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3rd fib International Congress-2010

INFLUENCE OF THE MINERAL ADDITION ON THE DURABILITY OF MEDIUM STRENGTH SELF-COMPACTING CONCRETE

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ABSTRACT

Self-compacting concrete (SCC) is an innovative concrete that does not require vibration for placing and compacting. SCC, developed in Japan in the 1980s, provides a present-day and attractive challenge for many researchers, as the long list of papers on the topic corroborates. Nevertheless, the durability of SCC, especially medium strength SCC, provides the researcher with opportunities for study in depth.

This paper deals with the durability of SCC: two SCC mixtures, with 30 MPa compressive strength, are studied. The main difference between the two SCCs is the type of the addition: fly ash and limestone powder.

The microstructure and products of hydration of the SCC are analyzed by means of Mercury Porosimetry Intrusion (MPI) and TG/DTA techniques. The penetration of water under pressure, chloride diffusion and CO₂ penetration are also analyzed at different ages of the SCC specimens. Fresh and mechanical properties are also evaluated.

Microstructure, durability properties and mechanical behaviour are analyzed together, and then associated with the role of the admixture (influence of the type of admixture). Conclusions are then presented.

Keywords: Self-compacting concrete, Durability, Pore size distribution, Hydration, Additions.

INTRODUCTION

Self-compacting concrete (SCC) was originally developed in Japan in the mid-1980s, under the leadership of Prof. Okamura¹. SCC has been increasingly used in ready-mix concrete and in the precast industry to improve several aspects of construction. Though SCC has been examined by many researchers, some questions remain unanswered, especially durability aspects in comparison with vibrated concrete (VC), with the few results available²⁻³ on this topic usually concerning high-performance concrete (HPC). Less attention has been devoted to the durability of low and medium strength concrete⁴, and the effect of the addition (filler) in such durability. High volume of addition on the mix proportioning of SCC modifies the porous structure⁵⁻⁹, and affects transport mechanisms of liquids and gases. These aspects require concerted analysis.

The purpose of this paper is to study the influence of the addition type on the mechanical properties, fresh behaviour and durability of low-strength SCC (characteristic compressive strength of 25 MPa). Two SCCs, with the same mix design, though with different types of addition were cast and tested. Fly ash and limestone powder were used to make the concrete. Properties of fresh mix concrete, mechanical properties of hardened concrete and durability tests were performed. Accelerated carbonation tests, Cl⁻ ion diffusion on saturated specimen and water penetration under pressure tests were performed for both SCCs. Moreover, the porous structure was determined by means of mercury intrusion porosimetry (MIP), and the hydrated products by means of ATD/DTG.

Whereas the following section introduces the materials used, the subsequent section examines the experimental programme. Results are then presented, with discussion and conclusions being provided at the end.

MATERIALS

Two mixes of SCC were studied. These mixes were cast using the same cement: I 42.5 R/SR (sulphate resistant). Siliceous rolled with grade 0/4 sand and 4/16 gravel were adopted. Siliceous fly ash and limestone powder were adopted as additions, with an upper limit of 250 kg/m³. Polycarboxylate superplasticiser (SIKA Viscocrete 3425) was used as an admixture. No viscosity modifying agent was needed.

Table 1 shows the mix proportions for both SCCs. The mix design was made to perform SCC with 25 MPa characteristic strength. Table 1 shows several differences on the quantity of cement, water/cement (*w/c*) ratio and admixture dose.

The concrete was mixed using a vertical-axis planetary mixer with a capacity of 100 L. Concrete samples were cast in various cylindrical moulds of two sizes (15 ϕ x 30 cm height and 10 ϕ x 20cm de height), corresponding to the programmed tests:

Table 1. Mix proportions (kg/m^3).

