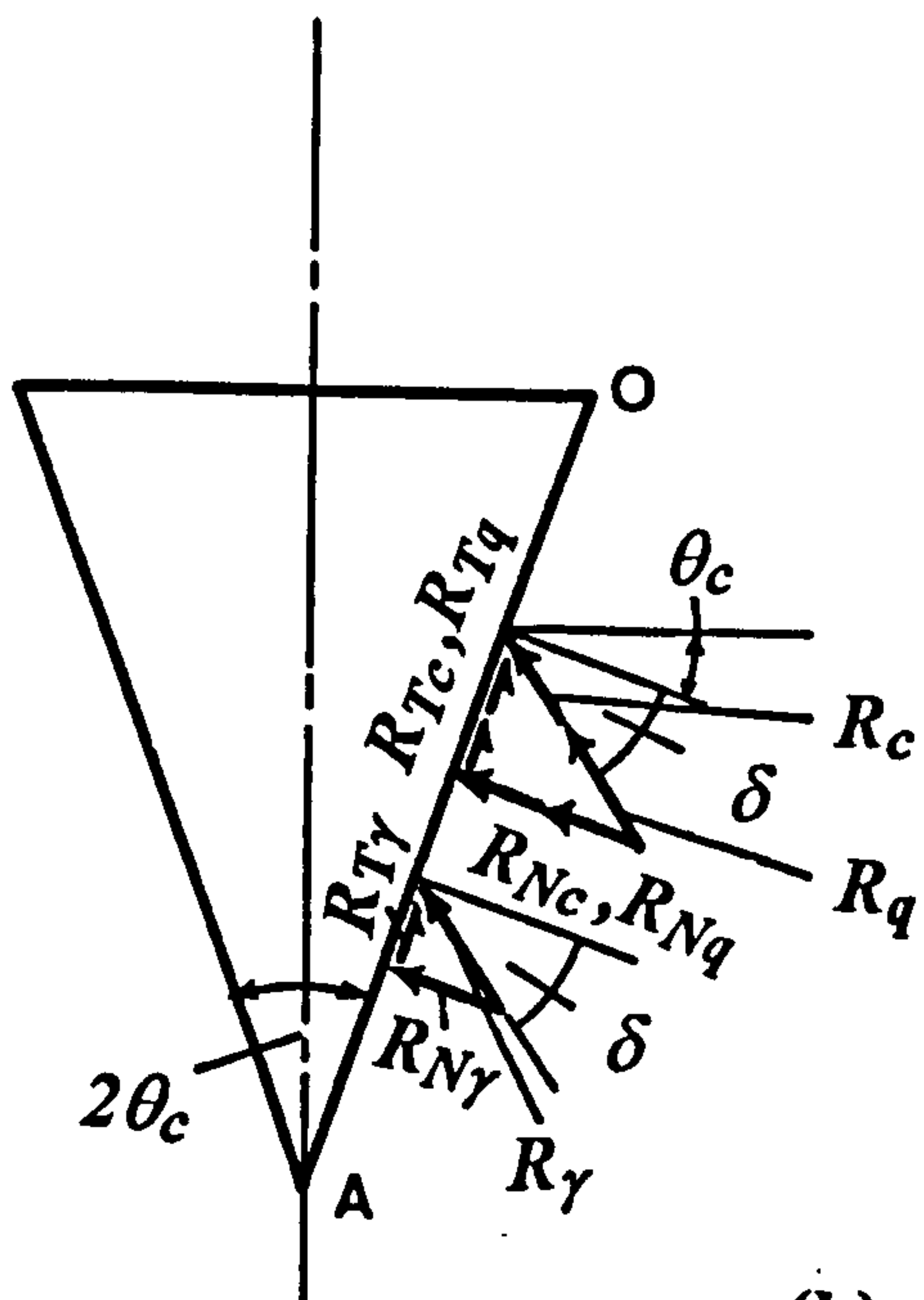


Fig. 4.21 Angle (α) of the opened-up cone.

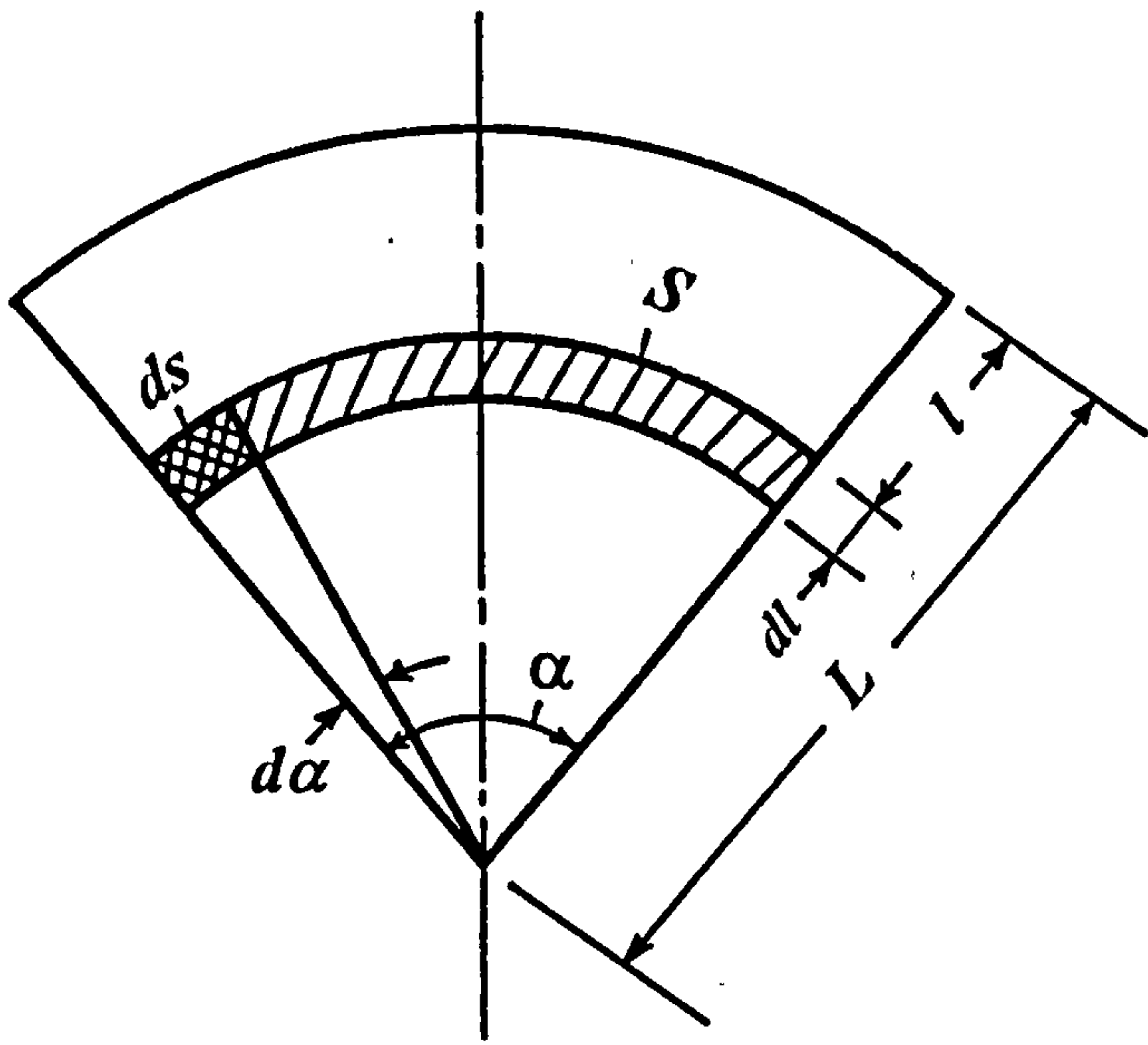


(a)

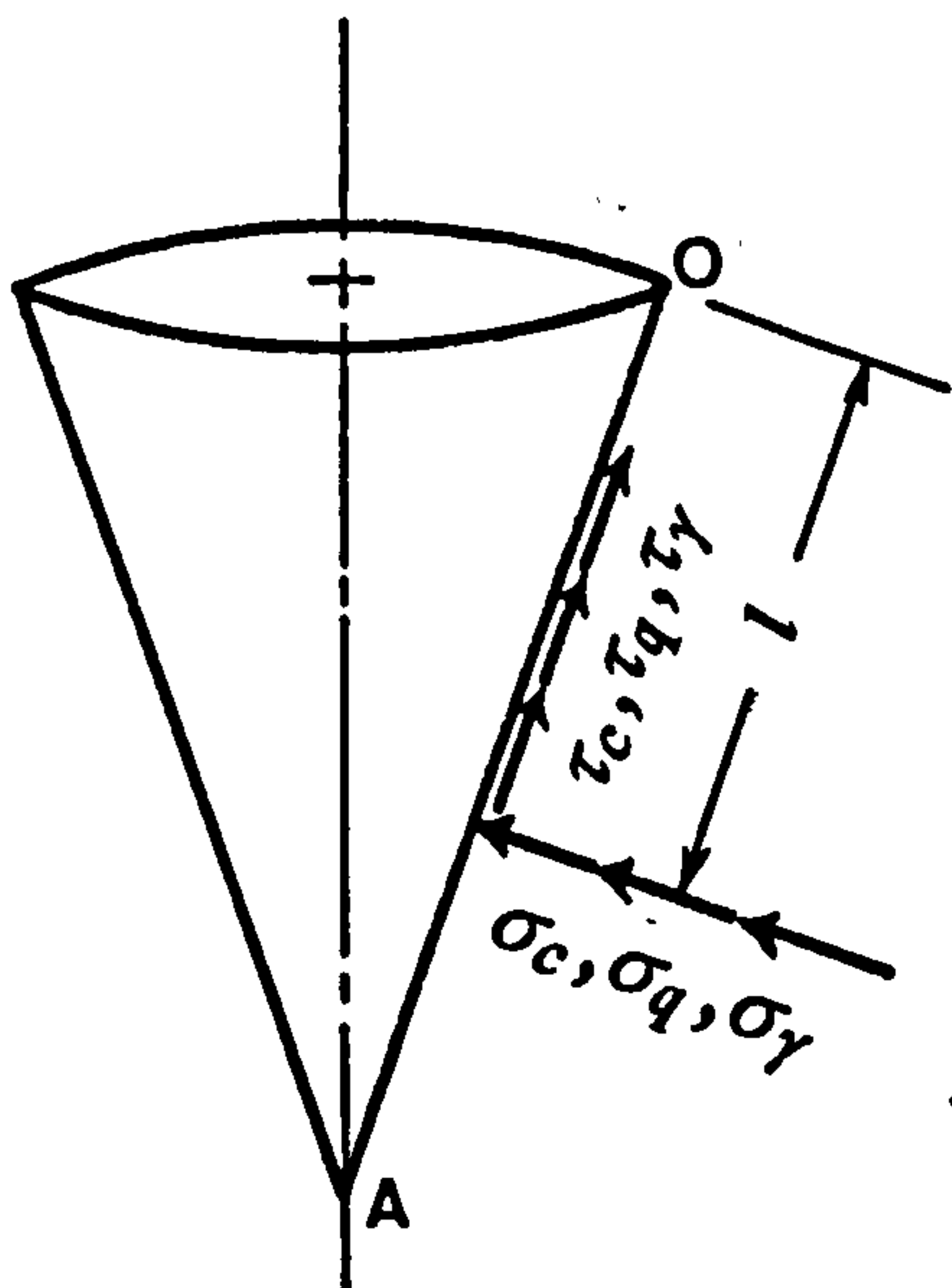


(b)

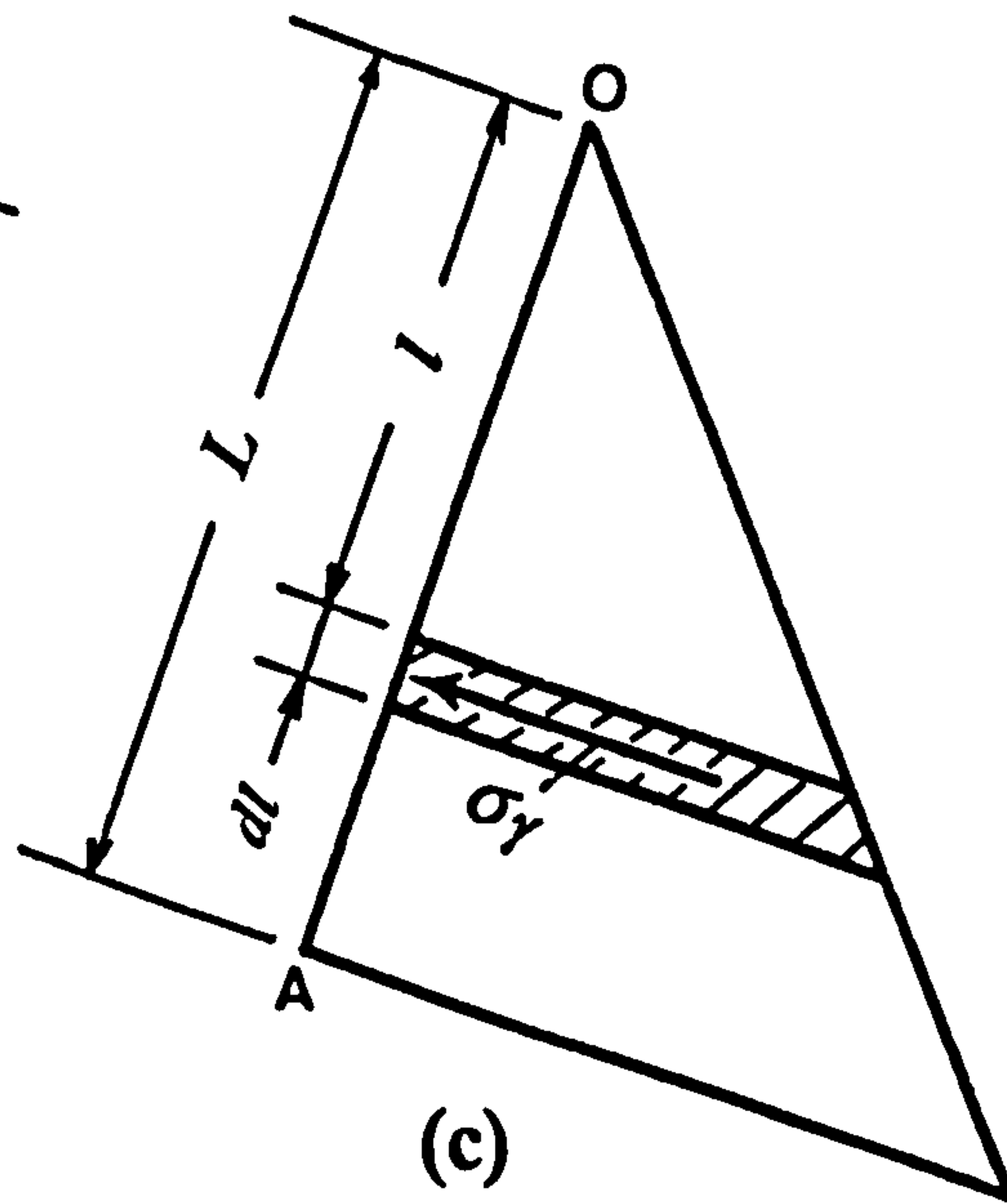
Fig. 4.22 The force components acting on the cone surface.



(a)

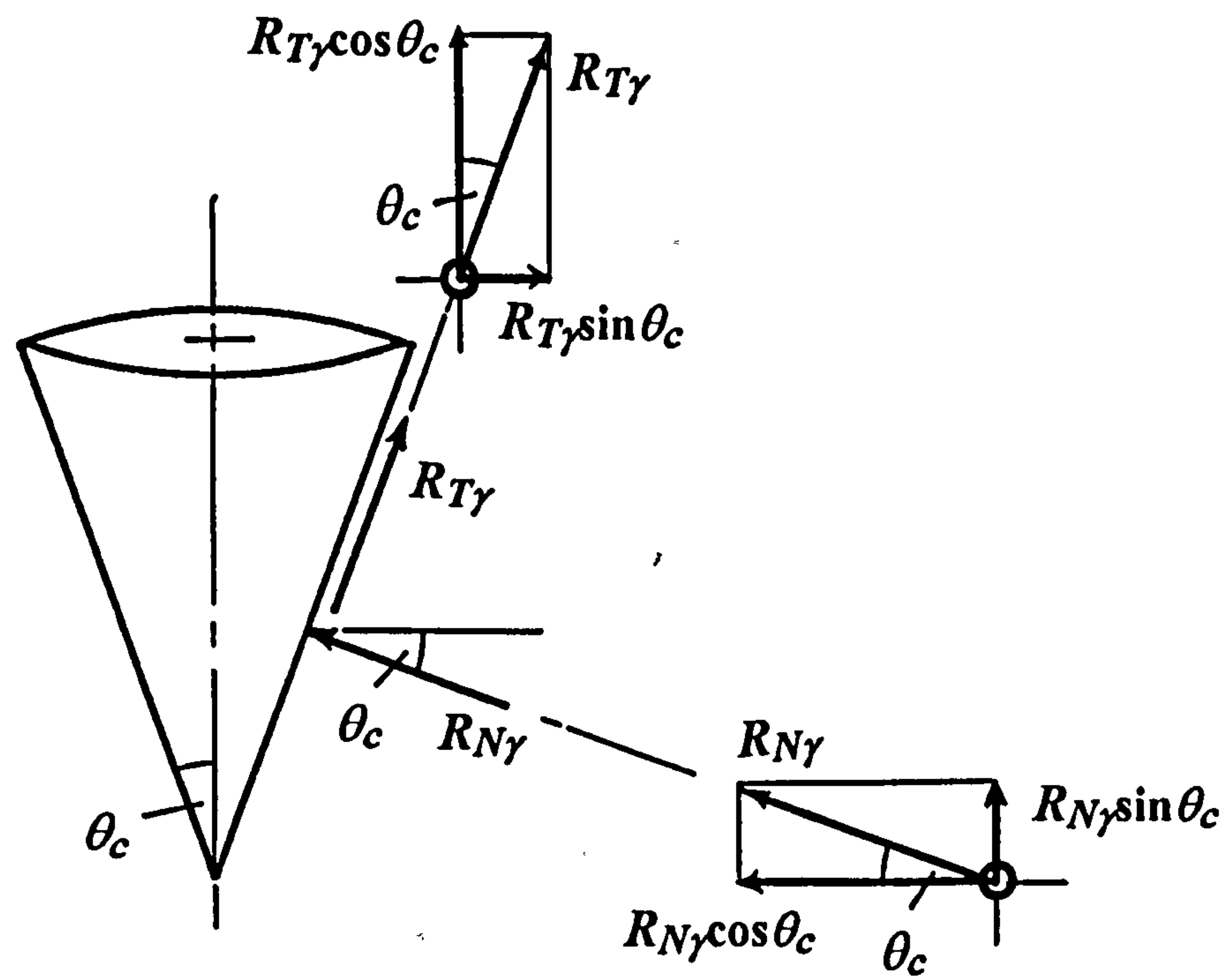


(b)

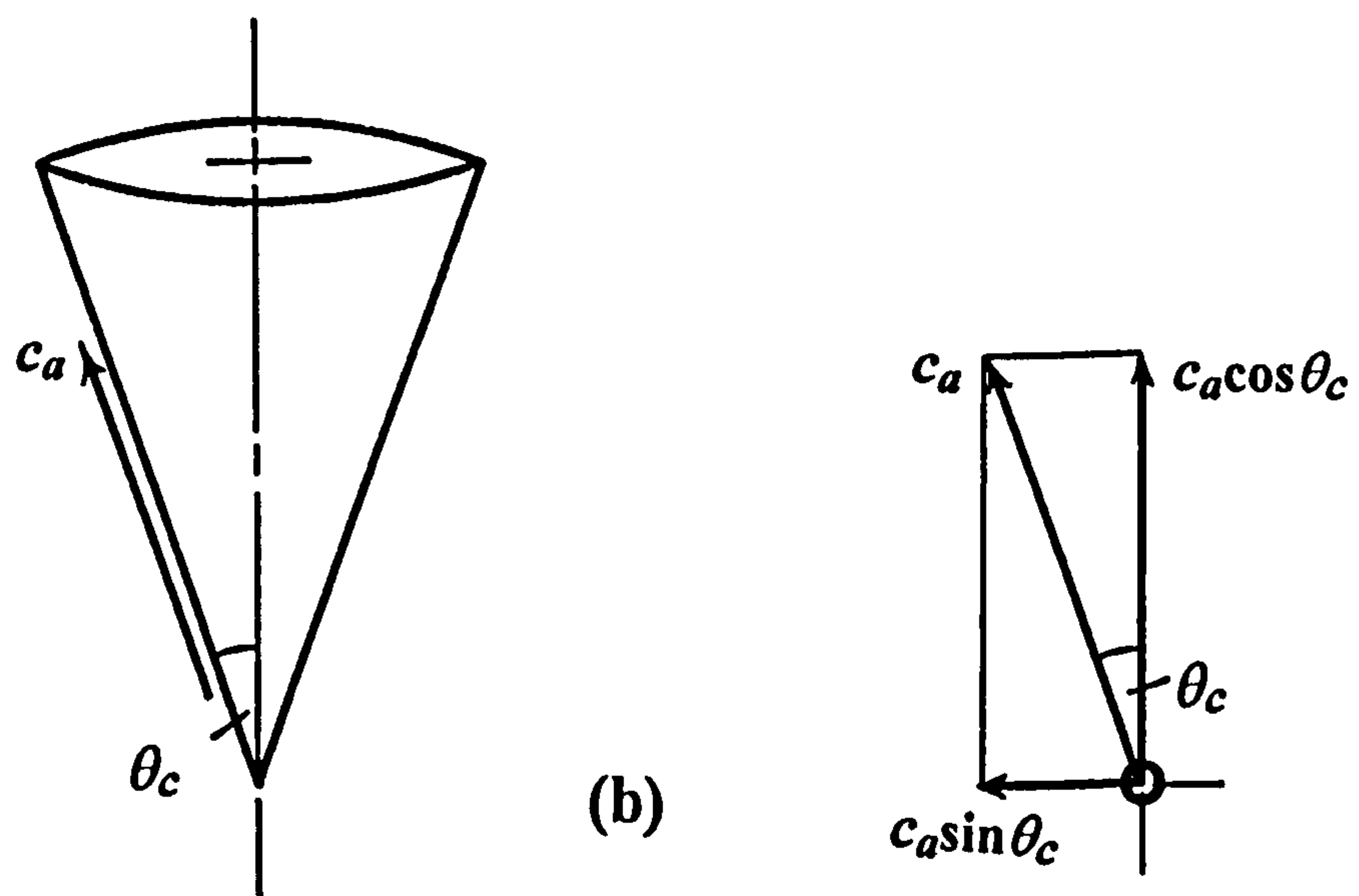


(c)

Fig. 4.23 (a) and (b) The elemental stress on cone. (c) Calculation for gravitational stress.



(a)



(b)

Fig. 4.24 (a) Determination of the vertical component of force. (b) Determination of the vertical component of adhesion.

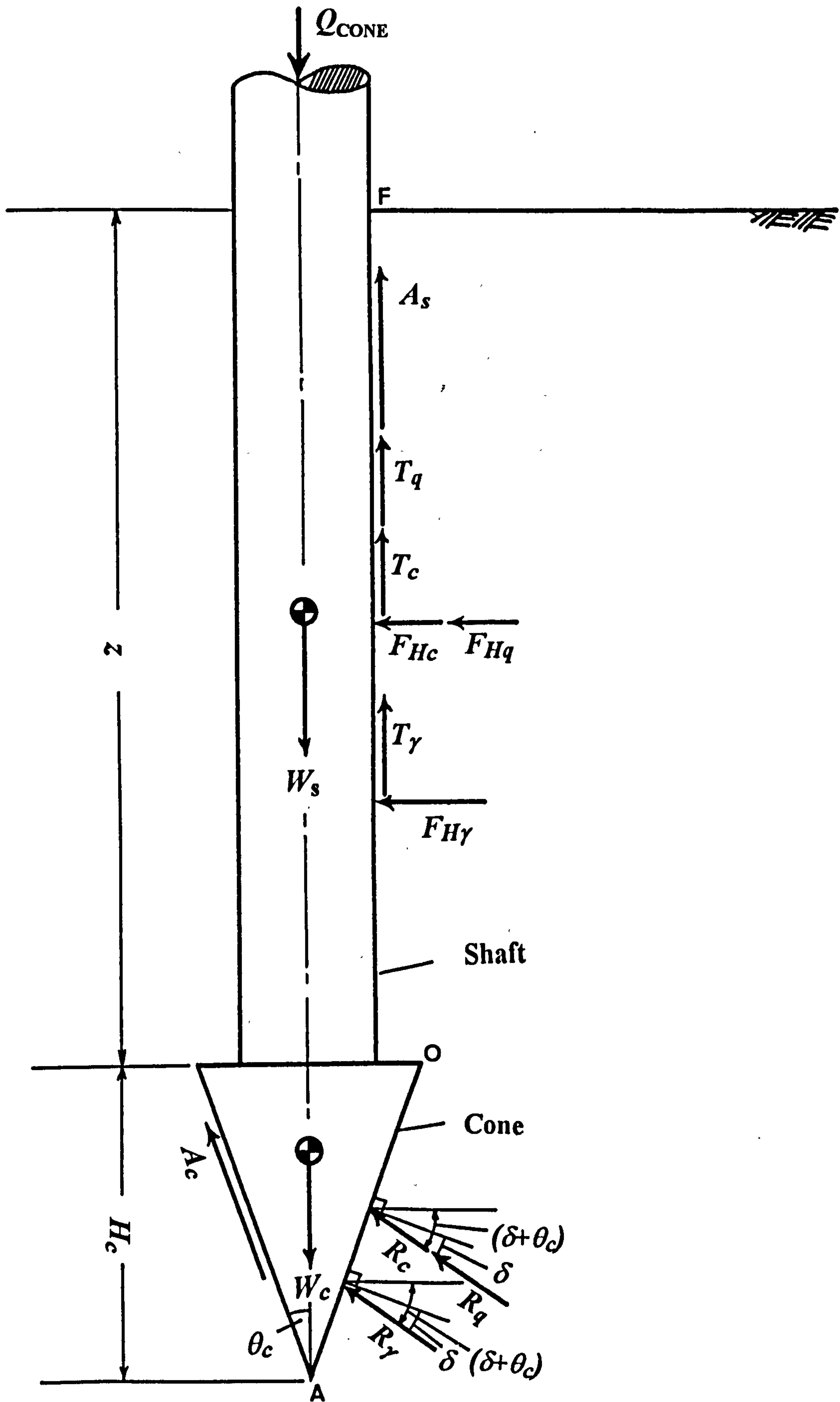


Fig. 4.25 Equilibrium of Cone and Shaft.

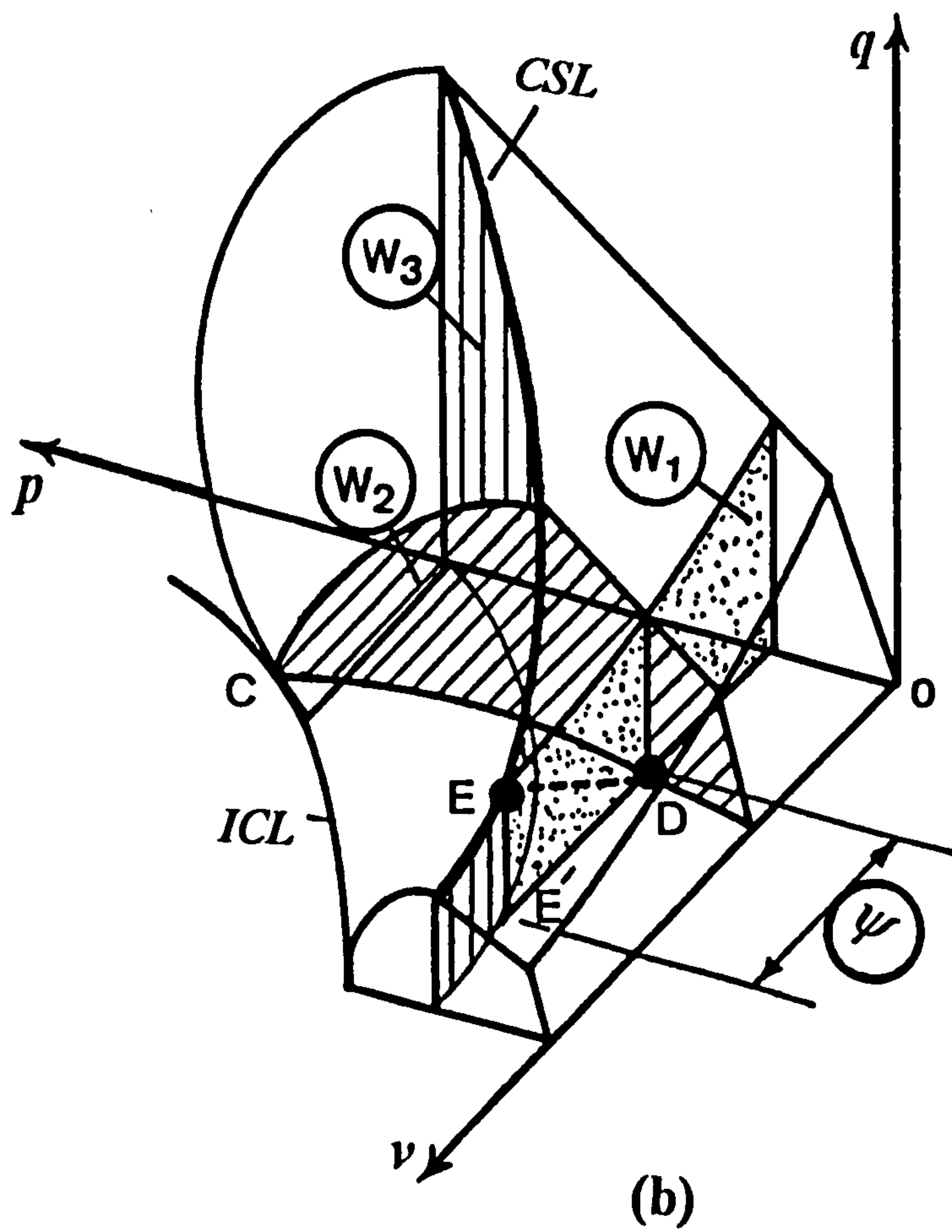
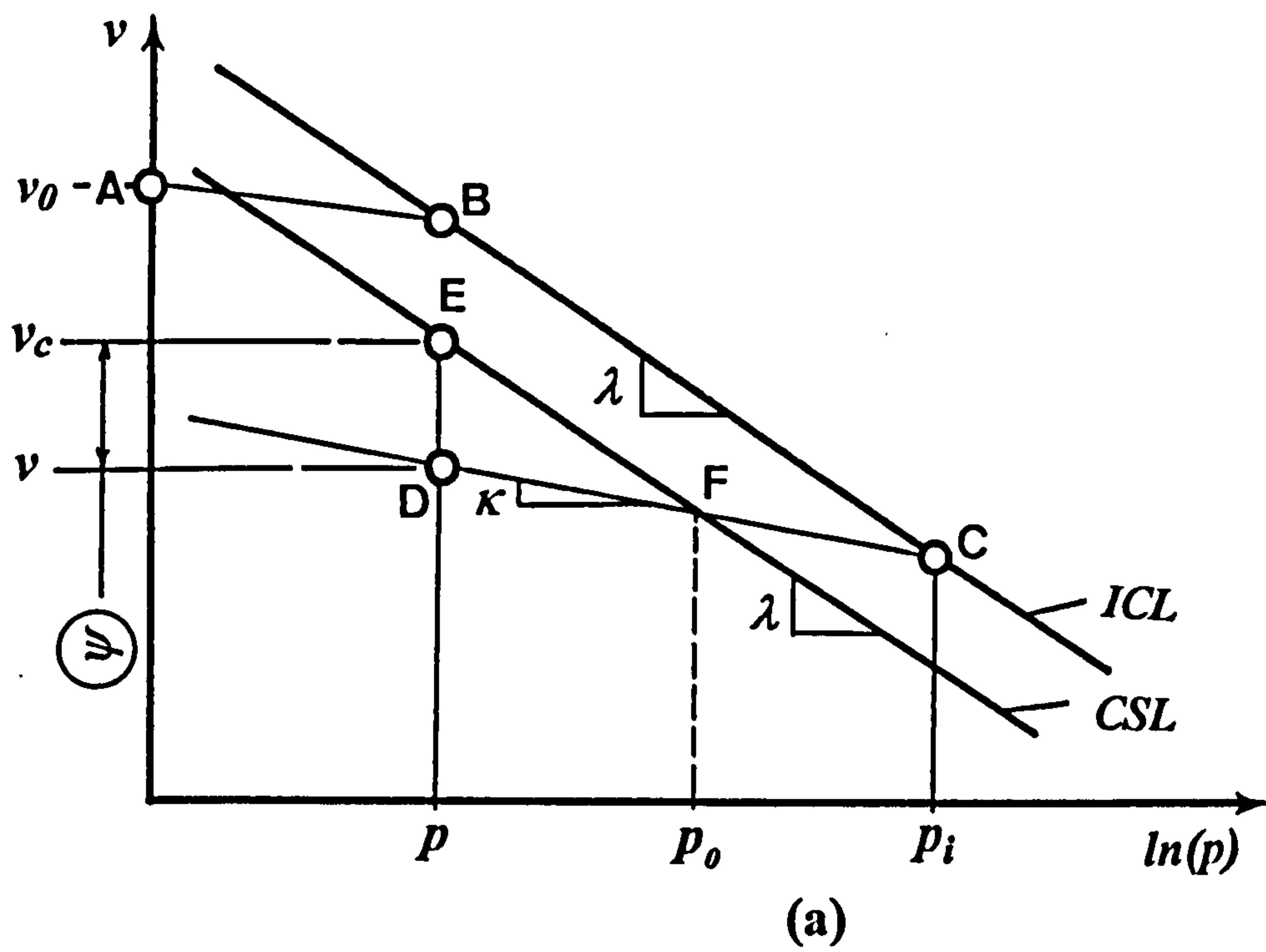


Fig. 5.1 (a) Definition of State parameter, ψ in v - $\ln(p)$ plane. (b) Elastic walls associated with State parameter in Critical state space.

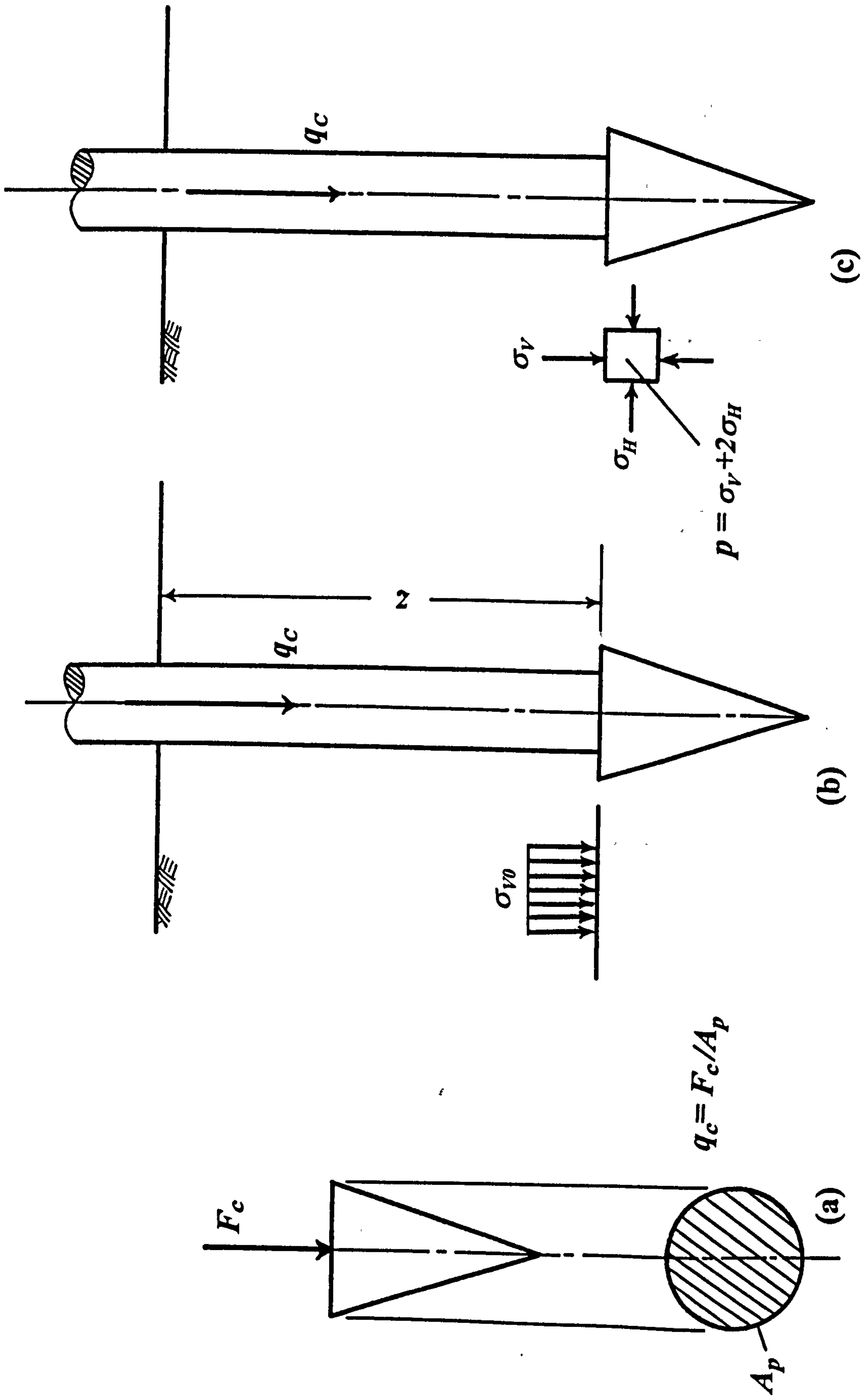


Fig. 5.2 (a) Cone index. (b) Geostatic stress on horizontal planes at cone level. (c) Mean normal stress at cone level.

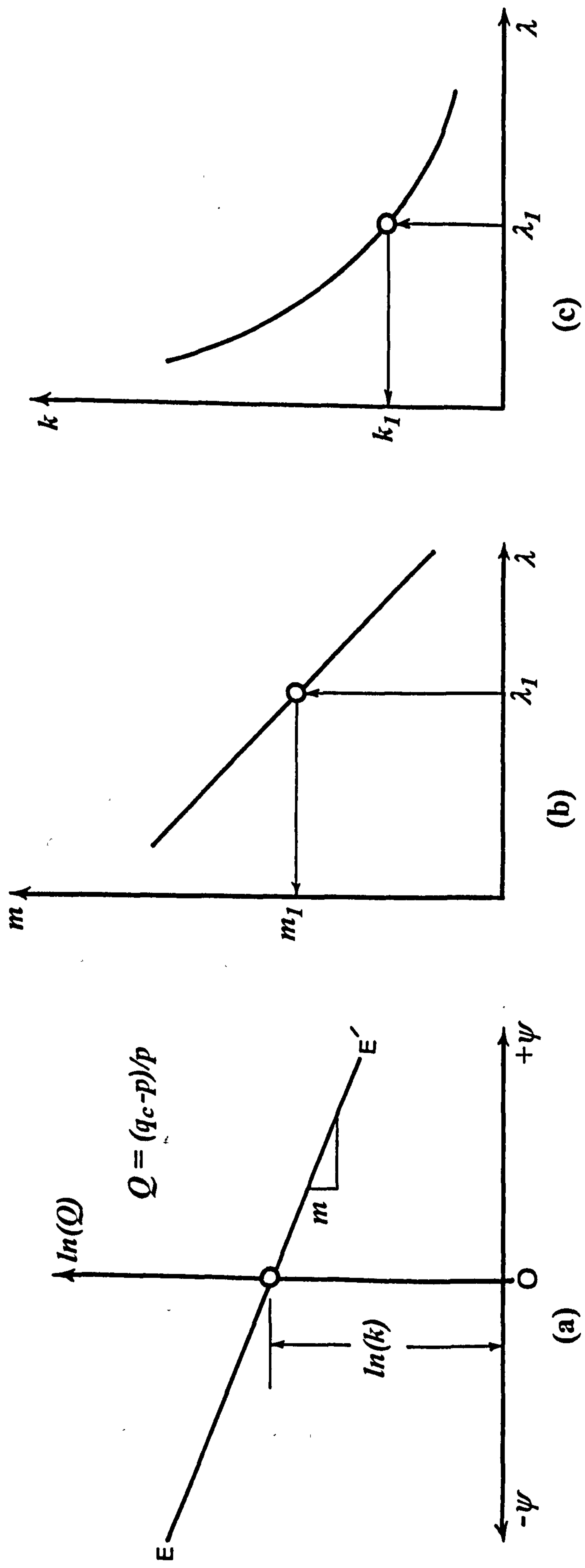


Fig. 5.3 (a) Relationship between State parameter, ψ and normalized Cone index, Q .
 (b) Variation of gradient m with critical state parameter λ . (c) Variation of intercept k with critical state parameter λ .

