

1 Stochastic Petri net-based modelling of the durability of renderings

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4
5 **Abstract:**

6 In this study, a methodology to model and predict the life-cycle performance of building
7 façades based on Stochastic Petri Nets is proposed. The proposed model evaluates the
8 performance of rendered façades over time, evaluating the uncertainty of the future
9 performance of these coatings. The performance of rendered façades is evaluated based
10 on a discrete qualitative scale composed of five condition levels, established according to
11 the physical and visual degradation of these elements. In this study, the deterioration is
12 modelled considering that the transition times between these condition states can be
13 modelled as a random variable with different distributions. For that purpose, a Stochastic
14 Petri Nets model is used, as a formal framework to describe this problem. The model's
15 validation is based on probabilistic indicators of performance, computed using Monte-
16 Carlo simulation and the probability distribution parameters leading to better fit are
17 defined as those maximizing the likelihood, computed using Genetic Algorithm. In this
18 study, a sample of 99 rendered façades, located in Portugal, is analysed, and the
19 degradation condition of each case study is evaluated through *in-situ* visual inspections.
20 The model proposed allows evaluating: i) the transition rate between degradation
21 conditions; ii) the probability of belonging to a given degradation condition over time;
22 and iii) the mean time of permanence in each degradation condition. The use of Petri Nets
23 shows to be more accurate than a more traditional approach based on Markov Chains, but

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24 also allows developing future research to consider different environmental conditions,
25 maintenance actions or inspections, amongst other aspects of life-cycle analysis of
26 existing assets.

27

28 **Keywords:** Petri nets; rendered façades; genetic algorithms; degradation.

29 **1. Introduction**

30 According to Jensen and Rozenberg (2012), the net theory can be seen as “a system
31 theory that aims at understanding systems whose structure and behaviour are determined
32 by a combinatorial nature of their states and changes”. The first proposal of nets of places
33 and transitions, proposed by C. A. Petri (Petri, 1962), allows developing a non-idealizing
34 methodology to concurrency and information flow, in organizational systems (Genrich
35 and Lautenbach, 1981). Petri nets are considered a mathematical and graphical tool for the
36 formal description of systems whose dynamics are characterized as being concurrent,
37 asynchronous, distributed, parallel, nondeterministic, and/or stochastic, mutual exclusive,
38 and conflicting, which are typical features of distributed environments (Murata, 1989).
39 Therefore, Petri nets allow capturing the static and the dynamic nature of a real system,
40 thus characterizing the rate of transition between states or conditions (Marsan et al.,
41 1994).

42 Due to their characteristics, Petri nets have been successfully applied in different fields
43 of knowledge, namely in robotics (Al-Ahmari, 2016), in the optimization of
44 manufacturing systems (Chen et al., 2014; Uzam et al., 2015), business process
45 management (van der Aalst, 2002), human computer interaction (Tang et al., 2008),
46 among others. Petri nets are not widely used in the construction industry, and
47 particularly in building asset modelling. Nevertheless, there are various works (Li,
48 1998; Cheng et al., 2011; Molinero and Núñez, 2011; Cheng et al., 2013; Rinke et al.,
49 2017) that use Petri nets to manage resources, to estimate equipment availability and
50 scheduling of tasks on the site-work during the building design process. On the other
51 hand, recent work has been published on the use of Petri Nets to model the deterioration
52 of other civil engineering infrastructures (Andrews, 2013; Rama and Andrews, 2013; Le
53 and Andrews, 2015; Le and Andrews, 2016; Leigh and Dunnett, 2016; Yianni et al.,

54 2017; Zhang et al., 2017). In the last decades, various authors proposed several
55 extensions and adaptations of ordinary Petri nets; all of them based on the basic Petri
56 net formalism, but presenting very different characteristics and assumptions, in order to
57 adapt themselves to the phenomena under analysis. Consequently, there is a reasonable
58 expertise in the application of Petri nets to different application domains, thus allowing
59 transferring knowledge and methodologies from one field to another (Girault and Valk,
60 2002).