Concrete	1	2
Cement type	I 42.5 R/SR	I 42.5 R/SR
Addition type	Fly ash	Limestone powder
Cement (kg)	280	325
Addition (kg)	200	220
Water (kg)	140	179
Sand (kg)	960	960
Gravel (kg)	695	695
Admixture (kg)	4.6 (1.7%)	7.1 (2.2%)
<i>w/c</i>	0.50	0.55

EXPERIMENTAL PROGRAMME

TESTS FOR FRESH SCC

Three tests were performed: the slump flow test (according to UNE 83361 standard), L box (in accordance with the UNE 83363 standard) and V-funnel (UNE 83364 standard). Fig. 1 shows the “slump-flow spread”. Fig. 2 shows the fresh concrete in the L Box after testing.



Fig. 1 “Slump-flow spread” after slump flow test.



Fig. 2 Concrete in L box after testing.

TESTS FOR HARDENED SCC

In mechanical terms, three properties were measured: compressive strength (EN 12390-3 standard), elasticity modulus (UNE 83316 standard) and tensile strength (EN 12390-6 standard). The tests were performed at seven, 28 and 91 days of the age of the samples.

TESTS FOR DURABILITY

The tests performed for durability characterization were the following: water penetration under pressure, accelerated carbonation and chloride diffusion on saturated sample. All began when the concrete age was at 91 days.

Water penetration under pressure test was performed with cylindrical $15\phi \times 30\text{cm}$ height samples, in accordance with the EN 12390-8 standard. Water applied pressure was 500kPa with a duration of three days. Fig. 3a shows the specimen under testing, and Fig. 3b the final depth profile of water inside of the split specimen.

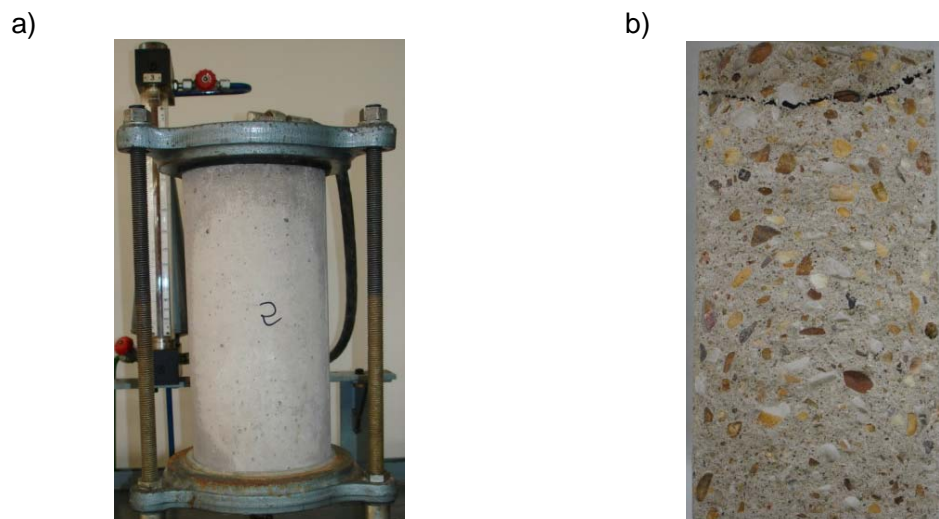


Fig.3 Water penetration under pressure tests: a) specimen under testing, and b) wet profile on the split tested specimen.

An accelerated carbonation test was performed with cylindrical $10\phi \times 20\text{cm}$ height samples. The specimens were placed in a carbonation chamber, with a constant temperature ($23^{\circ}\text{C} \pm 2^{\circ}\text{C}$) and relative humidity ($65\% \pm 1\%$). The tests were carried out at a CO_2 concentration of $6\% \pm 1\%$, with the difference being a maximum 100% N_2 . The carbonation depth was measured with phenolphthalein (UNE 112011 standard). Measurement was carried out at 120, 150 and 180 days of concrete life. Fig. 4a shows the carbonation chamber and Fig. 4b the carbonated ring on a sample after the phenolphthalein application.