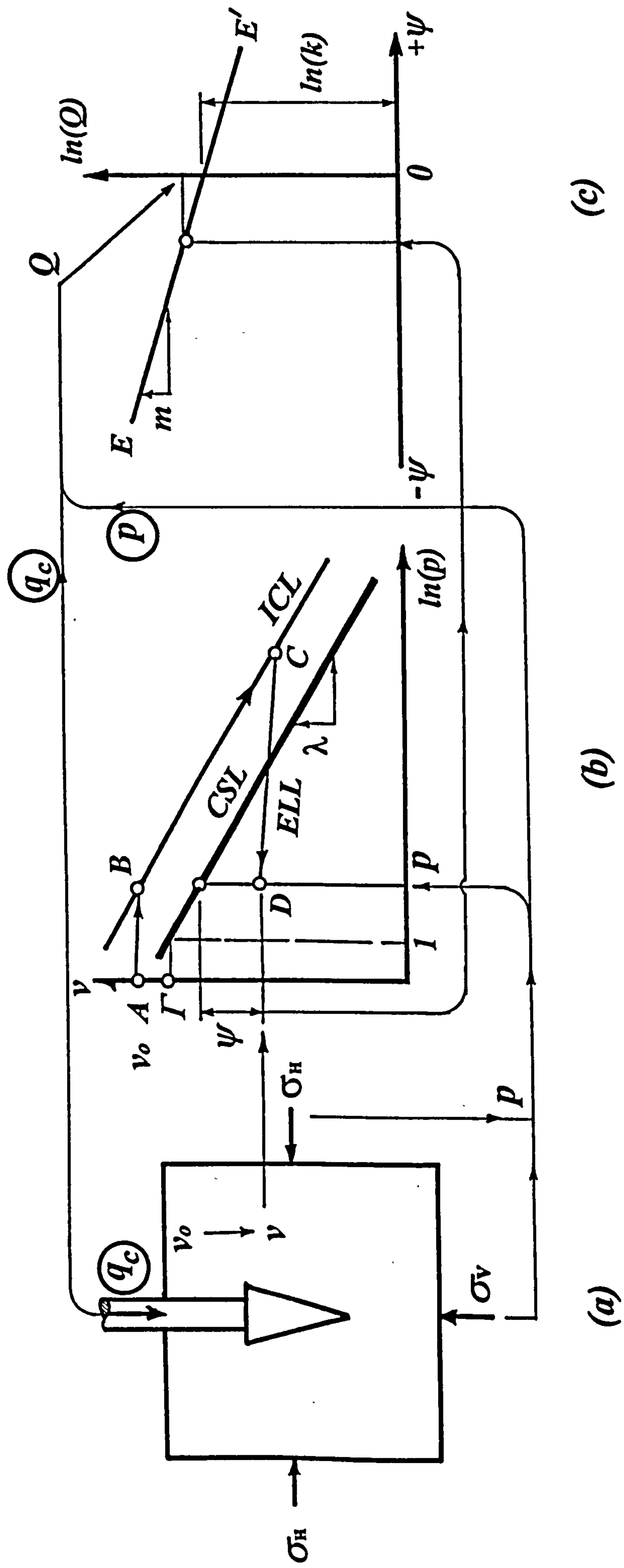


Fig. 6.1 (a) to (c) Steps in the calibration process.

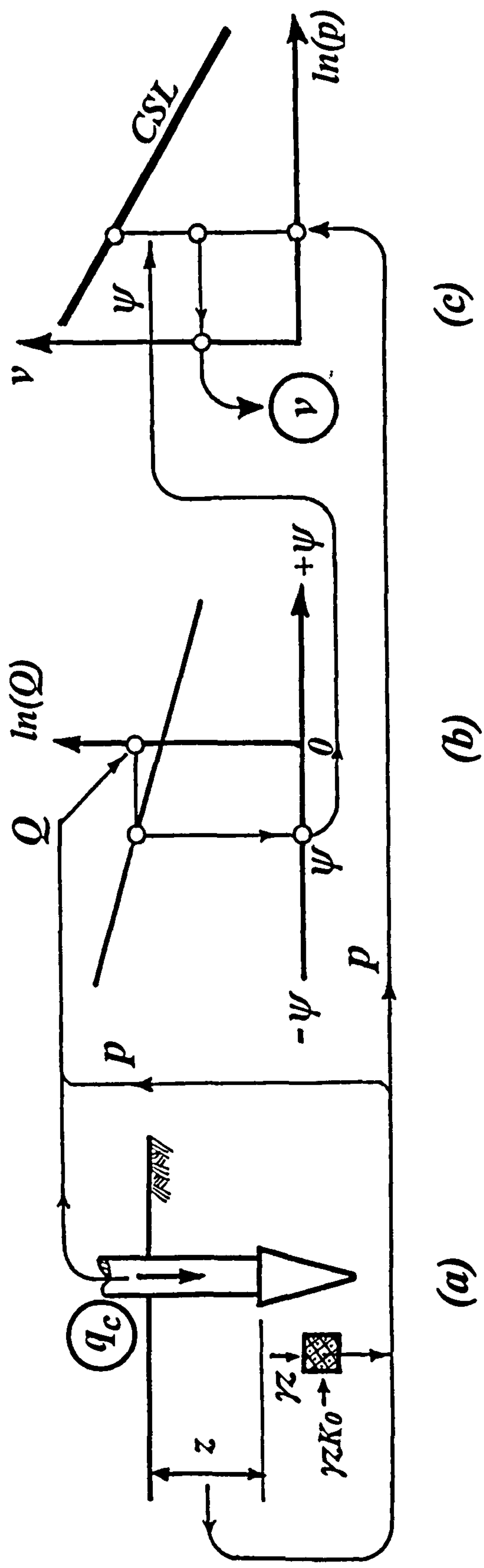


Fig. 6.2 (a) to (c) Conversion of Cone index to soil specific volume.

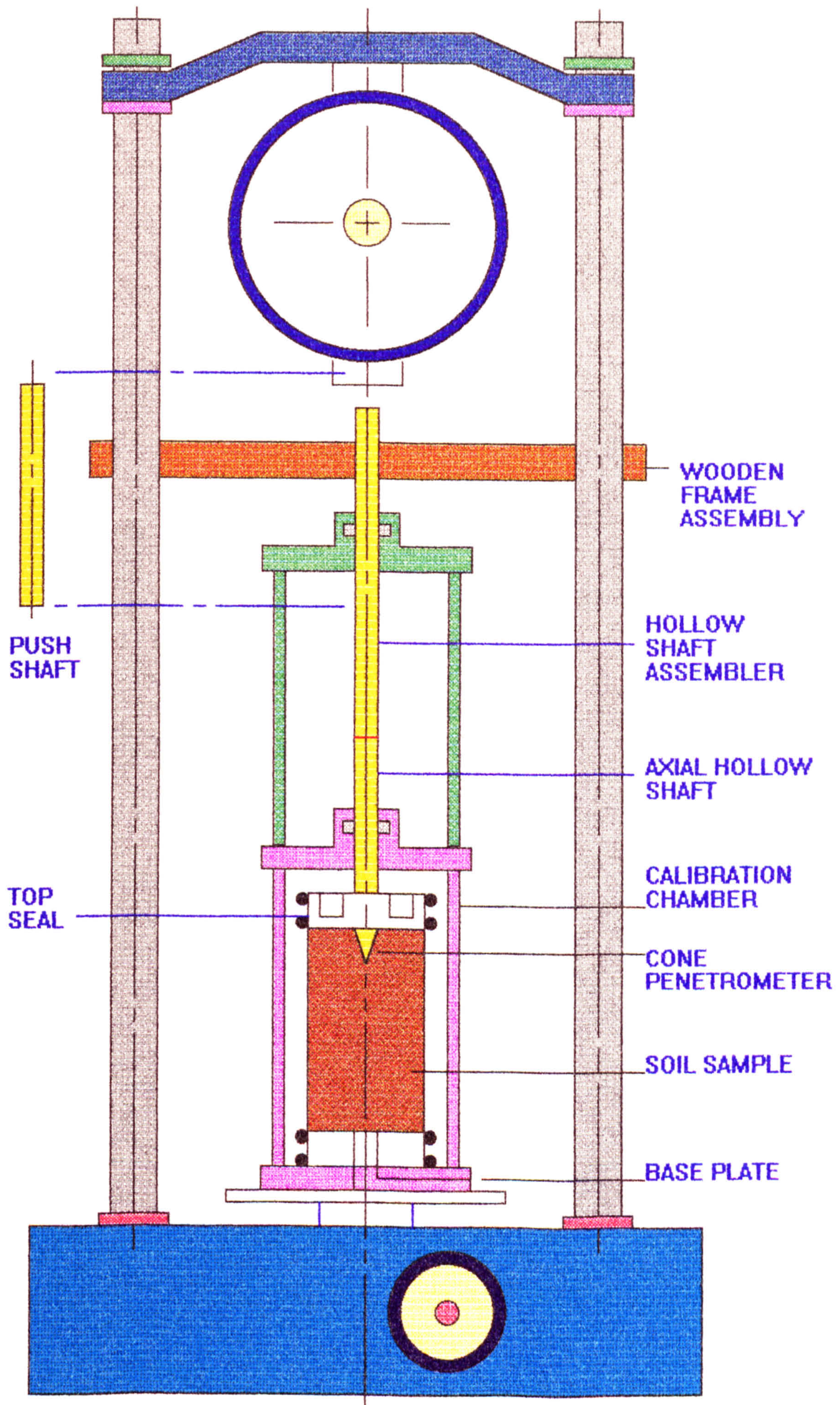
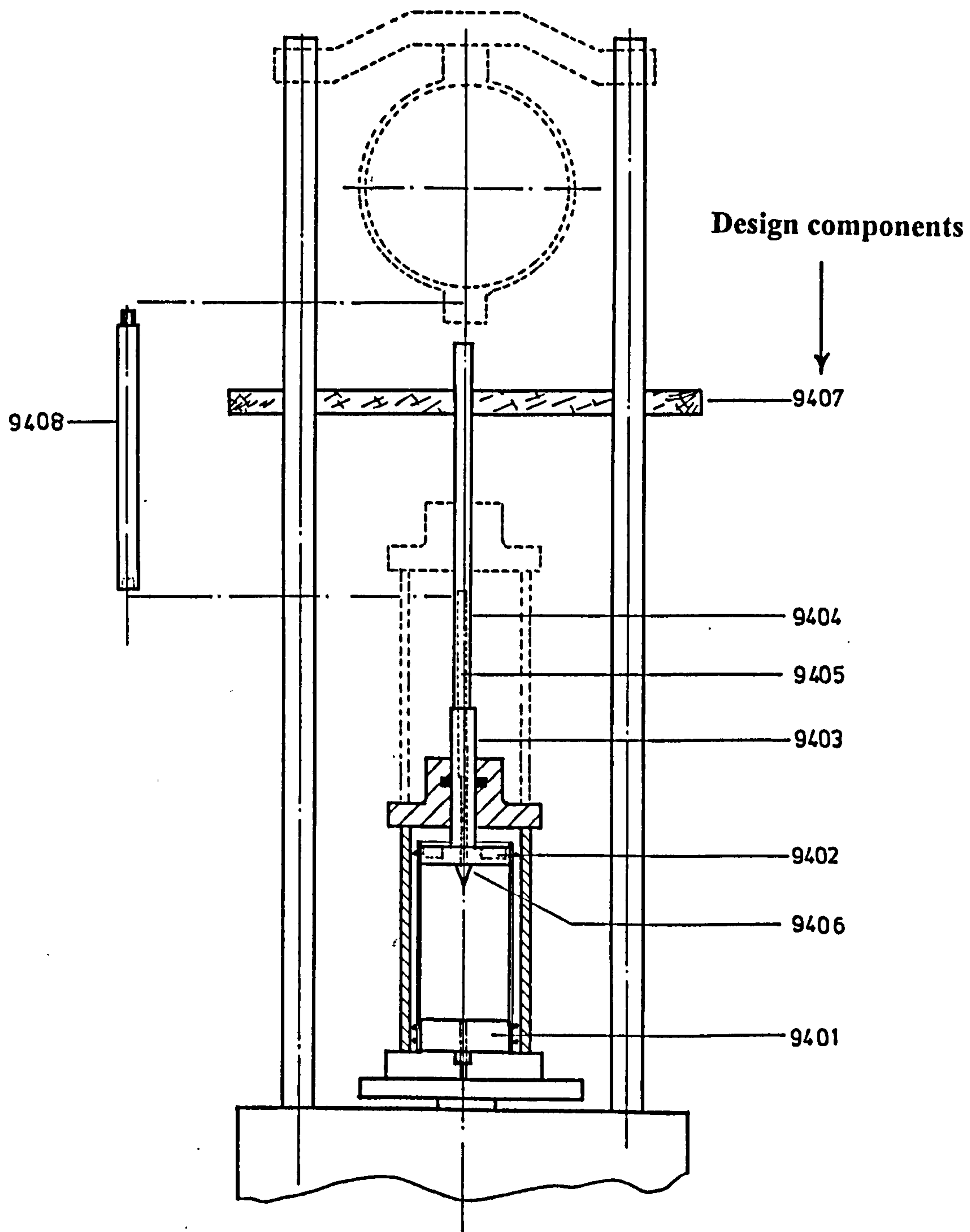


Fig. 6.3 Schematic arrangement for triaxial calibration chamber test.

TRIAXIAL CALIBRATION CHAMBER



All components are drawn to scale

Fig. 6.4 The revised design components for the triaxial Calibration chamber.

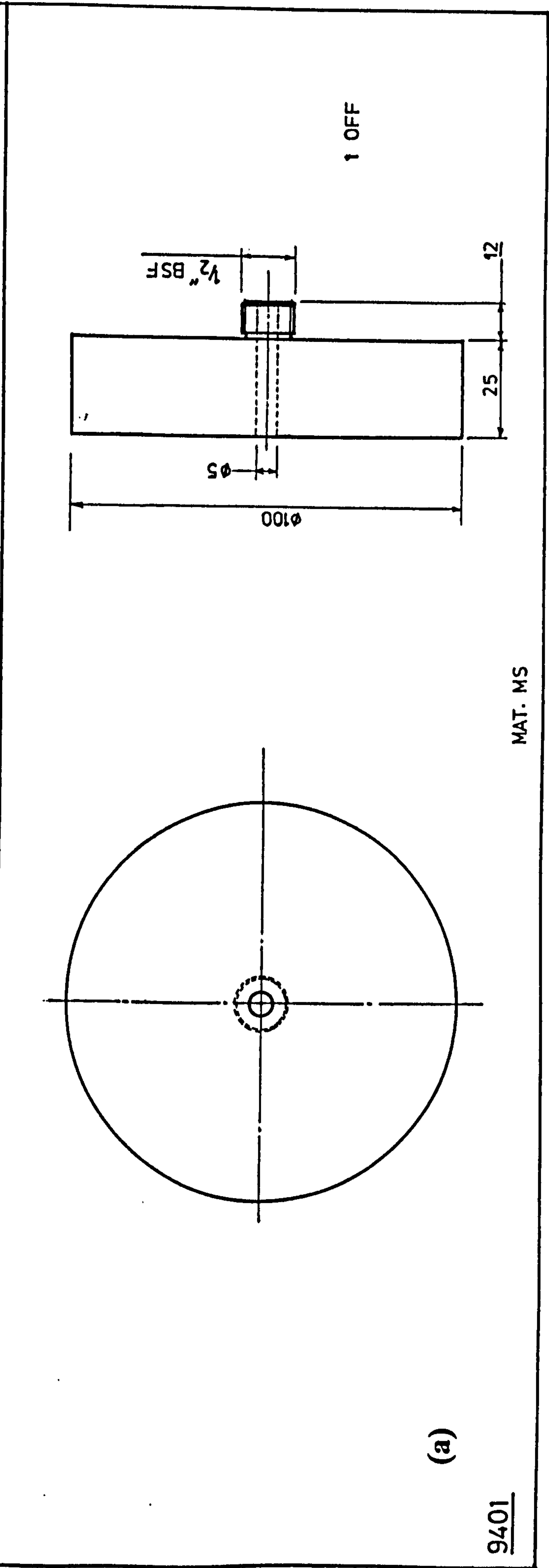
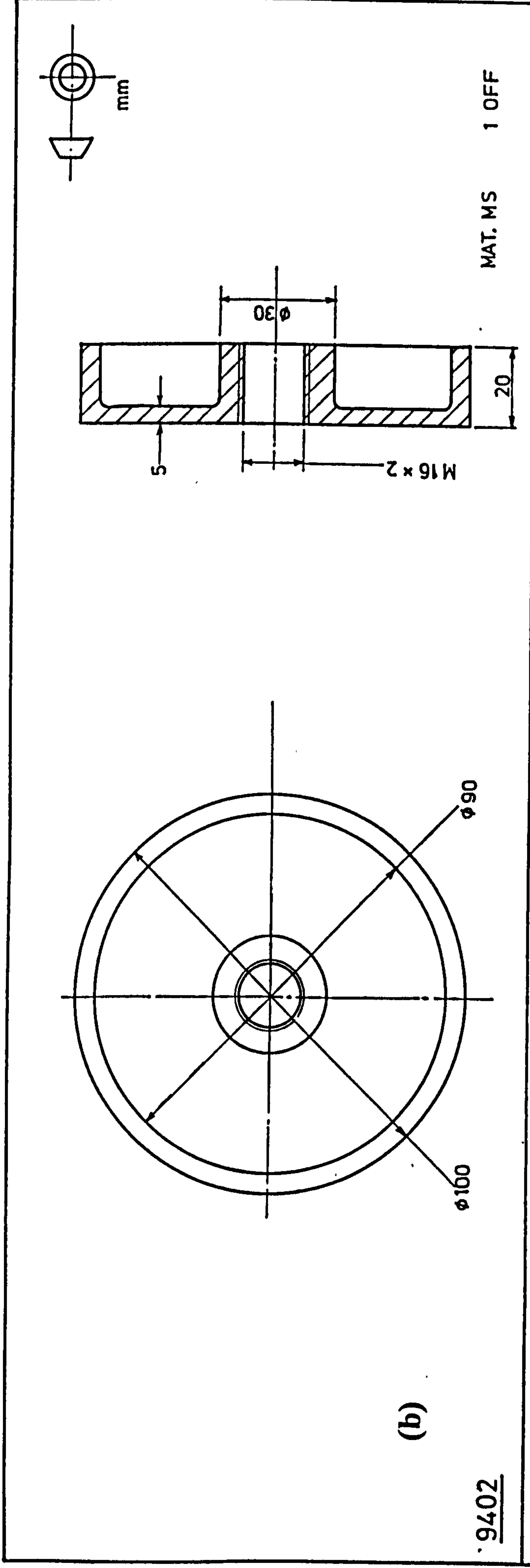


Fig. 6.5 (a) The design details for modified Base plate. (b) The design details for Top seal.

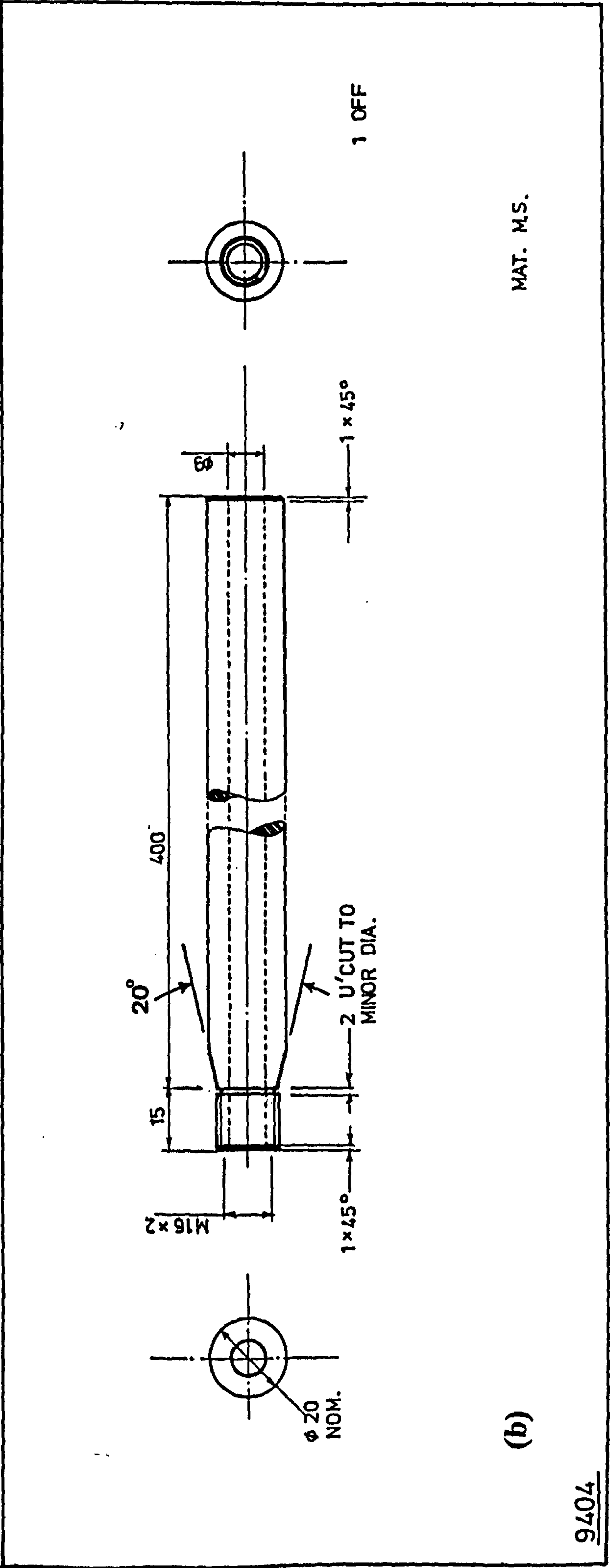
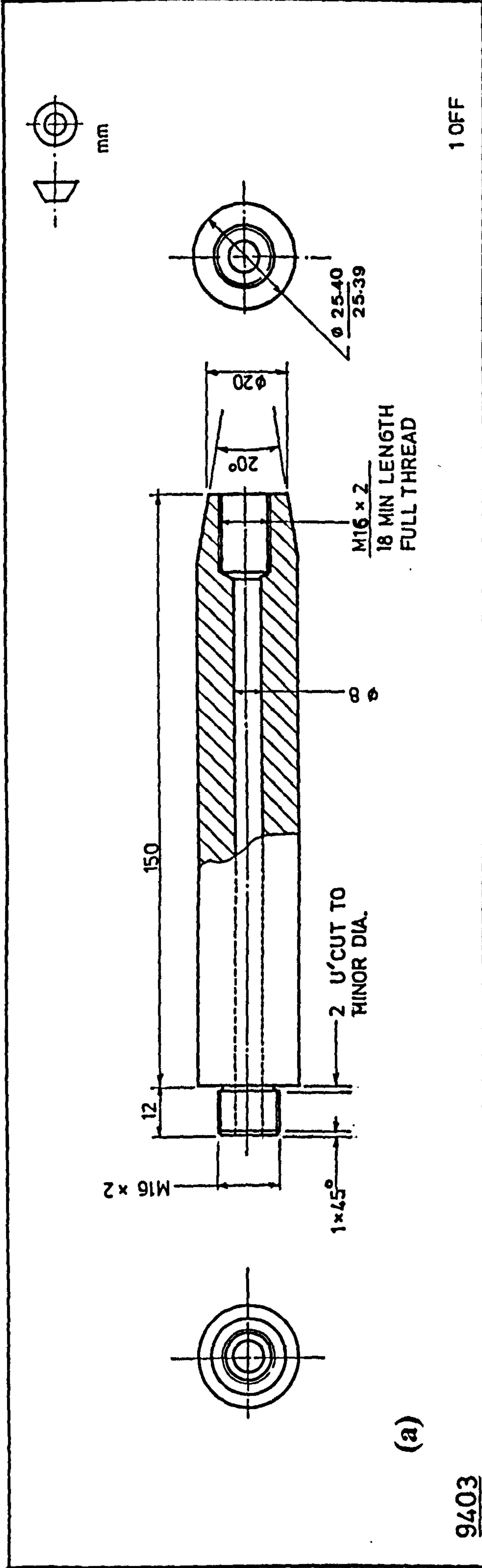


Fig. 6.6 (a) The design details for Axial hollow shaft. (b) The design details for Hollow shaft assembler.

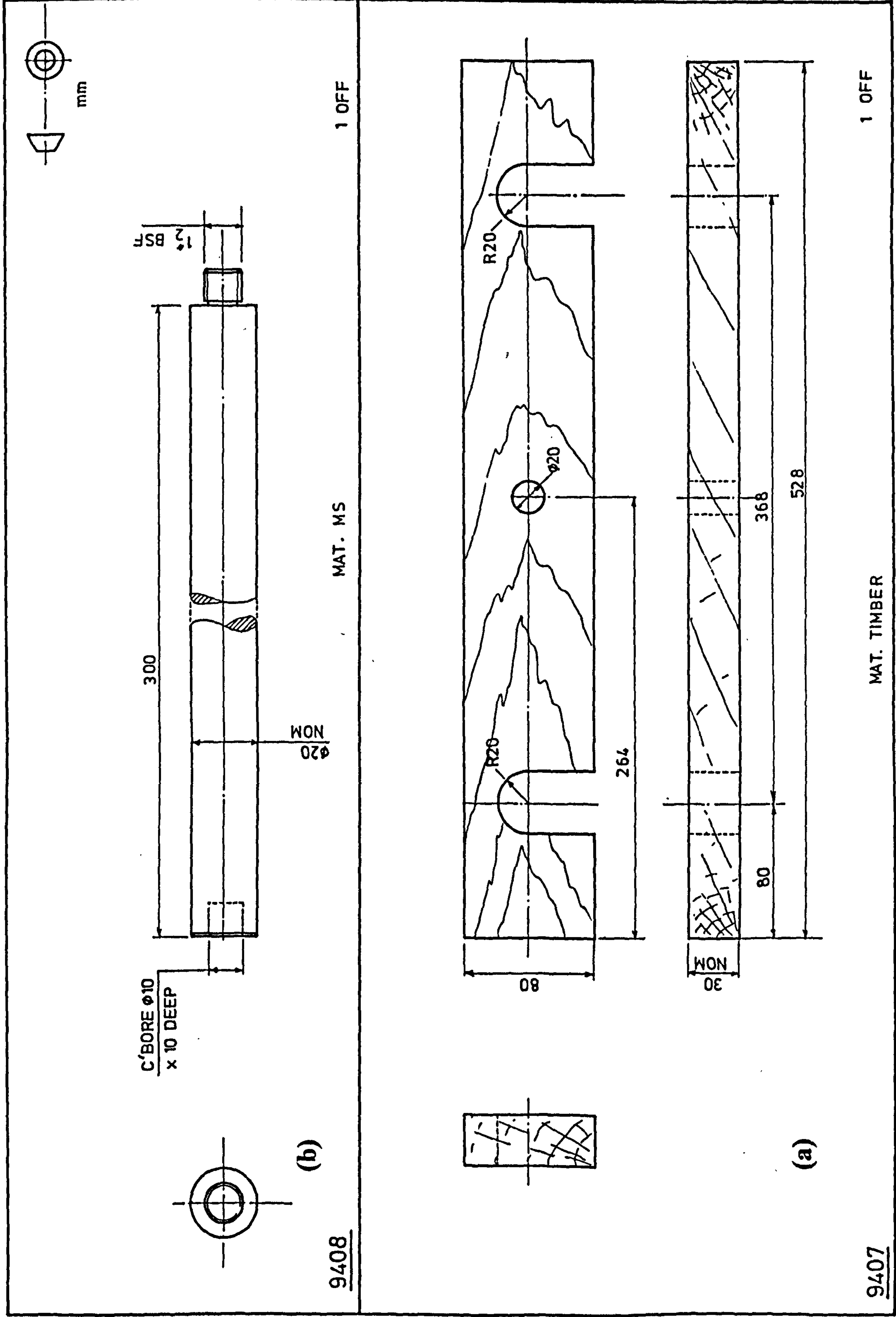


Fig. 6.7 (a) The design details for Wooden frame assembly. (b) The design details for Penetrometer aligner (Push-shaft).

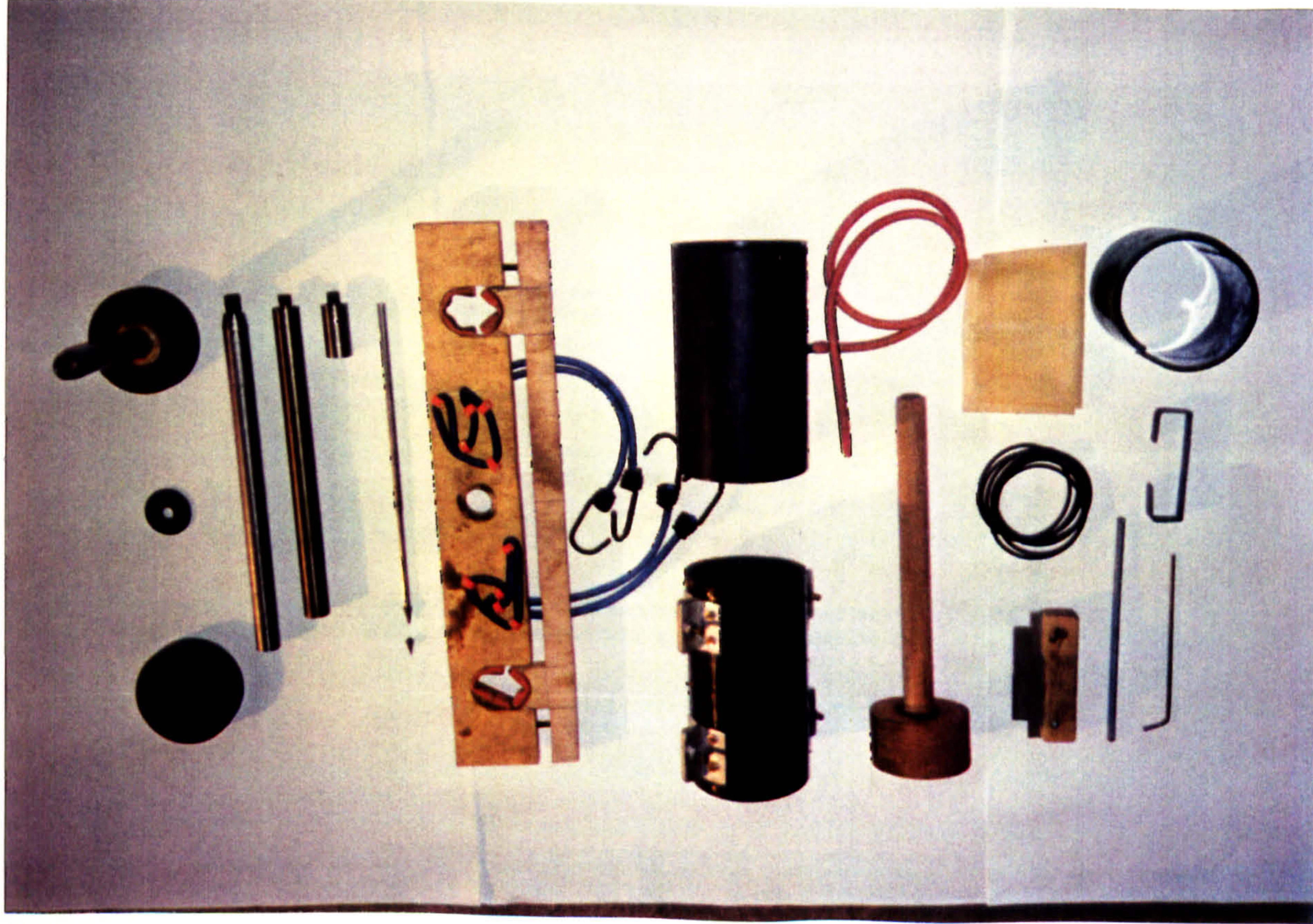


Fig. 6.9 (a) The revised design components and the sample preparation accessories for the triaxial calibration chamber test.

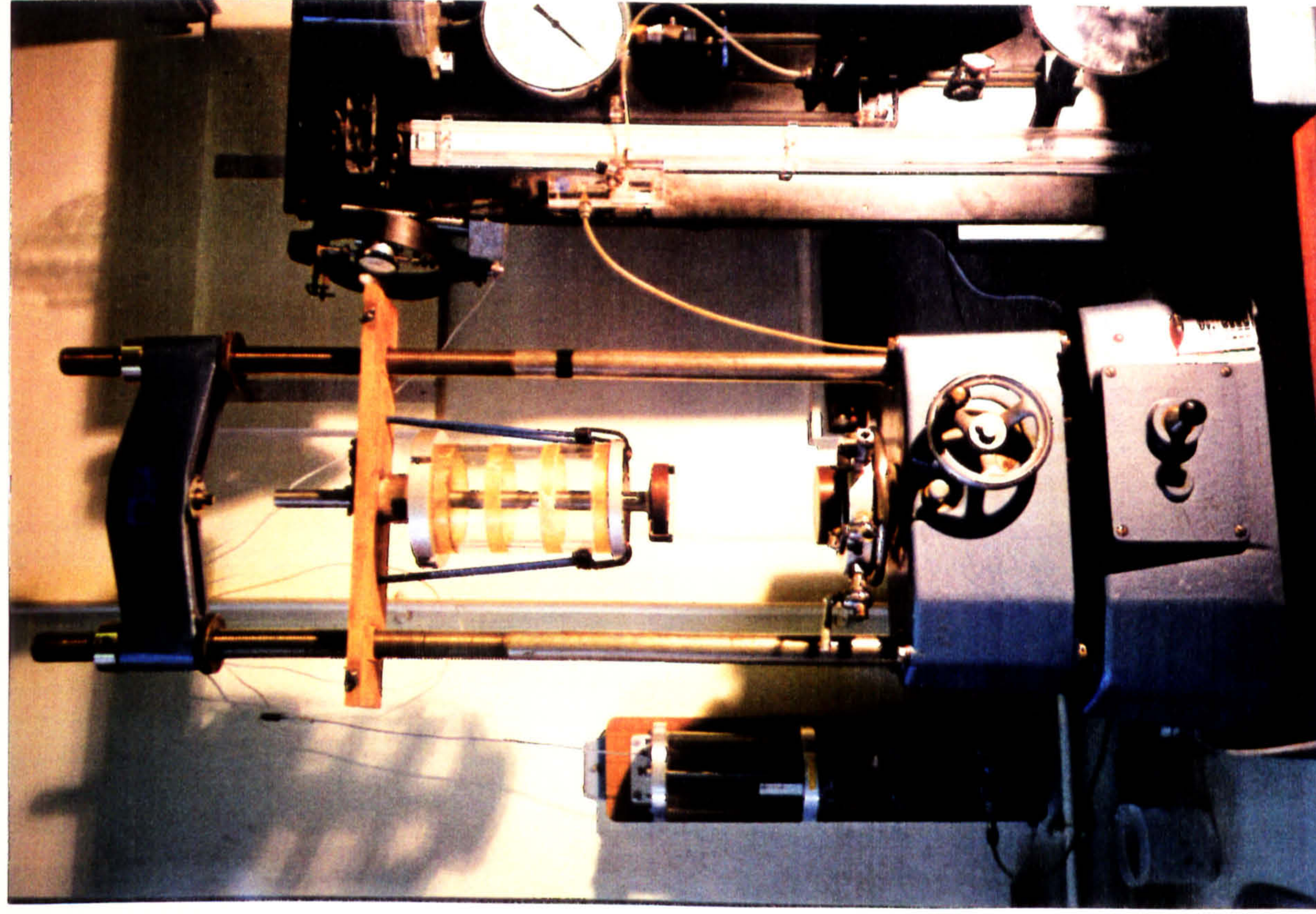


Fig. 6.9 (b) Steps in the working principle; Procedure for holding the triaxial calibration chamber with the Wooden frame assembly.

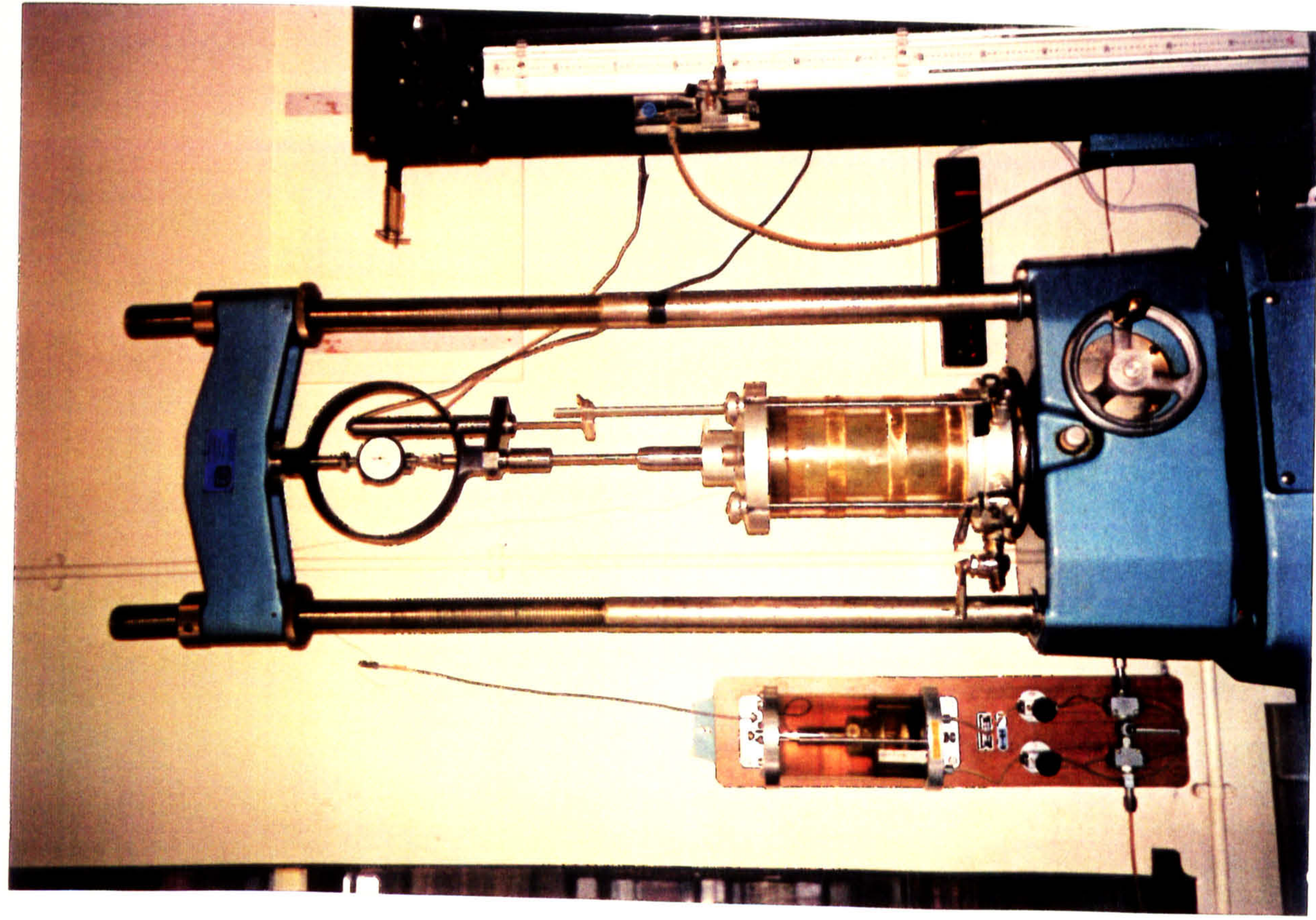
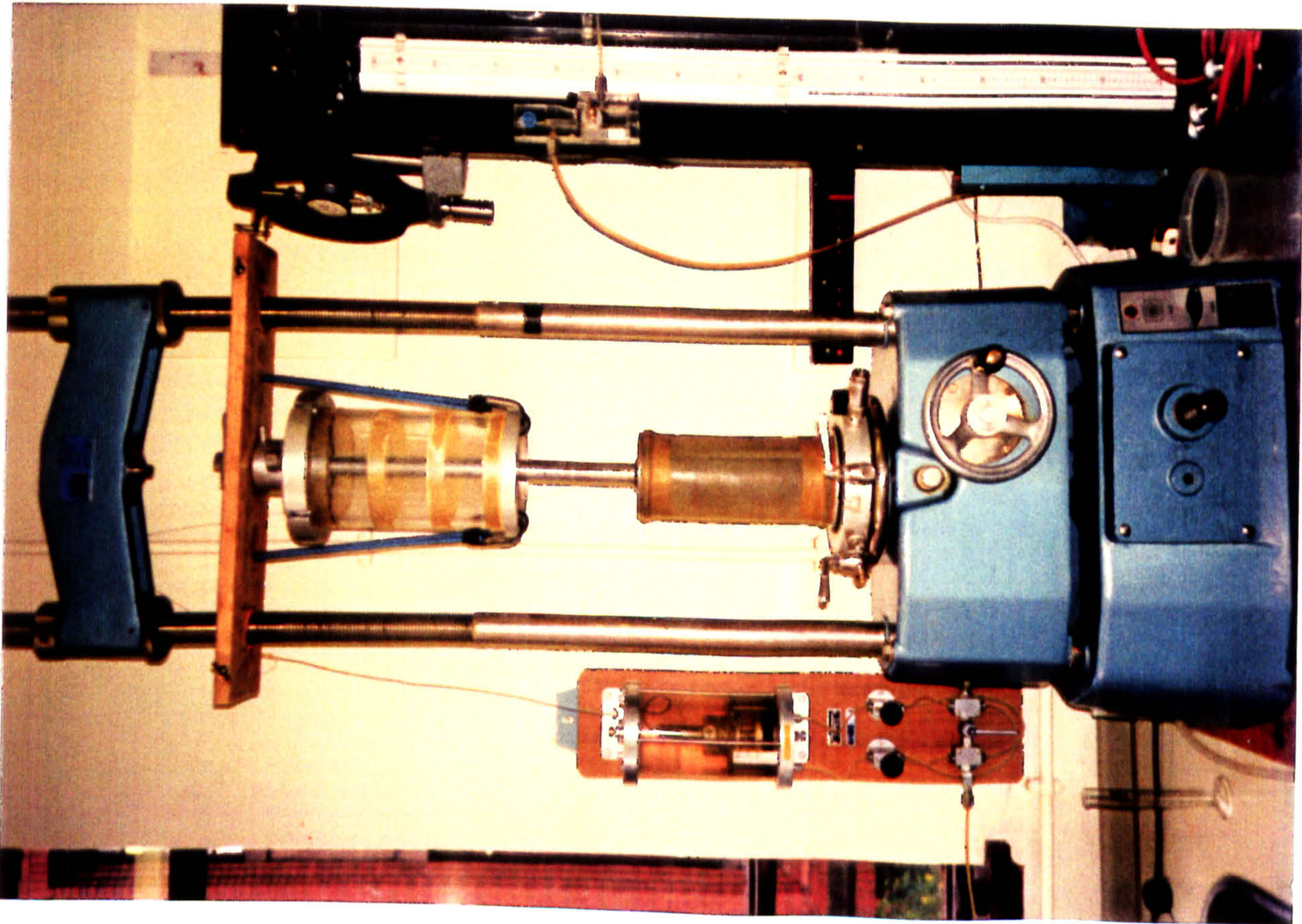


Fig. 6.9 (c) and (d) Procedure for placing the sample in the triaxial calibration chamber.

