PROBABILISTIC-BASED CHARACTERISATION OF THE MECHANICAL

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PROPERTIES OF CFRP LAMINATES

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13 Abstract

14 Fibre reinforced polymer (FRP) composites have been increasingly used worldwide in the strengthening of civil engineering structures. As FRP becomes more common in structural 15 strengthening, the development of probability-based limit state design codes will require accurate 16 17 models for the prediction of the mechanical properties of the FRPs. Existing models, however, are 18 based on small sample sizes and ignore the importance of the tail region for analyses and design. 19 Addressing these limitations, this paper presents a probabilistic-based characterisation of the 20 mechanical properties of carbon FRP (CFRP) laminates using a large batch of tension tests. The 21 analysed specimens were pre-cured laminates of carbon fibres embedded in epoxy matrices, which 22 is the most commonly used laminate for the strengthening concrete beams and slabs. Based on the 23 existing data, probabilistic models and correlations were established for the Young's modulus, 24 ultimate strain and tensile strength. Analyses demonstrate the suitability of the Weibull distribution 25 for the estimation of CFRP properties. Results also show that the statistical characterisation of the 26 mechanical properties should be performed with a focus on the tail region. The proposed 27 distributions constitute a set of validated probabilistic models that can be used for performing 28 reliability analyses of structures strengthened with CFRP laminates.

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30 Keywords: CFRP laminates; strengthening of structures; Mechanical properties; Probabilistic
 31 models.

32 **1. Introduction**

33 During the last decades, externally bonded reinforcement (EBR) of fibre-reinforced polymers 34 (FRP) has become a common technique to strengthen and upgrade civil engineering structures. FRP 35 is usually used in the form of wet lay-up sheets or pre-fabricated laminates due to their simplicity 36 and lower capital cost. The former system is based on the direct application of fibre sheets saturated 37 with resin, whereas the second uses pre-fabricated cured strips. There are also automated techniques 38 using vacuum (e.g. resin infusion techniques) or vacuum and heat (e.g. heated vacuum bag only) for 39 impregnation of fibres [1-3]. The characteristics of the FRP, namely its lightweight, high durability 40 in aggressive environments, ease of installation and cost effectiveness, are quite competitive for 41 strengthening purposes and constitute a good alternative to more traditional methods and materials, 42 such as EBR using steel plates or concrete jacketing [1]. There are several examples where FRPs 43 were used to increase the flexural, shear or axial capacity of structural members, such as beams, 44 slabs, columns, or joints [4-8].

The growing interest in FRP composites resulted in the development of several design guidelines 45 46 (e.g. CEB-FIB [9], TR-55 [10], CNR [11] and ACI 440.2R-08 [12]). These, however, are not 47 presently at a level of development comparable to those used in structural concrete and steel design. 48 Considering the uncertainties present in FRP applications, new guidelines are required to develop 49 probability-based limit state design codes and to support the acceptance of FRP materials in civil 50 engineering [13, 14]. Despite previous reliability studies (e.g. Ellingwood [13], Plevris, 51 Triantafillou [15], Okeil, El-Tawil [16], Monti and Santini [17], Atadero, Lee [18], Atadero and 52 Karbhari [19], Okeil, Belarbi [20], and Ali, Bigaud [21]) having addressed some of these uncertainties, the statistical information is still limited in the development of more accurate 53 54 probabilistic models.

55 A variety of factors affect the properties of FRP after manufacturing which create a degree of 56 uncertainty and must be considered in design [22]. Atadero [23] employed normal, log-normal, 57 Weibull and Gamma distributions to analyse the probabilistic properties of field-manufactured wet lay-up carbon and glass composites. Six sets, composed by one, three or four subsets resulting in 58 903 samples, were considered to assess the tensile strength, the Young's modulus and the laminate 59 60 thickness. Despite the large number of samples used, the need to divide them in smaller subsets of 61 different properties and manufacturing processes led to a significant reduction in the sample size available for the statistical analysis. From this study, the Weibull distribution was proposed to 62 model the tensile strength, whereas the Young's modulus and the laminate thickness were modelled 63 64 using a log-normal distribution. Zureick, Bennett [24] performed statistical analysis on over 600 samples of pultruded composite materials fabricated from E-glass fibres and polyester or vinylester 65 66 matrices. However, due to the differences in the properties of the specimens, each subset contained no more than 30 samples. Zureick, Bennett [24] investigated the longitudinal tensile and 67 compressive strengths, the longitudinal tensile and compressive modulus, the shear strength and 68 69 modulus. The Weibull distribution was proposed to model the strength and stiffness properties. 70 Further studies on the probabilistic properties of composites can be found in Jeong and Shenoi [25] 71 or Lekou and Philippidis [26].