Chloride diffusion test was performed with cylindrical $10\phi \times 20\text{cm}$ height samples, in accordance with the ASTM C1543 standard. This test was performed in saturated material,

under unidirectional diffusion (waterproofing vertical face and bottom base of the cylinder) in an aggressive solution (NaCl concentration of 3% in deionized water) at 20°C. Measurement of the chloride concentration inside the specimen was performed down to the depth of 30mm, with intervals of 6mm. The UNE 112010 standard was adopted for measurement of the chloride concentration.

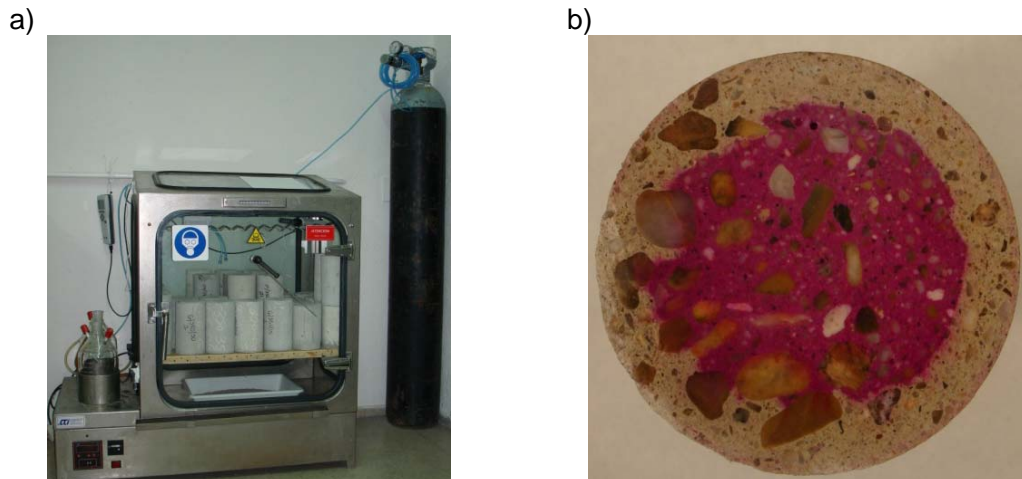


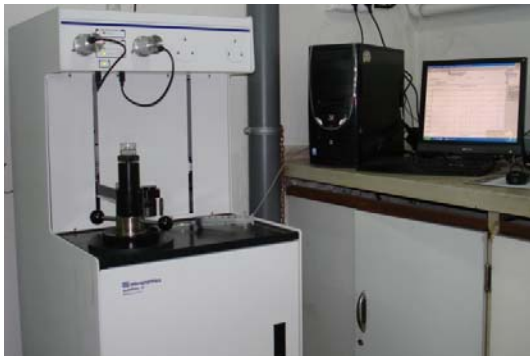
Fig. 4 Accelerated carbonation test: a) Carbonation chamber, and b) carbonated ring on a sample after the phenolphthalein application.

TESTS FOR POROUS STRUCTURE AND BINDER HYDRATION

MIP tests were performed with a Micromeritics porosimeter, Autopore IV 9500 model, which reached a pressure of 228 MPa, and measured diameter of pores from 0.006 μm to 175 μm . Tests were performed when the concrete age was at 91 days. The ASTM D4404 standard was adopted. The weight of the sample was 3.5 g \pm 0.3 g. The specimen was dried at 40°C and degasified. Fig. 5 shows the porosimeter. For modeling, the pores were idealized as cylindrical channels, and the Washburn-Laplace law was adopted to relate the mercury pressure with the radius of the pore.