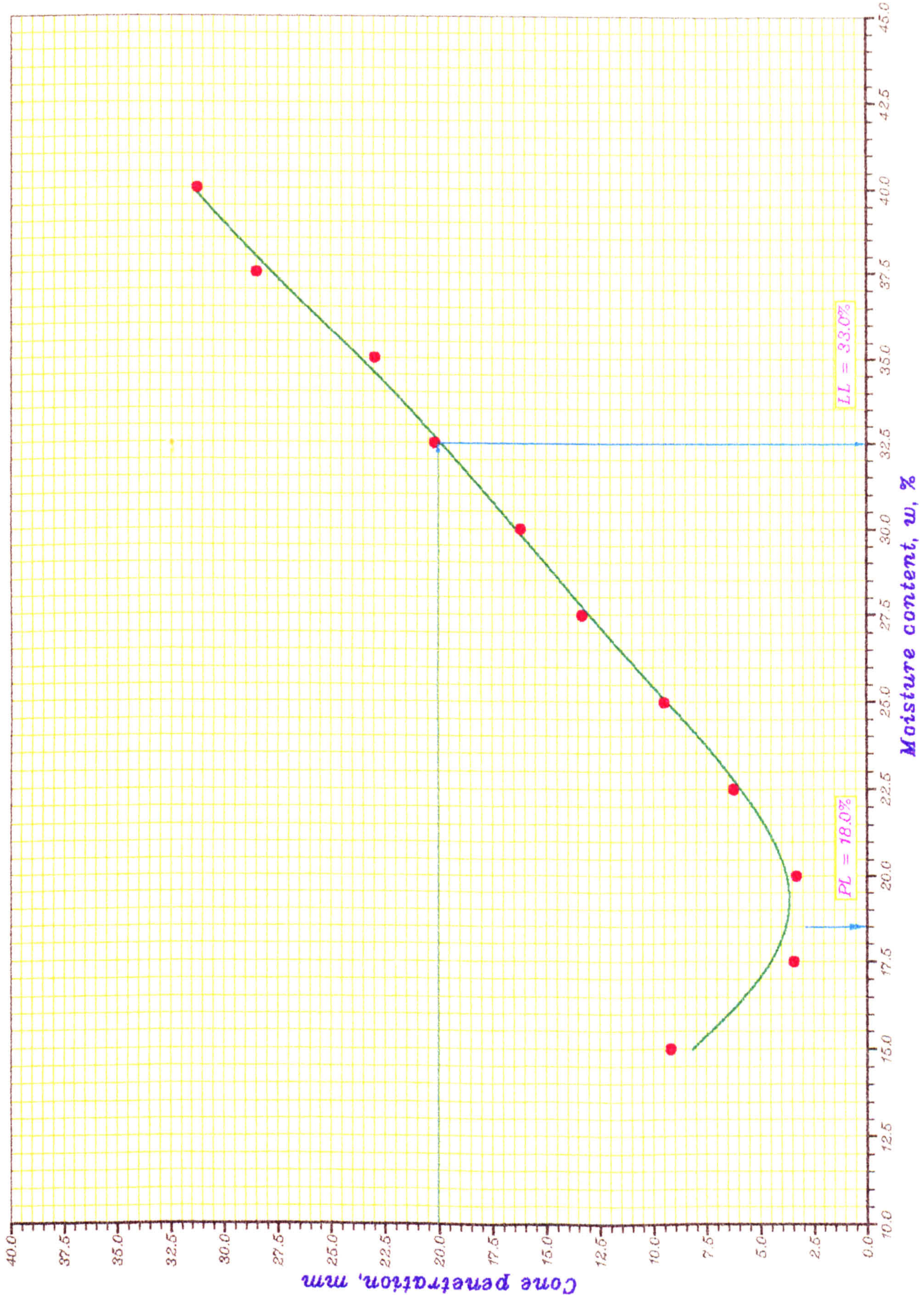


Fig. 7.1 Determination of Liquid limit and Plastic limit for the experimental soil

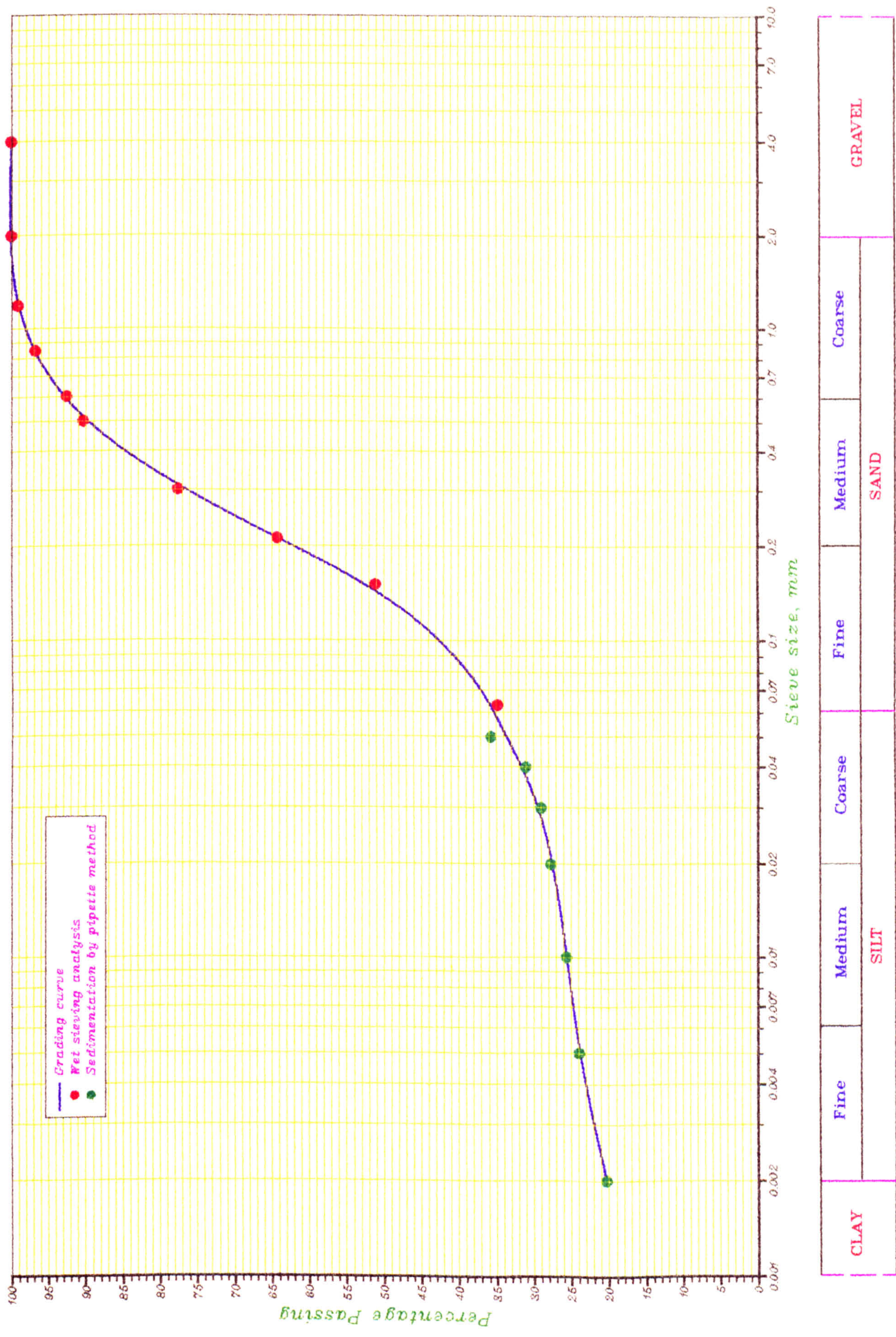


Fig. 7.2 Grading curve of the experimental soil

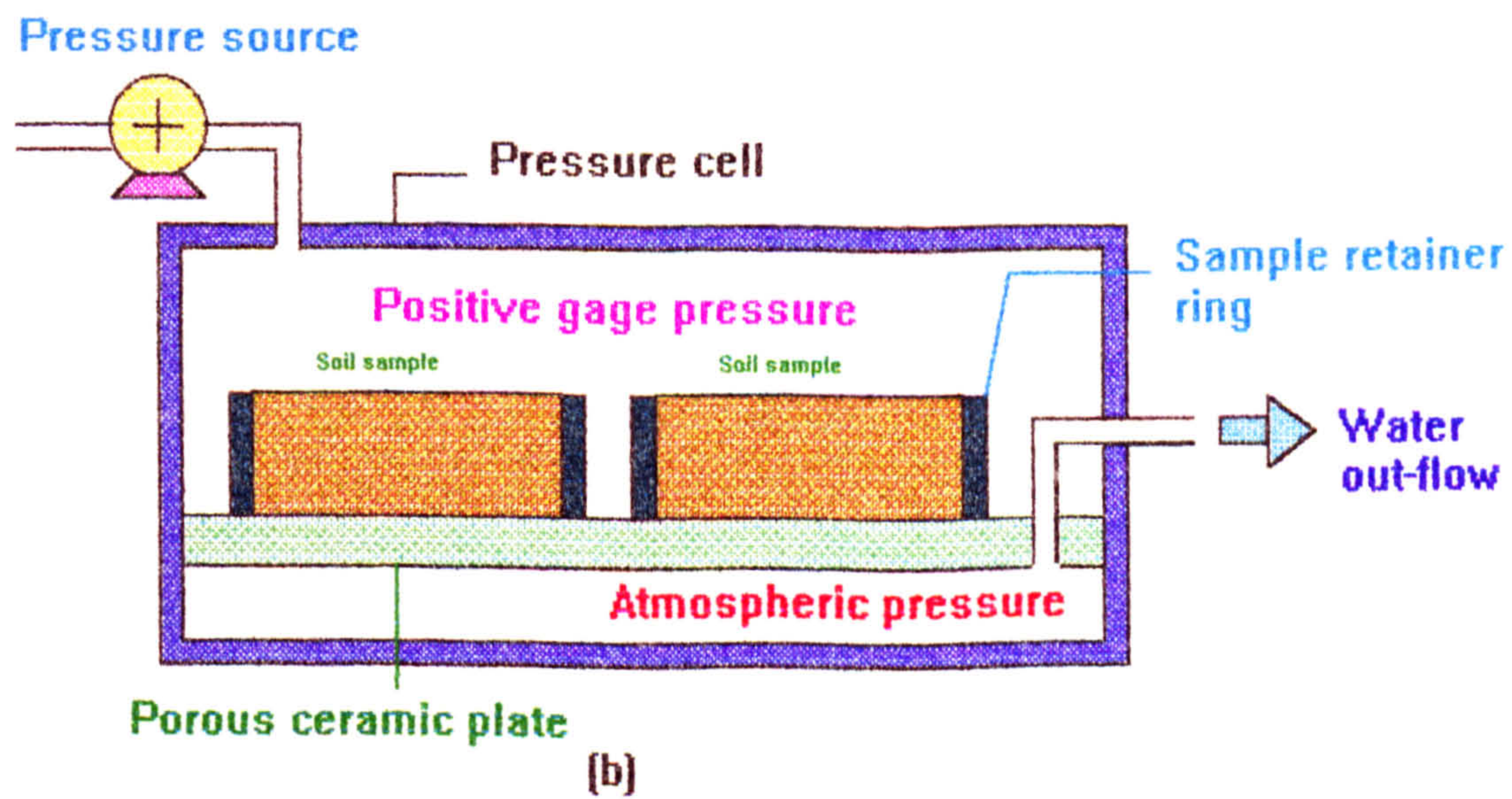
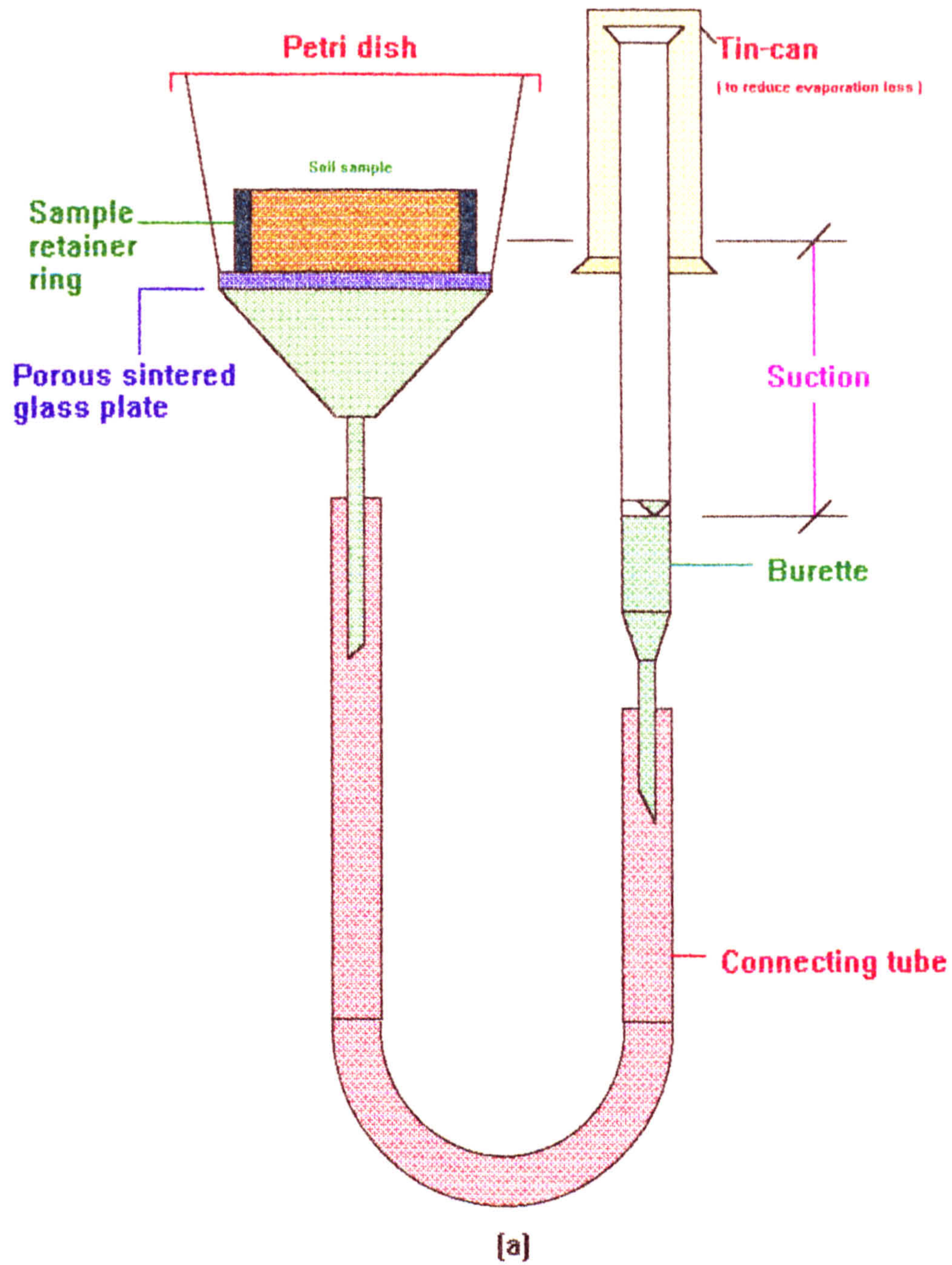


Fig. 7.3 (a) Schematic arrangement for Haine's Apparatus.
 (b) Schematic arrangement for Pressure Plate Extractor.

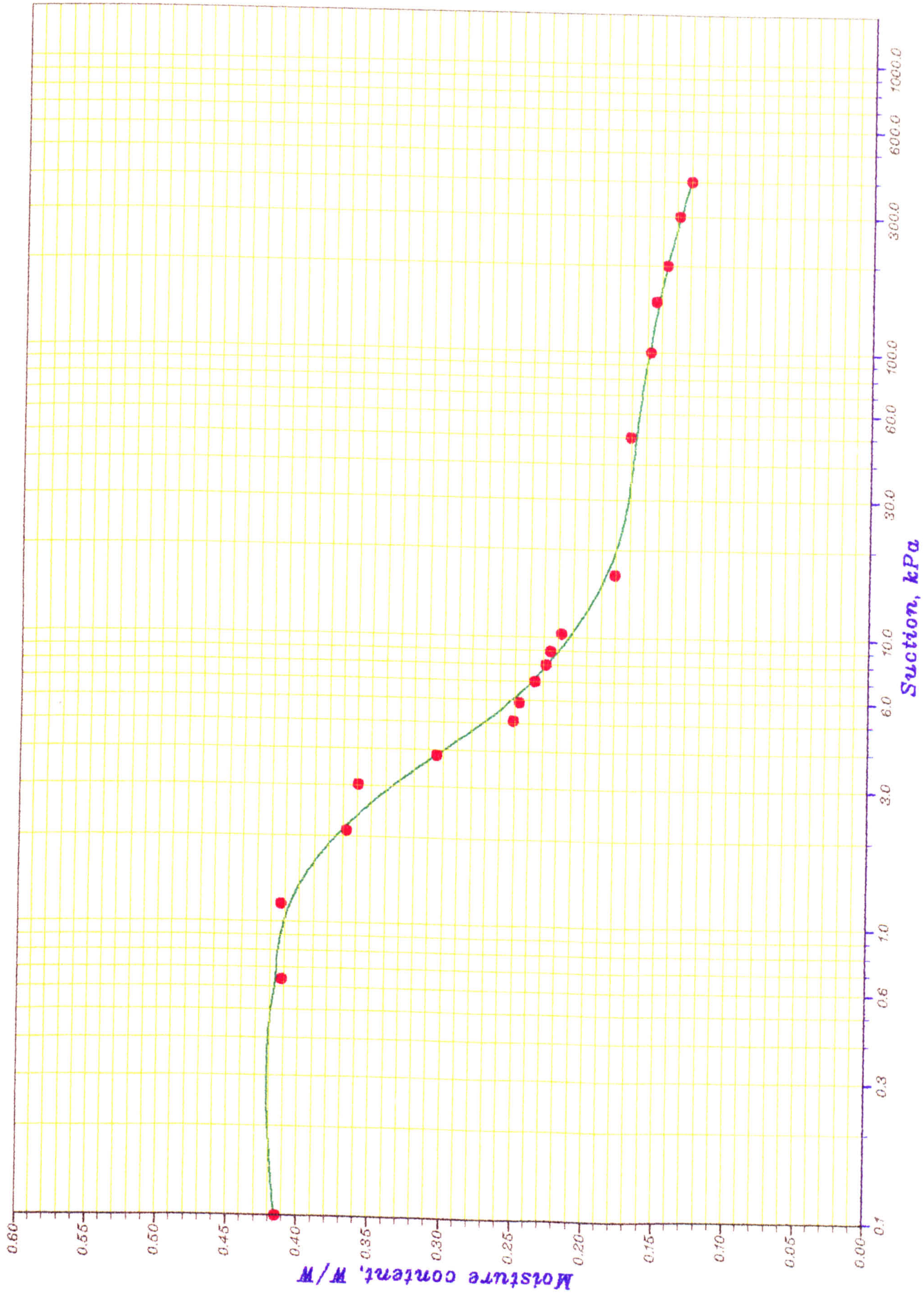


Fig. 7.4 Moisture release characteristics of the experimental soil

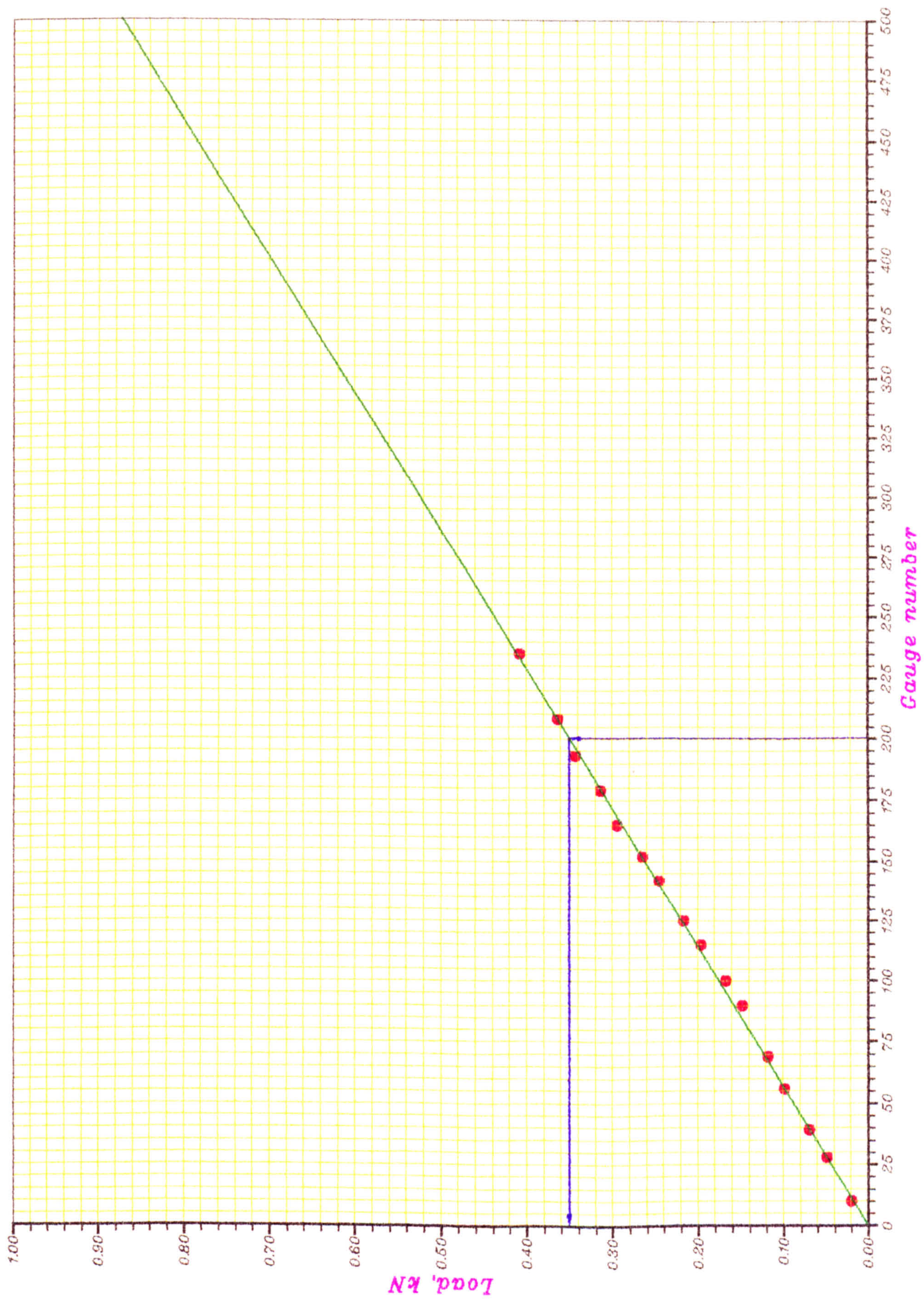


Fig. 7.5 Calibration (compression) for proving ring in the triaxial apparatus

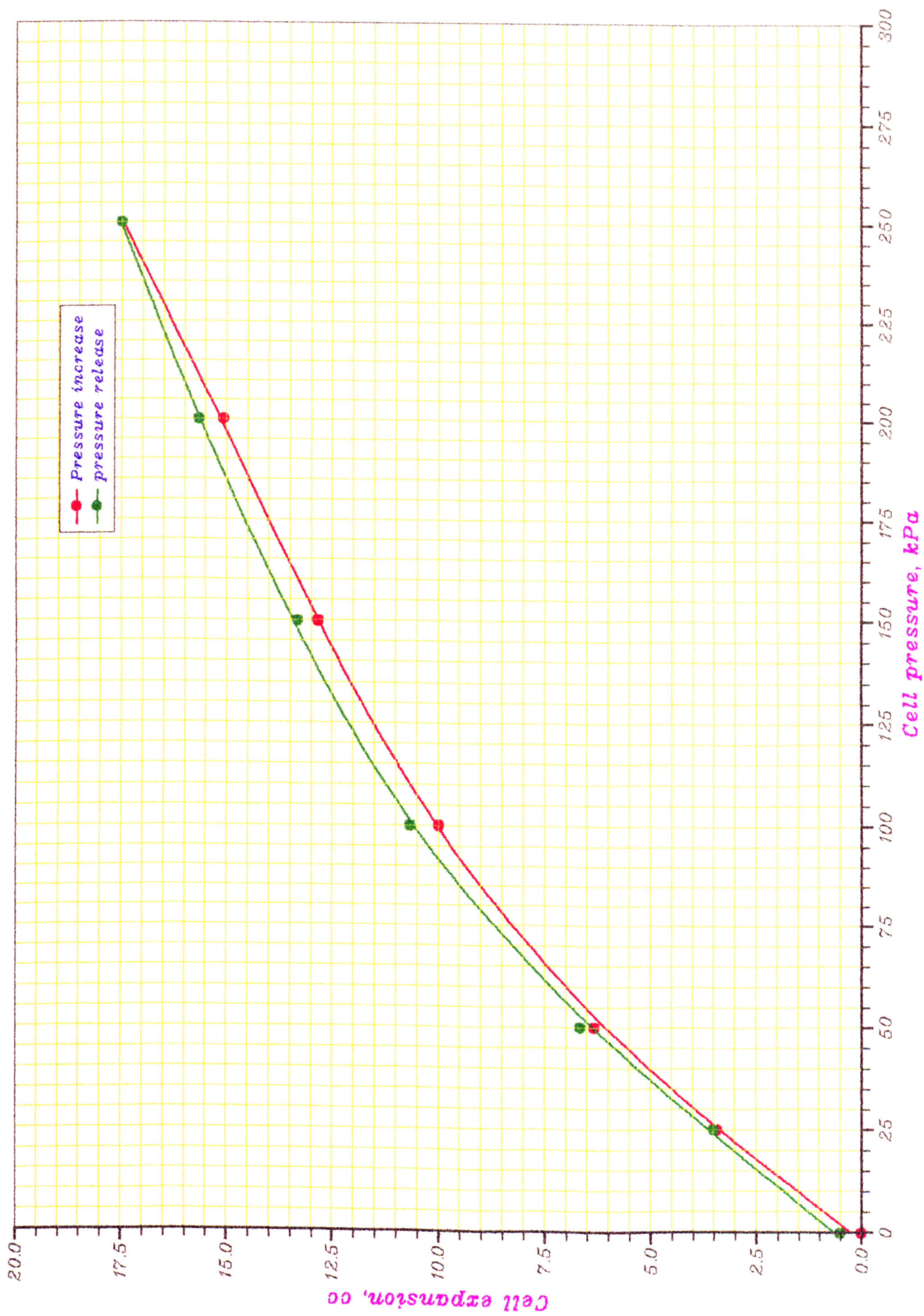


Fig. 7.6 Calibration for triaxial cell expansion due to hydrostatic pressure

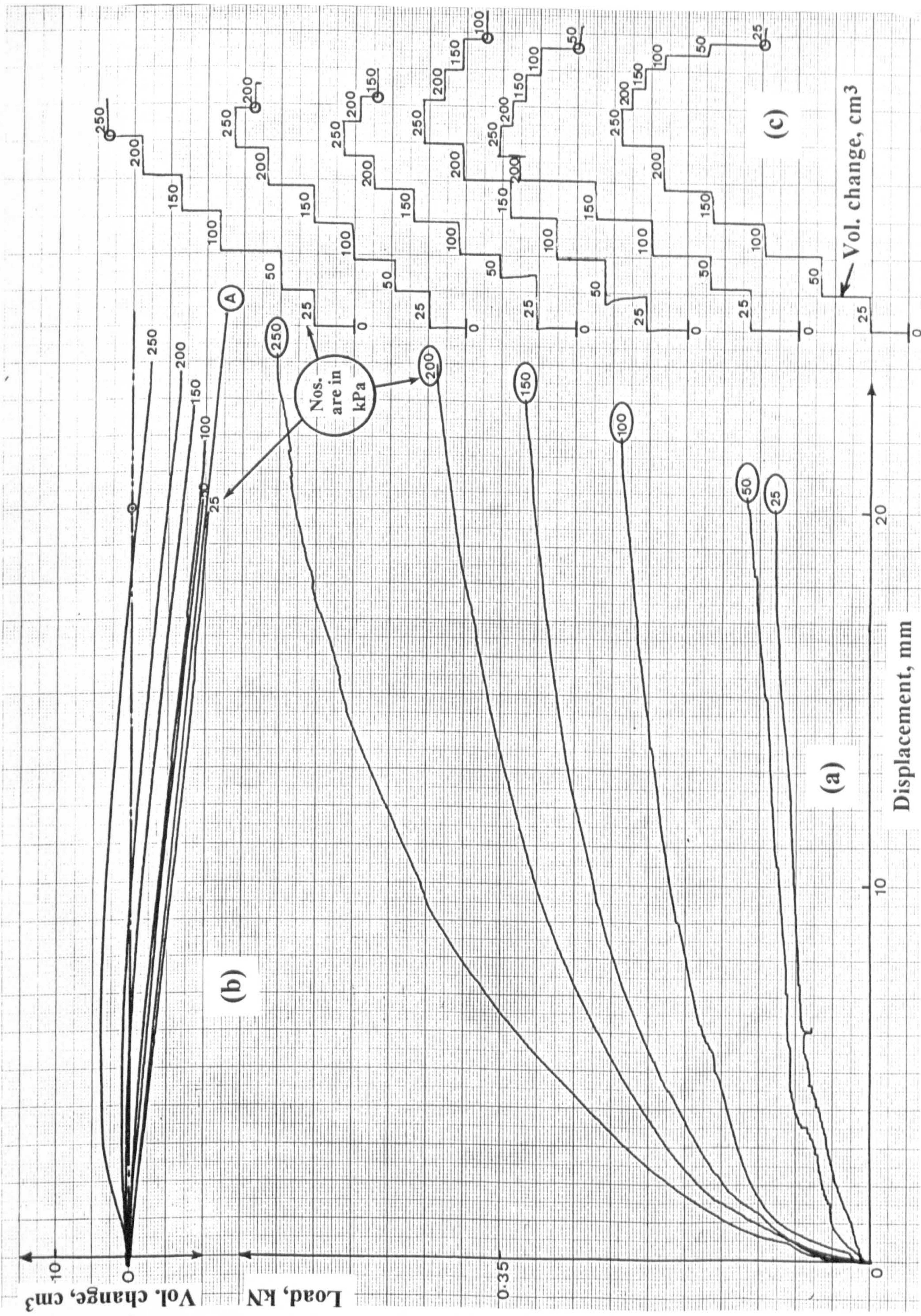
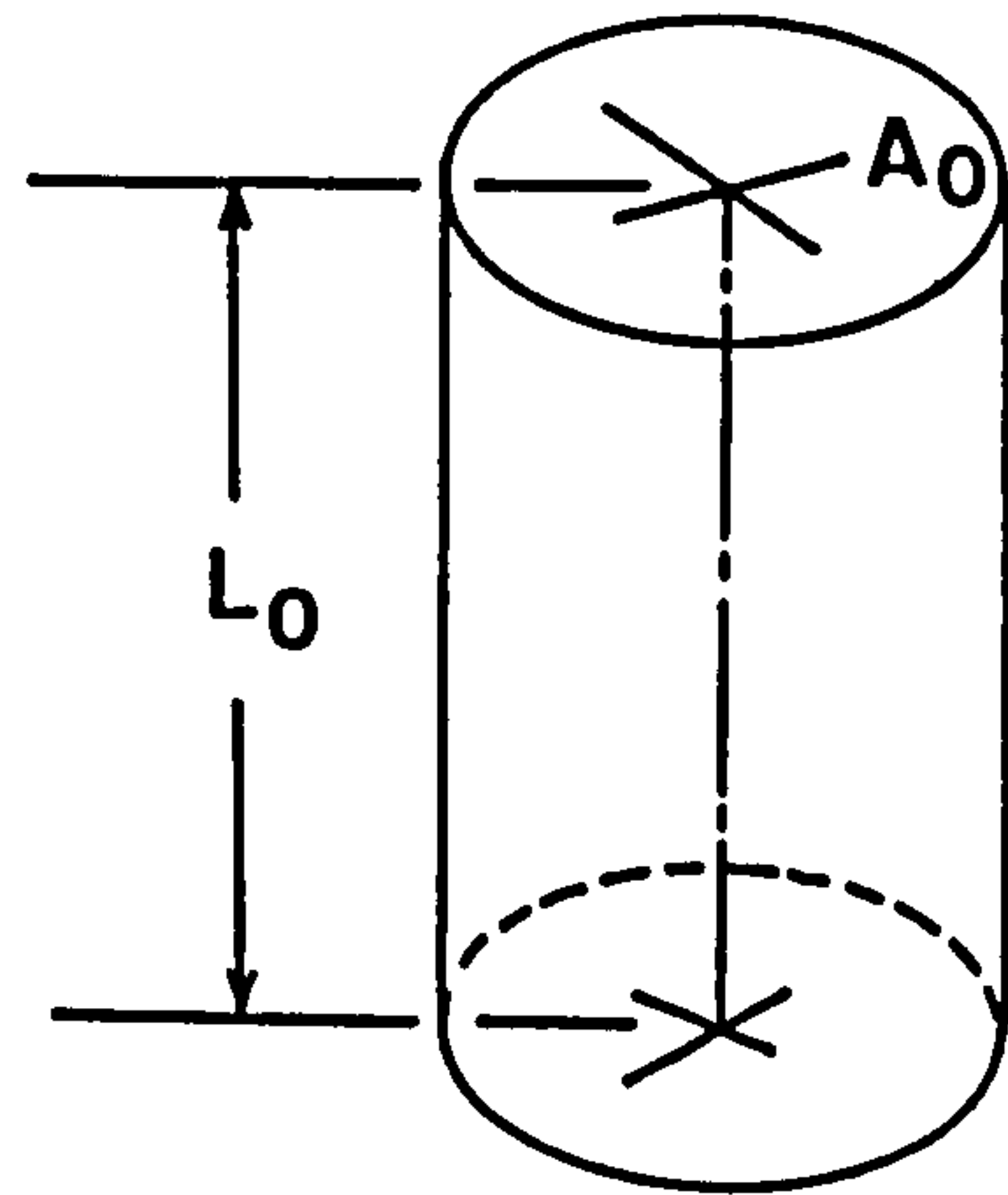
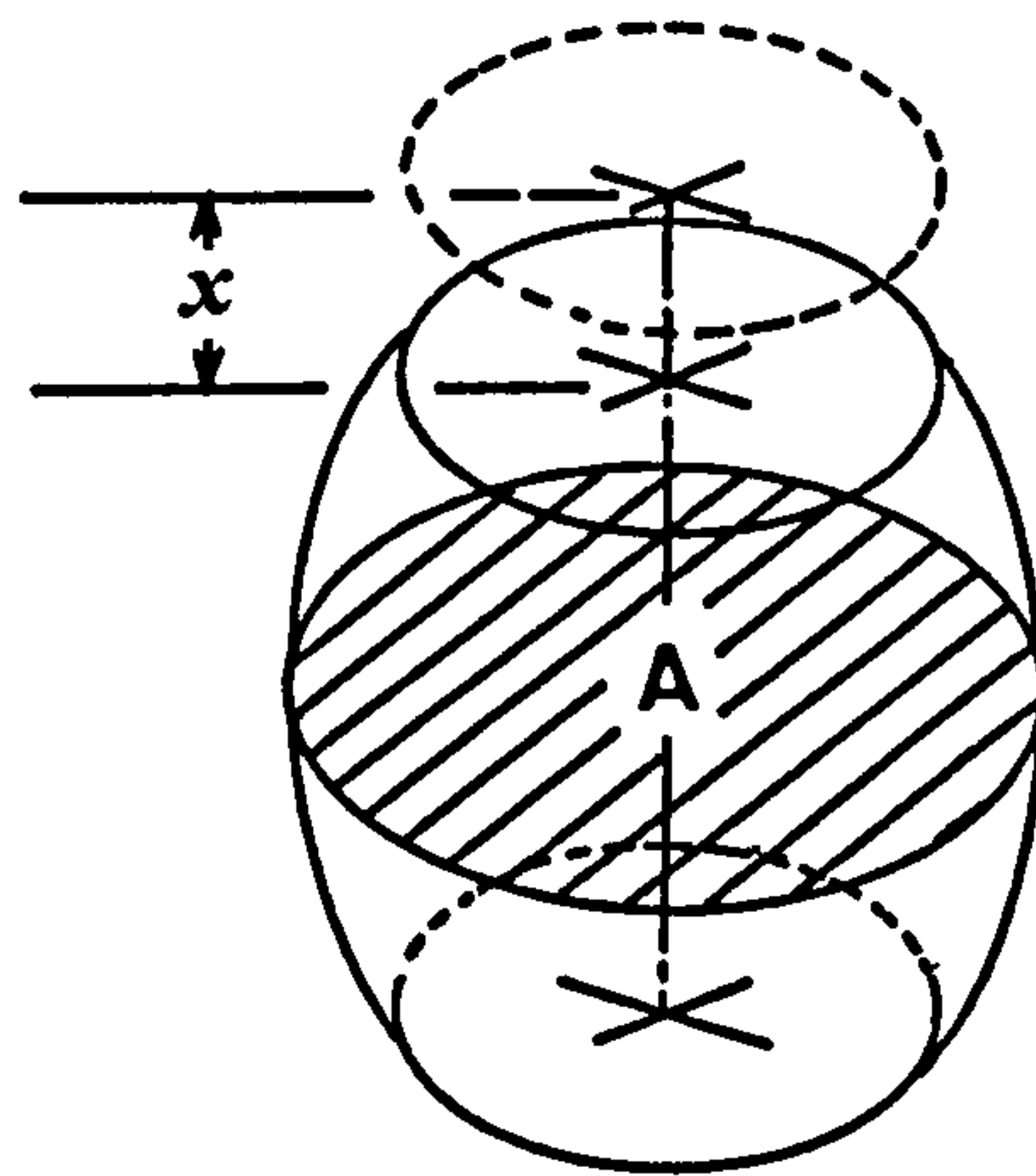


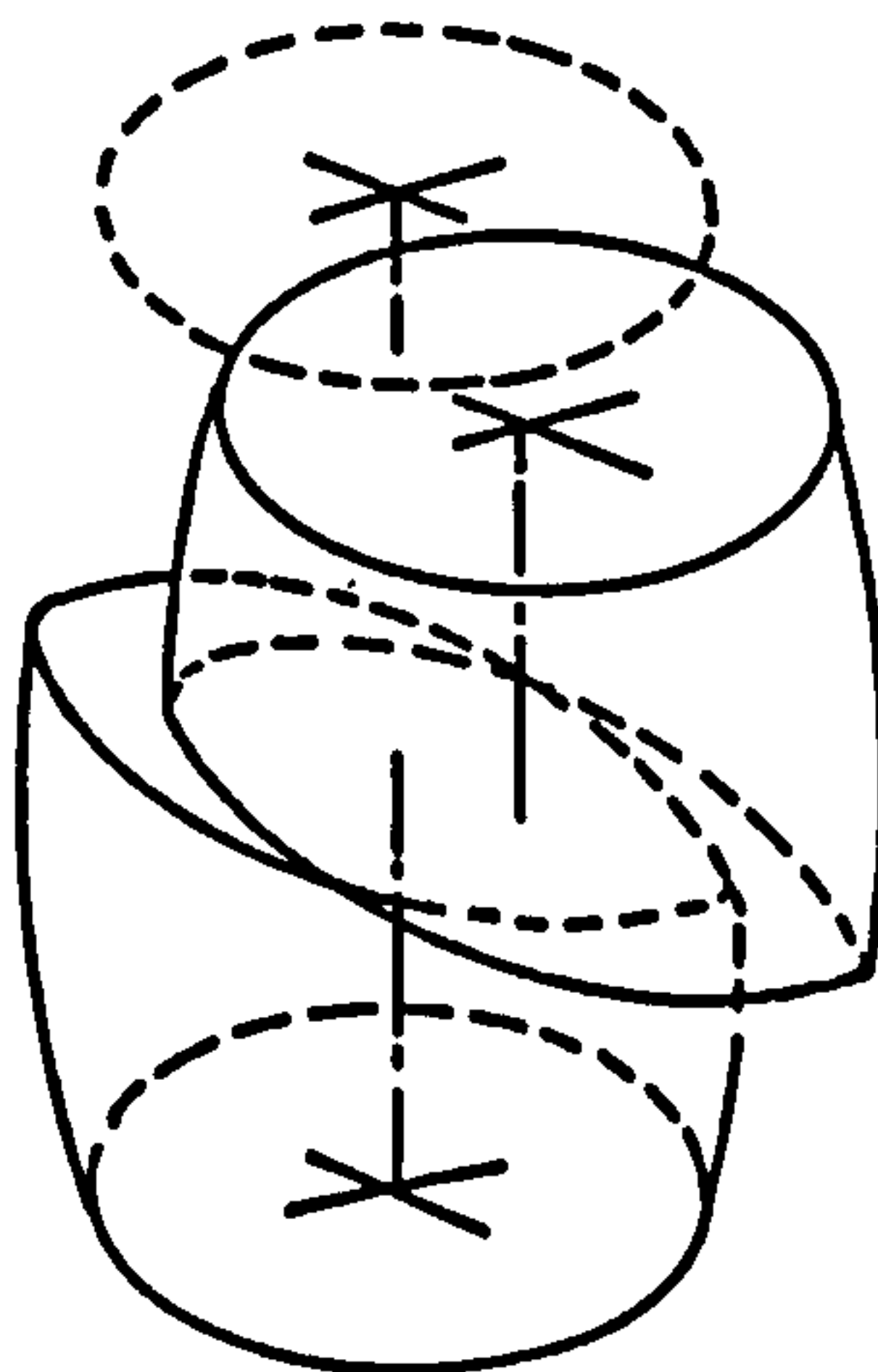
Fig. 7.7 A typical output on the X-Y-Y plotter from triaxial test; (a) Load versus displacement. (b) Volume change versus displacement. (c) Recording for the volume-change of the specimen during isotropic compression and subsequently allowed it for swelling.