72 **2. Research Significance**

The main limitations in previous studies are mainly related with the small size of the samples that 73 makes it difficult to accurately characterise probabilistic distributions. Previous models focused on 74 75 the entire sample distribution and ignored the importance of the tail region for probabilistic analysis. It is also difficult to obtain suitable probability distribution functions without sufficient 76 77 number of samples and to output accurate estimates for the tail region. As such, discrepancy 78 between existing models and experimental data could reach several orders of magnitude [27]. To 79 address these limitations, the main aim of this work is to validate and propose probabilistic models 80 for the mechanical properties of the carbon FRP (CFRP) laminates (i.e. Young's modulus, ultimate

strain and tensile strength) and to highlight the importance of the tail of the sample distribution. All
statistical analyses are performed on a large and homogeneous batch of samples.

83 **3. Experimental Tests**

The data used in the present study concerns pultruded laminates produced from the same manufacturer. The CFRP had a density of 1.4g/cm³ and a fibre content above 68% in volume, with a tensile design stress of 1000 MPa and 1300 MPa, respectively for 0.6% and 0.8% elongation. As part of the quality process of the manufacturer, the mechanical properties of the CFRP were consistently assessed in the fibre direction. In total, a large set of 1368 coupon samples were obtained for this process, collected from specimens with various cross sections (60-168 mm²) – see appendix A for complete sample characterisation.

The coupon configuration for tensile testing was based on the EN ISO 527-5 [28] standard (Table 1), with all the tensile tests being carried out according to same standard on a Zwick Z100 universal testing machine (Figure 1a). As part of the experimental procedure, a pre-load of 0.1 kN was applied to avoid any misalignment within the system. Then, each coupon sample was loaded at a constant displacement rate of 2 mm/min until failure. Both loading and CFRP strain were directly measured using a load cell and a strain gauge, respectively (Figure 1b).

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Table 1. Details of the tensile samples based on EN ISO 527-5 [28].

Detail	Values (mm)
FRP length	250
FRP width	15 (±0.5)
FRP thickness	1.0 (±0.2)
Tab extension	> 50
Tab thickness	0.5-2
Grip extension	≥ 7
Gauge length	50 (±1)
Bevel angle	90



It should be denoted that the pre-load was considered in the analyses described in the following sections. Furthermore, the data for statistical analysis was carefully selected to exclude invalid results arising from: (i) tab region failure; (ii) broken fibres in contact with the strain gauge; (iii) slippage of specimens from the jaws; and (iv) failure of specimens at or close to the jaws. The stress versus strain curves were plotted, and the tensile strength, modulus of elasticity, and ultimate strain of the FRP were calculated. Figure 2 illustrates typical raw stress-strain diagrams for coupon samples tested where the linear elastic behaviour can be observed nearly up to failure.



111

Figure 2. Raw stress-strain diagrams for five tested coupon samples.

112 4. Statistical Models

113 Three statistical distributions were considered to model the CFRP properties: (i) normal; (ii) log-114 normal; and (iii) Weibull. The probability density function (PDF) and the cumulative distribution 115 function (CDF) for each distribution were obtained from the following relationships.