Thermogravimetric analysis (TG/DTA) was performed to study the hydration of the binder and cement. The STA 791 device was used. The specimen was heated up to 1,000°C, with a velocity of 10°C/min. The reference material was $\alpha\text{-Al}_2\text{O}_3$, heated at 1,200°C. The tests were carried out in N_2 atmosphere with a flow of 80ml/min. Results, in accordance with the ASTM E1131 standard, were obtained when concrete life was at seven, 28 and 90 days.

a)



b)

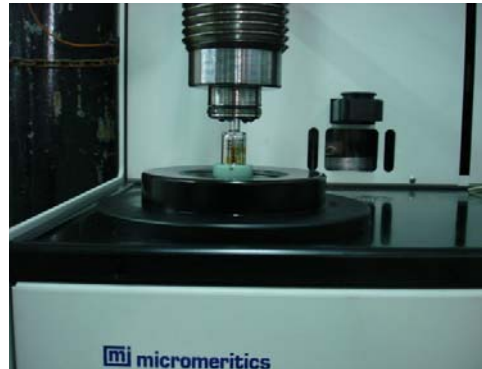


Fig. 5 Mercury porosimetry intrusion test: a) porosimeter, and b) penetrometer with a high pressure cell.

RESULTS

MECHANICAL RESULTS OF THE HARDENED CONCRETE

Fig. 6, 7 and 8 show the compressive strength, elasticity modulus and tensile strength (Brazilian test) of concrete life at seven, 28 and 91 days. Figures show the average values of three valid tests.

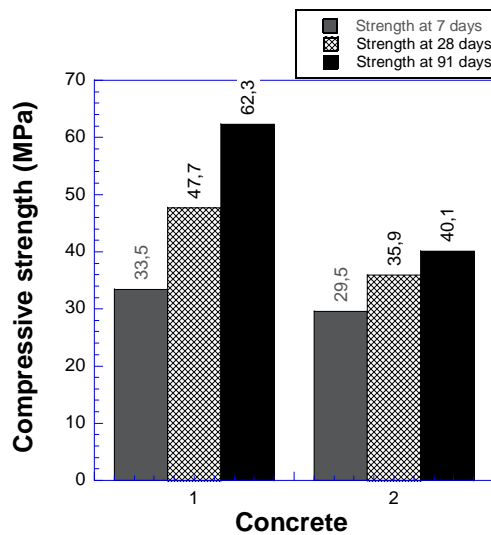


Fig. 6 Compressive strength.

Compressive strength at 28 days is in both cases greater than 35MPa. Concrete 1 reaches higher compressive strength at all points in its life. Modulus of elasticity shows similar

behaviour. With regard to Concrete 2, the elasticity modulus close to being equal at 28 and 91 days. Tensile strength shows similar behaviour.

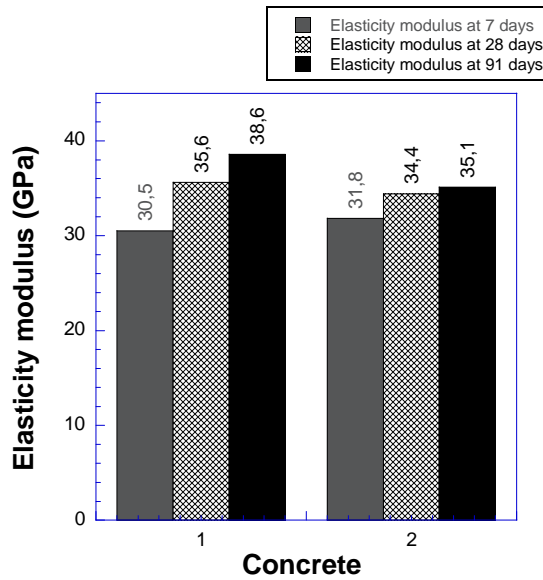


Fig. 7 Elasticity modulus.

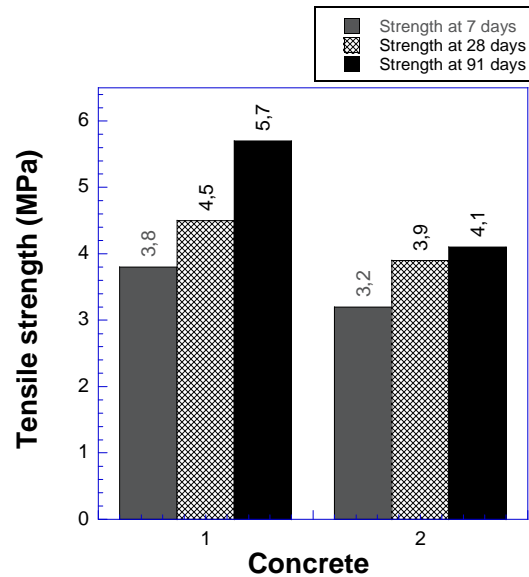


Fig. 8 Tensile strength (Brazilian test).