(a)



(b)



(c)

Fig. 7.8 Change in area in mid-section in the triaxial test sample during failure; (a) Original sample. (b) Plastic failure. (c) Brittle failure.

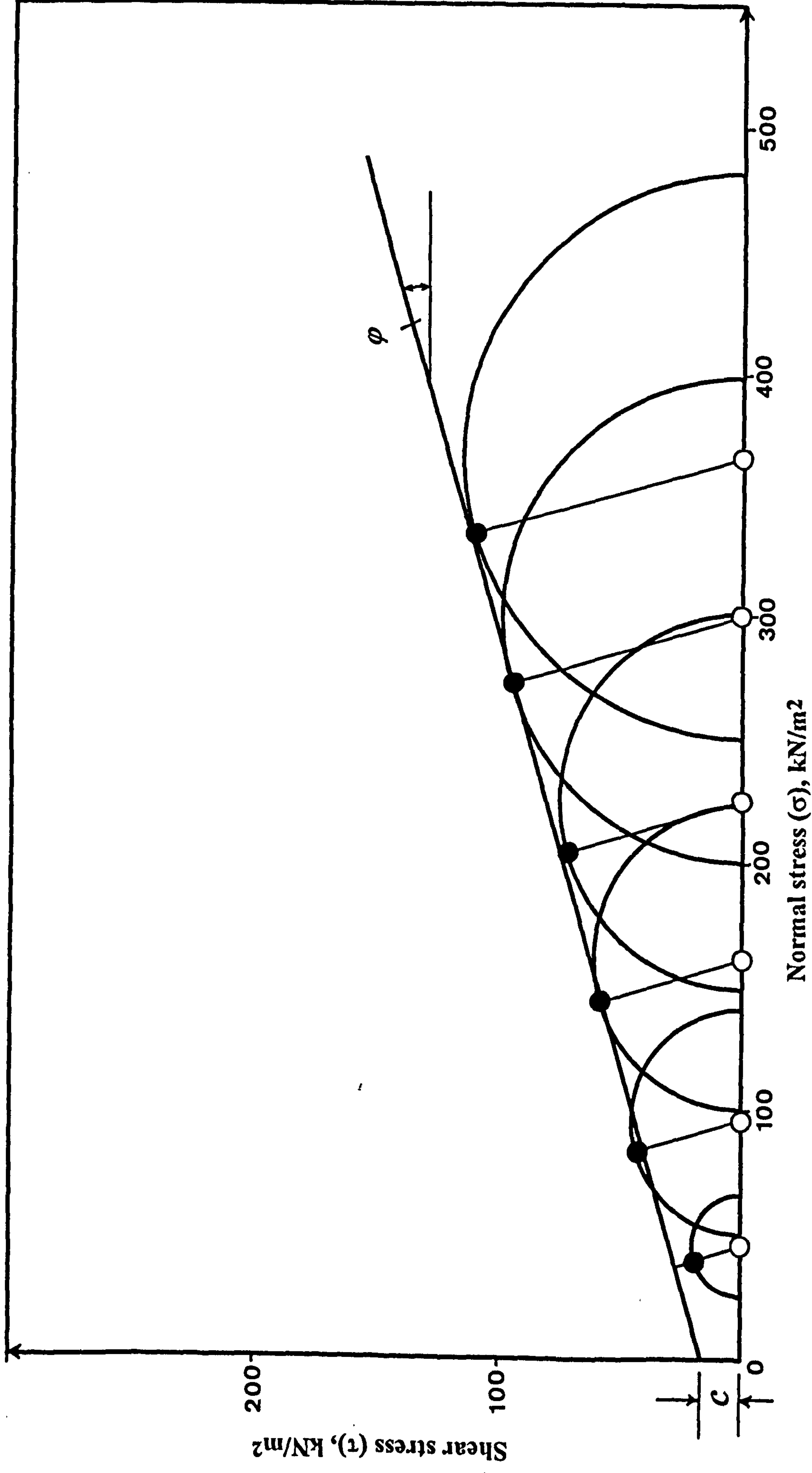


Fig. 7.9 Mohr's diagram (top part) for the experimental soil at 15.0% m.c.

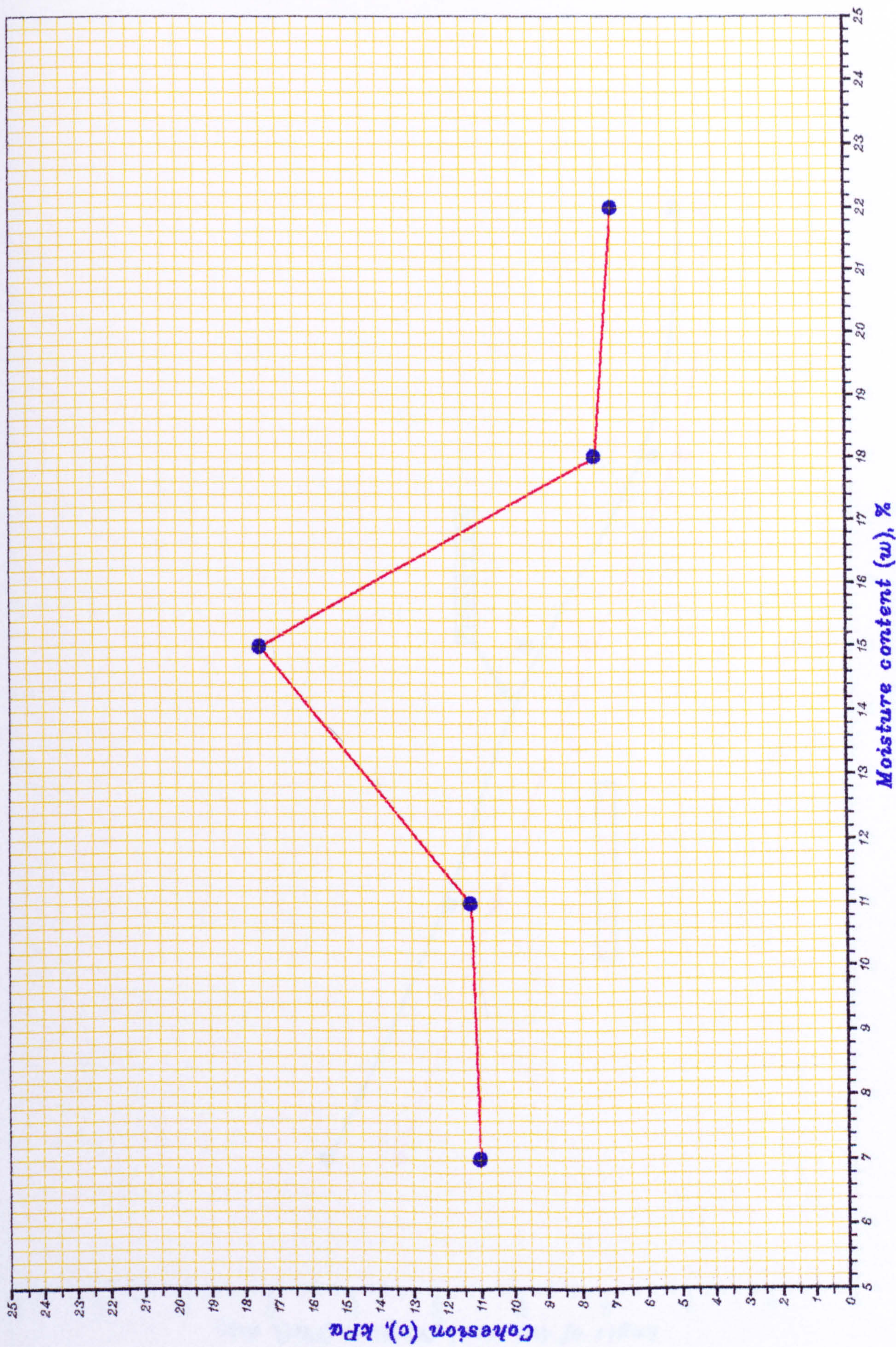


Fig. 7.10 Variation of cohesion, c with m.c.

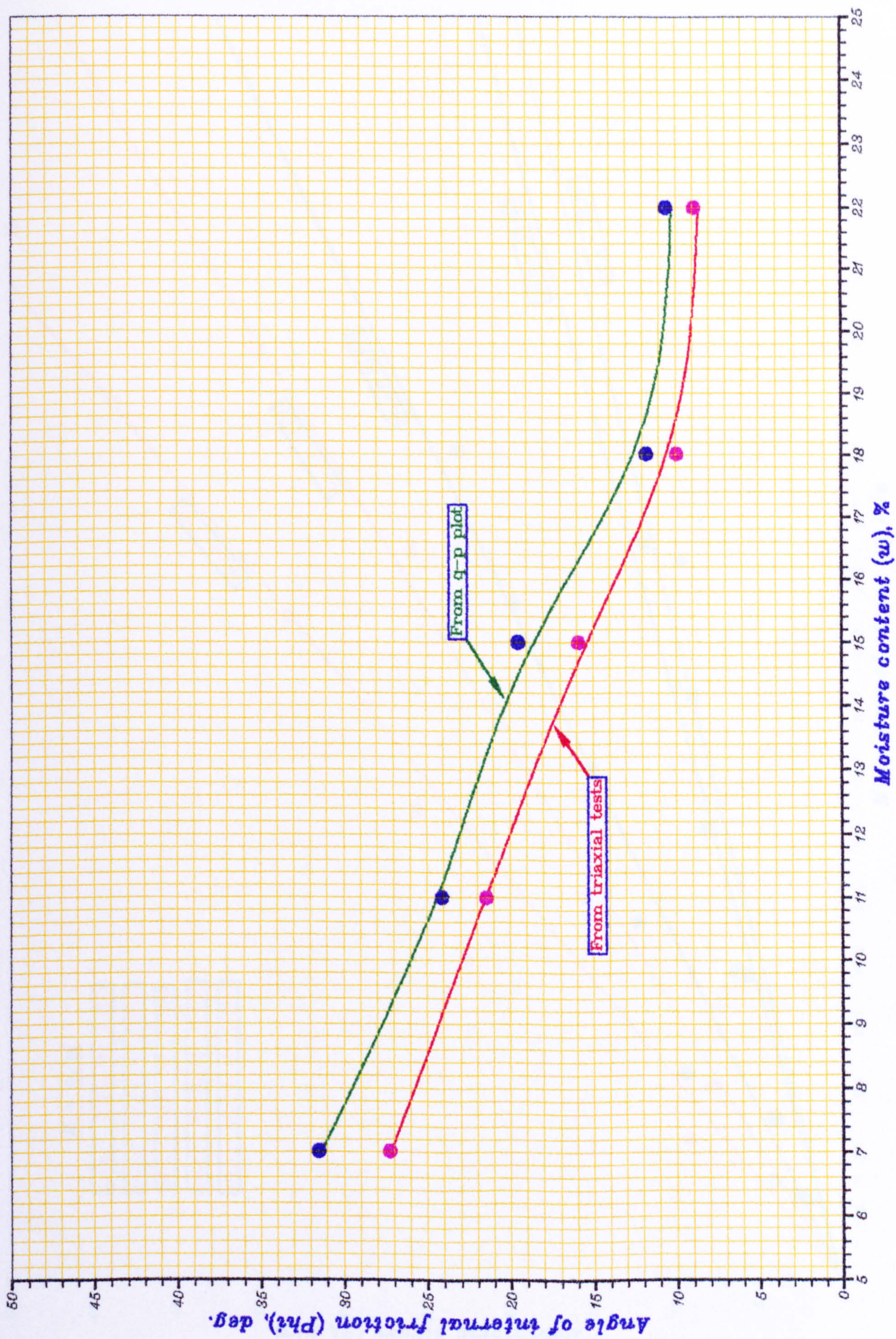


Fig. 7.11 Variation of the Angle of internal friction, Φ with $m.c.$

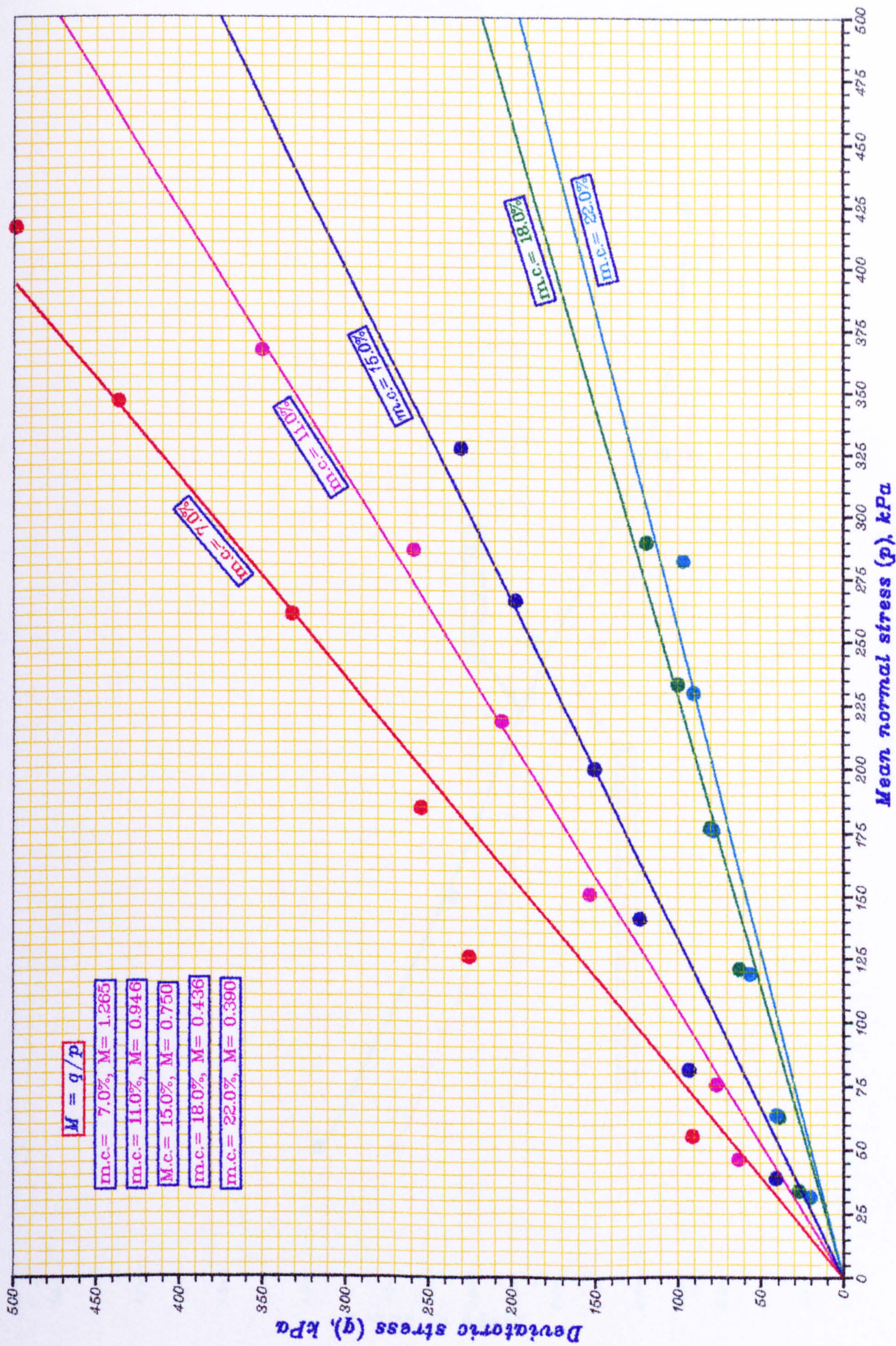


Fig. 7.12 The CSL at different m.c. in q-p plane

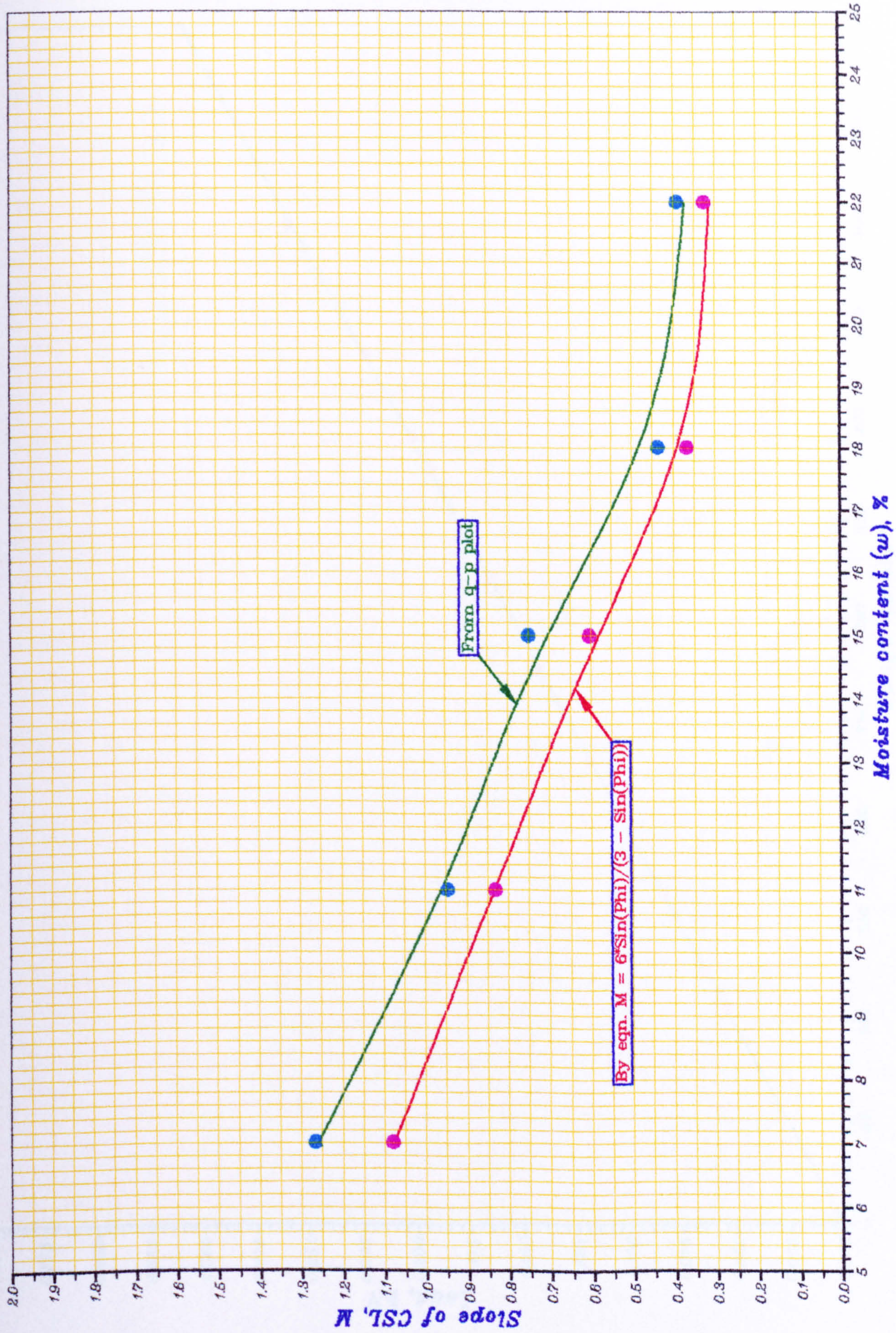


Fig. 7.13 Variation of the Slope of CSL, M with $m.c.$

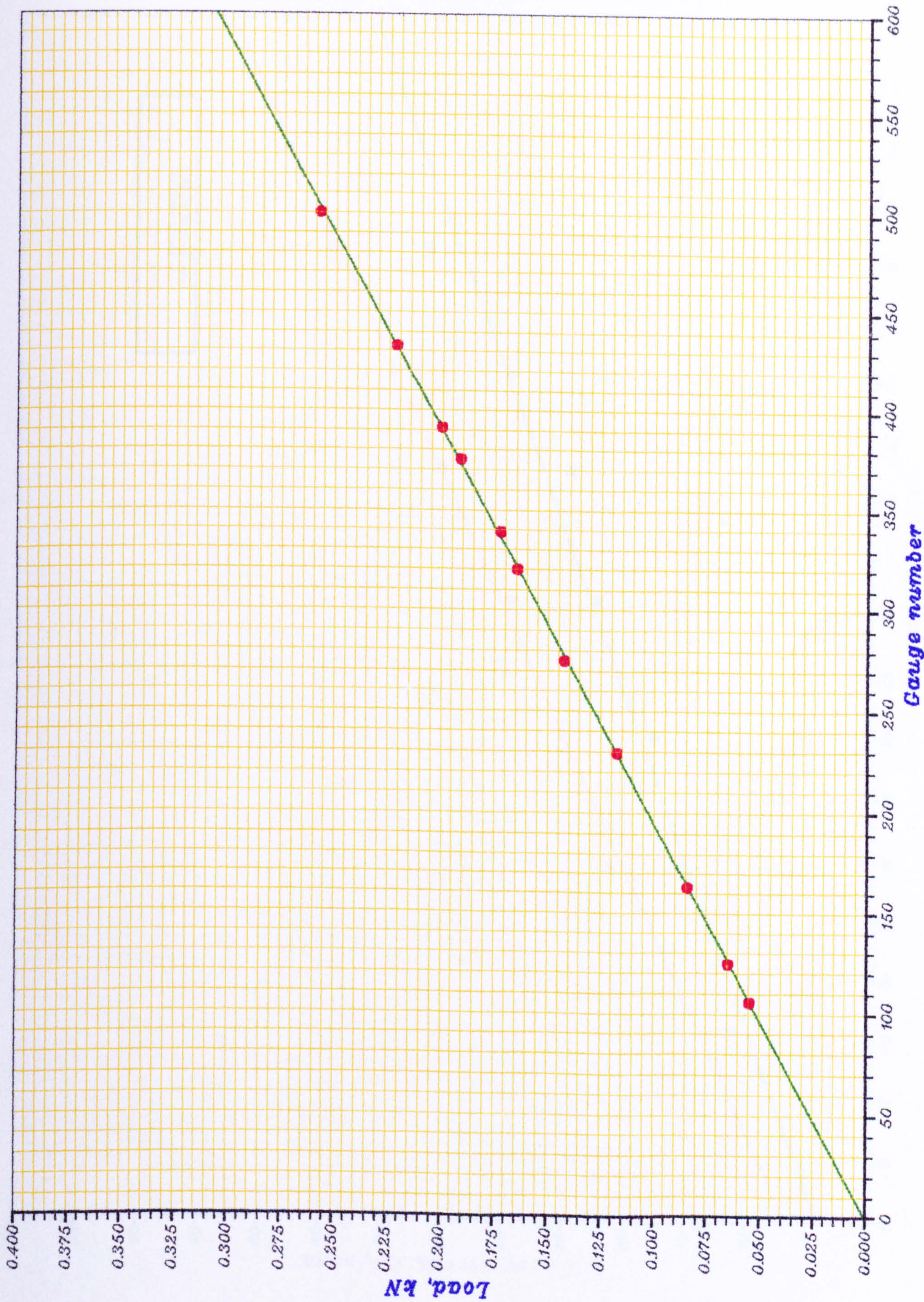


Fig. 7.14 The calibration (compression) of proving ring in the Shear box apparatus

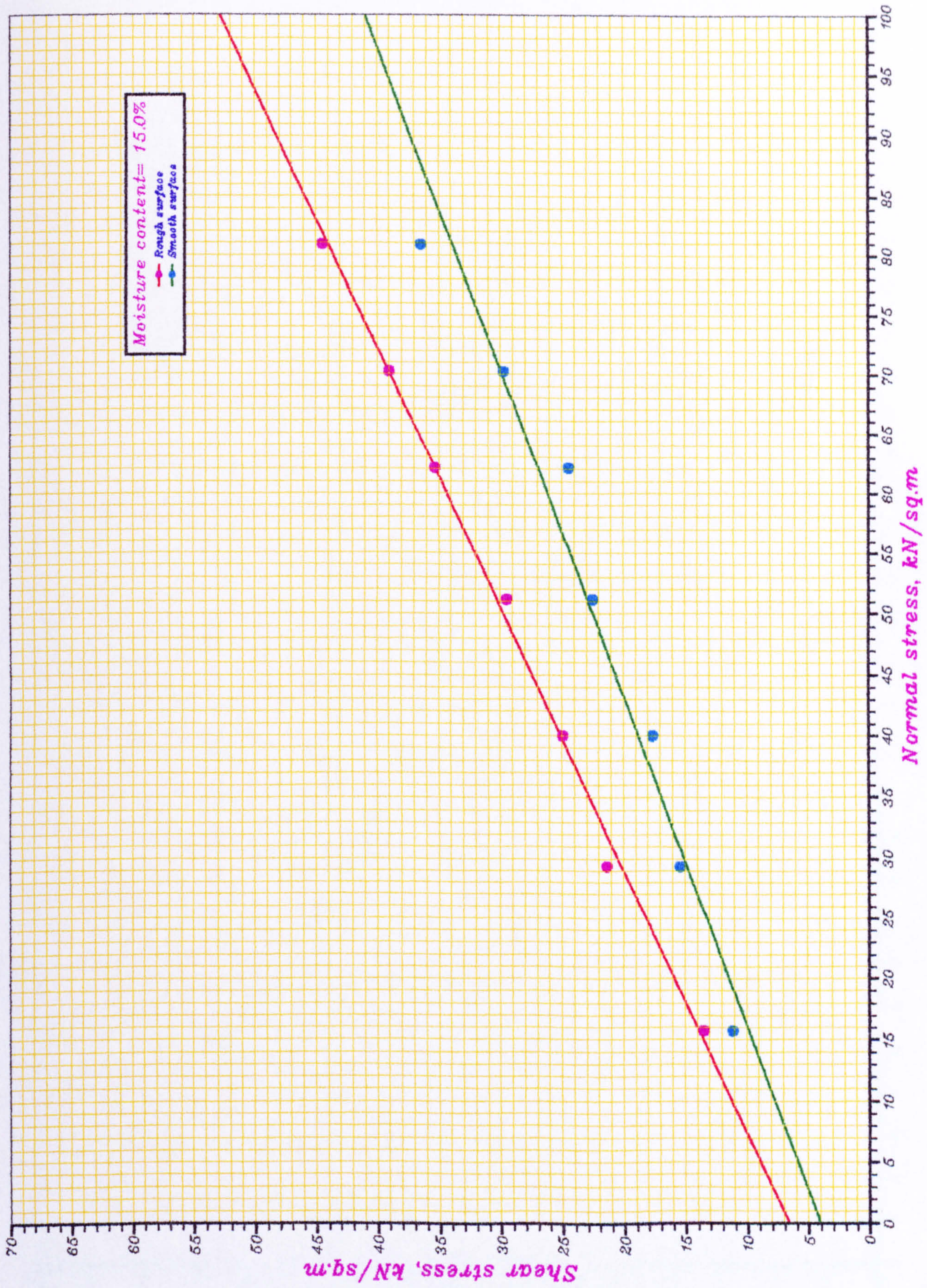


Fig. 7.15 A typical Normal stress vs Shear stress plot from Shear-box test at 15.0% m.c.

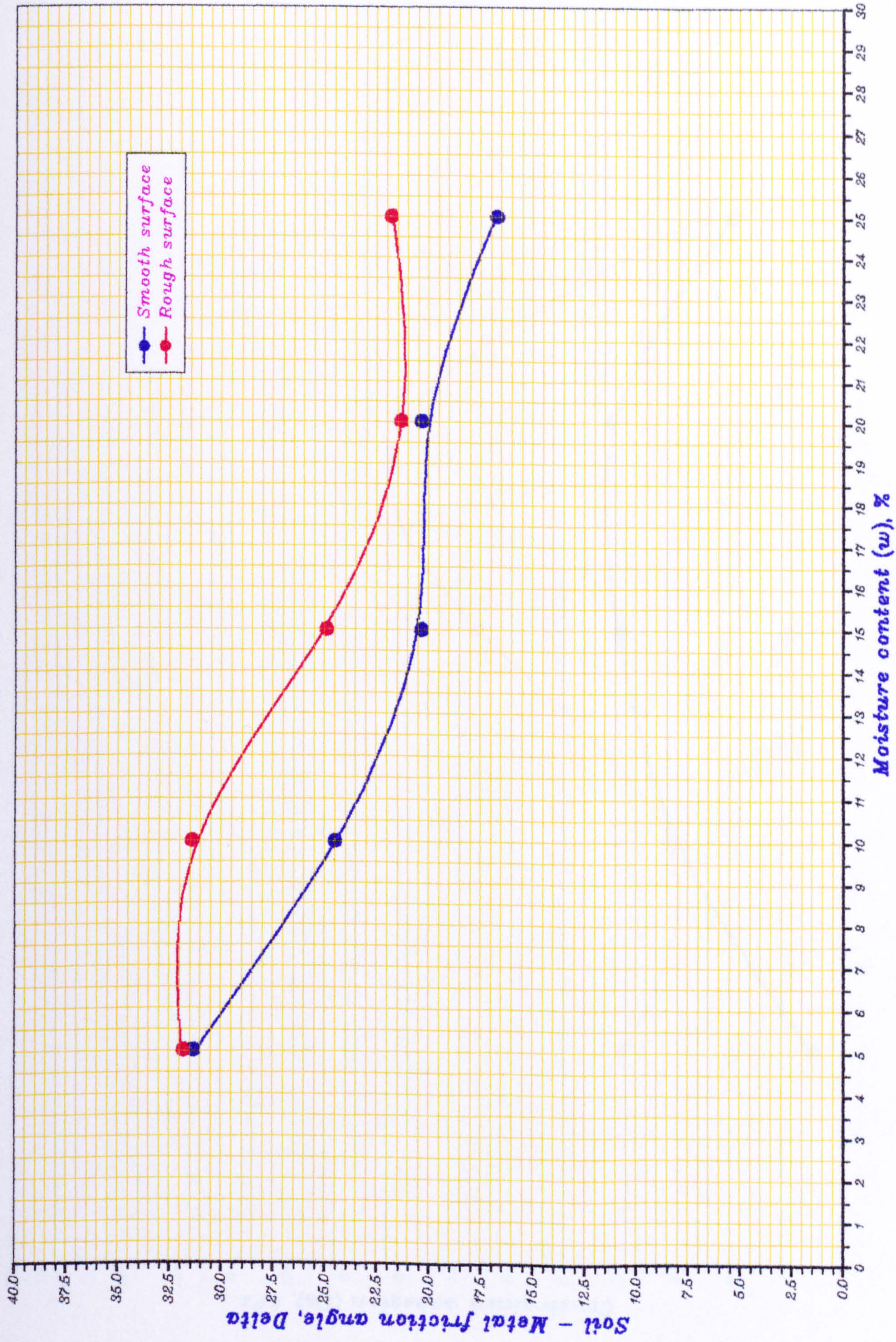


Fig. 7.16 The variation of Delta with m.c.

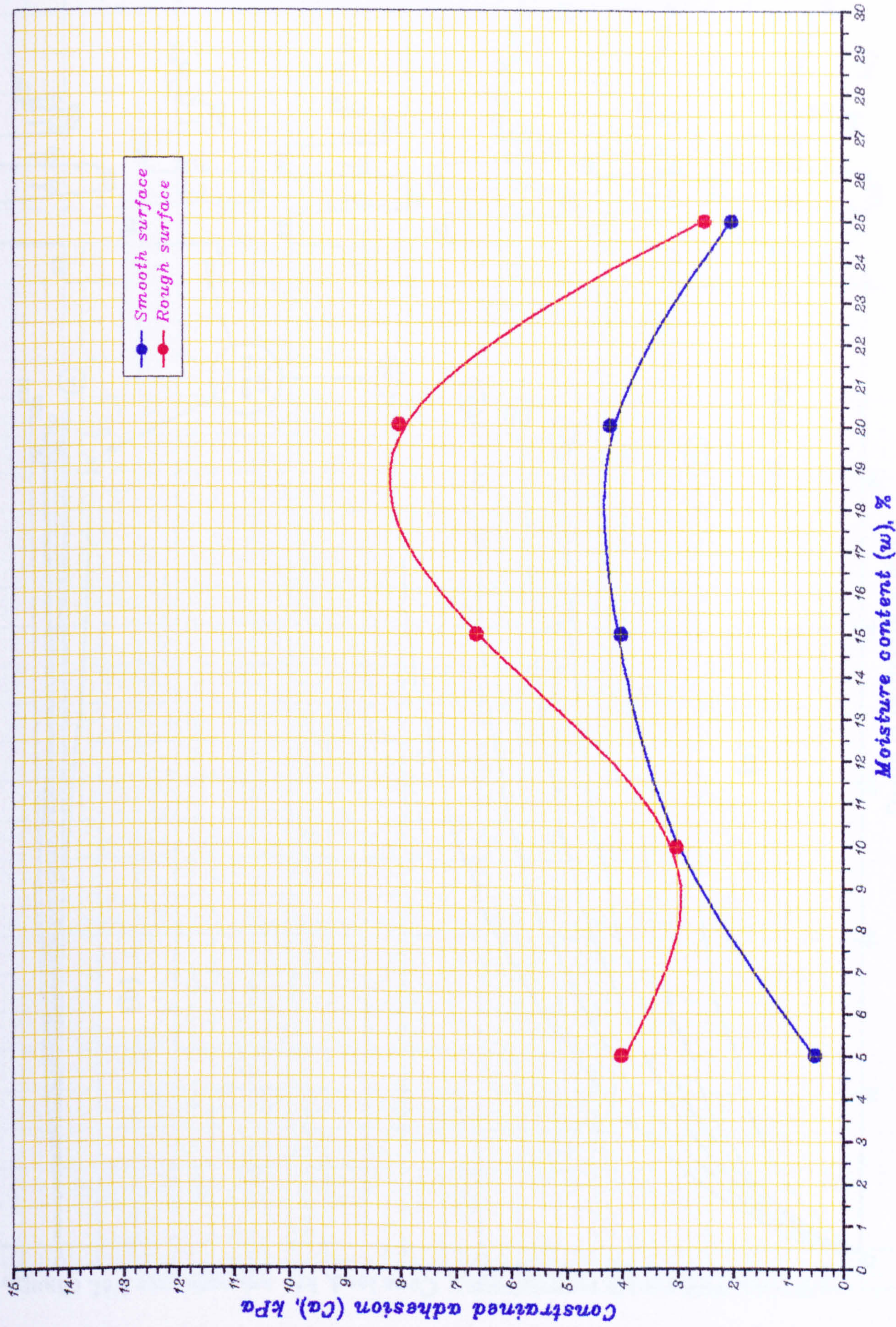


Fig. 7.17 The variation of constrained adhesion, Ca with m.c.

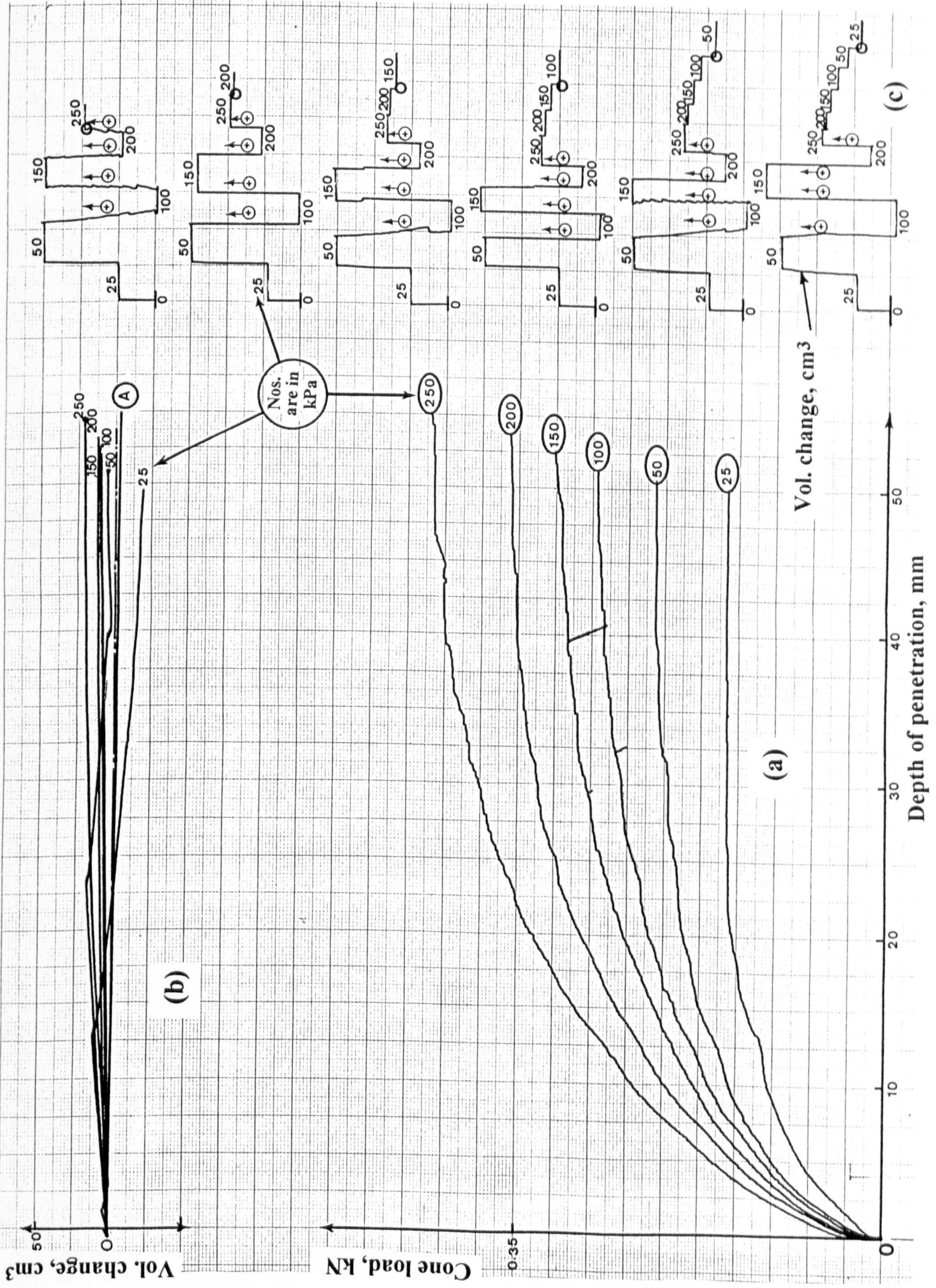


Fig. 7.18 A typical output on the X-Y-Y plotter from triaxial calibration chamber test; (a) Cone load versus displacement. (b) Volume-change versus displacement. (c) Recording for the volume-change of the specimen during isotropic compression and subsequently allowed it for swelling.