61 This study intends to evaluate the suitability and advantages of the use of Stochastic
62 Petri Nets (SPN) as deterioration models in building asset management. The main
63 advantages of SPN are their graphical representation, allowing a better and more
64 intuitive understanding of the modelling principles, and their versatility, allowing the
65 modelling complex stochastic processes. In the particular case of deterioration
66 modelling, and compared to the more traditional Markov Chains, SPN allow the
67 seamless use of different probabilistic distributions. Furthermore, their versatility allow
68 modelling, in a common framework, multiple aspects of asset management, including
69 deterioration, maintenance, inspection, and decision-making. In this study, a model to
70 predict the life-cycle performance of building façades based on stochastic Petri nets is
71 proposed. To analyse the degradation condition of rendered façades over time, a set of
72 Petri net models considering different probabilistic distributions are used to estimate the
73 transitions times between condition levels. Since there are no closed form expressions
74 for the probability distribution of the condition state at a certain time, Monte Carlo
75 simulation is used to compute the likelihood of each model. However, the errors
76 introduced by Monte Carlo simulation require the use of gradient-independent
77 optimization methods, like Genetic Algorithms, to identify the optimal parameters of
78 the probability distributions.

79 The sample analysed in this study comprises 99 renderings, located in Portugal, for which
80 degradation condition was evaluated through *in situ* visual inspections. The classification
81 system adopted in this study to evaluate the deterioration state of rendered façades is a
82 discrete qualitative scale divided in five condition levels, proposed by Gaspar and de Brito
83 (2008, 2011), ranging between “no visible degradation” (condition A) and “generalised
84 degradation” (condition E), which requires an immediate rehabilitation or maintenance
85 action.

86 In the first part of this study, a traditional method, based on Markov chains is applied, in
87 order to define a benchmark model. The benchmark model and the Petri net model with
88 transition times exponentially distributed are used to validate the methodology
89 proposed. The comparison of the models is possible since the stochastic Petri net with
90 transitions exponentially distributed is equivalent to a finite Markov chain. After that, a
91 set of probabilistic distributions are used to analyse the degradation condition of
92 rendered façades over time. The information obtained from the Petri net models allows
93 the identification of the degradation rate of rendered façades, characterizing the pattern
94 that characterizes the loss of performance of these claddings over time. This information
95 is crucial to identify the future need for interventions, optimizing the maintenance
96 needs, and thus avoiding unnecessary cost associated with urgent interventions.

97 The outline of this paper is as follows: Section 2 provides a literature review concerning
98 the classification system and modelling techniques used to model the evolution of the
99 degradation in rendered façades; Section 3 introduces the concept of Petri nets, as well
100 as the procedure used to predict the life-cycle performance of renderings. Finally, the
101 discussion of the results is presented in Section 4 and conclusions are drawn in Section
102 5.

103 **2. Literature review**

104 The façades can be seen as the skin of the building, i.e. they can be considered the first
105 layer of protection against the deterioration agents (Silva et al., 2015), thus being the
106 element more prone to degradation. According to Flores-Colen and de Brito (2010) the
107 claddings' degradation level can influence the quality of the urban environment, since it
108 affects the architectural appearance of buildings, which has a considerable effect on the
109 physical comfort of inhabitants of larger cities (Korjenic et al., 2016). Rendered façades
110 are the most common type of cladding in Portugal (Census, 2001). In the present
111 context of societies aiming at achieving a more sustainable use of resources, it is
112 increasingly important to define rational maintenance strategies so as to avoid
113 unnecessary costs (Wang and Xie, 2002; Arain and Pheng, 2006; Wong and Li, 2009).
114 For that purpose, it is essential to develop new and versatile tools to support the
115 decision-making process regarding the instant in which maintenance actions must be
116 performed, knowing the degree of uncertainty associated with the estimates (Frangopol,
117 2011). To achieve this, the present work focuses on the use of probabilistic based
118 methods for modelling performance, including Stochastic Petri Nets and Markov
119 Chains.

120 The definition of maintenance strategies is, in general, related with the users' demands,
121 i.e. more demanding users may demand a high level of performance, requiring that the
122 cladding be replaced as soon as it starts to deteriorate; on the other hand, some users
123 may accept a lower level of performance, thus minimizing the maintenance costs
124 (Shohet et al., 1999). Consequently, the definition of maintenance strategies requires the
125 condition assessment of rendered façades and the knowledge of their expected service
126 life. According to Hertlein (1999), condition-based maintenance by inspection planning
127 can be a useful tool to reduce the life cycle costs, achieving a more rational and efficient

128 way to manage maintenance budgets (Flores et al., 2011).