RESULTS OF FRESH TESTS

Table 2 shows the results of the characterization of fresh tests. Table 2 also includes the reference values for a SCC.

Table 2. Fresh concrete tests results and limits by EHE-08¹⁰.

Concrete		1	2	Limits (EHE-08) ¹⁰
Slump flow	$T_{500}(s)$	2.5	4.8	$T_{50} \leq 8 s$
	$\varnothing_m (mm)$	750	700	$550mm \leq d_f \leq 850mm$
V funnel	$T_V(s)$	9	12	$4 s \leq T_V \leq 20 s$
L box	$T_{600}(s)$	5.8	3	-
	H_2 / H_1	0.75	0.76	$0.75 \leq C_{bL} \leq 1.00$

DURABILITY RESULTS

Fig. 9 shows the maximum water penetration depth, corresponding to the water penetration under pressure test, and the compressive strength. Concrete 1 shows lower depth water penetration.

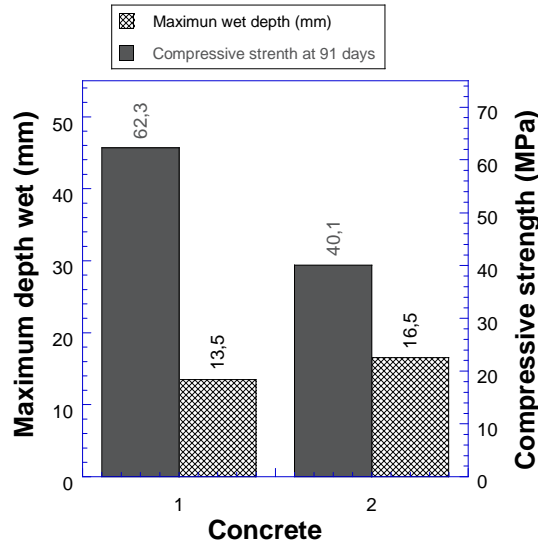


Fig. 9 Maximum wet depth in the water penetration under pressure test.

Fig. 10 shows the depth and velocity of carbonation. Carbonated depth and velocity of carbonation is higher in concrete 2. Table 3 shows the velocity of carbonation for both concretes.

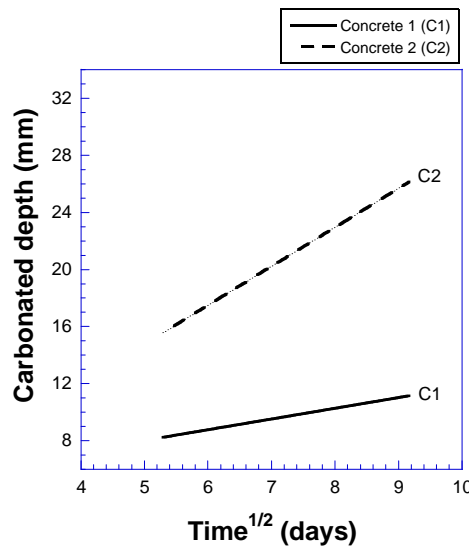


Fig. 10 Carbonated depth in the accelerated carbonation test.

Table 3. Carbonation velocity results.

Concrete	Carbonation velocity (mm/day ^{1/2})
1	0.4
2	1.1

Table 4 shows the effective coefficient of chloride diffusion. Concrete 2 shows a higher value of the effective coefficient of chloride diffusion, approximately double the amount, than concrete 1.

Table 4. Effective coefficient of chloride diffusion.