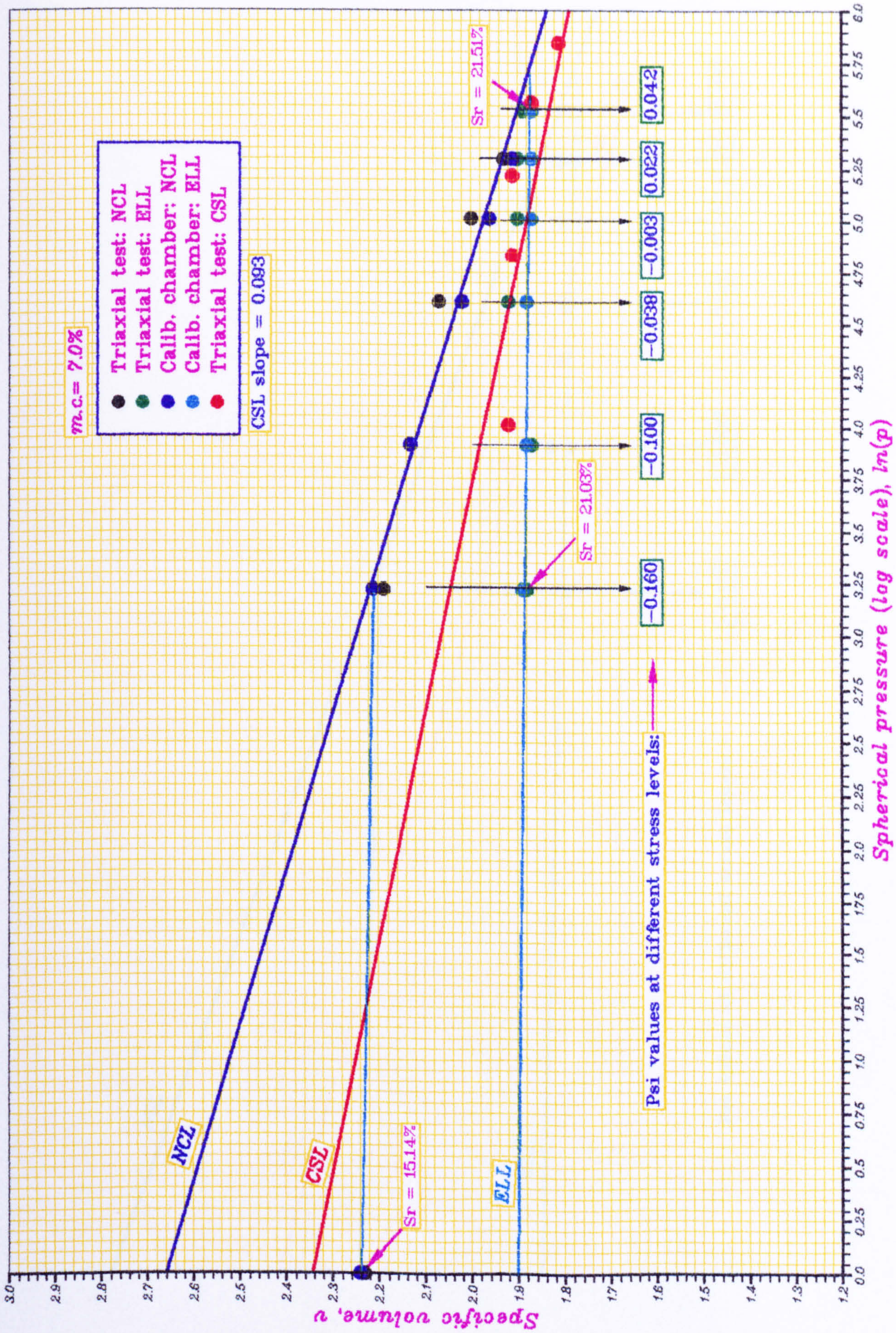


Fig. 7.19 A $v-\ln(p)$ plot from Triaxial and Calibration chamber tests at 7.0% m.c.

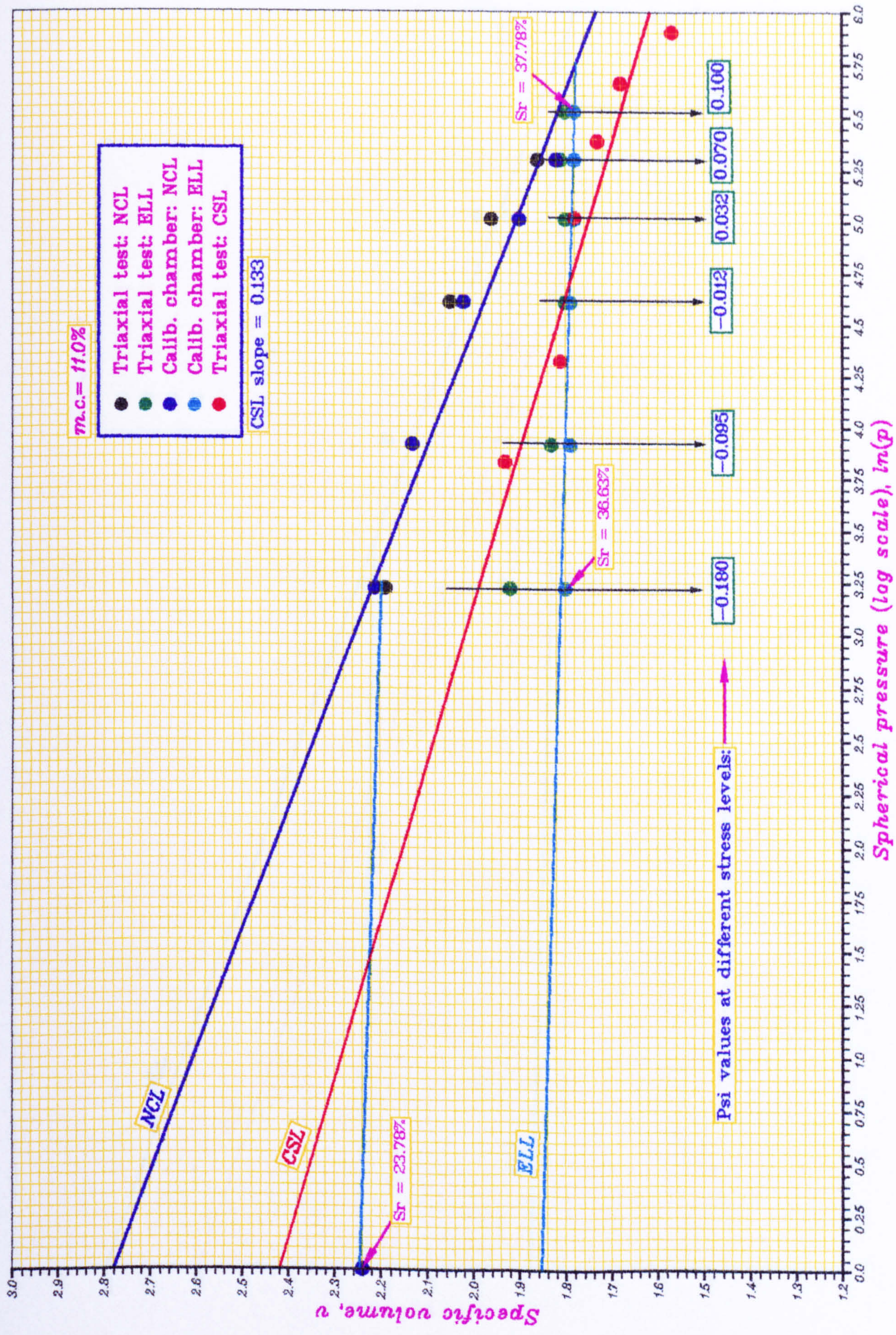


Fig. 7.20 A $v-\ln(p)$ plot from Triaxial and Calibration chamber tests at 11.0% m.c.

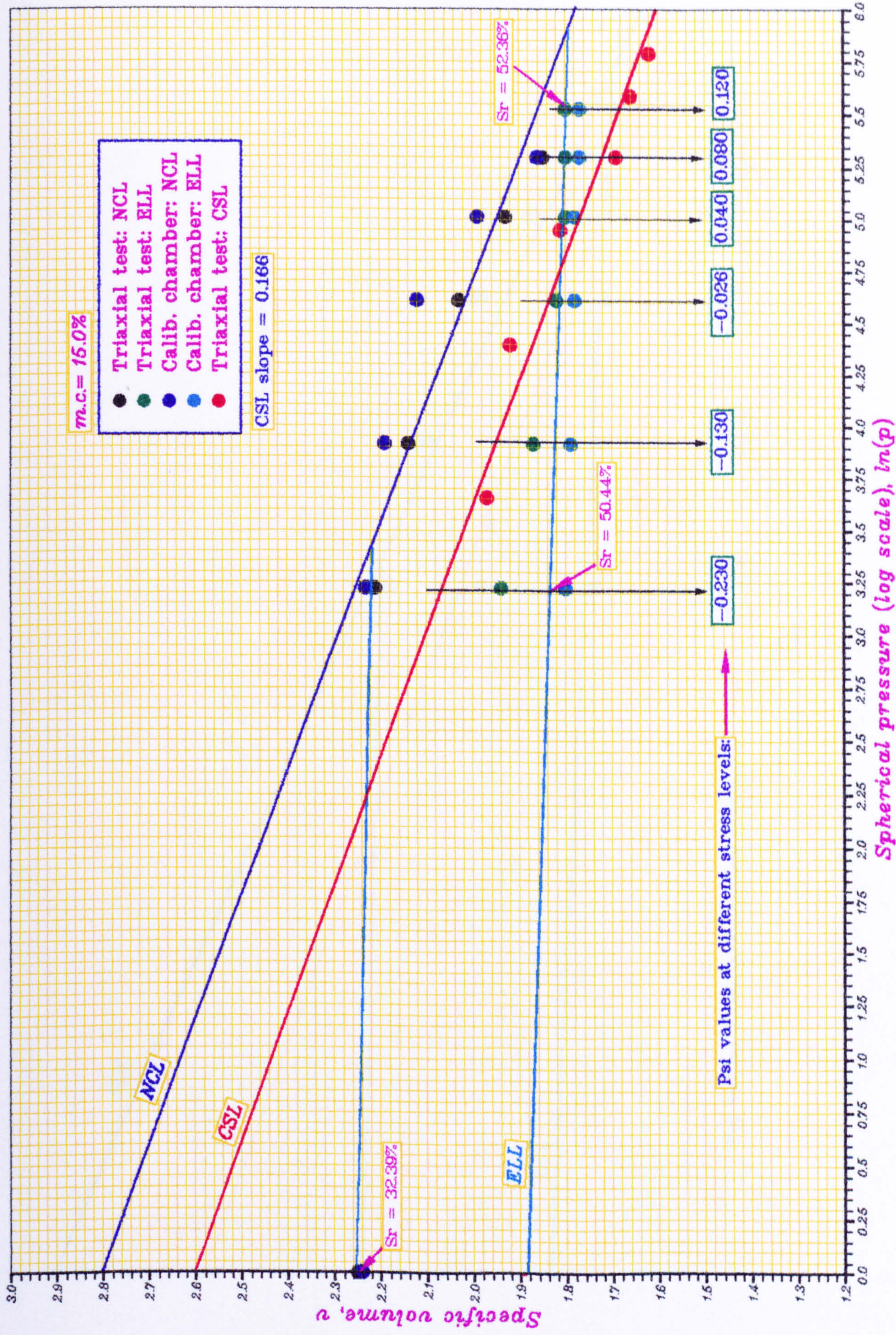


Fig. 7.21 A $v-\ln(p)$ plot from Triaxial and Calibration chamber tests at 15.0% m.c.

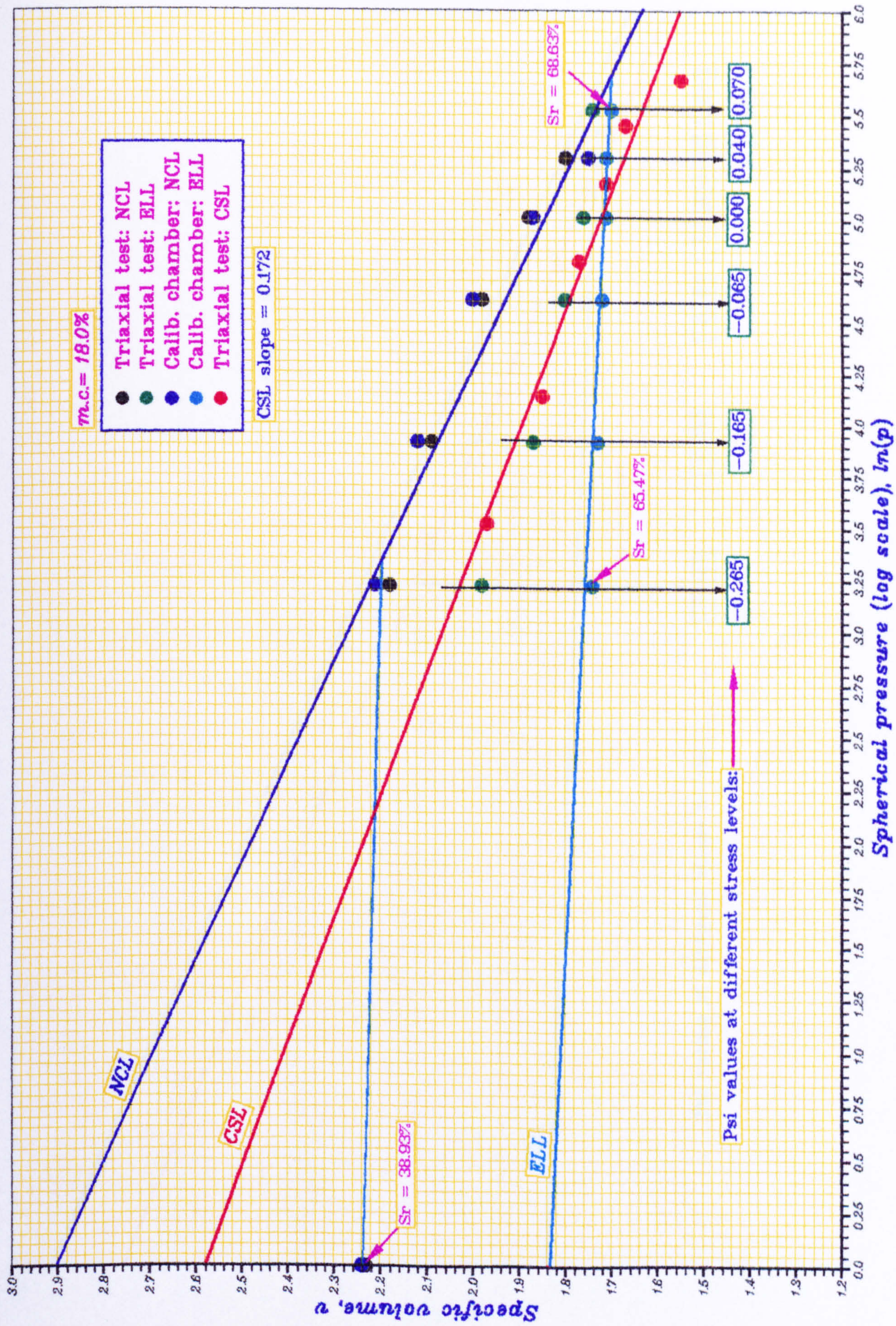


Fig. 7.22 A $v-\ln(p)$ plot from Triaxial and Calibration chamber tests at 18.0% m.c.

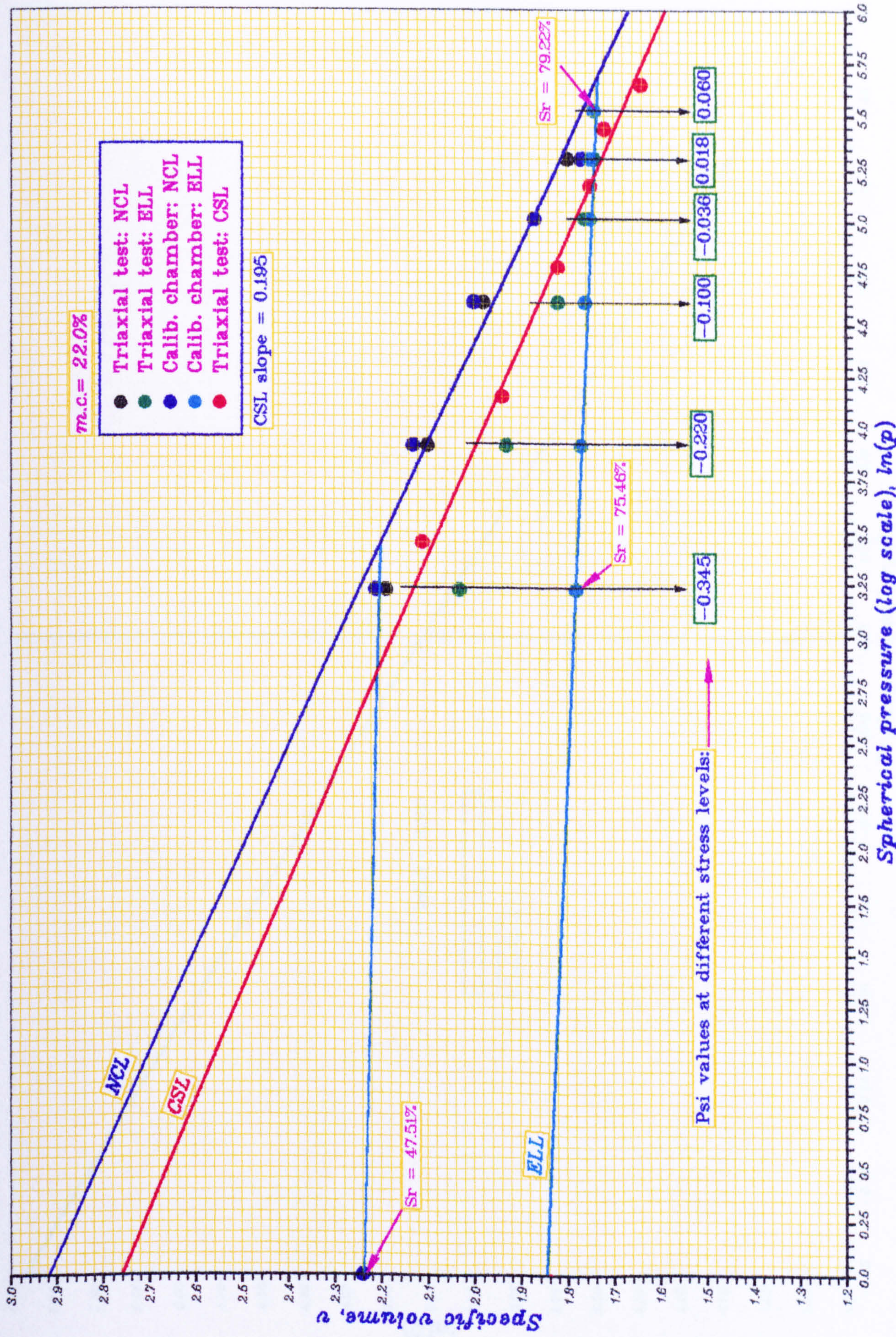


Fig. 7.23 A $v-\ln(p)$ plot from Triaxial and Calibration chamber tests at 22.0% m.c.

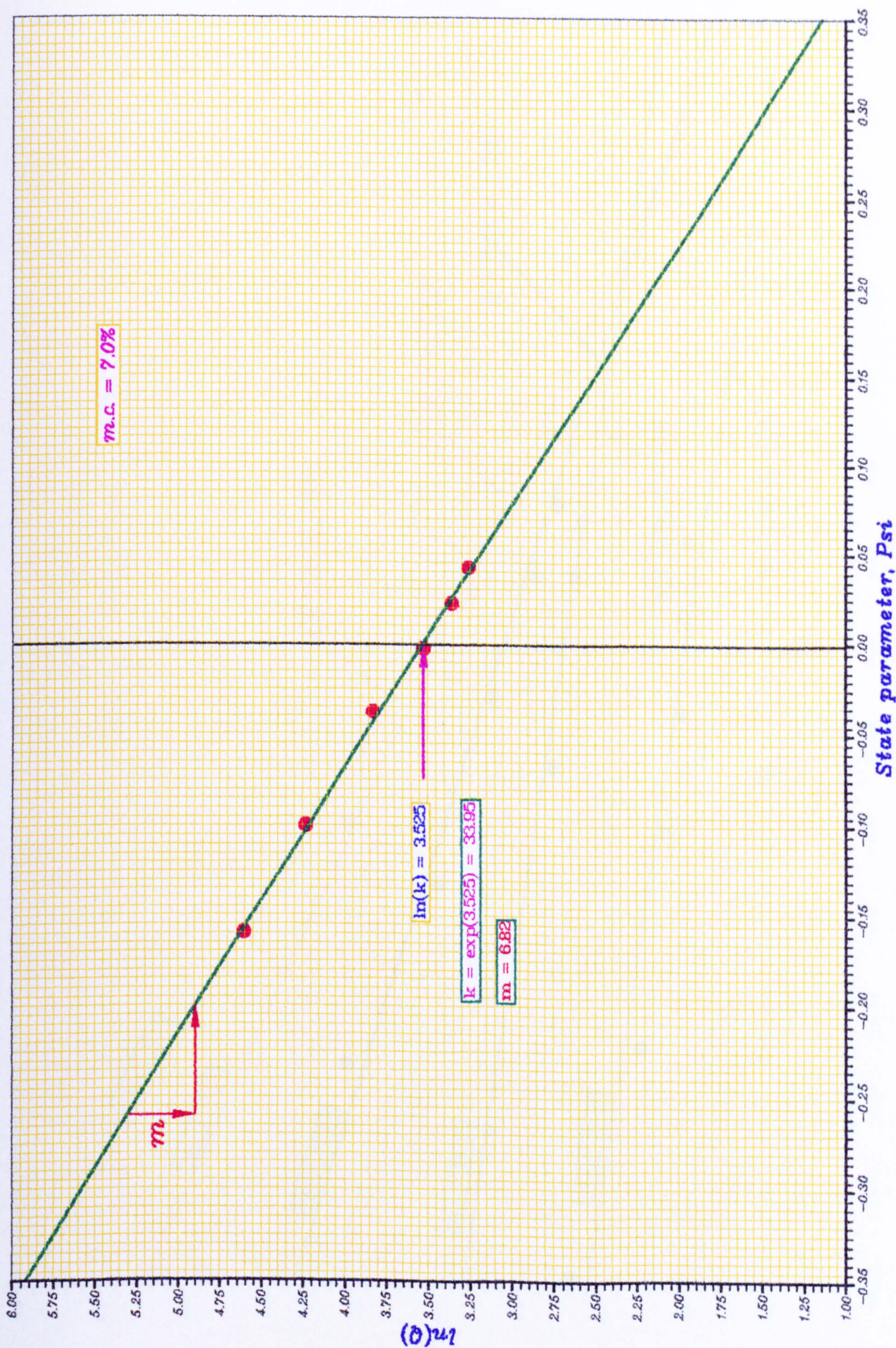


Fig. 7.24 A typical $\ln(Q)$ - Psi plot for 7.0% m.c.

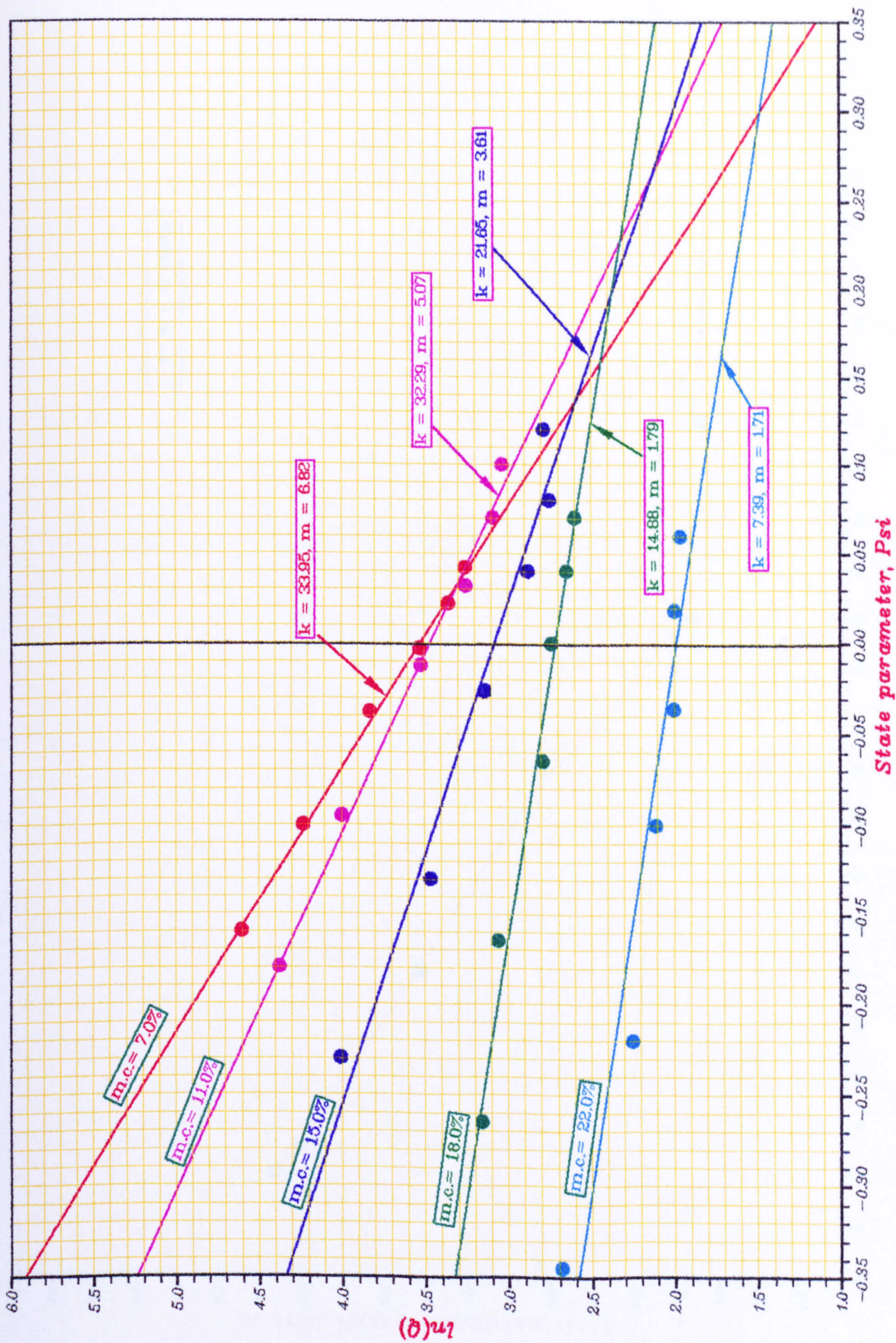


Fig. 7.25 The summarised $\ln(Q)$ - Psi plot for different m.c.

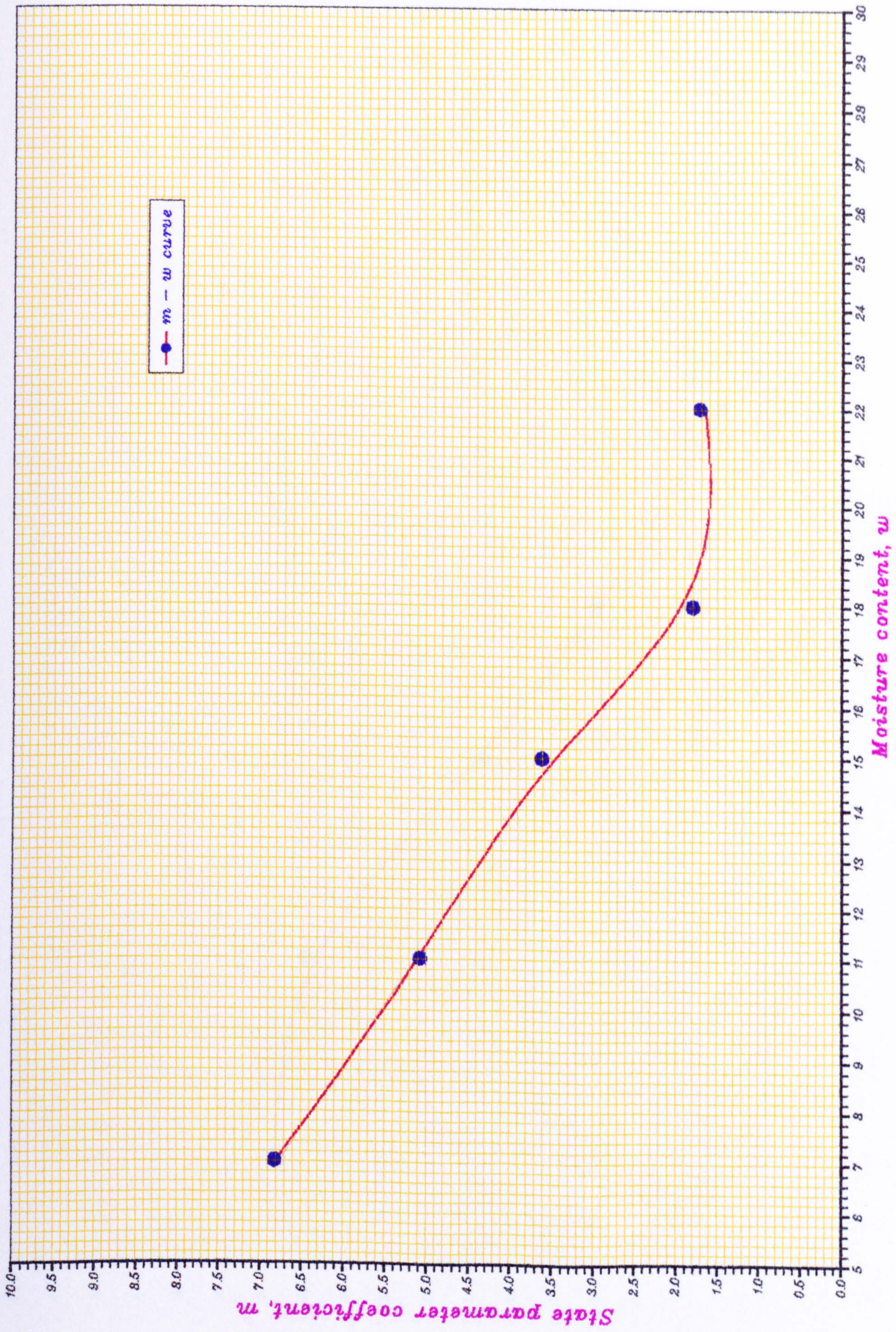


Fig. 7.26 The variation of State parameter coefficient, m with $m.c.$

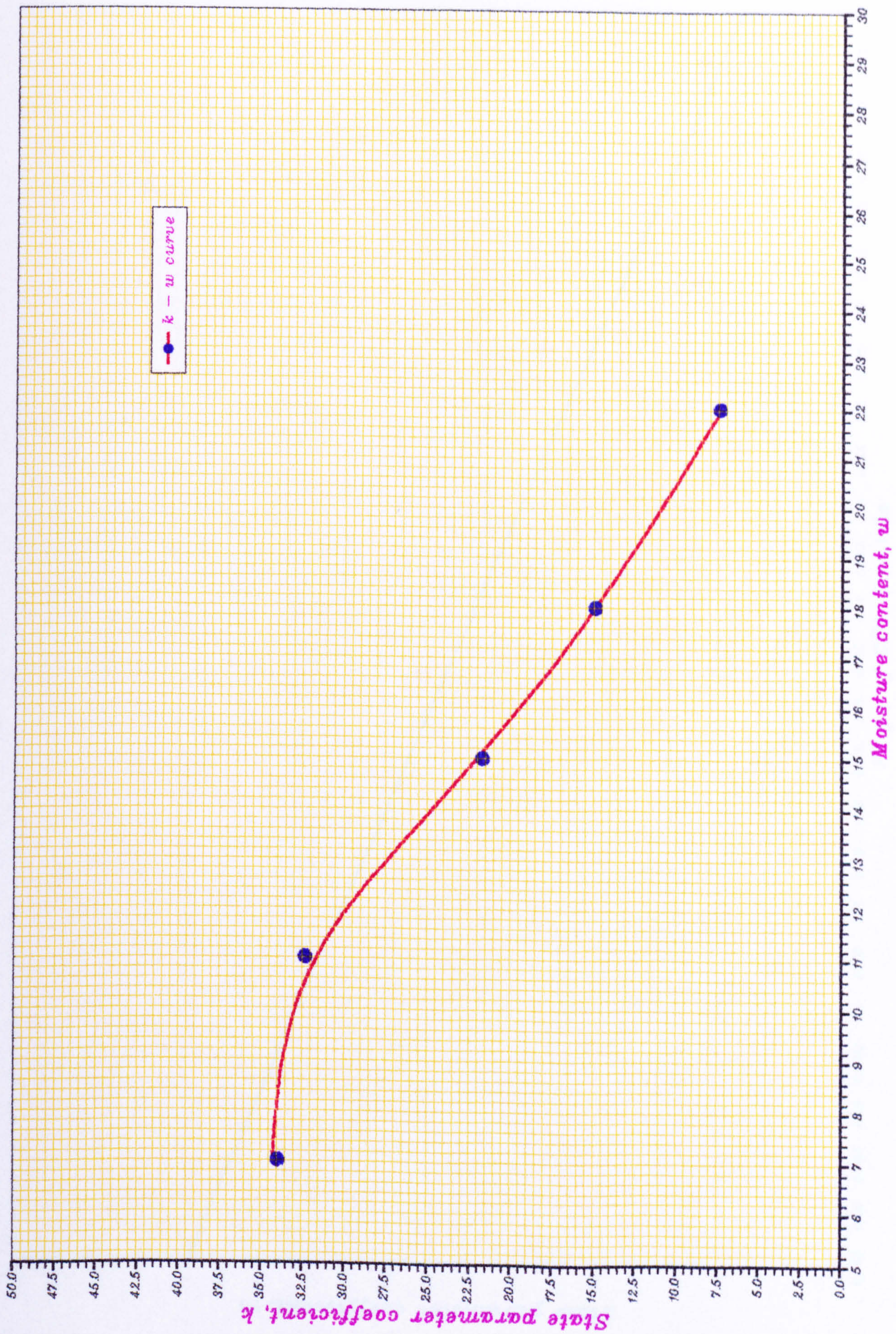


Fig. 7.27 The variation of State parameter coefficient, k with $m.c.$

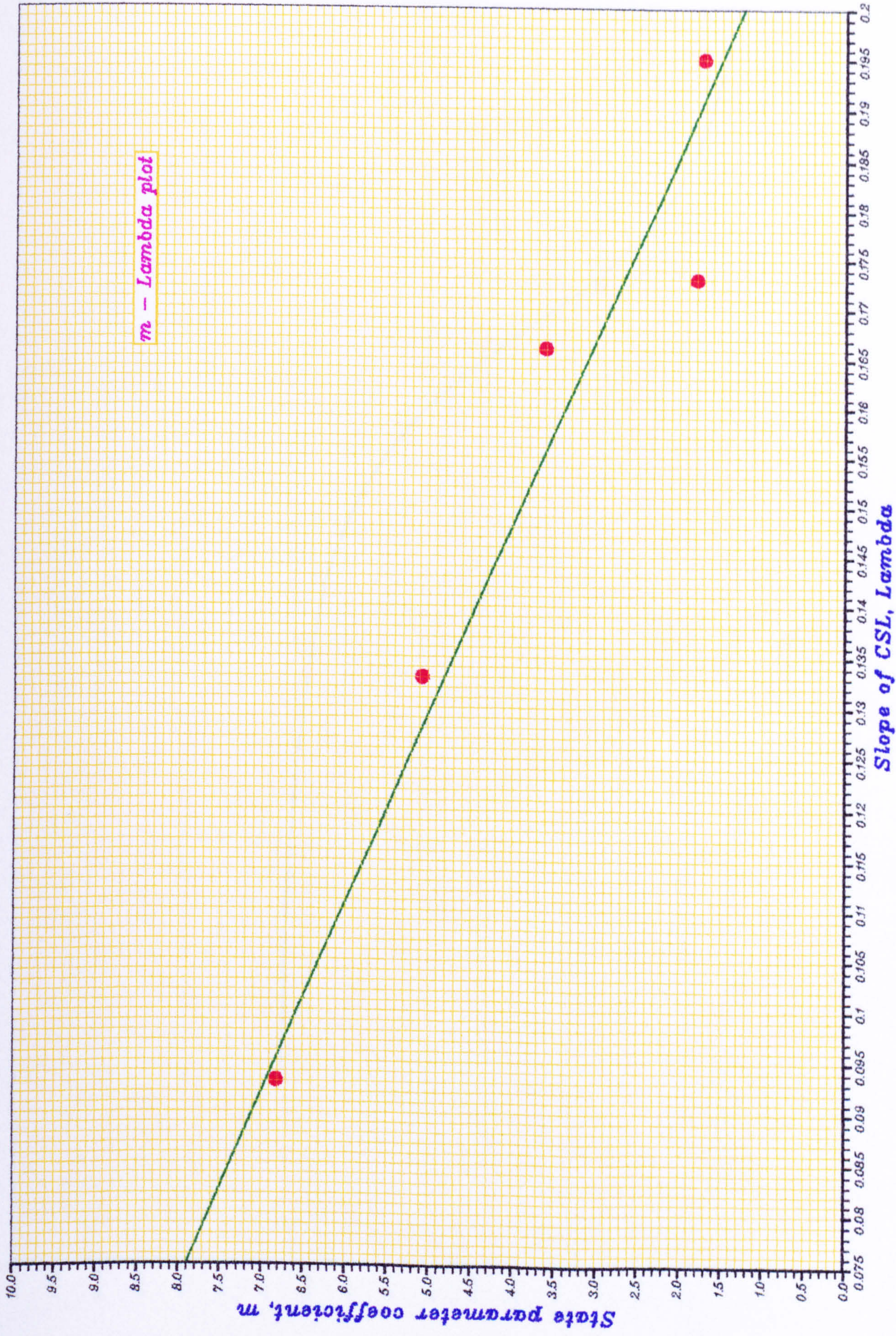


Fig. 7.28 The variation of State parameter coefficient, m with the slope of CSL

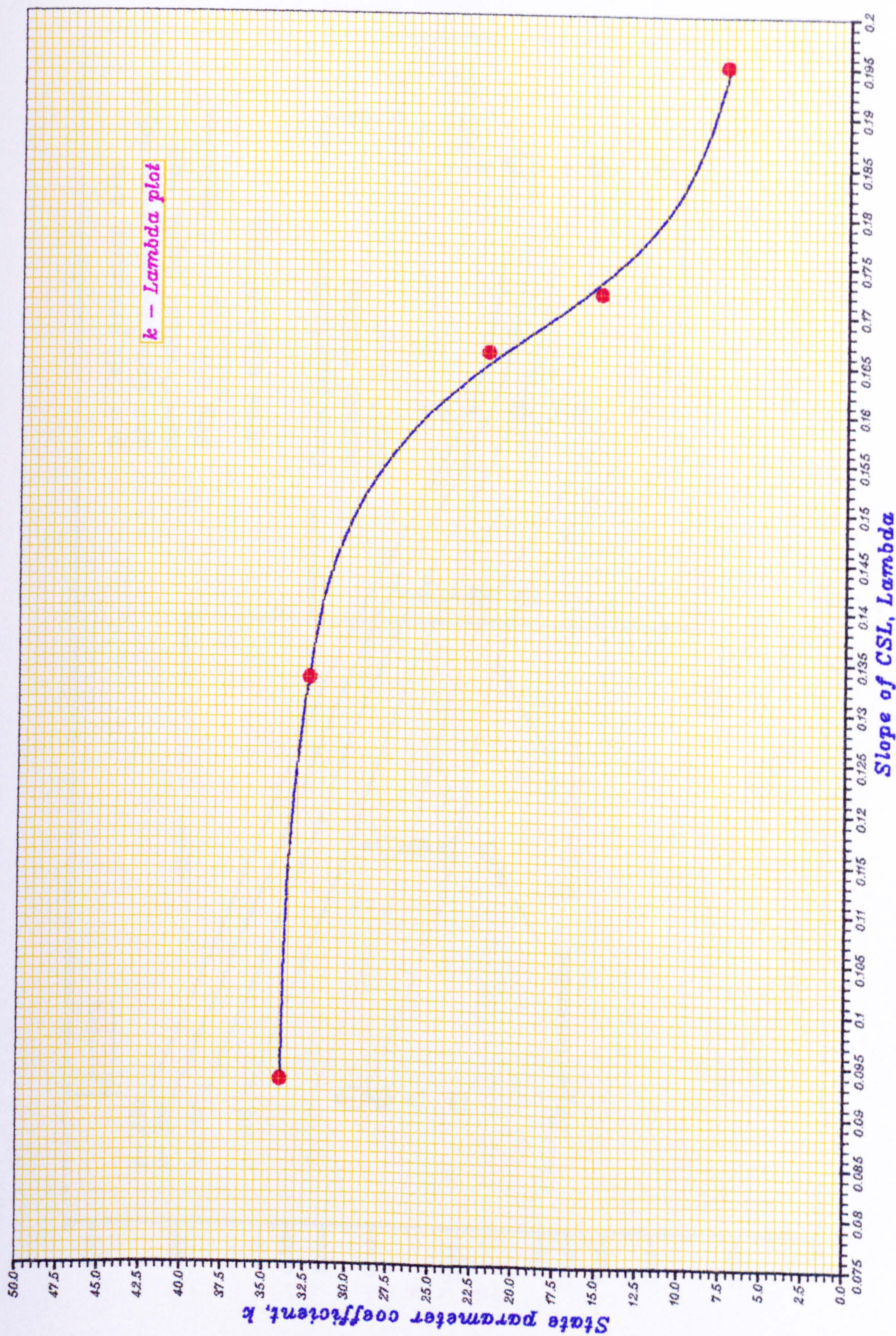


Fig. 7.29 The variation of State parameter coefficient, k with the slope of CSL

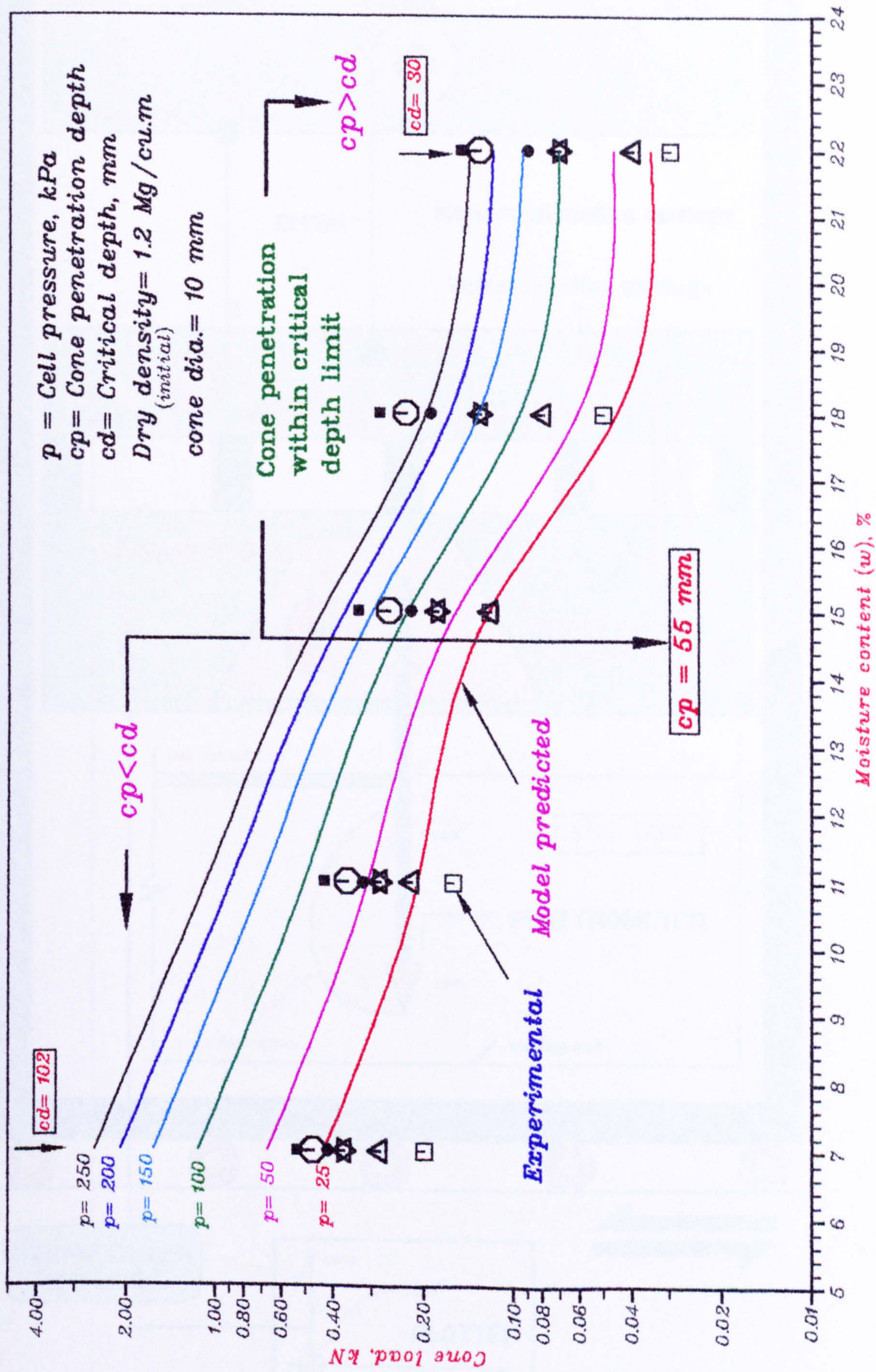


Fig. 7.30 A typical validation results of the cone penetrometer test in the Calib. chamber