129 In the last decades, different studies (Shohet et al., 2002; Shohet and Paciuk, 2004;
130 Gaspar and de Brito, 2008; Paulo et al., 2014; Paulo et al., 2016) propose visual and
131 physical scales to characterize the type, extension and severity of defects observed in
132 rendered façades. Gaspar and de Brito (2008) and Silva et al. (2014) proposed a discrete
133 scale to evaluate the degradation condition of rendered façades (Table 1).

134 This qualitative scale, based on the evaluation of the physical and visual degradation of
135 rendered façades analysed during a comprehensive fieldwork, can be associated with a
136 quantitative index that portrays the global performance of the façades. This numerical
137 index, initially proposed by Gaspar and de Brito (2008, 2011), expresses the global
138 degradation of façade coatings through the ratio between the degraded area weighted as
139 a function of its condition and a reference area, equivalent to the whole and having the
140 maximum degradation level possible - equation (1).

$$141 \quad S_w = \frac{\sum(A_n \times k_n \times k_{a,n})}{A \times k} \quad (1)$$

142 Where S_w is the degradation severity of the coating, expressed as a percentage; k_n is the
143 multiplying factor of anomaly n , as a function of their degradation level, within the
144 range $K = \{0, 1, 2, 3, 4\}$; $k_{a,n}$ is a weighting factor corresponding to the relative weight
145 of the anomaly detected ($k_{a,n} \in \mathbb{R}^+$); $k_{a,n} = 1$ by default; A_n is the area of coating affected
146 by an anomaly n ; A is the façade area; and k is the multiplying factor corresponding to
147 the highest degradation level of a coating of area A .

148 In this study, the anomalies that occur in rendered façades are grouped in three
149 categories: stains; cracking; and detachment. The coefficient $k_{a,n}$ allows establishing a
150 relative weight between these groups of anomalies, based on the cost of repair of each

151 anomaly, its aesthetic impact, the influence on the renderings' service life, the
152 fulfilment of performance requirements (e.g. watertightness) and its propensity to
153 generate new anomalies. In this study, the following $k_{a,n}$ values are adopted for the
154 different groups of anomalies: 1.0 for cracking; 1.5 for detachment; and 0.25 for stains
155 in condition B and 0.67 for stains in more serious condition levels (C, D and E).

156 Figure 1 shows the correlation between the condition of some façades inspected and the
157 numerical index, illustrating the visual conditions of rendering in each degradation
158 condition.

159 **2.1 Application of Markov chains to model the evolution of the degradation of** 160 **rendered façades**

161 Markov chains are widely used by researchers in several fields of civil engineering (Wang
162 and Zaniewski, 1996; Hawk and Small, 1998; Thompson et al., 1998; McDulling, 2006;
163 Ortiz-García et al., 2006). Particularly, continuous-time Markov chains are commonly
164 used in modelling the deterioration of civil engineering assets (Kallen and van Noortwijk,
165 2006). This modelling technique is considered an intuitive, simple and computationally
166 inexpensive stochastic process, since analytical solutions exist and the memoryless
167 property allows estimating the future performance only based on the current performance,
168 becoming particularly relevant when limited information is available.

169 Silva et al. (2015) used continuous-time Markov chains to evaluate the degradation
170 process of external render over time, based on the visual inspections of characteristics and
171 condition of façades located in Portugal. In this work, it is assumed that the progression of
172 damage is continuous and, over a small time interval, the condition of the façade can only
173 remain constant or deteriorate to the next condition state. The intensity matrix defines the
174 rate of transitions between states (Kalbfleisch and Lawless, 1985) as:

$$175 \quad \mathbf{Q} = \begin{bmatrix} -q_{1,2} & q_{1,2} & \cdots & 0 & 0 \\ 0 & \ddots & \ddots & 0 & 0 \\ \vdots & \ddots & \ddots & \ddots & \vdots \\ 0 & 0 & \ddots & -q_{n-1,n} & q_{n-1,n} \\ 0 & 0 & \cdots & 0 & 0 \end{bmatrix} \quad (2)$$