116 - Normal distribution

117 PDF:
$$f(x \mid \mu, \sigma) = \frac{1}{\sigma\sqrt{2\pi}} e^{-\frac{1}{2}\left(\frac{x-\mu}{\sigma}\right)^2}, \quad \sigma > 0, -\infty < x, \mu < +\infty,$$
 (1)

118 CDF:
$$F(x \mid \mu, \sigma) = \frac{1}{\sqrt{2\pi}} \int_{-\infty}^{x} e^{-\frac{1}{2} \left(\frac{t-\mu}{\sigma}\right)^2} dt$$
, (2)

119 where μ is the mean and σ is the standard deviation, and t is a real variable.

120 - Log-normal distribution

121 PDF:
$$f(x \mid \mu, \sigma) = \frac{1}{x\sigma\sqrt{2\pi}} e^{-\frac{(\ln x - \mu)^2}{2\sigma^2}}, \quad x > 0, \sigma > 0, -\infty < \mu < +\infty,$$
 (3)

122 CDF:
$$F(x \mid \mu, \sigma) = \frac{1}{\sigma\sqrt{2\pi}} \int_0^x \frac{1}{t} e^{-\frac{1}{2}\left(\frac{\ln t - \mu}{\sigma}\right)^2} dt.$$
 (4)

123 - Weibull distribution

Since previous studies [29] showed that the statistical characterisation of the CFRP does not improve using a three-parameter Weibull distribution, a two-parameter approach was adopted here. This is defined by the following expressions:

127 PDF:
$$f(x \mid \alpha, \beta) = \frac{\alpha}{\beta} \left(\frac{x}{\beta}\right)^{\alpha-1} e^{-\left(\frac{x}{\beta}\right)^{\alpha}}, \quad \alpha, \beta \ge 0, 0 \le x < +\infty,$$
 (5)

128 CDF:
$$F(x \mid \alpha, \beta) = 1 - e^{-\left(\frac{x}{\beta}\right)^{\alpha}}$$
, (6)

129 where α and β are the shape and the scale parameters, respectively.

130 The best-fit distributions were found following the censored maximum likelihood estimation 131 (MLE) [30]. This method allows estimating parameters θ of a statistical distribution for a sample, 132 considering the following:

133
$$L(\theta \mid \hat{x}_1, \hat{x}_2, \dots, \hat{x}_n) = \prod_{i=1}^n f_X(\hat{x}_i \mid \theta),$$
 (7)

134 in which L(.) is the likelihood that the parameters $\theta = \theta_1, \theta_2, ..., \theta_n$ properly describe the sample 135 $\hat{x} = \hat{x}_1, \hat{x}_2, ..., \hat{x}_n$, and f_X is the joint PDF of a sample. The maximum likelihood estimators are 136 computed from the set of parameters that maximise the likelihood function by considering all 137 possible cases of θ .

Since the tail region is critical for structural reliability analysis and prediction, especial attention is given to this region in the statistical analysis of the tensile tests. The adopted technique considers explicitly the values of the lower tail that are smaller than a predefined bound, whereas the remaining values are used implicitly [31]. The censored MLE can be defined as follows:

$$142 L = L1 \times L2, (8)$$

143 with

144
$$L1 = \prod_{i=1}^{J} f(x_i | \theta),$$
 (9)

145
$$L2 = P(X \ge x_G \mid \theta)^{n-j},$$
 (10)

146
$$P(X \ge x_G \mid \theta) = 1 - F(x_G \mid \theta), \tag{11}$$

147 where L1 is the likelihood associated with the *j* observations of values equal or lower than the 148 bound value x_G . L2 is the likelihood associated with the observations of values higher than the 149 bound value x_G . $F(x_G | \theta)$ is the CDF of x_G given the PDF θ , *n* is the total number of observations and n-j is the total number of observations exceeding the bound value x_G . The best fit can be computed iteratively through the optimisation problem of maximising *L*.

For each property, the distributions families were adjusted for the entire sample and the lower percentiles of: 20th, 25th, 30th, 35th and 40th. The 20th percentile is considered to be a reasonable choice for reliability studies in this research, since it includes the region of interest without decreasing the sample size to statistically meaningless values.