Concrete	effective coefficient of chloride diffusion (mm ² /s)
1	1.36E-8
2	2.99E-8

Fig. 11 shows the total mercury porosity when concrete age was at 91 days. Significant differences can be observed, with concrete 1 being more impermeable. Fig. 12 shows the average pore size and maximum depth water penetration (a water penetration under pressure test), and clearly concrete 2 shows the larger pores size. Table 5 shows the meso and macro pore volume, with notable differences being observed between concretes.

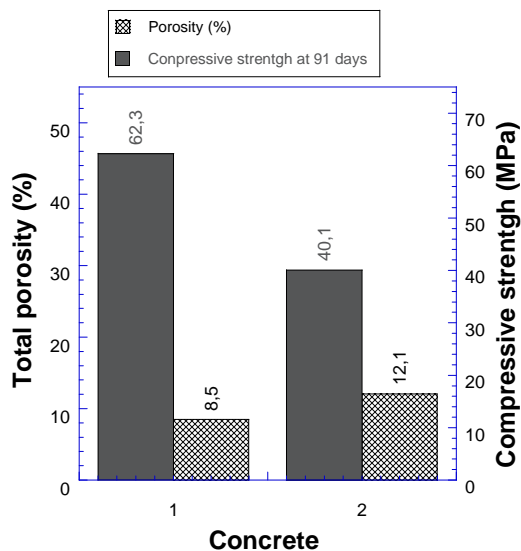


Fig. 11 Total mercury porosity and compressive strength at 91 days.

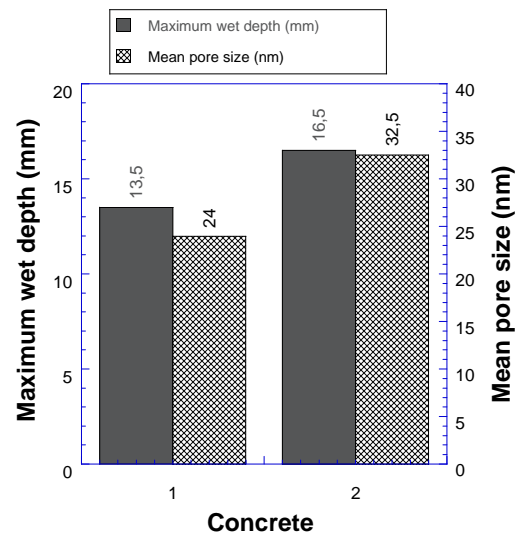


Fig. 12 Average pore size and maximum depth water penetration.

Table 5. Macro and meso pores volume.

Concrete	Macro-pore volume (ml/g)	Meso-pore volume (ml/g)
1	0.012	0.025
2	0.019	0.036

RESULTS OF TG/DTA

Fig. 13 shows the loss of water at seven, 28 and 91 days, with differences being notably clear. Fig. 14 shows the Portlandite (Ca(OH)_2). The influence of fly ash on concrete 1 is evident. Fig. 15 shows the grade of hydration.