176 The estimation of the optimal intensity matrix, leading to the best fit between the model
 177 and the observed condition, was based on the concept of maximum likelihood described
 178 by Kalbfleisch and Lawless (1985). Likelihood is defined as the predicted probability of
 179 occurrence of the observed transitions:

$$180 \quad L(\mathbf{Q}) = \prod_{i=1}^m \prod_{j=1}^k p_{ij} \quad (3)$$

181 Where i is the condition level in the initial instant, j is the condition level in the final
 182 instant, m is the number of elements, k is the number of intervals between inspections,
 183 and p_{ij} is the probability of transition from condition level i to condition level j , (i, j)
 184 entry of the transition probability matrix, \mathbf{P} , given by:

$$185 \quad \mathbf{P}(\Delta t) = e^{\mathbf{Q} \cdot \Delta t} \quad (4)$$

186 Where Δt is the time interval between inspections.

187 The optimization of the intensity matrix, \mathbf{Q} , was performed using the active set algorithm
 188 implemented in MATLAB[®]. The aim of the optimization algorithm is to find the
 189 parameters of the intensity matrix, \mathbf{Q} , which maximize the fitness function, $\log V =$
 190 $\max(\sum \sum \log p_{ij})$, while keeping all terms of matrix \mathbf{Q} positive, $q_{i-1,i} > 0$ where
 191 $i \in \{2, \dots, n\}$. This optimization algorithm is a reasonable tool for problems that use
 192 analytical expressions. In situations where analytical expressions are not available, the
 193 numerical estimation of the functions can lead to convergence problems and lack of
 194 robustness of the solution.

195 3. Petri nets

196 3.1 Conventional Petri nets

197 The concept of Petri nets was originally present by Carl A. Petri, who in his doctoral
198 thesis developed a new model of information flow and control in systems (Petri, 1962).
199 Petri nets are a graphical and mathematical modelling tool, suitable for characterizing
200 concurrent, asynchronous, distributed, parallel, nondeterministic, and/or stochastic
201 systems (Murata, 1989).

202 An ordinary Petri net is considered a directed, weighted, bipartite graph with an initial
203 state called the initial marking, M_0 (Murata, 1989; Schneeweiss, 2004). It is called a
204 bipartite graph because nodes are divided into two different types: places, usually
205 represented by circles, and transitions, usually represented by rectangles. Both nodes are
206 linked by directed arcs, from places to transitions (input arcs) and from transitions to
207 places (output arcs) (Peterson, 1977; Murata, 1989; Schneeweiss, 2004). The third
208 element of a Petri nets are tokens, usually represented by black dots, which represent the
209 elements in the system (Peterson, 1977; Murata, 1989). Figure 2 shows a simple Petri net
210 model. Transition T_1 has two input places (P_1 and P_2) and one output place (P_3). The
211 arcs that connect the input places to the transition and the transition to the output place
212 represent the pre- and post-conditions of the transition, respectively. When all input
213 places are occupied by a token the transition is said to be enabled. At this point, the
214 transition fires, the tokens are removed from the input places, and new tokens are created
215 in the output places. In this example, transition T_1 is not enabled because the pre-
216 conditions of the transition are not complied with, i.e. there is a token in place P_1 , but no
217 token at place P_2 . Once tokens exist in both P_1 and P_2 , transition T_1 will fire, tokens
218 from places P_1 and P_2 will be removed and a token will be placed in place P_3 .

219 In the context of this study, places represent resources or conditions while transitions can
220 represent actions or events that cause the system to change (Murata, 1989). Tokens are

221 stored in places representing the present state of the system and transitions allow the
222 tokens to move between two places modelling, in this way, the dynamic behaviour of the
223 system.

224 When analysing a PN, conflicts might occur when two or more transitions are enabled
225 from a common place and the firing of one transition disables the other transitions
226 (Bowden, 2000). In the literature, there are several proposals for solving conflicts, either
227 deterministically, for example through the introduction of a priority transition by the
228 user, or probabilistically, by assigning probabilistic properties to the conflicting
229 transitions (David and Alla, 2010; Wang, 2012). However, in timed Petri nets, the most
230 common way to solve conflicts is through firing times, assuming that the transition with
231 the shortest delay will fire first (Murata, 1989).