The goodness of fit for all distributions was examined using the Anderson-Darling test for the: (i) entire samples; and (ii) samples with right-censored data. The Anderson-Darling test was adopted since it provides adequate comparison tools for tail regions [32]. The statistic for the rightcensored data and entire data can be obtained respectively by [33]:

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$$A^{2} = -\frac{1}{n} \sum_{i=1}^{r} (2i-1) \Big[\ln Z_{(i)} - \ln(1-Z_{(n+1-i)})) \Big] - n,$$
(12)

161
$$A_{r,n}^{2} = -\frac{1}{n} \sum_{i=1}^{r} (2i-1) \left[\ln Z_{(i)} - \ln \{1 - Z_{(i)}\} \right] - 2 \sum_{i=1}^{r} \ln \{1 - Z_{(i)}\} - \frac{1}{n} \left[(r-n)^{2} \ln \{1 - Z_{(r)}\} - r^{2} \ln Z_{(r)} + n_{(r)}^{2} \right]$$
162 (13)

where *r* is the uncensored observation, *n* is the total number of observations and *Z* denotes the CDF of the probability distribution. The statistic values (A^2) were then compared with the critical values (CV) presented by Stephens and D'Agostino [33]. The null hypothesis (*H0*) of the data following the distribution tests was not rejected if the statistic value was lower than the critical value. The critical values for different percentiles are given in Table 2. To minimise Type I errors, which occur when *H0* was wrongly rejected, or Type II errors, in which *H0* was wrongly accepted, the significance level (α) was set at 10%.

Table 2. Critical values for different percentiles.

Percentile	20%	25%	30%	35%	40%	100%
CV	0.436	0.545	0.651	0.756	0.857	1.933

171 **4.1. Young's modulus**

The Young's modulus is one of the significant parameters related with the structural safety of the FRP for rehabilitation of structures, particularly in situations where failure is expected to occur at tensile stresses significantly lower than the ultimate strength of the FRP. This type of failure usually occurs when debonding of the CFRP or concrete crushing are the dominant failure mechanisms [9].

176 The best fit for each PDF for the Young's modulus is illustrated in Figure 3. As it can be seen in Figure 3a, when the distributions were fitted to the entire sample, significant differences existed in 177 178 the range of the lower and upper values. Considering the importance of the tail regions in safety assessment, clear improvements were achieved by applying the approach described above firstly to 179 180 the lower 20th percentile region – see Figure 3b. Both normal and log-normal distributions provided similar results, whereas the Weibull distribution showed the closest fit to the data. For more clarity, 181 the Q-Q curves were plotted for three distributions in Figure 4. The Weibull distribution was able to 182 183 approximate the experimental data with high precision in both 20th percentile lower tail and entire 184 range regions (Figure 4e and Figure 4f).



Figure 3. PDF for the Young's modulus of: (a) the entire data fit, (b) the 20th percentile lower tail
fit; and (c) the 20th percentile upper tail fit.

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Figure 4. Q-Q plot of the Young's modulus based on: normal distribution adjusted to (a) the entire range, and (b) the 20th lower and (c) 20th upper percentile; log-normal distribution adjusted to (d) the entire range, and (e) the 20th lower and (f) 20th upper percentile; and Weibull distribution adjusted to (g) the entire range, and (h) the 20th lower and (i) 20th upper percentile.



- values, meaning that the average squared distance between the data and the fitted distribution was
- also the lowest.

207Table 3. Statistical values for the Anderson-Darling goodness of fit test for each percentile and

208

Percentile	Normal	Log-normal	Weibull
20%	0.279	0.373	0.123
25%	0.598	0.696	0.427
30%	1.587	1.697	1.327
35%	1.587	1.697	1.327
40%	4.226	4.351	3.767
100%	18.236	23.568	13.678

distribution.