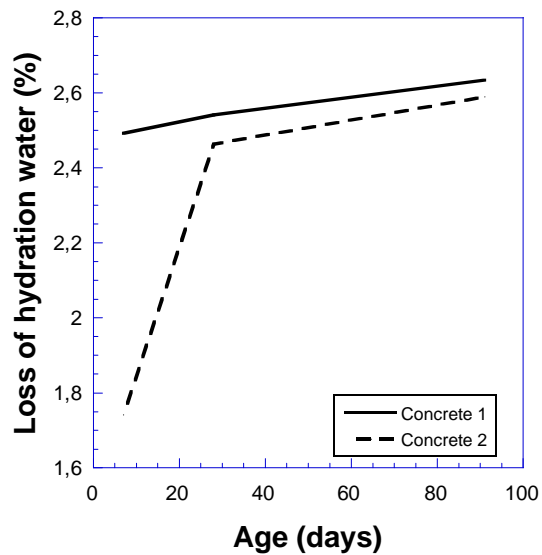
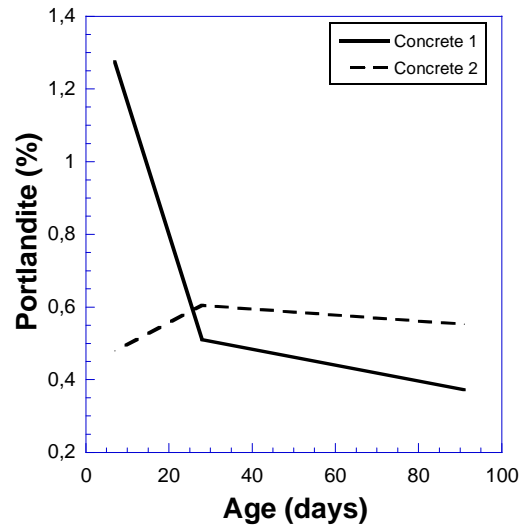


Fig. 13 Loss of hydration water.

Fig. 14 Portlandite (Ca(OH)_2) content.

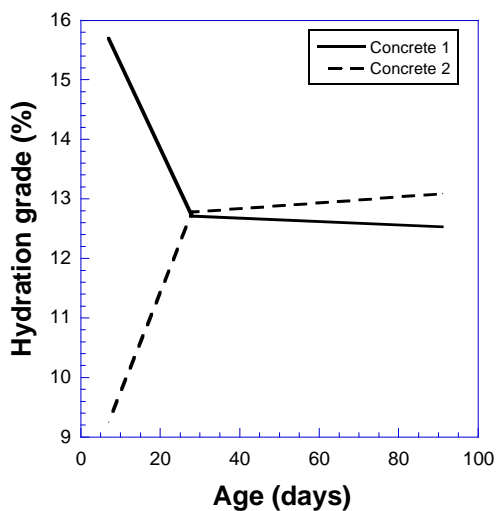


Fig. 15 Hydration grade

DISCUSSION

The main different between the concretes is the addition: fly ash (concrete 1) and limestone powder (concrete 2). Table 1 shows the mix proportions for each concrete. Clearly, the compressive strength evolution until 91 days is different. Similar behavior is shown by tensile strength and minor difference by modulus of elasticity.

Fig. 15 shows differences in the hydration process. The C-S-H gel caused by the pozzolanic reaction of fly ash is different (concrete 1) from the gel generated in concrete 2. Concrete 1 shows high hydration grade at seven days, and one lower at 28 days; concrete 2 shows low hydration grade at seven days, and a higher one at 28 days. In both cases there is no significant increment from 28 to 91 days.

The loss of hydration water (related with C-S-H content) and portlandite content are coherent, as Fig. 13 and 14 show. In concrete 1 the content of portlandite diminishes from seven to 28, due to the reaction of fly ash.

Compressive strength fit with C-S-H content.

The differences in wet depth (a water under pressure test), chloride diffusion and carbonated depth are also congruent with the values of total porosity and distribution of pore sizes. Both concretes have low permeability and good durability (for the performed tests), though concrete 1 is of a better nature in these aspects. Fly ash improves mechanical properties and durability when it is adopted as addition for SCC.

CONCLUSIONS

- Fly ash is a more suitable addition for SCC; it improves the mechanical properties and potential durability.
- The origin seems to be the pozzolanic nucleuses generated by fly ash, which compacts the matrix of concrete.
- The obtained results confirm for SCC that macro-pores affect compressive strength, water penetration under pressure and carbonated depth. Meso-pores affect chloride diffusion.

ACKNOWLEDGEMENTS

The authors gratefully acknowledge the financial support for the research provided by the Spanish Ministerio de Ciencia e Innovación under grant BIA-2008-03523. In addition, they also thank the Ministerio de Fomento under grant MFOM-01/07, and the Instituto Español del Cemento y sus Aplicaciones (IECA) for the complementary financial support received.

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