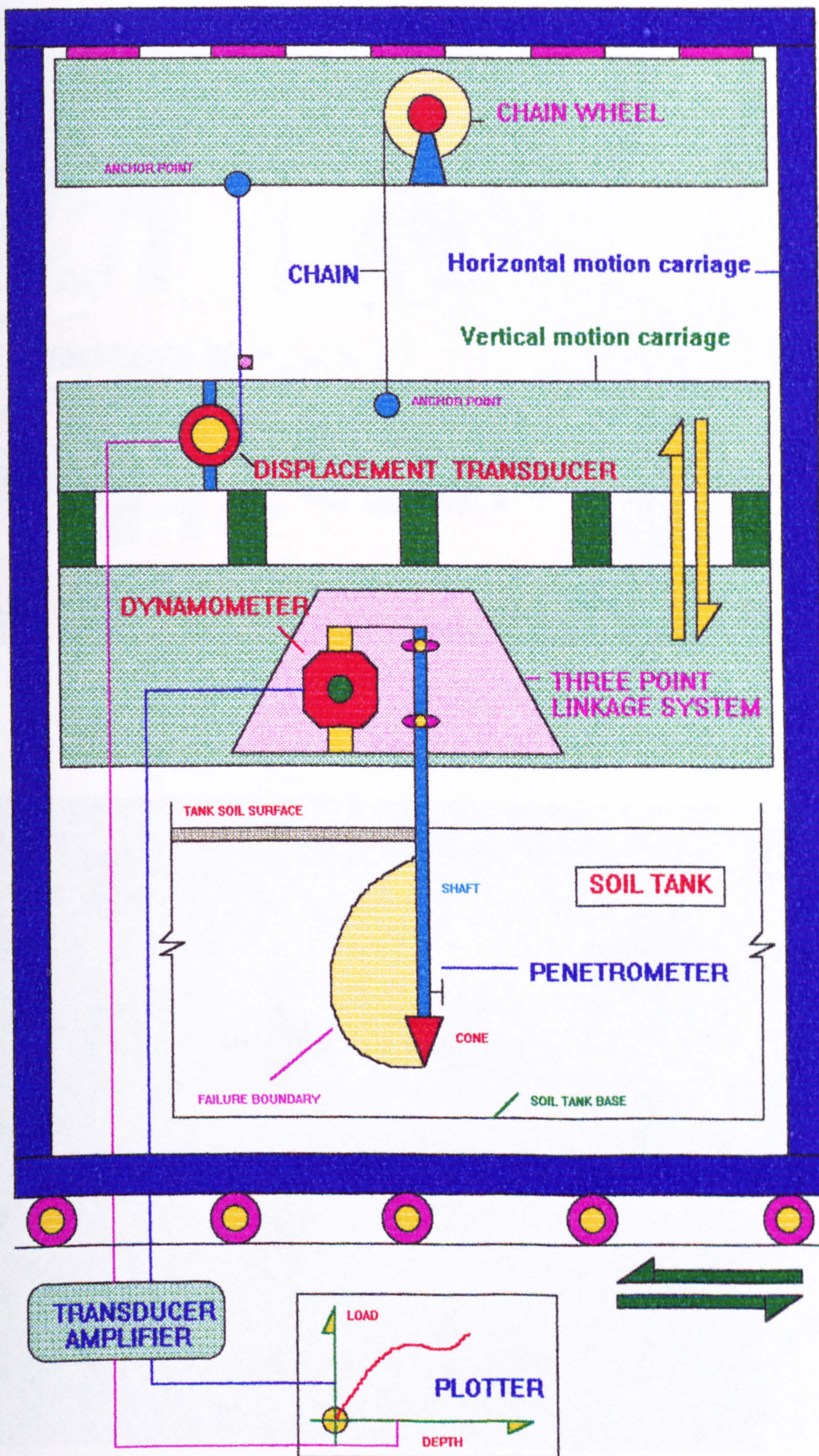


Fig. 7.31 The schematic arrangement of the experimental set-up for cone penetrometer test in the soil tank.

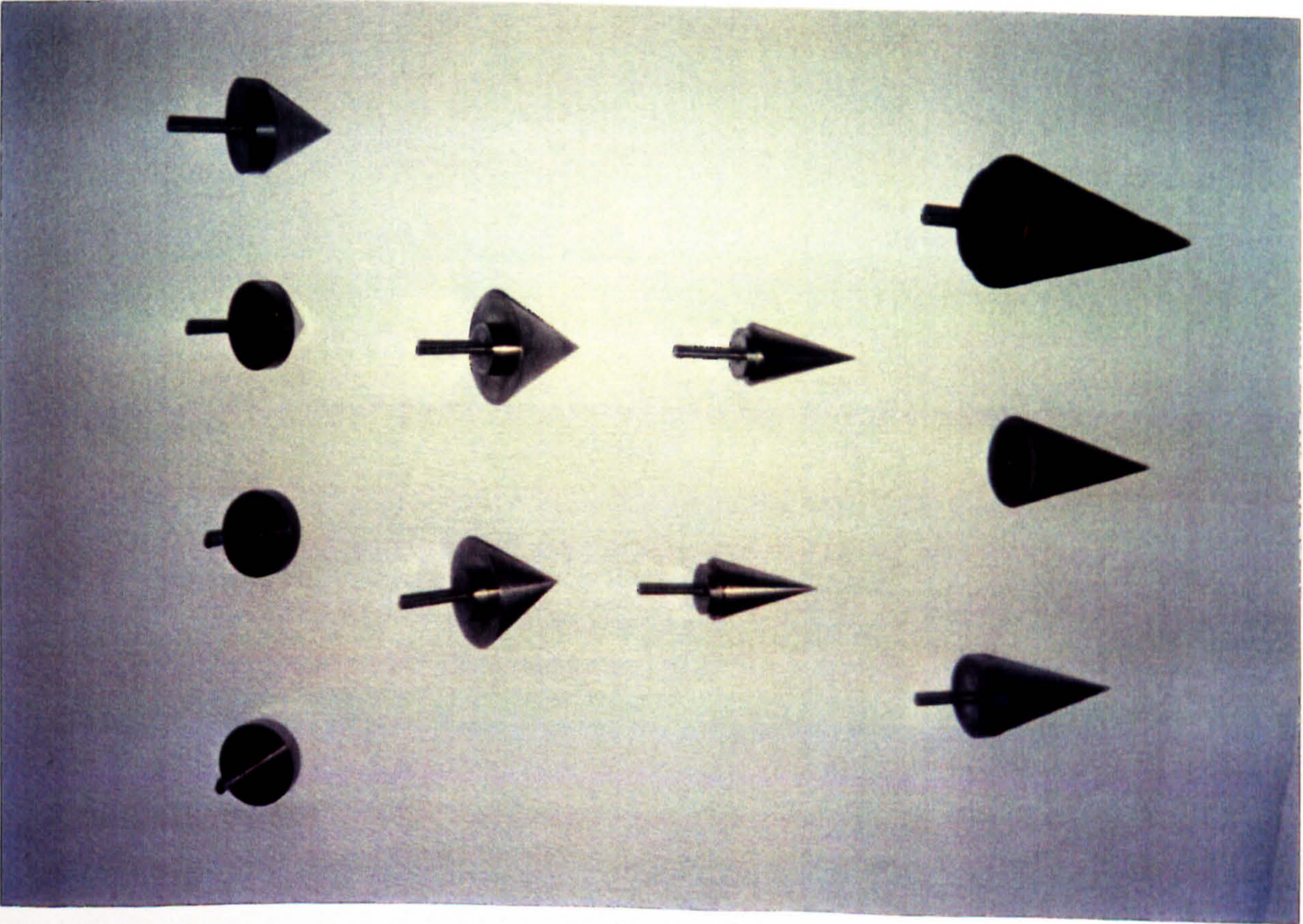


Fig. 7.32 (a) Cones with different geometry and surface roughness used in the Soil-tank experiment. (b) The complete experimental set-up of the cone penetrometer test in the Soil-tank.

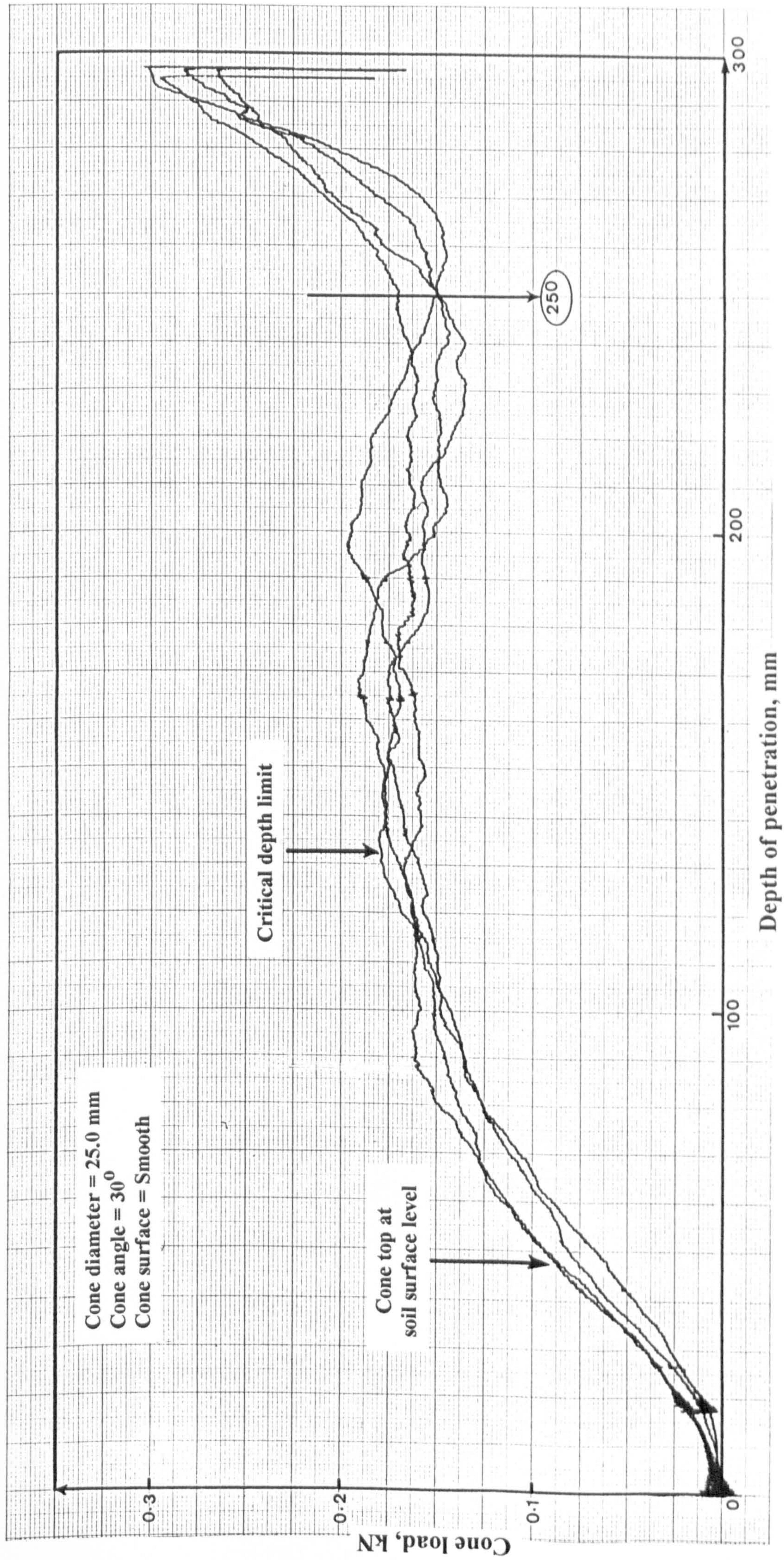


Fig. 7.33 A typical output on the X-Y plotter from the Soil-tank experiment.

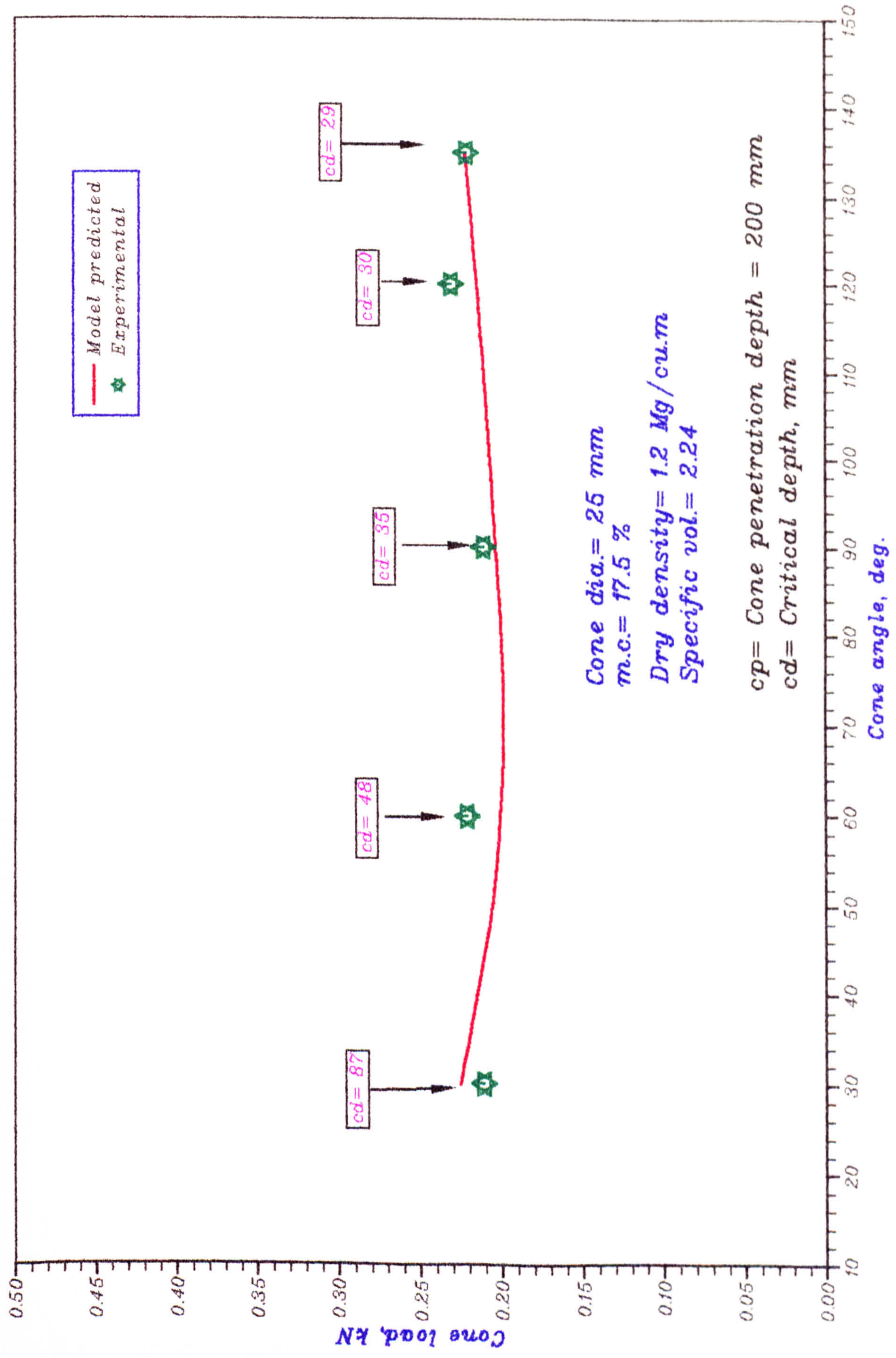


Fig. 7.34 A typical validation results of the cone penetrometer test in the Soil tank

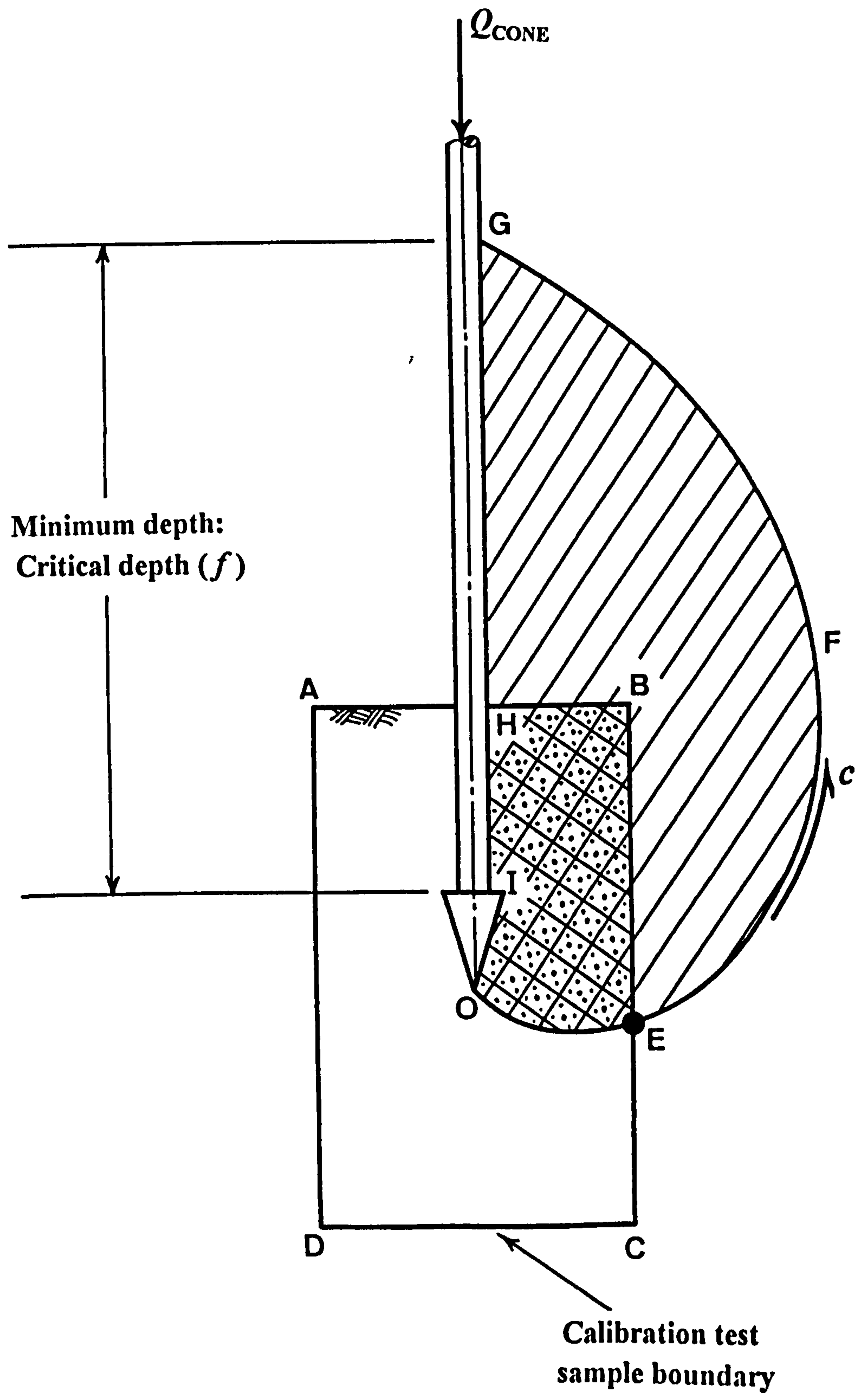


Fig. 8.1 An example of critical depth limit (f) in validation of the model for triaxial calibration chamber.

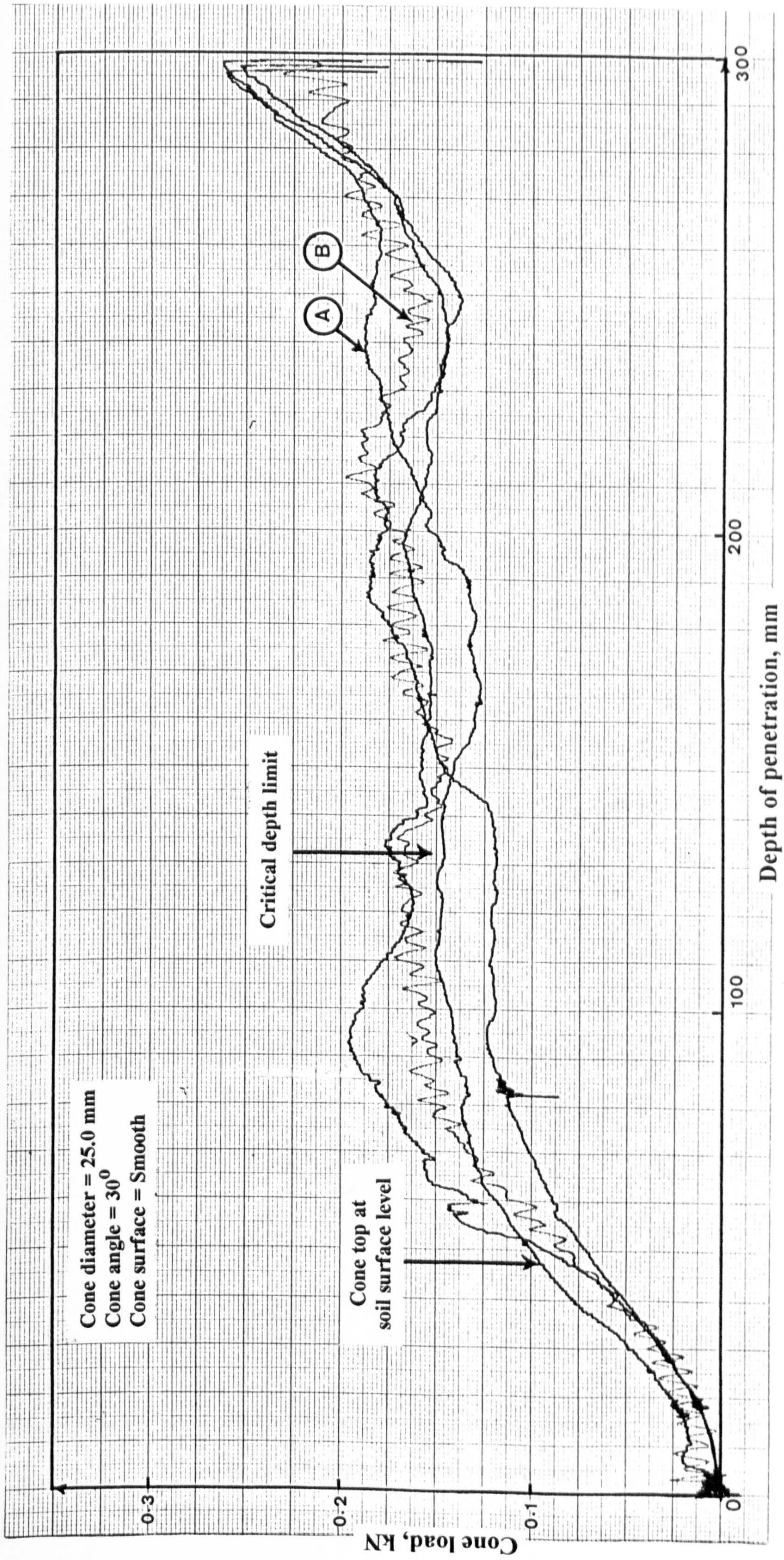


Fig. 8.2 A typical output on the X-Y plotter from the Soil-tank experiment showing the effect of penetration rate on penetration load.

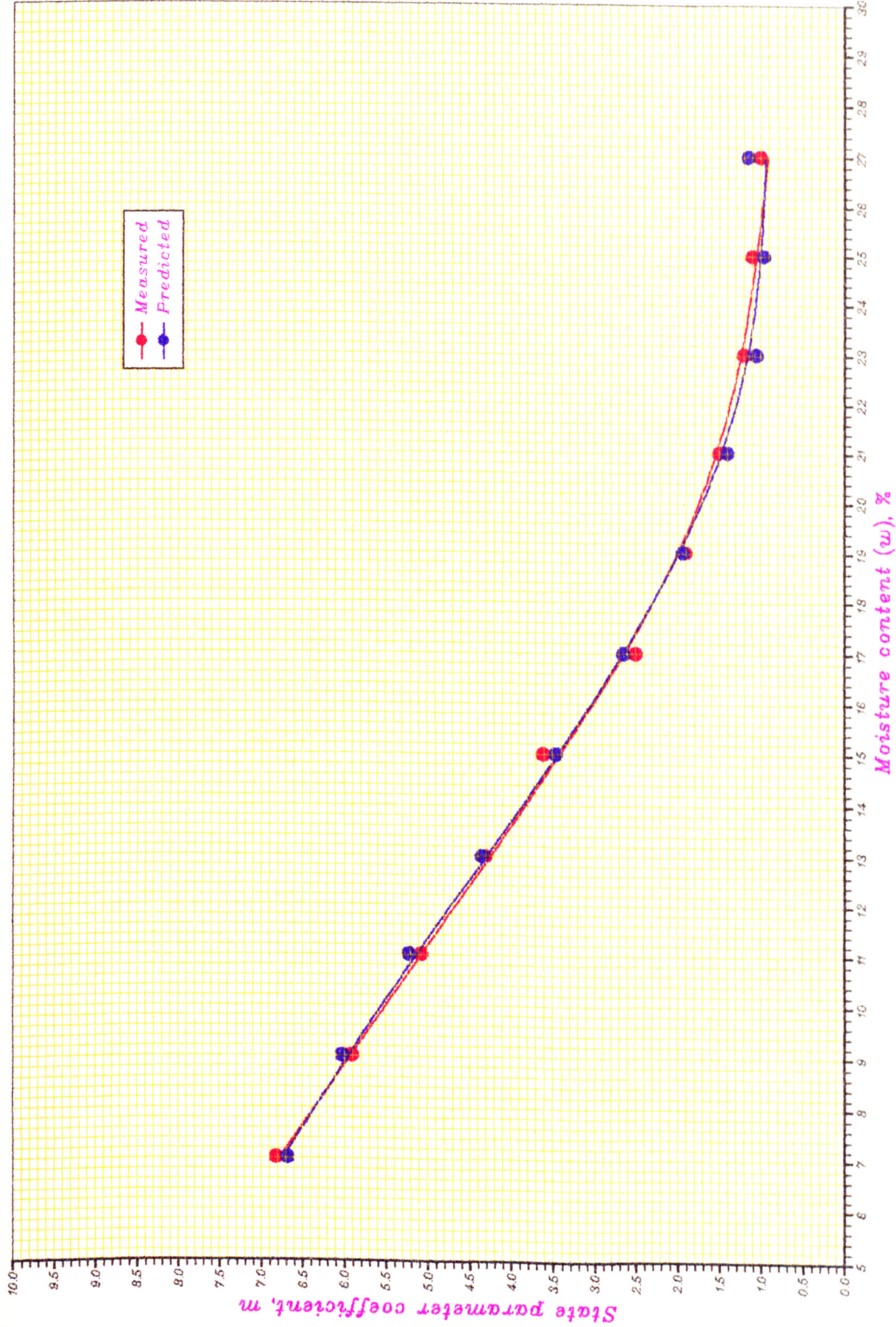


Fig. 9.1 Characterization of State parameter coefficient, m as a function of $m.c.$

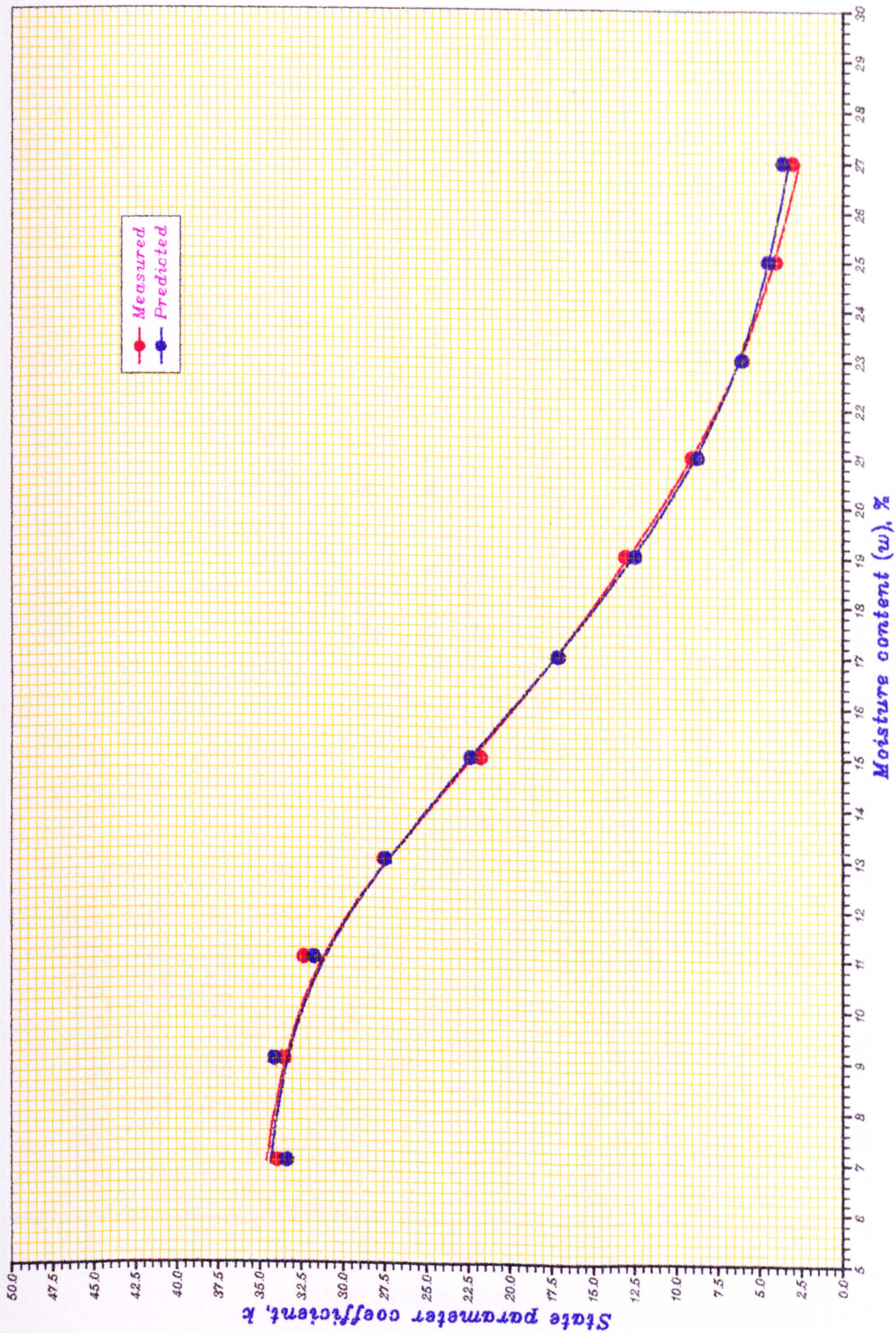


Fig. 9.2 Characterization of State parameter coefficient, k as a function of m.c.

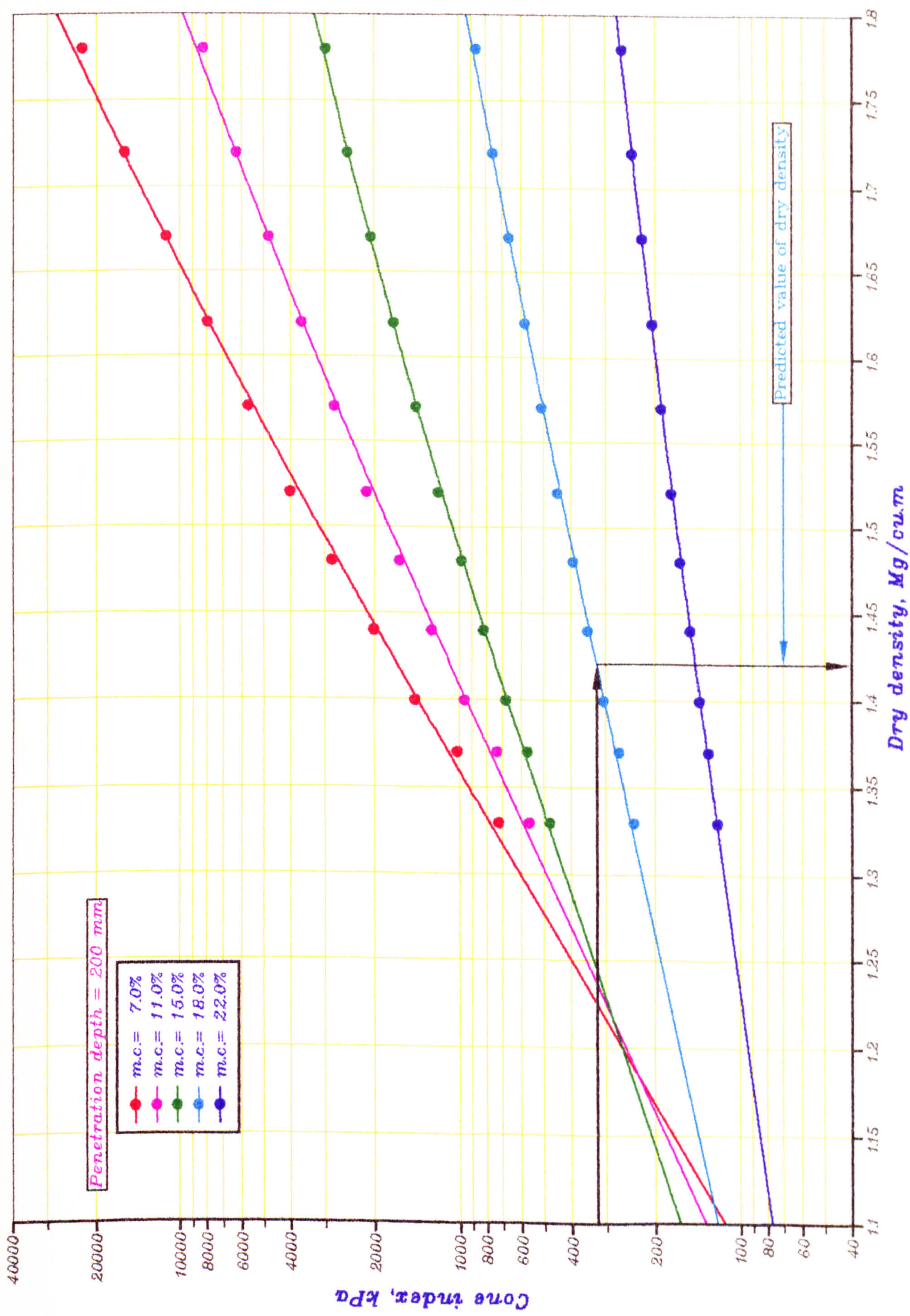


Fig. 9.3 Prediction of Cone index against Dry density at different m.c.