232 **3.2 Stochastic Petri net**

233 In the original definition of Petri nets, the concept of time is not explicitly included (Murata,
234 1989). However, many applications are time dependent and the introduction of time delays
235 has to be considered. The notion of time in Petri nets was initially introduced by
236 Ramamoorthy and Ho (1980) and Zuberek (1980). In these two works, deterministic time
237 intervals are used for each transition, creating a delay between the instant the transition
238 becomes enabled and the firing instant. Molly (1982) introduced the concept of stochastic
239 Petri net by assigning an exponentially distributed firing rate to each transition for
240 continuous time systems. After that, several classes of stochastic Petri nets have been
241 proposed for performance and reliability analysis of systems, the more relevant of which
242 are: the generalized stochastic Petri net (Marsan et al., 1984), the extended stochastic Petri
243 net (Dugan et al., 1984), and the deterministic and stochastic Petri net (Marsan and Chiola,
244 1987).

245 The model employed in this work considers Petri nets with transitions times defined as
246 a random variable, as proposed by Molly (1982). However, the results obtained showed
247 that the exponential distribution for the firing times, proposed by Molly (1982), were
248 not always adequate. To overcome this limitation, the proposal of Dugan et al. (1984),
249 allowing any probability distribution to be used to model the firing rate was used.

250 Mathematically, the theory behind the stochastic Petri net is the same as the Petri net;
251 their mode of operation is identical, applying the same firing rules. The only difference
252 is the random time interval between the transition becoming enabled and firing.

253 **3.3 Deterioration Petri net model**

254 Deterioration can be modelled with Petri nets by considering that each place is a
255 condition state of the classification system adapted, tokens indicate the current
256 condition of an element, and timed transitions define the movement between condition
257 states (Le, 2014, Yianni et al., 2016, 2017). In this work, a five condition levels Petri net
258 scheme is defined. Since maintenance actions are not considered, the condition level of
259 the infrastructure deteriorates continuously over time until it reaches the worst condition
260 level defined in the performance scale.

261 The time dependent nature of the problem is included by defining timed transitions. The
262 time specified by each transition represents the sojourn time in the condition level, i.e., the
263 time that infrastructure spends in condition level i before moving to condition level $i + 1$.
264 The timed transitions are modeled by probability distributions.

265 **3.4 Parameter estimation**

266 The probability distribution that best describes the deterioration process of an
267 infrastructure is that resulting in higher probabilities of occurrence of observed transitions.

268 In order to identify the parameters of the probability distribution that provide a best fit,
269 parameter estimation is required. The parameters of the probability distribution are fitted
270 to historical data through the maximum likelihood method proposed by Kalbfleisch and
271 Lawless (1985) and shown in equation (3). To simplify the computations and improve
272 robustness, the logarithm of the likelihood is maximized.

273 **3.4.1 Monte Carlo simulation**

274 The probability of occurrence of the observed transition, p_{ij} , is estimated by Monte Carlo
275 simulation. This is a helpful approach to compute numerical approximations in situations
276 where it is not feasible to obtain analytical solutions and can be used to consider the
277 propagation of uncertainties during the lifetime of the infrastructure. This method allows
278 generating random sojourn times to each condition level from the inverse CDF
279 (cumulative distribution function) of probability distribution.

280 The proposed procedure for computing the probability of occurrence of the observed
281 transition, p_{ij} , is illustrated in Figure 4. The procedure depicted is repeated for each
282 transition observed in the historical database. The input for the algorithm includes the
283 information about each observed transition: time interval between observations, Δt ,
284 condition level in the initial instant, i , and condition level in the final instant, j . The
285 condition level in the initial instant, i , is used to define the initial marking, M_0 , of the Petri
286 net, the time interval between observations, Δt , is the time horizon of the analysis, and the
287 condition level in the final instant, j , is used to compute the probability of occurrence at
288 the end of the procedure. The first transition to fire is identified by checking which
289 transitions are enabled. When more than one transition is enabled, the transition with less
290 time delay is the first to fire. However, since the Petri net defined for the deterioration
291 model is arranged in sequential manner and there is only one token in the Petri net, i.e.

292 conflicts do not need to be considered. In the next analysis step, the sojourn time in the
293 initial condition level is computed, and the Petri net and time are updated. The process is
294 repeated until Δt is reached. The output of the procedure is the condition index at the time
295 horizon for each sample. Using Monte-Carlo simulation the distribution of the final
296 condition can be computed and the probability of the observed transition occurring can be
297 calculated.