209

Based on the statistical analysis of the experimental data, the following shape and scale parameters were proposed to model the Young's modulus based on the Weibull distribution adjusted to the 20th percentile:

213
$$E_f \sim W(26.2, 180.9)$$
 GPa. (14)

Depending on the design situation, the upper percentile of the Young's modulus might also be required. For example, in situations of debonding failure, an higher value for this material parameter can provide more conservative estimates on the capacity of the structural member. For this reason, the study described in this section was similarly applied to obtain the best fit distribution for the 20th upper percentile. Results are shown in Figs. 3 and 4, whereas the Weibull distribution adjusted to the upper tail region was given by the following equation:

220
$$E_f \sim W(20.4, 174.4) \text{ GPa.}$$
 (15)

Using the distributions shown in Eqs. (14) and (15), the characteristic values for the Young's modulus were determined as 161.5 GPa and 184.0GPa, respectively corresponding to the 5th and 95th percentiles. It should be mentioned that the lower value was only slightly below the design value provided by the manufacturer (165 GPa). Results also showed that the coefficient of variation
was reduced, i.e., 0.04.

226 **4.2.** Ultimate strain

227 The ultimate strain of the FRP is another important parameter in structural safety since the material typically exhibits elastic behaviour until failure. The same procedure described above was 228 229 followed to analyse this material parameter from the tensile tests. Conversely to what was observed 230 for the Young's modulus, the statistical analysis showed that (Figure 5a) none of the selected 231 distributions could fit well the lower tail when using the entire sample. Figure 5b shows the ultimate 232 strain probability density functions adjusted to the lower tail, where the Weibull distribution was the 233 one that provided the best results. The same trend could be seen in the corresponding Q-Q plots 234 illustrated in Figure 6.





Figure 5. PDF for the ultimate strain of the entire data fit (a) and 20th percentile lower tail fit (b).

The Anderson-Darling goodness of fit test presented in Table 4 shows that the Weibull was the only distribution not rejected for the highest percentile (in this case the 40th), whereas the null hypothesis was rejected for all the distributions adjusted to the entire sample. Based on these results, the Weibull distribution adjusted to the 20th percentile was proposed to model the ultimate strain with a coefficient of variation of 0.06, and the following parameters:

243
$$\varepsilon_{fu} \sim W(17.1, 1.5) \%$$
 (16)



percentile.

Percentile	Normal	Log-normal	Weibull
20%	0.206	0.351	0.050
25%	0.237	0.393	0.057
30%	0.433	0.656	0.101
35%	0.771	1.148	0.126
40%	1.371	2.056	0.136
100%	2.5453	5.485	8.9873

distribution.

257 **4.3.** Tensile strength

The tensile strength of the FRP is important in situations where failure occurs within the laminate. This can be particularly critical for prestressed FRP laminates, since the prestress loading often represents a high percentage of the tensile strength [34, 35]. Preliminary results of the distributions adjusted to the entire sample showed that all selected distributions were unable to provide a good fit in the lower tail, as illustrated in Figure 7a. An improvement could be obtained when the procedure based on fitting the CDF to the lower tail is followed – see Figure 7b. The Weibull distribution performed better in both cases.



Figure 7. PDF for the tensile strength of (a) the entire data fit (b) and the 20th percentile lower tail fit.

The Q-Q plots showed the similarity between normal and log-normal distributions – see Figure 8a-d – and that using the entire sample was not suitable for the lower tail region. The good

fit obtained with the Weibull distribution in this region can be noticed by comparing Figure 8e and f. Despite these observations, the goodness of fit results for the lowest tail fit (Table 5) did not reject any of the distributions for the 20th and 25th percentiles. However, since the Weibull presented a better result than the other models overall, it was adopted here as the distribution model for the tensile strength with the following parameters:

276
$$f_f \sim W(15.9, 2777.0)$$
 MPa. (17)

The 5th characteristic value using the proposed distribution was 2304.2 MPa, which was only 0.3% higher than the experimental value (2299.0 MPa). The coefficient of variation was also very small, i.e. 0.08. The selected distribution is in agreement with the works from Atadero [23] and Zureick, Bennett [24] for prediction of the tensile strength based on the entire data fit.





refeelitile	I WIIIIIIIII	Log-normai	weibuli
20%	0.050	0.068	0.064
25%	0.342	0.366	0.333
30%	0.894	0.941	0.817
35%	2.518	2.658	2.154
40%	4.160	4.429	3.404
100%	5.453	5.485	9.897

293 **5.** Correlation Analysis

This section presents a correlation analysis on the mechanical properties discussed in the previous section. Within the linear elastic range, strain, stress and Young's modulus are naturally related with each other by the Hooke's law. When approaching ultimate values – i.e. the material strength – the standard relation may no longer hold and more suitable relationships may need to be recommended for reliability analysis. The following pairs were considered: (i) tensile strength and ultimate strain, (ii) tensile strength and Young's modulus, and (iii) Young's modulus and ultimatestrain.