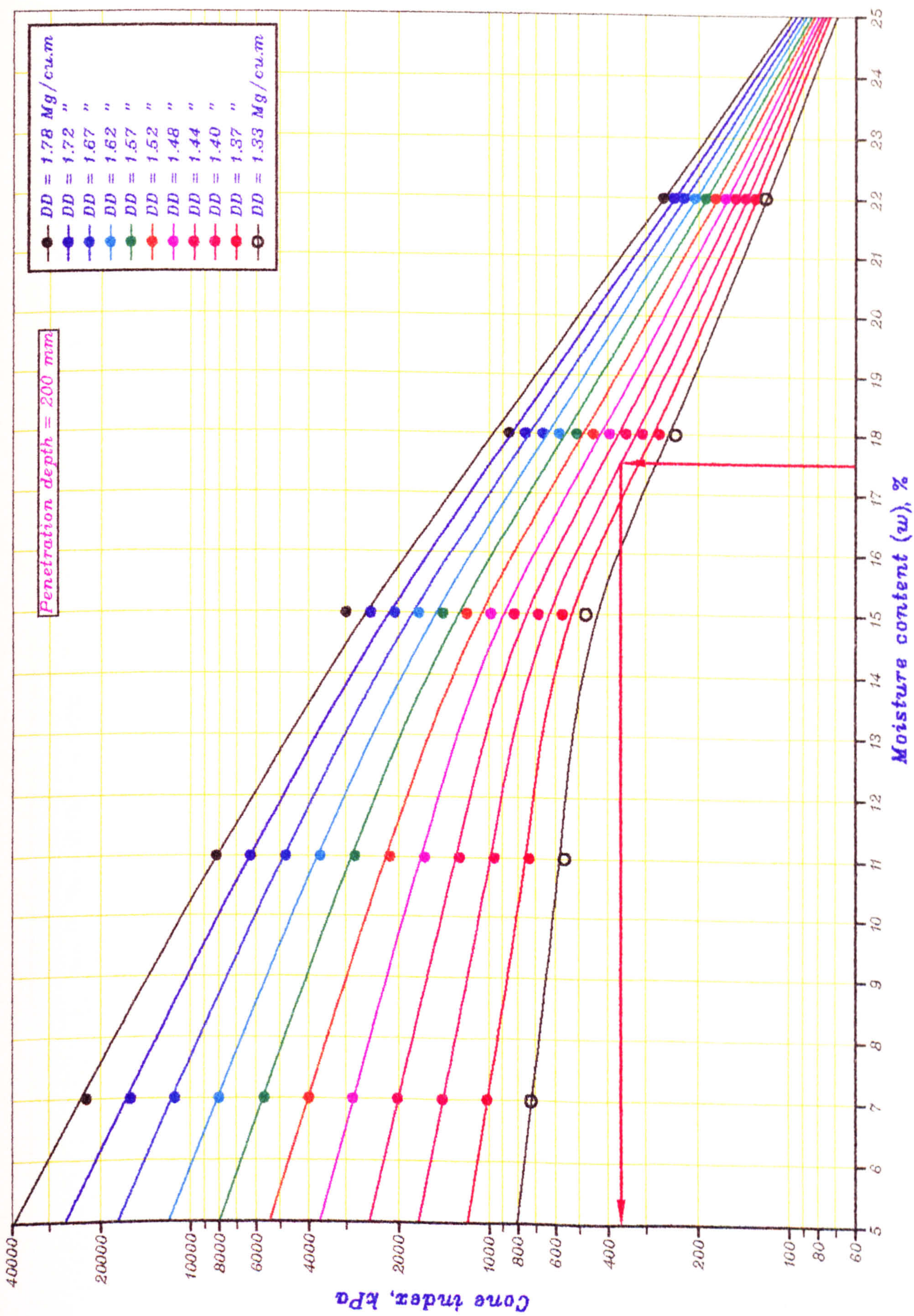


Fig. 9.4 Prediction of Cone index against Moisture content at different dry densities

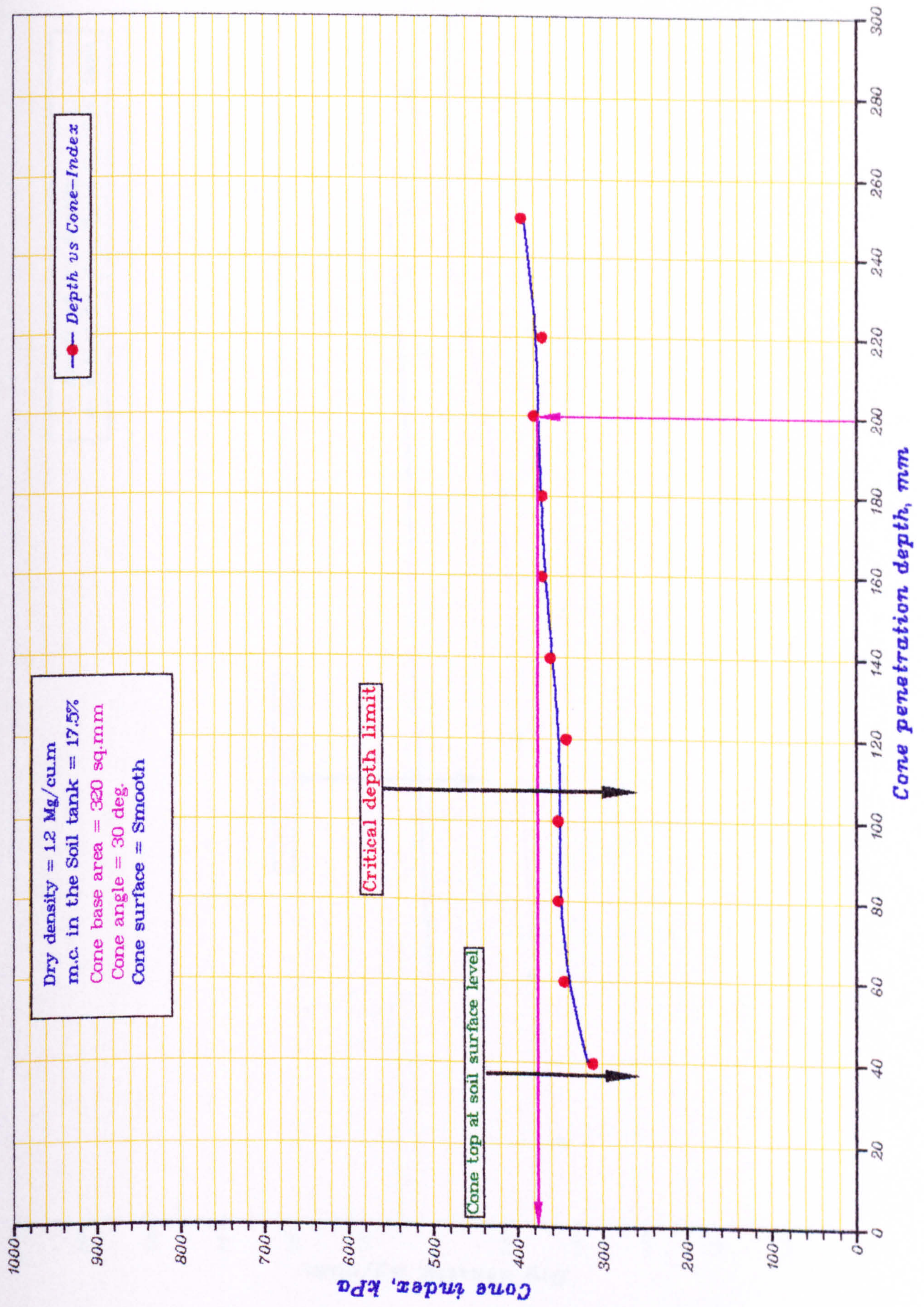


Fig. 9.5 Conversion of cone load to cone index at different depths for a 30 deg. cone

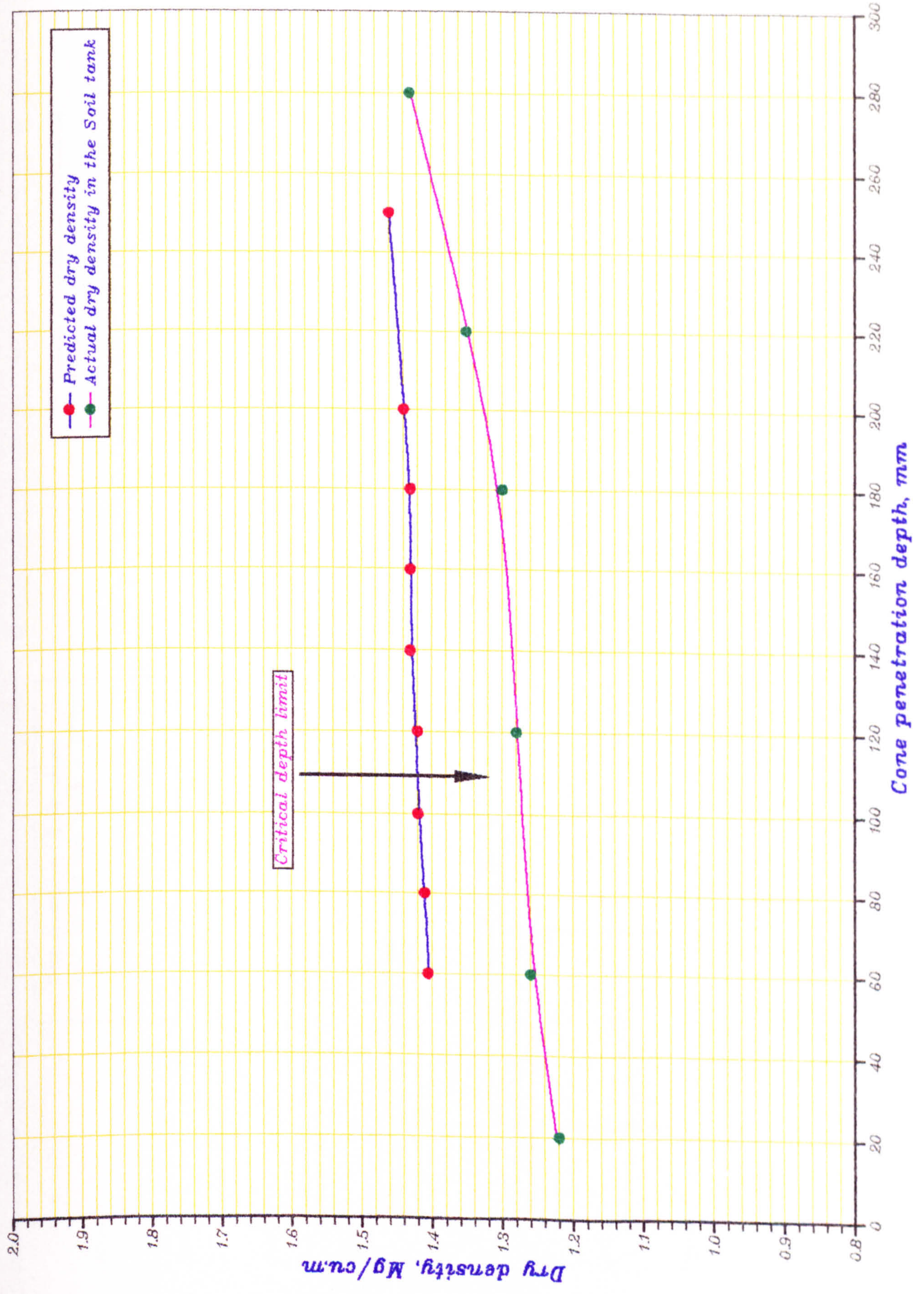


Fig. 9.6 Predicted values of dry density at different depths (using 30 deg. cone)

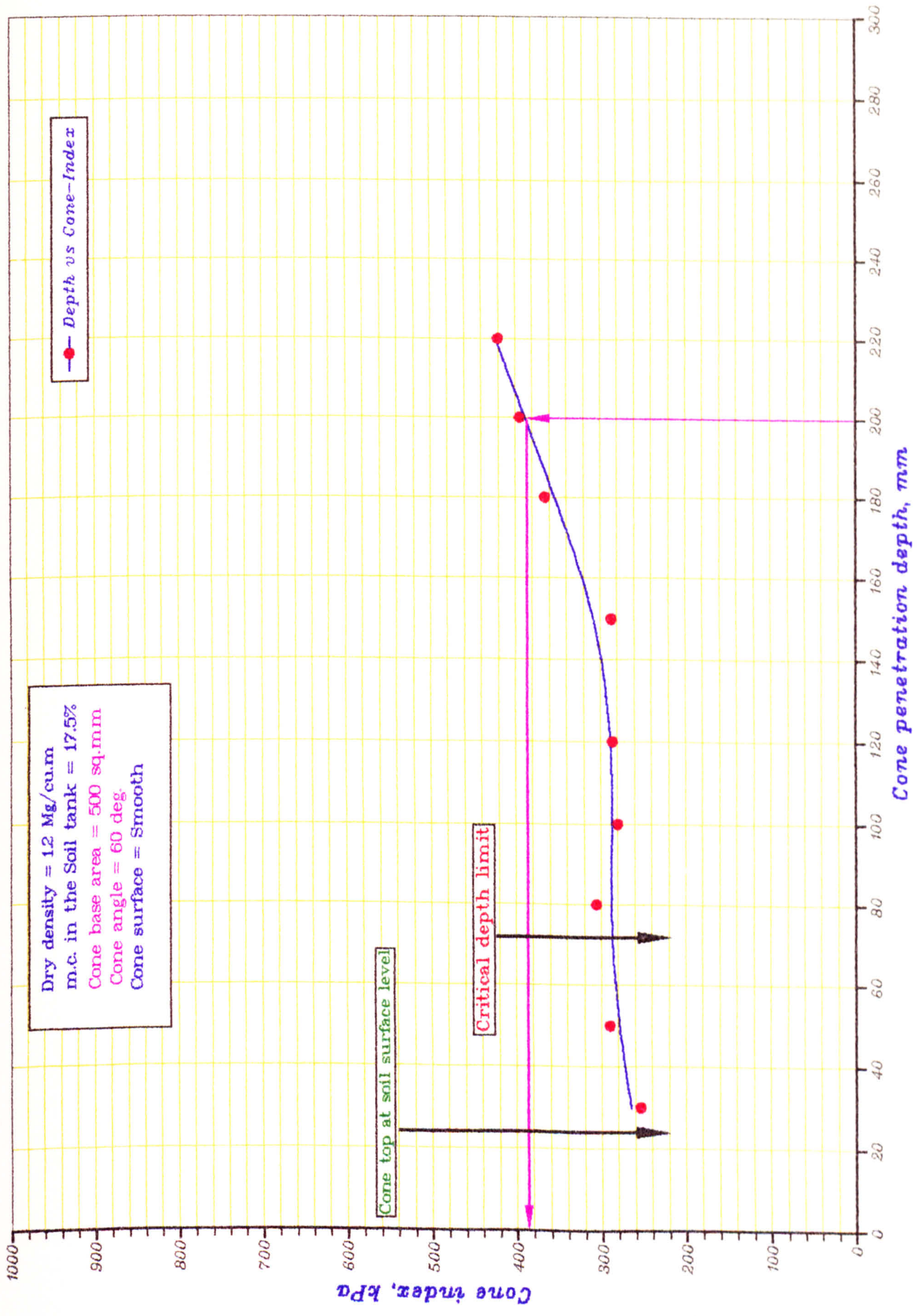


Fig. 9.7 Conversion of cone load to cone index at different depths for a 60 deg. cone

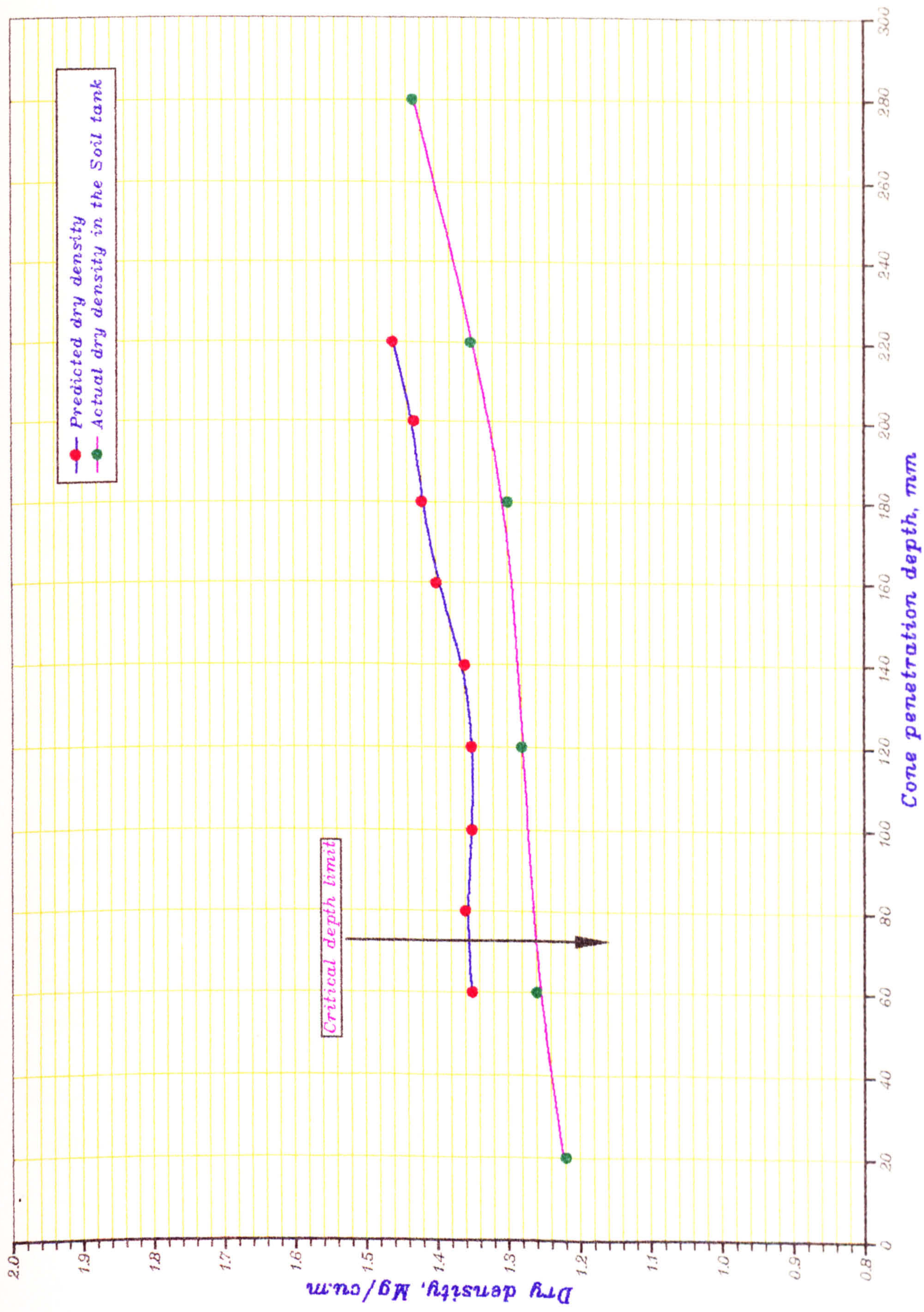


Fig. 9.8 Predicted values of dry density at different depths (using 60 deg. cone)

Table -3.1 Soil properties

Soil type	Sandy loam	Clay loam	clay
Soil series	Darvel ^a	Winton ^a	Evesham ^b
Sample depth, mm	0 -150	0 -200	0 -200
Liquid limit, g/kg	290	400	600
Plastic limit, g/kg	240	300	390
Cone penetrometer plastic limit ^c , g/kg	170	260	350
Organic matter (H ₂ O ₂), g/kg	30	31	41
Sand (2.0 - 0.06 mm), g/kg	645	375	184
Silt (0.06 - 0.002 mm), g/kg	259	382	212
Clay (<0.002 mm), g/kg	96	243	604

^aRagg & Futty (1967), ^bPalmer (1982). ^cCampbell (1976).

Table -3.2 Soil parameters

Soil series	w, %	N	λ_N	Γ	λ	M	κ	h^e
Evesham clay	24.5	3.752	0.275	3.812	0.308	1.700	0.000	0.225
	28.0	4.063	0.325	3.973	0.333	1.710	0.000	0.182
	31.5	4.200	0.360	4.063	0.363	1.500	0.000	0.150
	35.0	4.250	0.375	4.125	0.387	1.180	0.000	0.120
	38.5	4.250	0.375	4.162	0.416	0.790	0.000	0.100
Winton clay loam	13.0	2.750	0.175	2.688	0.174	1.710	0.000	0.975
	15.6	2.750	0.184	2.690	0.189	1.680	0.000	0.925
	18.2	2.750	0.199	2.750	0.214	1.550	0.000	0.880
	20.8	2.760	0.210	2.760	0.234	1.150	0.000	0.820
	23.4	2.760	0.225	2.770	0.260	0.780	0.000	0.700
Darvel sandy loam	8.5	2.375	0.116	2.375	0.130	1.420	0.000	0.620
	10.2	2.376	0.120	2.375	0.132	1.420	0.000	0.600
	11.9	2.382	0.124	2.380	0.136	1.401	0.000	0.600
	13.6	2.392	0.124	2.390	0.138	1.362	0.000	0.600
	15.3	2.382	0.116	2.380	0.136	1.291	0.000	0.580

(after O'sullivan *et al.*, 1994), ^eHatibu (1987)

Table- 7.1 Typical output of PROGRAMME-4

SAMPLE INFORMATION:

sample height mm	sample dia mm	sample mass gms	water content %
81.0	38.0	118.0	7.0

ISOTROPIC COMPRESSION:

sph.press	sp.vol	VCC	VCX	Net vol.ch.
0.00	2.23	0.00	0.00	0.00
25.00	2.19	-5.00	3.42	-1.58
50.00	2.13	-10.50	6.33	-4.17
100.00	2.07	-16.75	10.00	-6.75
150.00	2.00	-22.25	12.83	-9.42
200.00	1.93	-27.33	15.08	-12.25
250.00	1.89	-31.74	17.50	-14.24
200.00	1.90	-29.44	15.66	-13.78
150.00	1.90	-26.94	13.33	-13.61
100.00	1.92	-23.61	10.66	-12.95
50.00	1.88	-21.36	6.66	-14.70
25.00	1.87	-18.36	3.50	-14.86

TRIAxIAL COMPRESSION:

sigma3	sigma1	C.cir.	VCS	sp.vol	sph.press	dev.str.
25.00	115.64	70.32	2.00	1.92	55.21	90.64
50.00	275.40	162.70	1.50	1.91	125.13	225.40
100.00	353.75	226.87	.50	1.93	184.58	253.75
150.00	481.47	315.74	-1.50	1.87	260.49	331.47
200.00	636.63	418.31	-3.50	1.81	345.54	436.63
250.00	748.35	499.17	-7.00	1.72	416.12	498.35

Note: Details of the abbreviated headings are given in the Programme

Table- 7.2 Typical output of PROGRAMME-5

SAMPLE INFORMATION:

sample height mm	sample dia mm	sample mass gms	water content %
185.0	100.0	2120.0	22.0

ISOTROPIC COMPRESSION:

sph.press	sp.vol	VCC	VCX	Net vol.ch.
0.00	2.24	0.00	0.00	0.00
25.00	2.21	-25.00	3.42	-21.58
50.00	2.13	-78.75	6.33	-72.42
100.00	2.00	-163.75	10.00	-153.75
150.00	1.87	-252.50	12.83	-239.67
200.00	1.77	-320.75	15.08	-305.67
250.00	1.74	-340.00	17.50	-322.50
200.00	1.75	-335.00	15.66	-319.34
150.00	1.75	-330.87	13.33	-317.54
100.00	1.76	-321.62	10.66	-310.96
50.00	1.77	-310.50	6.66	-303.84
25.00	1.78	-302.00	3.50	-298.50

Note: Details of the abbreviated headings are given in the Programme

Table- 7.3 Typical output of the PROGRAMME-6

DIAC = 10.0 mm, ACONE ANGLE = 30.0 deg., DIAS = 5.0 mm

Cell pressure = 25.00 kPa

m.c	Phi	C	Gamma	F, mm	Ng	Nca	Nq	Qcone
7.0	27.22	11.00	14.87	101.04	.506	31.586	17.173	.43518
11.0	21.43	11.25	16.18	67.37	1.717	16.696	6.859	.20182
15.0	15.86	17.50	16.82	46.92	1.131	9.799	2.849	.13552
18.0	9.93	7.50	17.86	32.57	.548	6.084	1.074	.04074
22.0	8.88	7.00	18.01	30.57	.472	5.650	.895	.03481

Cell pressure = 50.00 kPa

m.c	Phi	C	Gamma	F, mm	Ng	Nca	Nq	Qcone
7.0	27.22	11.00	14.93	101.04	.506	31.586	17.173	.67134
11.0	21.43	11.25	16.27	67.37	1.717	16.696	6.859	.29615
15.0	15.86	17.50	16.89	46.92	1.131	9.799	2.849	.17470
18.0	9.93	7.50	17.97	32.57	.548	6.084	1.074	.05551
22.0	8.88	7.00	18.09	30.57	.472	5.650	.895	.04712

Cell pressure = 100.00 kPa

m.c	Phi	C	Gamma	F, mm	Ng	Nca	Nq	Qcone
7.0	27.22	11.00	14.97	101.04	.506	31.586	17.173	1.14362
11.0	21.43	11.25	16.33	67.37	1.717	16.696	6.859	.48479
15.0	15.86	17.50	16.95	46.92	1.131	9.799	2.849	.25305
18.0	9.93	7.50	18.05	32.57	.548	6.084	1.074	.08505
22.0	8.88	7.00	18.21	30.57	.472	5.650	.895	.07174

Cell pressure = 150.00 kPa

m.c	Phi	C	Gamma	F, mm	Ng	Nca	Nq	Qcone
7.0	27.22	11.00	15.00	101.04	.506	31.586	17.173	1.61589
11.0	21.43	11.25	16.36	67.37	1.717	16.696	6.859	.67342
15.0	15.86	17.50	17.01	46.92	1.131	9.799	2.849	.33141
18.0	9.93	7.50	18.13	32.57	.548	6.084	1.074	.11459
22.0	8.88	7.00	18.32	30.57	.472	5.650	.895	.09636

Cell pressure = 200.00 kPa

m.c	Phi	C	Gamma	F, mm	Ng	Nca	Nq	Qcone
7.0	27.22	11.00	15.04	101.04	.506	31.586	17.173	2.08817
11.0	21.43	11.25	16.40	67.37	1.717	16.696	6.859	.86205
15.0	15.86	17.50	17.05	46.92	1.131	9.799	2.849	.40976
18.0	9.93	7.50	18.17	32.57	.548	6.084	1.074	.14412
22.0	8.88	7.00	18.35	30.57	.472	5.650	.895	.12098

Cell pressure = 250.00 kPa

m.c	Phi	C	Gamma	F, mm	Ng	Nca	Nq	Qcone
7.0	27.22	11.00	15.05	101.04	.506	31.586	17.173	2.56043
11.0	21.43	11.25	16.41	67.37	1.717	16.696	6.859	1.05068
15.0	15.86	17.50	17.11	46.92	1.131	9.799	2.849	.48812
18.0	9.93	7.50	18.22	32.57	.548	6.084	1.074	.17366
22.0	8.88	7.00	18.40	30.57	.472	5.650	.895	.14560

Table- 7.4 Typical output of PROGRAMME-7

DIAS = 16.00 mm

WC, % = 17.50 PHI, deg. = 11.00 C, kN/sq.m = 11.00 GAMMA, kN/cu.m = 16.28

Acone, deg.	Dcone, mm	Coeff.	F, mm	Ng	Nca	Nq	Qcone, kN
30.00	20.00	.75	66.02	.368	3.789	.836	.127
30.00	20.00	1.00	71.13	.412	4.368	.949	.146
30.00	25.00	.75	86.86	.464	4.986	1.100	.220
30.00	27.00	.75	93.81	.493	5.385	1.188	.257
30.00	38.00	1.00	142.25	.700	8.736	1.897	.584
60.00	25.00	.75	47.31	.451	4.413	.989	.195
60.00	33.00	.75	62.45	.577	5.825	1.305	.340
60.00	33.00	1.00	67.29	.617	6.380	1.413	.372
90.00	25.00	.75	35.20	.467	4.480	1.002	.198
120.00	25.00	.75	30.24	.493	4.727	1.050	.209
135.00	25.00	.75	29.08	.509	4.885	1.080	.216

Table - 9.1 Typical output of the PROGRAMME-8

1. Analysis for polynomial coefficients (for k):

X	Y(actual)	Y(theo.)	deviation	% deviation
7.0	33.95	33.3538	.5962	1.7560
9.0	33.50	34.0962	-.5962	-1.7796
11.0	32.29	31.6938	.5962	1.8463
13.0	27.50	27.3993	.1007	.3661
15.0	21.65	22.2462	-.5962	-2.7536
17.0	17.00	17.0491	-.0491	-.2887
19.0	13.00	12.4038	.5962	4.5859
21.0	9.00	8.6874	.3126	3.4738
23.0	6.00	6.0576	-.0576	-.9608
25.0	4.00	4.4538	-.4538	-11.3459
27.0	3.00	3.5962	-.5962	-19.8721

Polynomial coefficients for k

A0 = -17.226213
 A1 = 15.062773
 A2 = -1.433905
 A3 = 0.048894
 A4 = -0.000570

2. Analysis for polynomial coefficients (for m):

X	Y(actual)	Y(theo.)	deviation	% deviation
7.0	6.82	6.6749	.1451	2.1269
9.0	5.90	6.0170	-.1170	-1.9823
11.0	5.07	5.2151	-.1451	-2.8610
13.0	4.30	4.3420	-.0420	-.9776
15.0	3.61	3.4649	.1451	4.0181
17.0	2.50	2.6451	-.1451	-5.8022
19.0	1.90	1.9379	-.0379	-1.9937
21.0	1.50	1.3932	.1068	7.1214
23.0	1.20	1.0549	.1451	12.0879
25.0	1.10	.9614	.1386	12.5988
27.0	1.00	1.1451	-.1451	-14.5054

Polynomial coefficients for m

B0 = 6.658403
 B1 = 0.378318
 B2 = -0.067790
 B3 = 0.002117
 B4 = -0.000015

* **PROGRAMME - 1**

* *****

* krrr.f

*

* This programme is used to calculate the value of
* p , q and v at every point on the state surface for any
* given set of value of soil parameters.

*

```
SUBROUTINE PCORD (V,S,T,PT,PC,PN,QT,QC)
COMMON ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
T = GAMMA - ALAM * ALOG((3.0 - H)/(AMU - H))
PN = EXP((ANU - V)/ALAMN)
S = V + AKAPPA * ALOG(PN)
PT = EXP((T - S)/(ALAM - AKAPPA))
PC = EXP((GAMMA - S)/(ALAM - AKAPPA))
QT = 3.0*PT
QC = AMU*PC
RETURN
END
```

*

```
SUBROUTINE TCOS (V,S,P,Q)
COMMON ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
PT = EXP((T - S)/(ALAM - AKAPPA))
Q = 3.0*P
RETURN
END
```

*

```
SUBROUTINE HVORS (V,S,P,Q)
COMMON ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
PC = EXP((GAMMA - S)/(ALAM - AKAPPA))
Q = (AMU-H)*PC + H*P
RETURN
END
```

*

```
SUBROUTINE ROSCOE (V,S,P,Q)
COMMON ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
PC = EXP((GAMMA - S)/(ALAM - AKAPPA))
PN = EXP((ANU - V)/ALAMN)
Q = SQRT((AMU*AMU*PC*PC
+ *(1.0-((P-PC)*(P-PC)/((PN-PC)*(PN-PC))))))
RETURN
END
```

*

* **MAIN PROGRAMME**

* PARAMETER (RADN = 3.14159265/180.0, NY=50, NX=5000)
* REAL P(NY,NX), V(NY), Q(NY,NX)

*

```
COMMON ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
```



```

*
* Read : Soil parameters
*
102 READ(5,'(3F10.2)') ALAM, GAMMA, AMU
103 READ(5,'(2F10.2)') ALAMN, ANU
104 READ(5,'(F10.2)') AKAPPA
105 READ(5,'(F10.2)') H
*
IF (H .LT. 0.0) THEN
NROUTE = IFIX(ABS(H)+0.01)
GO TO (102,103,104,120), NROUTE
END IF
*
* Write : Soil parameters
*
WRITE(6,500)
WRITE(6,504) ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
*
* Format Statements
*
-----
500 FORMAT(/',9X,'Lambda',4X,'Gamma',7X,'M',
+6X,'Lambda*n',5X,'N',9X,'k',9X,'h/')
502 FORMAT(/',11X,'V',9X,'Pt',7X,'Pc',
+6X,'Pn',9X,'Qt',7X,'Qc/')
503 FORMAT(/',8X,'P',8X,'V',9X,'Q/')
504 FORMAT(' ',3X,7F10.3/)
507 FORMAT(3F10.2)
*
-----
*
DO 200 J=1,21
*
V(J) = REAL(J-1)*0.02 + 1.60
V(J) = REAL(J-1)*0.02 + 1.90
VY = V(J)
*
CALL PCORD (VY,SY,TY,PT,PC,PN,QT,QC)
*
DO 201 K=1,101
P(J,K) = REAL(K-1)*10.0
IF (P(J,K) .LE. PT) THEN
PTCO = P(J,K)
*
CALL TCOS (VY,SY,PTCO,QTCO)
Q(J,K) = QTCO
ELSE IF (P(J,K) .GT. PT.AND. P(J,K) .LE. PC) THEN
PH = P(J,K)
*
CALL HVORS (VY,SY,PH,QH)
Q(J,K) = QH

```



```
ELSE IF (P(J,K) .GT. PC.AND. P(J,K) .LE. PN) THEN  
PR = P(J,K)
```

```
*
```

```
CALL ROSCOE (VY,SY,PR,QR)
```

```
Q(J,K) = QR
```

```
END IF
```

```
*
```

```
201 CONTINUE
```

```
200 CONTINUE
```

```
*
```

```
* Write : output values
```

```
*
```

```
WRITE(6,503)
```

```
*
```

```
DO 205 L =1,21
```

```
DO 204 M =1,101
```

```
WRITE(6,507) P(L,M)/10.0, V(L), Q(L,M)
```

```
204 CONTINUE
```

```
205 CONTINUE
```

```
*
```

```
120 STOP
```

```
END
```



```

* PROGRAMME - 2
* *****
* kqqq.f
*
* This programme is used to calculate the value of  $p$  against  $v$ 
* (ranging from 1.90 to 2.30 for Evesham clay and 1.60 to 2.00
* for both Winton clay loam and Darvel sandy loam) firstly for
* a set of value of Eta ( $0 < \eta < M$ ) for Roscoe surface,
* secondly Eta ( $M < \eta < 3$ ) for Hvorslev surface, thirdly Eta
* ( $\eta = 3$ ) for Tension Cut-off surface.
*
* Parameters for:
*     NCL =>  $\lambda_N$  (ALAMN),  $N$  (ANU)
*     CSL =>  $\lambda$  (ALAM),  $\Gamma$  (GAMMA),  $M$  (AMU)
*     ELL =>  $\kappa$  (AKAPPA)
*
* REAL V(200), P(200,200), ALP(200,200), ETA(200)
*
* Data for specific volume:
* DATA (V(NA), NA = 1,41)
*
* For Evesham clay
* +/2.30,
* + 2.29, 2.28, 2.27, 2.26, 2.25, 2.24, 2.23, 2.22, 2.21, 2.20,
* + 2.19, 2.18, 2.17, 2.16, 2.15, 2.14, 2.13, 2.12, 2.11, 2.10,
* + 2.09, 2.08, 2.07, 2.06, 2.05, 2.04, 2.03, 2.02, 2.01, 2.00,
* + 1.99, 1.98, 1.97, 1.96, 1.95, 1.94, 1.93, 1.92, 1.91, 1.90/
*
* For Winton clay loam and Darvel sandy loam
* +/2.00,
* + 1.99, 1.98, 1.97, 1.96, 1.95, 1.94, 1.93, 1.92, 1.91, 1.90,
* + 1.89, 1.88, 1.87, 1.86, 1.85, 1.84, 1.83, 1.82, 1.81, 1.80,
* + 1.79, 1.78, 1.77, 1.76, 1.75, 1.74, 1.73, 1.72, 1.71, 1.70,
* + 1.69, 1.68, 1.67, 1.66, 1.65, 1.64, 1.63, 1.62, 1.61, 1.60/
*
* Data for Eta ( $= q/p$ ) values:
* DATA (ETA(NB), NB = 1,17)
* +/0.000,
* + 0.200, 0.400, 0.600, 0.800, 1.000,
* + 1.200, 1.400, 1.600, 1.700, 1.800, 2.000,
* + 2.200, 2.400, 2.600, 2.800,
* + 3.000/
*
* Read : Soil parameters:
* 101 READ(5,'(3F10.2)') ALAM,GAMMA,AMU
* 102 READ(5,'(2F10.2)') ALAMN,ANU
* 103 READ(5,'(F10.2)') AKAPPA
* 104 READ(5,'(F10.2)') H

```



```

*
  IF (H. LT. 0.0) THEN
    NROUTE = IFIX(ABS(H)+0.01)
    GO TO (101,102,103,120), NROUTE
  END IF
*
*   Format Statements
* -----
502 FORMAT(' ',7F10.3)
503 FORMAT(' ',F10.3)
505 FORMAT(' ',3F10.2)
506 FORMAT('/ ',9X,'Va',7X,'Pa',6X,'Log(Pa)'/)
507 FORMAT('/ ',6X,'Eta=')
508 FORMAT('/ ',7X,'Lambda',3X,'Gamma',
  +8X,'M',5X,'Lambda*n',5X,'N',9X,'k',9X,'h'/)
509 FORMAT('/ ',3X,'Roscoe surface')
510 FORMAT('/ ',3X,'Hvorslev surface')
511 FORMAT('/ ',3X,'Normal Consolidation Line')
512 FORMAT('/ ',3X,'Critical State Line')
513 FORMAT('/ ',3X,'Tension Cut-off')
514 FORMAT(' ',9X,17F7.1,/)
515 FORMAT(' ',F6.2,2X,17F7.1)
516 FORMAT(' ',4X,'v',6X,'c1',5X,'c2',5X,'c3',5X,'c4',5X,'c5',
  +5X,'c6',5X,'c7',5X,'c8',5X,'c9',4X,'c10',4X,'c11',4X,'c12',
  +4X,'c13',4X,'c14',4X,'c15',4X,'c16',4X,'c17')
* -----
*
*   WRITE(6,508)
*   WRITE(6,502) ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
*   WRITE(6,516)
*
  DO 602 NA = 1,41
    VA = V(NA)
    DO 601 NB = 1,17
*
      PN = EXP((ANU - VA)/ALAMN)
      S  = VA + AKAPPA*ALOG(PN)
*
      IF (ETA(NB) .LT. AMU) THEN
        GO TO 301
      ELSE IF (ETA(NB) .GT. AMU .AND. ETA(NB) .LE. 3.0) THEN
        GO TO 302
      END IF
* ----- : Calculations for Roscoe surface
*   Roscoe surface
*   (0.0 <= η <= M)
*
301 PC = EXP((GAMMA - S)/(ALAM - AKAPPA))
  IF (ETA(NB) .LT. 1.0E-06) THEN

```


PA = PN
GO TO 105
END IF

*

F = AMU*PC/ETA(NB)
A = (PN - PC)
GG = F*F/(A*A)
C1 = 1.0 + GG
C2 = 2.0*PC*GG
C3 = F*F - GG*PC*PC
C4 = SQRT(C2*C2 + 4.0*C1*C3)
PA = (C2 + C4)/(2.0*C1)

105 P(NA,NB) = PA
ALP(NA,NB) = LOG(PA)
GO TO 601

*

* ----- : Calculations for Hvorslev surface

* Hvorslev surface

* ($M \leq \eta \leq 3.0$)

*

302 PCH = EXP((GAMMA - S)/(ALAM - AKAPPA))
QCH = AMU*PCH
Q1H = H*PCH
PAH = (QCH - Q1H)/(ETA(NB) - H)
P(NA,NB) = PAH
ALP(NA,NB) = LOG(PAH)

*

601 CONTINUE
602 CONTINUE

*

* Write: output values

*

107 WRITE(6,514) (ETA(NB), NB=1,17)
DO 10 NA=1,41
WRITE(6,515) V(NA), (P(NA,NB), NB=1,17)
10 CONTINUE

*

108 WRITE(6,514) (ETA(NB), NB=1,17)
DO 20 NA=1,41
WRITE(6,515) V(NA), (ALP(NA,NB), NB=1,17)
20 CONTINUE

*

120 STOP
END

* **PROGRAMME - 3**

* *****

* krr.f

*

* This programme is used to calculate the value of
* p/p_n and q/p_n (with ν ranging from 1.9 to 2.3 for
* Evesham clay and 1.6 to 2.0 for both Winton clay
* loam and Darvel sandy loam) for any given set of
* value of soil parameter.

*

```
SUBROUTINE PCORD (V,S,T,PT,PC,PN,QT,QC)
COMMON ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
T = GAMMA - ALAM * ALOG((3.0 - H)/(AMU - H))
PN = EXP((ANU - V)/ALAMN)
S = V + AKAPPA * ALOG(PN)
PT = EXP((T - S)/(ALAM - AKAPPA))
PC = EXP((GAMMA - S)/(ALAM - AKAPPA))
QT = 3.0*PT
QC = AMU*PC
END
```

*

```
SUBROUTINE TCOS (V,S,P,Q)
COMMON ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
PT = EXP((T - S)/(ALAM - AKAPPA))
Q = 3.0*P
END
```

*

```
SUBROUTINE HVORS (V,S,P,Q)
COMMON ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
PC = EXP((GAMMA - S)/(ALAM - AKAPPA))
Q = (AMU-H)*PC + H*P
END
```

*

```
SUBROUTINE ROSCOE (V,S,P,Q)
COMMON ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
PC = EXP((GAMMA - S)/(ALAM - AKAPPA))
PN = EXP((ANU - V)/ALAMN)
CONST = AMU*AMU*PC*PC
Q = SQRT(CONST*(1.0 - ((P-PC)*(P-PC)/((PN-PC)*(PN-PC))))))
END
```

*

* **MAIN PROGRAMME**

*

```
PARAMETER (RADN = 3.14159265/180.0, NY=50, NX=5000)
REAL P(NY,NX), V(NY), Q(NY,NX)
REAL PE(NY)
```

*


```

COMMON ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
*
* Read : Soil parameters
*
102 READ(5,'(3F10.2)') ALAM, GAMMA, AMU
103 READ(5,'(2F10.2)') ALAMN, ANU
104 READ(5,'(F10.2)') AKAPPA
105 READ(5,'(F10.2)') H
*
IF (H .LT. 0.0) THEN
NROUTE = IFIX(ABS(H)+0.01)
GO TO (102,103,104,120), NROUTE
END IF
*
* Write : Supplied soil parameters
WRITE(6,500)
WRITE(6,504) ALAM,GAMMA,AMU,ALAMN,ANU,AKAPPA,H
*
* Format Statements
* -----
500 FORMAT(/ ' ',9X,'Lambda',4X,'Gamma',7X,'M',
+6X,'Lambda*n',6X,'N',9X,'h/')
502 FORMAT(/ ' ',11X,'V',9X,'Pt',7X,'Pc',
+6X,'Pn',9X,'Qt',7X,'Qc'/)
503 FORMAT(/ ' ',8X,'P',8X,'V',9X,'Q'/)
504 FORMAT(' ',2X,6F10.2)
507 FORMAT(3F10.3)
* -----
*
DO 200 J=1,5
V(J) = REAL(J-1)*0.10 + 1.9
VY = V(J)
CALL PCORD (VY,SY,TY,PT,PC,PN,QT,QC)
PE(J) = PN
*
DO 201 K=1,101
P(J,K) = REAL(K-1)*PT/100.0
PTCO = P(J,K)
CALL TCOS (VY,SY,PTCO,QTCO)
Q(J,K) = QTCO
201 CONTINUE
*
DO 202 K=101,200
P(J,K) = REAL(K-100)*(PC-PT)/100.0 + PT
PH = P(J,K)
CALL HVORS (VY,SY,PH,QH)
Q(J,K) = QH
202 CONTINUE