298 **3.4.2 Optimization**

299 The optimization of the parameters of the probability distributions is performed using
300 Genetic Algorithms (GA), which were selected for being widely available, robust and
301 efficient for objective functions computed using Monte-Carlo simulation. In fact, by
302 using only information on the objective function, not requiring the computation of
303 gradients, GA avoid the potential consequences of numerical errors, significantly
304 simplifying the problem (Man et al, 1999; Morcoux and Lounis, 2005).

305 The GA used for optimization of the parameters of the probability distributions is
306 simply depicted in Figure 5. The optimization procedure begins with the definition of
307 optimization variables, objective function, and constraints. The objective function is
308 used to measure the degree of “goodness” of each individual of the population (Man et
309 al, 1999; Morcoux and Lounis, 2005). All parameters of the probability distributions are
310 defined as problem parameters, and the Monte-Carlo procedure described above is used
311 to compute the objective function.

312 In the following step, the initial population is randomly generated. A population is
313 composed by a set of individuals, where each individual is a potential solution of the
314 problem. All individuals of the initial population are evaluated through the objective
315 function, where the best individual is the one with the highest value of the likelihood. At

316 each step of the optimization process, the GA uses the best individual of the current
317 population to create the offspring generation (MatLab, 2016), using the crossover and
318 mutation procedures. The new population generated is then evaluated using the objective
319 function and used as a new parent population. This process is repeated iteratively until a
320 predefined stopping criteria is satisfied.

321 In this study, the optimization of the parameters of the probability distributions was
322 performed using the GA available in MATLAB[®] (MatLab, 2016). The parameters used in
323 the GA are the following:

- 324 • Size of the population: 50 individuals when the number of optimization
325 variables is less than or equal to 5; and 200 individuals otherwise;
- 326 • Stopping criteria: the algorithm stops if the average relative change in the best
327 fitness function value over 50 generations (minimum number of generations) is
328 less than or equal to 10^{-6} ;
- 329 • Mutation procedure was performed using the Gaussian algorithm implemented
330 in MATLAB[®].

331 In the extension of Petri nets proposed by Molly (1982), the stochastic sojourn time is
332 modelled as an exponentially distributed random variable. In this case, a stochastic Petri
333 net is isomorphic to a finite Markov chain.

334 **4. APPLICATION TO RENDERED FAÇADES**

335 The deterioration Petri net model for façades is illustrated in Figure 3. It is composed of five
336 places C_1 to C_5 each representing one of the five discrete states that characterize the
337 degradation condition of external render façades defined in section 2. Level A means there
338 is no visible degradation and Level E indicates the presence of extensive damage in the
339 render façade. The transitions T_1 to T_4 represent the time interval required for the façades to

340 progress to a more deteriorated state.

341 Since Markov chains are widely used to evaluate the condition level over time and taking
342 into account the isomorphism between Markov chains and stochastic Petri nets, the Petri
343 net model proposed can be validated by comparison with the Markov chains model
344 proposed by Silva et al. (2015). In this manner, the efficiency of the numerical procedure
345 and the optimization algorithm described in section 3.4 can be evaluated.

346 The data presented by Silva et al. (2015) is therefore used to calibrate both the Markov
347 chain model and the Petri net models. The database is composed of 99 visual inspection
348 records of external render façades located in Portugal. For each façade, only the initial
349 condition level (assuming that at time zero the render is in Level A) and final condition,
350 corresponding to the inspection date, are known.

351 **4.1 Validation of the Petri net model**

352 In Table 2 the optimal transition rates considering a Markov chain model, implemented
353 using analytical expressions, and a Petri Net model with exponentially distributed sojourn
354 times, are compared. The values of the parameters for each condition level are quite
355 similar as expected. The differences obtained are due to sampling errors associated with
356 the Monte-Carlo simulation used in conjunction with the Petri Net model (Figure 6).

357 Table 3 shows the number of observed and predicted façade in each condition level for
358 both models. The results show that both models are suitable to model the deterioration
359 process of the external façade renders. The biggest relative error is obtained for the Level
360 D (15.3% for Markov chains and 16.5% for Petri nets).

361 Taking into account the results obtained by Petri nets, it is confirmed that the proposed
362 model is suitable to evaluate the degradation of external façade renders.