A linear regression analysis was firstly performed between tensile strength and ultimate strain without constraints. Results showed high correlation between these two properties ($R^2 = 0.75$) as illustrated in Figure 9a. Additionally, the residual standard deviation related with the uncertainty of the proposed model was 0.062%, which means that a probabilistic model could indeed describe the correlation between the two mechanical parameters. The corresponding model was defined as follows:

$$307 \qquad \varepsilon_{fu} = 0.17 + 0.0005014 f_f + 0.0618 Z(\%), \tag{18}$$

308 where f_f is the tensile strength in MPa, ε_{fu} is the ultimate strain and $Z \sim N(0,1)$.

Based on the results above, a second correlation analysis was performed by constraining the linear relation to the origin. The results and observations were quite similar, as shown in Figure 9b. The latter model had a standard deviation of 0.063% and was defined by the following expression:

312
$$\varepsilon_{fu} = 0.0005646 f_f + 0.0633 Z(\%)$$
. (19)

The last expression can be recommended in practice to relate the two expressions, since it provides good results and is relatively simple. It should be mentioned that such result shows that the ultimate strain and tensile strength are highly correlated variables. However, since both are not deterministic, the numerical value in the equation should not be directly compared with the inverse ratio of the Young's modulus – although both are similar given the linear nature of the correlation found.

It should be highlighted that from this study, the tensile strength and Young's modulus were found to have a small correlation – see representation in Figure 10a. Similar observation was also found between the Young's modulus and ultimate strain (Figure 10b). This suggests that the variables could be considered as independent in both situations.



Figure 9. Scatter diagram of tensile strength versus ultimate strain of (a) the regression without



322 323

constraints and (b) the regression across the origin.



328 Figure 10. Scatter diagram of (a) tensile strength versus Young's modulus (f_f , E_f) and (b) 329 Young's modulus versus ultimate strain (E_f , ε_{fu}).

330 6. Conclusions

This manuscript presented a statistical analysis on mechanical properties of prefabricated CFRP laminates obtained from a large set of tests. Results showed that the Weibull distribution can be adopted to model the Young's modulus, the ultimate strain and the tensile strength of CFRP laminates. Furthermore, it was shown that the statistical characterisation of the CFRP should be carried out giving particular attention to the tail region. In fact, although an overall good fit of any
selected distribution can be achieved in most cases, the approximation obtained in the tail region is
not acceptable.

A low variability in the mechanical properties was also observed in this study, which is most significant in terms of structural safety. The lowest coefficient of variation is found for the Young's modulus, with the characteristic values from experimental data and proposed distributions being also very similar.

The correlation analysis between mechanical properties demonstrated that a probabilistic model relating the tensile strength and ultimate strain can be proposed. However, despite the strain, stress and Young's modulus being related by the Hooke's law in the linear elastic region, no probabilistic model could be proposed between tensile strength or ultimate strain and Young's modulus. In fact, these pairs of variables can be considered as independent.

As a final note, it should be mentioned that the distributions given in this paper can be used for carrying out reliability analyses aimed at proposing partial safety factors for the future revision of design codes.

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357 Appendix A: sample distribution

- 358 Table A-1 provides the sample size and geometrical data for the 1368 coupon samples studied in
- 359 this paper.

360

Cross-section	Area	Sample size
(mm^2)	(mm^2)	(#)
50×1.2	60	85
50×1.4	70	422
60×1.4	8.4	54
80×1.2	96	110
80×1.4	112	122
90×1.4	126	41
100×1.2	120	144
100×1.4	140	192
120×1.2	144	43
120×1.4	168	155

Table A-1. Details of coupon samples.

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