```



```
*
DO 203 K=201,300
P(J,K) = REAL(K-200)*(PN-PC)/100.0 + PC
PR    = P(J,K)
CALL ROSCOE (VY,SY,PR,QR)
Q(J,K) = QR
203 CONTINUE
*
200 CONTINUE
*
*   Write : output values
WRITE(6,503)
*
DO 205 L =1,5
DO 204 M=1,300
WRITE(6,507) P(L,M)/PE(L), V(L), Q(L,M)/PE(L)
204 CONTINUE
205 CONTINUE
*
120 STOP
END
```


* **PROGRAMME - 4**

* *****

* ttest.f

*

* This programme is used to calculate the value of bulk density,
* void ratio and specific volume from triaxial test for a given
* cell pressure. And finally arrange the values in $v-p$ and $q-p$
* plane to find out the soil parameters

*

* Notations:

*

* DEVIST = Deviatoric stress due to shear, kPa
* = SIGMA1 - SIGMA3

* GS = Specific gravity of soil particle

*

* SAAREA = Sample area, sq.mm

* SAHT = Sample height, mm

* SADIA = Sample diameter, mm

* SAMASS = Sample mass, gms

* WC = Water content, gms/100gms

*

* SAVOL = Sample volume, cu.mm

* BULKD = Bulk density, kg/cm.m

* VOIDR = Void ratio

* SPVOL = Specific volume

* VOL1 = Volume after consolidation, cc

* VOL2 = VOL1 renamed to use when shearing, cc

* VOLS2 = Volume after shear, cc

* WATERD = Density of water

* = 1000.0 kg/cu.m

* SIGMA1 = Major principal stress, kPa

* SIGMA2 = Intermediate principal stress, kPa

* = SIGMA3 for triaxial test condition

* SIGMA3 = Minor principal stress, kPa

* SPHERP = Spherical pressure, kPa

* = $(1/3) * (\text{SIGMA1} + 2.0 * \text{SIGMA3})$

* Y = Displacement at failure of specimen, mm

* VCX = Volume change due to expansion of triaxial cell, cc

* VCC = Volume change due to consolidation of soil sample, cc

* VCCN = Net volume change due to consolidation of sample, cc

* = VCC + VCX

* VCS = Volume change due to shear, cc

*

* Note: -ve sign indicates compacting

* +ve sign indicates dilating

*

REAL VOL2(15)


```

OPEN (UNIT=5, FILE='dttest')
OPEN (UNIT=6, FILE='rttest')
*
*   NSLU = Nos.of steps in loading and then unloading the
*   soil specimen
*
*   NSLU = 12
*
*   WATERD = 1000.0
*   GS     = 2.68
*
*   Read: Sample information
101 READ(5,'(2F10.2)') SAHT, SADIA
102 READ(5,'(F10.2)') SAMASS
103 READ(5,'(F10.4)') WC
*
*   PI   = 3.14159265
*   SAAREA = (PI/4.0)*(SADIA/1000.0)*(SADIA/1000.0)
*   SAVOL  = SAAREA*(SAHT/1000.0)
*
*   WRITE(6,508)
*
*   WRITE(6,505) SAHT, SADIA, SAMASS, WC
*
*   ===== Step 1: Soil sample normally consolidated =====
*   Spherical pressure, p= SIGMA3 with no deviatoric stress
*
*   WRITE(6,509)
*
*   DO 601 JJ = 1,NSLU
*
105 READ(5,'(3F10.2)') SIGMA3,VCC,VCX
*
*   VCCN   = VCC+VCX
*   VOL1   = SAVOL+(VCCN/1000000.0)
*   BULKD1 = (SAMASS/1000.0)/VOL1
*   VOIDR1 = GS*(1.0 + WC)*(WATERD/BULKD1) - 1.0
*   SPVOL1 = 1.0 + VOIDR1
*
*   VOL2(JJ) = VOL1
*   WRITE(6,507) SIGMA3, SPVOL1, VCC, VCX, VCCN
*
601 CONTINUE
*
*   ===== Step 2: Same soil sample subjected to shear =====
*   Spherical pressure, p= (1/3)*(SIGMA1 + 2*SIGMA3)
*

```



```

WRITE(6,510)
*
DO 602 KK = NSLU, (NSLU/2+1), -1
*
110 READ(5,'(2F10.2,F10.4,F10.2)') SIGMA3,VCS,ALOAD,Y
*
VOLS2 = VOL2(KK) + (VCS/1000000.0)
BULKD2 = (SAMASS/1000.0)/VOLS2
VOIDR2 = GS*(1.0 + WC)*(WATERD/BULKD2) - 1.0
SPVOL2 = 1.0 + VOIDR2
*
DEVIST = ALOAD*((SAHT - Y)/1000.0)/SAVOL
SIGMA1 = DEVIST + SIGMA3
SPHERP = (1/3.0)*(SIGMA1 + 2.0*SIGMA3)
CCIRCL = SIGMA3 + (DEVIST/2.0)
*
WRITE(6,506) SIGMA3,SIGMA1,CCIRCL,VCS,SPVOL2,SPHERP,DEVIST
*
602 CONTINUE
*
*   Format Statements
* -----
502 FORMAT(' ',2F10.3)
503 FORMAT(' ',3F14.4)
504 FORMAT(' ',4F14.4)
505 FORMAT(' ',4F15.4/)
506 FORMAT(' ',5F9.2,F10.2,F10.2)
507 FORMAT(' ',5F13.2)
508 FORMAT('/ ',6X,'sample height',4X,'sample dia',
+4X,'sample mass',5X,'water content'/)
509 FORMAT('/ ',6X,'sph.press',7X,'sp.vol',
+7X,'VCC',10X,'VCX',7X,'Net vol.ch.'/)
510 FORMAT('/ ',4X,'sigma3',3X,'sigma1',3X,'C.cir.',5X,
+'VCS',4X,'sp.vol',3X,'sph.press',3X,'dev.str.'/)
514 FORMAT('/ ',6X,17F10.3,/)
515 FORMAT(' ',F6.2,17F10.2)
* -----
*
120 STOP
END

```


* **PROGRAMME - 5**

* *****

* tt.f

*

* This programme is used to calculate the value of bulk density,
* void ratio and specific volume from calibration chamber test
* for a given cell pressure. And finally arrange the values in
* in $v-p$ space to find out the soil parameters.

*

* Notations:

*

* DEVIST = Deviatoric stress due to shear, kPa
* = SIGMA1 - SIGMA3

* GS = Specific gravity of soil particle

*

* SAAREA = Sample area, sq.mm

* SAHT = Sample height, mm

* SADIA = Sample diameter, mm

* SAMASS = Sample mass, gms

* WC = Water content, gms/100gms

*

* SAVOL = Sample volume, cu.mm

* BULKD = Bulk density, kg/cm.m

* VOIDR = Void ratio

* SPVOL = Specific volume

* VOL1 = Volume after consolidation, cc

* VOL2 = VOL1 renamed to use when shearing, cc

* VOLS2 = Volume after shear, cc

* WATERD = Density of water

* = 1000.0 kg/cu.m

* SIGMA1 = Major principal stress, kPa

* SIGMA2 = Intermediate principal stress, kPa

* = SIGMA3 for triaxial test condition

* SIGMA3 = Minor principal stress, kPa

* SPHERP = Spherical pressure, kPa

* = $(1/3) * (SIGMA1 + 2.0 * SIGMA3)$

* Y = Displacement at failure of specimen, mm

* VCX = Volume change due to expansion of triaxial cell,cc

* VCC = Volume change due to consolidation of soil sample,cc

* VCCN = Net volume change due to consolidation of sample,cc

* = VCC + VCX

* VCS = Volume change due to shear, cc

*

* Note: -ve sign indicates compacting

* +ve sign indicates dilating

REAL VOL2(15)

*


```

OPEN (UNIT=5, FILE='dtf')
OPEN (UNIT=6, FILE='rtt')
*
*   NSLU = Nos.of steps in loading and then unloading the
*       soil specimen
*
NSLU = 12
*
WATERD = 1000.0
GS     = 2.68
*
101 READ(5,'(2F10.2)') SAHT, SADIA
102 READ(5,'(F10.2)')  SAMASS
103 READ(5,'(F10.4)')  WC
*
PI     = 3.14159265
SAAREA = (PI/4.0)*(SADIA/1000.0)*(SADIA/1000.0)
SAVOL  = SAAREA*(SAHT/1000.0)
*
WRITE(6,508)
WRITE(6,505) SAHT, SADIA, SAMASS, WC
*
* ===== Soil sample normally consolidated =====
* Spherical pressure,  $p = \text{SIGMA3}$  with no deviatoric stress
*
*
WRITE(6,509)
*
DO 601 JJ = 1,NSLU
*
105 READ(5,'(3F10.2)') SIGMA3,VCC,VCX
*
VCCN  = VCC+VCX
VOL1  = SAVOL+(VCCN/1000000.0)
BULKD1 = (SAMASS/1000.0)/VOL1
VOIDR1 = GS*(1.0 + WC)*(WATERD/BULKD1) - 1.0
SPVOL1 = 1.0 + VOIDR1
*
VOL2(JJ) = VOL1
WRITE(6,507) SIGMA3, SPVOL1, VCC, VCX, VCCN
*
601 CONTINUE
*
*   Format Statements
* -----
502 FORMAT(' ',2F10.3)
503 FORMAT(' ',3F14.4)
504 FORMAT(' ',4F14.4)

```


505 FORMAT(' ',4F15.4/)

506 FORMAT(' ',4F10.2,2X,F10.2,2X,F10.2)

507 FORMAT(' ',5F13.2)

*

508 FORMAT(/' ',6X,'sample height',4X,'sample dia',
+4X,'sample mass',5X,'water content'/)

509 FORMAT(/' ',6X,'sph.press',7X,'sp.vol',
+7X,'VCC',10X,'VCX',7X,'Net vol.ch.'/)

510 FORMAT(/' ',5X,'sigma3',4X,'sigma1',6X,'VCS',
+5X,'sp.vol',4X,'sph.press',3X,'dev.stress'/)

514 FORMAT(/' ',6X,17F10.3,/)

515 FORMAT(' ',F6.2,17F10.2)

* -----

*

120 STOP

END


```

* PROGRAMME - 5A
* *****
*   ttl.f
*
* Programme for calculating the degree of saturation
* initially and at different confining pressure along
* rebound line (i.e. ELL = Elastic Loading Line)
*
* GS   = Specific gravity of soil particle
* SAAREA = Sample area, sq.mm
* SAHT  = Sample height, mm
* SADIA  = Sample diameter, mm
* SAMASS = Sample mass, gms
* WC     = Water content, gms/100gms
*
* SAVOL = Sample volume, cu.mm
* BULKD  = Bulk density, kg/cm.m
* VOIDR  = Void ratio
* SPVOL  = Specific volume
* VOL    = Volume after consolidation, cc
* WATERD = Density of water
*         = 1000.0 kg/cu.m
* SPP    = Confining pressure, kPa
* VCX    = Volume change due to expansion of triaxial cell,cc
* VCC    = Volume change due to consolidation of soil sample,cc
* NVC    = Net volume change due to consolidation of sample,cc
*         = VCC + VCX
* SR     = Degree of saturation
*
* Note: -ve sign indicates compacting
*       +ve sign indicates dilating
*
* OPEN (UNIT=5, FILE='dttl')
* OPEN (UNIT=6, FILE='rttl')
*
* WATERD = 1000.0
* GS     = 2.68
*
* Read: Sample information
101 READ(5,'(2F10.2)') SAHT, SADIA
102 READ(5,'(F10.2)') SAMASS
103 READ(5,'(F10.4)') WC
*
* PI    = 3.14159265
* SAAREA = (PI/4.0)*(SADIA/1000.0)*(SADIA/1000.0)
* SAVOL  = SAAREA*(SAHT/1000.0)
*
* WRITE(6,508)

```



```

*
  WRITE(6,505) SAHT, SADIA, SAMASS, WC
*
* ===== Soil sample normally consolidated =====
* Spherical pressure, p= SIGMA3 with no deviatoric stress
*
  WRITE(6,509)
*
  DO 601 KK = 1,7
105 READ(5,'(2F10.2)') SPP, NVC
*
  VOL    = SAVOL+(NVC/1000000.0)
  BULKD  = (SAMASS/1000.0)/VOL
  VOIDR  = GS*(1.0 + WC)*(WATERD/BULKD) - 1.0
*
  SR1    = WC*GS/VOIDR
  SR     = 100.0*SR1
  WRITE(6,502) SPP, SR
*
601 CONTINUE
*
*   Format Statements
* -----
502 FORMAT(' ',2F14.2)
505 FORMAT(' ',4F15.4/)
507 FORMAT(' ',5F13.2)
508 FORMAT('/ ',6X,'sample height',4X,'sample dia',
  +4X,'sample mass',5X,'water content'/)
509 FORMAT('/ ',7X,'Conf.press.',8X,'Sr'/)
510 FORMAT('/ ',5X,'sigma3',4X,'sigma1',6X,'VCS',
  +5X,'sp.vol',4X,'sph.press',3X,'dev.stress'/)
* -----
*
120 STOP
  END

```



```

* PROGRAMME - 5B
* *****
*   tts.f
*
* This programme is used to calculate the value ln(Q) from
* calibration chamber test data and finally arrange the values
* in ln(Q) - Psi space to find out the parameters m and k.
*
* Notations:
*
* CDIA   = Cone diameter, mm
* CAMM   = Cone area, sq.mm
* CAREA  = Cone area, sq.m
* FC     = Force at cone base level, kN
*
REAL P(10)
REAL Q(10)
REAL QL(10)
*
OPEN (UNIT=5, FILE='dtts')
OPEN (UNIT=6, FILE='rtts')
*
* NSL = Nos. of stress levels at which the specimen is
*      subjected to cone penetration
*      NSL = 6
*
100 READ(5,'(F10.2)') WC
    WRITE(6,506) WC
101 READ(5,'(F10.2)') CDIA
*
PI   = 3.14159265
CAMM = (PI/4.0)*(CDIA*CDIA)
CAREA = (PI/4.0)*(CDIA/1000.0)*(CDIA/1000.0)
*
WRITE(6,508)
WRITE(6,505) CDIA, CAMM
WRITE(6,509)
*
DO 601 JJ = 1,NSL
105 READ(5,'(F10.2,F10.4)') CP,FC
*
P(JJ) = CP
QC    = FC/CAREA
Q(JJ) = (QC - CP)/CP
QL(JJ) = ALOG(Q(JJ))
601 CONTINUE
*
* Write: output values

```



```
DO 602 KK = 1,NSL  
WRITE(6,507) P(KK), QL(KK)
```

```
*
```

```
602 CONTINUE
```

```
*
```

```
* Format Statements
```

```
*
```

```
-----  
505 FORMAT(' ',F15.2,2X,F15.4/)
```

```
506 FORMAT('/ ',5X,'Moisture content,% =',2X,F5.2)
```

```
507 FORMAT(' ',F15.2,2X,F15.4)
```

```
*
```

```
508 FORMAT('/ ',6X,'cone dia,mm',4X,'cone area,sq.mm')
```

```
509 FORMAT('/ ',6X,'conf.press.,kPa',4X,'ln(Q), kPa')
```

```
*
```

```
*
```

```
120 STOP
```

```
END
```


* **PROGRAMME - 6**

* *****

* ccam.f

*

* **Analysis of forces on CONE PENETROMETER**

- * C = Cohesion, kN/sq.m
- * APHI = Angle of internal friction, deg.
- * GAMMA = Soil unit weight, kN/cu.m
- * ATHEC = Half of cone angle, deg.
- * ACONE = Cone angle, deg.
- * DIAC = Cone diameter, m
- * DIAS = Shaft diameter, m
- * WTC = Weight of cone, kN
- * WTS = Weight of shaft, kN
- * CA = Soil-Cone constrained adhesion, kN/sq.m
- * CAS = Soil-Shaft constrained adhesion, kN/sq.m
- * ADELTA = Angle of Soil-Cone friction, deg.
- * ADELTS = Angle of Soil-Shaft friction, deg.
- * Q = Surcharge, kN/sq.m (optional)
- * Z = Depth of penetration, m
- * Qcone = Total force on cone penetrometer, kN
- * HC = Cone height, m
- * SC = Cone face length, m
- * R = Interface length, m
- * F = Rupture distance (i.e. Critical depth), m

*

*

SUBROUTINE ZLIM (PHI, C, CA, THEC, DIAC, DELTAC, F)

*

RADN = 3.14159265/180.0
DELC = ASIN(SIN(DELTAC)/SIN(PHI))
AMUC = 0.5*(DELC + DELTAC)
PSI = 45.0*RADN + 0.5*PHI
THETAP = PSI + AMUC
THETAM = PSI - AMUC
SC = DIAC/(2.0*SIN(THEC))
ETA = 270.0*RADN - (90.0*RADN-THEC) - THETAM
R = SC*SIN(THETAP)/COS(PHI)
F = R*EXP(ETA*TAN(PHI))
RETURN
END

*

**SUBROUTINE CONE (C, PHI, GAMMA, THEC, DIAC, WTC, DIAS, WTS,
+ DELTAC, DELTAS, CA, CAS, Q, Z, TFG, TFCA, TFQ, QCONE)**

*

RADN = 3.14159265/180.0
PI = 3.14159265
DELC = ASIN(SIN(DELTAC)/SIN(PHI))

AMUC = 0.5*(DELC + DELTAC)
 PSI = 45.0*RADN + 0.5*PHI
 THETAP = PSI + AMUC
 THETAM = PSI - AMUC
 DELS = ASIN(SIN(DELTAS)/SIN(PHI))
 AMUS = DELS + DELTAS
 AK11 = (1.0+SIN(PHI)*COS(AMUS))
 AK12 = (1.0-SIN(PHI)*COS(AMUS))
 AK1 = AK11/AK12
 AK2 = COS(PHI)*COS(AMUS)*(AK1+1.0)
 SC = DIAC/(2.0*SIN(THEC))
 ANGLE = 2.0*PI*SIN(THEC)
 HC = DIAC/(2.0*TAN(THEC))
 TC = SC*SIN(THETAM)/COS(PHI)
 ETA = 270.0*RADN - (90.0*RADN-THEC) - THETAM
 R = SC*SIN(THETAP)/COS(PHI)
 F = R*EXP(ETA*TAN(PHI))
 D1 = (1.0/2.0)*F
 D3 = (1.0/2.0)*R*COS(PHI)
 D4 = (2.0/3.0)*R*COS(PHI)
 D2 = F * ((3.0*Z - 2.0*F)/(3.0*(2.0*Z - F)))

*

* Equations for gravitational moment, AMW

AA = -3.0*TAN(PHI)
 AMW1 = GAMMA*F*F*F/(3.0*(AA*AA+1.0))
 AMW2 = EXP(AA*ETA)*(AA*SIN(ETA)-COS(ETA)) + 1.0
 AMW = AMW1*AMW2

* Equation for cohesive moment, AMC

IF (PHI .EQ. 0.0) THEN
 AMC = C*R*R*ETA
 ELSE
 AMC = C*(F*F-R*R)/(2.0*TAN(PHI))
 END IF

* Normal force (per unit length) to the shaft surface, kN/m

FHG = 0.5*GAMMA*AK1*F*(2.0*Z - F)
 FHC = C*F*AK2
 FHQ = Q*F*AK1

*

* Total shear force to the shaft surface, kN

WIDTHS = PI*DIAS
 AREAS = PI*DIAS*Z
 STG = FHG * TAN(DELTAS) * WIDTHS
 STC = FHC * TAN(DELTAS) * WIDTHS
 STQ = FHQ * TAN(DELTAS) * WIDTHS

*

AS = CAS*AREAS

*


```

* Forces on soil block: kN/m width
PG  = (AMW + FHG*D2)/D4
PC  = (AMC + FHC*D1)/D3
PQ  = FHQ*D1/D3
*
A1  = R*C
A2  = TC*C
AC  = SC*CA
*
* Weight of wedge, WW, kN/m width
HW  = TC*SIN(THETAP)
WW  = 0.5*GAMMA*SC*HW
E1  = 90.0*RADN - DELTAC - THETAM
E2  = THETAM - THEC
*
* Forces on cone face: kN/m width
RG  = (PG*COS(PHI) + WW*SIN(E2))/SIN(E1)
RC  = (PC*COS(PHI) + AC*SIN(THETAM) + A2*COS(PHI))/SIN(E1)
RQ  = (PQ*COS(PHI))/SIN(E1)
*
* Total vertical component of RG,RC,RQ
FVG = (ANGLE*SC*RG/3.0) * SIN(DELTAC+THEC)
FVC = (ANGLE*SC*RC/2.0) * SIN(DELTAC+THEC)
FVQ = (ANGLE*SC*RQ/2.0) * SIN(DELTAC+THEC)
*
* Vertical component of AC
VAC = CA*PI*HC*HC*TAN(THEC)
*
* Equilibrium of cone+shaft
TFG = FVG + STG - (WTC + WTS)
TFCA = FVC + STC + AS + VAC
TFQ = FVQ + STQ
*
QCONE = TFG + TFCA + TFQ
RETURN
END

*      MAIN
*      PROGRAM
*      *****
*
REAL SIG3(10)
REAL WC(10)
*
OPEN (UNIT=5, FILE='dccam')
OPEN (UNIT=6, FILE='rccam')
*
RADN = 3.14159265/180.0
PI    = 3.14159265

```



```

*
* Cone:
  DIAC = 0.010
* WTC = 0.000038
  WTC = 0.0
  ATHEC = 15.00
*
* Shaft:
  DIAS = 0.008
* WTS = 0.000965
  WTS = 0.0
*
* Format statements
* -----
501 FORMAT(' ',4X,'m.c',4X,'Phi',5X,'C ',5X,'Gamma',3X,'F, mm',
  +3X,'Ng',5X,'Nca',5X,'Nq',6X,'Qcone')
502 FORMAT(' ',3X,67(1H-))
503 FORMAT(' ',3X,67(1H_))
504 FORMAT(/)
505 FORMAT(' ',3X,20(1H*))
507 FORMAT(' ',3X,'DIAC =',F5.3)
508 FORMAT(' ',3X,'ATHEC =',F6.2)
509 FORMAT(' ',3X,'ACONE =',F6.2)
510 FORMAT(' ',3X,14(1H*))
512 FORMAT(' ',3X,'CA =',F5.2,3X,'CAS =',F5.2/)
515 FORMAT(' ',3X,'DIAC =',F4.3,3X,
  +'ACONE =',F5.2)
517 FORMAT(' ',3X,'DIAS =',F4.3)
518 FORMAT(' ',3X,'Cell pressure =',F7.2)
520 FORMAT(' ',3X,'APHI =',F5.2,3X,'C =',F5.2,3X,
  +'GAMMA =',F5.2/)
525 FORMAT(F8.1,4F8.2,3F7.3,F10.5)
530 FORMAT(F10.2,3F10.5,F10.5)
* -----
*
  THEC = ATHEC*RADN
  ACONE = 2.0*ATHEC
*
  WRITE(6,504)
  WRITE(6,515) DIAC, ACONE
  WRITE(6,517) DIAS
  WRITE(6,504)
*
  DATA (SIG3(NA), NA = 1, 6)
  +/25.0, 50.0, 100.0, 150.0, 200.0, 250.0/
*
  DATA (WC(NC), NC = 1, 5)
  +/7.0, 11.0, 15.0, 18.0, 22.0/
*

```



```

DO 601 NA = 1,6
Q1 = SIG3(NA)
*
WRITE(6,504)
WRITE(6,518) Q1
*
WRITE(6,502)
WRITE(6,501)
WRITE(6,502)
*
DO 701 NC = 1,5
102 READ(5,'(3F10.2)') APhi, C, GAMMA
*
WATERC = WC(NC)
PHI = APhi*RADN
*
ADELTC = (3.0/4.0)*APhi
DELTAC = ADELTC*RADN
CA = C*TAN(DELTAC)/TAN(PHI)
*
ADELTS = (3.0/4.0)*APhi
DELTAS = ADELTS*RADN
CAS = C*TAN(DELTAS)/TAN(PHI)
*
CALL ZLIM (PHI, C, CA, THEC, DIAC, DELTAC, F)
Z = 0.055
Q = Q1 + GAMMA*Z
*
CALL CONE (C, PHI, GAMMA, THEC, DIAC, WTC, DIAS, WTS,
+ DELTAC, DELTAS, CA, CAS, Q, Z, TFG, TFCA, TFQ, QCONE)
*
ANG = TFG/(GAMMA*Z*Z*DIAC)
ANCA = TFCA/(C*Z*DIAC)
ANQ = TFQ/(Q*Z*DIAC)
*
WRITE(6,525) WATERC,APhi,C,GAMMA,F*1000.0, ANG,ANCA,ANQ,
          QCONE
*
701 CONTINUE
    WRITE(6,502)
*
601 CONTINUE
*
150 STOP
    END

```


* PROGRAMME - 7
* *****
* denm.f
*

* -----
* SUBROUTINE ZLIM (PHI, C, CA, THEC, DIAC, DELTAC, F)
*

RADN = 3.14159265/180.0
DELC = ASIN(SIN(DELTAC)/SIN(PHI))
AMUC = 0.5*(DELC + DELTAC)
PSI = 45.0*RADN + 0.5*PHI
THETAP = PSI + AMUC
THETAM = PSI - AMUC
SC = DIAC/(2.0*SIN(THEC))
ETA = 270.0*RADN - (90.0*RADN-THEC) - THETAM
R = SC*SIN(THETAP)/COS(PHI)
F = R*EXP(ETA*TAN(PHI))
RETURN
END

* -----
* SUBROUTINE CONE (C, PHI, GAMMA, THEC, DIAC, WTC, DIAS, WTS,
+ DELTAC, DELTAS, CA, CAS, Q, Z, TFG, TFCA, TFQ, QCONE)
*

RADN = 3.14159265/180.0
PI = 3.14159265
DELC = ASIN(SIN(DELTAC)/SIN(PHI))
AMUC = 0.5*(DELC + DELTAC)
PSI = 45.0*RADN + 0.5*PHI
THETAP = PSI + AMUC
THETAM = PSI - AMUC
DELS = ASIN(SIN(DELTAS)/SIN(PHI))
AMUS = DELS + DELTAS
AK11 = (1.0+SIN(PHI)*COS(AMUS))
AK12 = (1.0-SIN(PHI)*COS(AMUS))
AK1 = AK11/AK12
AK2 = COS(PHI)*COS(AMUS)*(AK1+1.0)
SC = DIAC/(2.0*SIN(THEC))
ANGLE = 2.0*PI*SIN(THEC)
HC = DIAC/(2.0*TAN(THEC))
TC = SC*SIN(THETAM)/COS(PHI)
ETA = 270.0*RADN - (90.0*RADN-THEC) - THETAM
R = SC*SIN(THETAP)/COS(PHI)
F = R*EXP(ETA*TAN(PHI))
D1 = (1.0/2.0)*F
D3 = (1.0/2.0)*R*COS(PHI)
D4 = (2.0/3.0)*R*COS(PHI)

*
D2 = F * ((3.0*Z - 2.0*F)/(3.0*(2.0*Z - F)))

*

* Equations for gravitational moment, AMW

$$AA = -3.0 * \text{TAN}(\text{PHI})$$

$$AMW1 = \text{GAMMA} * F * F * F / (3.0 * (AA * AA + 1.0))$$

$$AMW2 = \text{EXP}(AA * \text{ETA}) * (AA * \text{SIN}(\text{ETA}) - \text{COS}(\text{ETA})) + 1.0$$

$$AMW = AMW1 * AMW2$$

*

* Equation for cohesive moment, AMC

IF (PHI .EQ. 0.0) THEN

$$AMC = C * R * R * \text{ETA}$$

ELSE

$$AMC = C * (F * F - R * R) / (2.0 * \text{TAN}(\text{PHI}))$$

END IF

* Normal force (per unit length) to the shaft surface, kN/m

$$\text{FHG} = 0.5 * \text{GAMMA} * AK1 * F * (2.0 * Z - F)$$

$$\text{FHC} = C * F * AK2$$

$$\text{FHQ} = Q * F * AK1$$

* Total shear force to the shaft surface, kN

$$\text{WIDTHS} = \text{PI} * \text{DIAS}$$

$$\text{AREAS} = \text{PI} * \text{DIAS} * Z$$

$$\text{STG} = \text{FHG} * \text{TAN}(\text{DELTAS}) * \text{WIDTHS}$$

$$\text{STC} = \text{FHC} * \text{TAN}(\text{DELTAS}) * \text{WIDTHS}$$

$$\text{STQ} = \text{FHQ} * \text{TAN}(\text{DELTAS}) * \text{WIDTHS}$$

$$\text{AS} = \text{CAS} * \text{AREAS}$$

* Forces on soil block: kN/m width

$$\text{PG} = (\text{AMW} + \text{FHG} * D2) / D4$$

$$\text{PC} = (\text{AMC} + \text{FHC} * D1) / D3$$

$$\text{PQ} = \text{FHQ} * D1 / D3$$

*

$$A1 = R * C$$

$$A2 = \text{TC} * C$$

$$\text{AC} = \text{SC} * \text{CA}$$

* Weight of wedge, WW, kN/m width

$$\text{HW} = \text{TC} * \text{SIN}(\text{THETAP})$$

$$\text{WW} = 0.5 * \text{GAMMA} * \text{SC} * \text{HW}$$

$$E1 = 90.0 * \text{RADN} - \text{DELTAC} - \text{THETAM}$$

$$E2 = \text{THETAM} - \text{THEC}$$

*

* Forces on cone face: kN/m width

$$\text{RG} = (\text{PG} * \text{COS}(\text{PHI}) + \text{WW} * \text{SIN}(E2)) / \text{SIN}(E1)$$

$$\text{RC} = (\text{PC} * \text{COS}(\text{PHI}) + \text{AC} * \text{SIN}(\text{THETAM}) + A2 * \text{COS}(\text{PHI})) / \text{SIN}(E1)$$

$$\text{RQ} = (\text{PQ} * \text{COS}(\text{PHI})) / \text{SIN}(E1)$$

*

* Total vertical component of RG,RC,RQ

$$\text{FVG} = (\text{ANGLE} * \text{SC} * \text{RG} / 3.0) * \text{SIN}(\text{DELTAC} + \text{THEC})$$


```

FVC = (ANGLE*SC*RC/2.0) * SIN(DELTAC+THEC)
FVQ = (ANGLE*SC*RQ/2.0) * SIN(DELTAC+THEC)
*
* Vertical component of AC
VAC = CA*PI*HC*HC*TAN(THEC)

* Equilibrium of cone+shaft
TFG = FVG + STG - (WTC + WTS)
TFCA = FVC + STC + AS + VAC
TFQ = FVQ + STQ
*
QCONE = TFG + TFCA + TFQ
RETURN
END

*
* MAIN
* PROGRAM
*
OPEN (UNIT=5, FILE='ddenm')
OPEN (UNIT=6, FILE='rdenm')
RADN = 3.14159265/180.0
PI = 3.14159265
*
WTC = 0.0
DIAS = 0.016
WTS = 0.0
*
* Format statements
* -----
501 FORMAT(' ',2X,'Acone,deg.',1X,'Dcone,mm',2X,'Coeff.',2X,
+'F, mm',5X,'Ng',5X,'Nca',6X,'Nq',4X,'Qcone,kN')
502 FORMAT(' ',2X,69(1H-))
503 FORMAT(' ',2X,69(1H_))
504 FORMAT(/)
505 FORMAT(' ',3X,20(1H*))
507 FORMAT(' ',3X,'DIAC =',F5.3)
508 FORMAT(' ',3X,'ATHEC =',F6.2)
509 FORMAT(' ',3X,'ACONE =',F6.2)
510 FORMAT(' ',3X,14(1H*))
512 FORMAT(' ',3X,'CA =',F5.2,3X,'CAS =',F5.2/)
515 FORMAT(' ',2X,'WC,%=' ,F5.2,2X,'PHI,deg.=' ,F5.2,2X,
+'C,kN/sq.m=' ,F5.2,2X,'GAMMA,kN/cu.m=' ,F5.2)
517 FORMAT(' ',2X,'DIAS, mm =',F5.2)
518 FORMAT(/' ',3X,'Cell pressure =',F7.2)
520 FORMAT(' ',3X,'APHI =',F5.2,3X,'C =',F5.2,3X,
+'GAMMA =',F5.2/)
525 FORMAT(1X,F9.2,2X,F9.2,F7.2,F9.2,3F8.3,F9.3)
530 FORMAT(F10.2,3F10.5,F10.5)
* -----

```



```

*
WRITE(6,504)
WRITE(6,517) DIAS*1000.0
WRITE(6,504)
*
101 READ(5,'(F10.2)') WC
102 READ(5,'(3F10.2)') APhi, C, GAMMA
*
WRITE(6,515) WC, APhi, C, GAMMA
Phi = APhi*RADN
WRITE(6,502)
WRITE(6,501)
WRITE(6,502)
*
103 READ(5,'(3F10.2)') ATHEC, DCONE, COEFF
*
IF (ATHEC .LT. 0.0) THEN
GO TO 140
END IF
*
DIAC = DCONE/1000.0
THEC = ATHEC*RADN
ACONE = 2.0*ATHEC
*
ADELTC = COEFF*APHI
DELTAC = ADELTC*RADN
CA = C*TAN(DELTAC)/TAN(PHI)
ADELTS = 0.0
DELTAS = ADELTS*RADN
CAS = C*TAN(DELTAS)/TAN(PHI)
*
CALL ZLIM (PHI, C, CA, THEC, DIAC, DELTAC, F)
Z = 0.200
Q = GAMMA*Z
*
CALL CONE (C, PHI, GAMMA, THEC, DIAC, WTC, DIAS, WTS,
+ DELTAC, DELTAS, CA, CAS, Q, Z, TFG, TFCA, TFQ, QCONE)
*
ANG = TFG/(GAMMA*Z*Z*DIAC)
ANCA = TFCA/(C*Z*DIAC)
ANQ = TFQ/(Q*Z*DIAC)
*
WRITE(6,525) ACONE,DCONE,COEFF,F*1000.0,ANG,ANCA,ANQ,QCONE
GO TO 103
140 WRITE(6,502)
*
150 STOP
END

```


* PROGRAMME - 8

* *****

* pna.f

*

INTEGER N, M1
PARAMETER (N=11, M1=5)

*

DOUBLE PRECISION YY(N), DEV(N), PDEV(N)
DOUBLE PRECISION REF
DOUBLE PRECISION A(M1), X(N), Y(N)

*

DO 101 J = 1, 11
READ(5, '(2F10.0)') X(J), Y(J)
101 CONTINUE

*

CALL E02ACF(X, Y, N, A, M1, REF)

*

WRITE(6, 515)
WRITE(6, 500)
WRITE(6, 503)
WRITE(6, 502) (A(I), I=1, M1)
WRITE(6, 503)
WRITE(6, 515)

* Format:

* -----

500 FORMAT (4X, 'Polynomial coefficients')
502 FORMAT (6X, F10.6)
503 FORMAT (' ', 3X, 23(1H-))
504 FORMAT (' ', 9X, 53(1H-))
505 FORMAT (13X, 'X', 5X, 'Y(actual)', 3X, 'Y(theo.)',
+2X, 'deviation', 2X, '% deviation')
510 FORMAT (6X, 5F11.4)
515 FORMAT (///)

* -----

*

WRITE(6, 505)
WRITE(6, 504)
DO 102 K = 1, 11
YY(K) = A(1) + A(2)*X(K) + A(3)*X(K)**2 + A(4)*X(K)**3 + A(5)*X(K)**4
DEV(K) = Y(K) - YY(K)
PDEV(K) = (DEV(K)/Y(K))*100.0
WRITE(6, 510) X(K), Y(K), YY(K), DEV(K), PDEV(K)
102 CONTINUE
WRITE(6, 504)

*

STOP
END

* PROGRAMME - 9

* *****

* final3.f

*

REAL SPV(500), UWT(500), QINDEX(500,500)

*

GS = 2.68

WATERD = 0.997

PI = 3.14159

RADN = 3.14159/180.0

*

CONED = 0.027

CONEA = (PI/4.0)*(CONED**2)

Z = 0.300

*

A0 = -17.226213

A1 = 15.062773

A2 = -1.433905

A3 = 0.048894

A4 = -0.000570

*

*

B0 = 6.658403

B1 = 0.378318

B2 = -0.067790

B3 = 0.002117

B4 = -0.000015

*

*

DO 601 KK = 1,5

*

READ(5,'(4F10.0)') WC, GAMAI, ALAMDA, APHI

*

AK0 = 1.0 - SIN(APHI*RADN)

*

AK = A0 + A1*WC + A2*WC**2 + A3*WC**3 + A4*WC**4

AM = B0 + B1*WC + B2*WC**2 + B3*WC**3 + B4*WC**4

*

DO 602 JJ = 11,1,-1

*

SPVOL = FLOAT(JJ - 1)*0.05 + 1.5

DRYD = GS*WATERD/SPVOL

BULKD = DRYD*(1.0 + (WC/100.0))

UNITWT = 9.80665*DRYD

SPV(JJ) = SPVOL

UWT(JJ) = UNITWT

*

P = UNITWT*Z*(1.0 + 2.0*AK0)


```

*
  SPVOLC = GAMAI - ALAMDA*ALOG(P)
*
  PSI   = SPVOL - SPVOLC
*
  Q     = AK*EXP((-AM)*PSI)
  QC    = Q*P + P
  QFORCE = QC*CONEA
*
  QINDEX(JJ,KK)= QC
*
602 CONTINUE
*
  AK0 = 0.0
  AK   = 0.0
  AM   = 0.0
601 CONTINUE
*
* -----
501 FORMAT (26X,'CONE INDEX, kN/sq.m at different m.c')
502 FORMAT (6X,F10.6)
503 FORMAT (' ',3X,67(1H-))
504 FORMAT (' ',23(1H-))
505 FORMAT (5X,'SPVOL',4X,'UNITWT',6X,'7.0%',5X,'11.0%',
  +5X,'15.0%',5X,'18.0%',5X,'22.0%')
510 FORMAT (4X,3F11.2,F11.5)
515 FORMAT (///)
516 FORMAT (/)
518 FORMAT (7F10.2)
* -----
*
  WRITE(6,515)
  WRITE(6,503)
  WRITE(6,501)
  WRITE(6,505)
  WRITE(6,503)
*
* Write: output values
*
  DO 701 NA= 11,1,-1
    WRITE(6,518) SPV(NA), UWT(NA), (QINDEX(NA,NB), NB=1,5)
701 CONTINUE
    WRITE(6,503)
*
300 STOP
  END

```


* PROGRAMME - 10

* *****

* final4.f

*

REAL WCT(50), QINDEX(500,500)

*

GS = 2.68

WATERD = 0.997

PI = 3.14159

RADN = 3.14159/180.0

*

CONED = 0.027

CONEA = (PI/4.0)*(CONED**2)

Z = 0.300

*

A0 = -17.226213

A1 = 15.062773

A2 = -1.433905

A3 = 0.048894

A4 = -0.000570

*

*

B0 = 6.658403

B1 = 0.378318

B2 = -0.067790

B3 = 0.002117

B4 = -0.000015

*

*

DO 601 KK = 1,5

*

READ(5,'(4F10.0)') WC, GAMAI, ALAMDA, APhi

*

WCT(KK) = WC

*

AK0 = 1.0 - SIN(APhi*RADN)

*

AK = A0 + A1*WC + A2*WC**2 + A3*WC**3 + A4*WC**4

AM = B0 + B1*WC + B2*WC**2 + B3*WC**3 + B4*WC**4

*

DO 602 JJ = 1,11

*

SPVOL = FLOAT(JJ - 1)*0.05 + 1.5

DRYD = GS*WATERD/SPVOL

BULKD = DRYD*(1.0 + (WC/100.0))

UNITWT = 9.80665*DRYD

*

P = UNITWT*Z*(1.0 + 2.0*AK0)


```

SPVOLC = GAMAI - ALAMDA*ALOG(P)
*
PSI  = SPVOL - SPVOLC
Q    = AK*EXP((-AM)*PSI)
QC   = Q*P + P
QFORCE = QC*CONEA
*
QINDEX(JJ,KK)= QC
*
602 CONTINUE
*
  AK0 = 0.0
  AK  = 0.0
  AM  = 0.0
601 CONTINUE
*
*   Format statements
* -----
501 FORMAT (26X,'CONE INDEX, kN/sq.m at different Dry density,
+kN/cu.m')
502 FORMAT (6X,F10.6)
503 FORMAT (' ',94(1H-))
504 FORMAT (' ',23(1H-))
505 FORMAT (2X,'m.c',3X,'d17.5',4X,'d16.9',3X,'d16.4',
+3X,'d15.9',3X,'d15.4',3X,'d15.0',3X,'d14.6',3X,'d14.2',
+3X,'d13.8',3X,'d13.4',3X,'d13.1')
510 FORMAT (4X,3F11.2,F11.5)
515 FORMAT (///)
516 FORMAT (/)
518 FORMAT (F5.1,2F9.2,9F8.2)
* -----
  WRITE(6,515)
  WRITE(6,503)
  WRITE(6,501)
  WRITE(6,505)
  WRITE(6,503)
*
*   Write: output values
*
  DO 701 NA= 1,5
  WRITE(6,518) WCT(NA), (QINDEX(NB,NA), NB=1,11)
701 CONTINUE
  WRITE(6,503)
*
300 STOP
  END

```