363 **4.2 Probabilistic analysis**

364 **4.2.1 Two-parameter distributions**

365 When using Petri net models, in addition to the exponential distribution, four distributions
366 were studied: Weibull, Lognormal, Gumbel, and Normal. Table 4 shows the optimal
367 parameters obtained in all probability distribution analysed as the likelihood computed for
368 each set of optimal parameters. All the studied distributions lead to a better likelihood
369 than the exponential distribution.

370 Table 5 shows the number of observed and predicted façades in each condition level for
371 each probability distribution and Table 6 shows the relative error obtained for each case.
372 The values obtained for the relative error are low and, in all cases, acceptable; the largest
373 errors occur for the exponential distribution. Amongst the alternative distributions, the
374 largest error is associated with the Gumbel distribution in Level A (8.6%). The results in
375 those two tables show that the exponential distribution is the one with the largest mean
376 relative error for all states (7.0%), while the smallest mean relative error for all states is
377 for the lognormal distribution (2.1%). The normal distribution presents a mean relative
378 error for all states of 3.3% (second lower value).

379 Figure 7 presents the average predicted condition profile of the external render façade over
380 time for each probability distribution analysed. The profiles obtained for the four
381 distributions are showing some differences to the profile obtained for the exponential
382 distribution.

383 The deterioration curve obtained by exponential distribution does not have inflection
384 points (concave up). The other distribution curves have two inflection points (Figure
385 7a). In the transition between levels B and C there is an inflection point, where the
386 concavity of the curve changes. The second inflection point occurs between levels C

387 and D. In terms of dispersion of the results (Figure 7b), any of the distributions
388 (Weibull, Lognormal, Gumbel, Normal) has lower dispersion values over the simulated
389 period than the exponential distribution. In fact, the exponential distribution has a mean
390 value equal to the standard deviation. There is no physical reason indicating this occurs
391 for the sojourn times. As a result, the use of Markov chains has limited ability to model
392 the variability of performance, frequently overestimating it.

393 These differences between the degradation curves also have high impact on the
394 probabilistic distribution of the degradation condition level over time (Figure 8a-c).
395 Despite the peaks occurring, approximately, in the same years, their values are quite
396 different.

397 For level A, the predicted probabilities for all distributions are similar, beginning with
398 probability equal to 1 and decreasing rapidly over time; at year 10 the probability of a
399 render façade being in level A is near zero (Figure 8a). Also, for level B (Figure 8a), the
400 predicted probabilities for all distributions are similar; the maximum probability of a
401 façade belonging to level B occurs between years 3 and 4; after that, the value of the
402 probability decreases rapidly. In level C significant differences can be observed between
403 models (Figure 8b). The maximum probability of belonging to level C is close to 0.50 for
404 the exponential distribution while for the other distributions it varies between 0.70 and
405 0.80. After the maximum probability is achieved, the slope of the exponential distribution
406 is softer, when compared with the other distributions. For level D (Figure 8b), the
407 exponential distribution has a smoother growth when compared to other distributions,
408 then the peak occurs in all distributions between years 18 and 19 (the maximum
409 probability of belonging to level D is close to 0.40 for the exponential distribution while
410 for the other distributions it varies between 0.70 and 0.80). After that, the slope of the
411 exponential distribution is softer. Finally, as expected, the probability of belonging to

412 level E (Figure 8c) increases over time; however, the increase for the exponential
413 distribution is softer than for the other distributions. At year 40, for the other distributions,
414 the probability of a façade belonging to level E is bigger than 0.95 while, for the
415 exponential distribution, it is closer of 0.80.

416 In the analysis of the service life and durability of rendered façades, it is assumed that level
417 D corresponds to the end of the service life of rendered façades, beyond which a
418 maintenance action must be performed. Figure 8b shows the probabilistic distribution of the
419 degradation condition D over time. The results reveal that the exponential models and,
420 consequently, the Markov chain models, are less accurate in predicting the behaviour of
421 deteriorated serious conditions, due to the reduced number of samples available. According
422 to the Markov chain model proposed by Silva et al. (2015), the probability of a rendering
423 belonging to level D reaches a peak at 15 years. In this study, using a Petri net model, this
424 peak is between 18 and 19 years. These values seem coherent with physical reality, in
425 agreement with the results present in the literature: i) Shohet et al. (1999) obtained an
426 expected service life for cementitious renders of 20 years; ii) Shohet and Paciuk (2004)
427 estimated a predicted service life of 15 years for a stricter level of demand (with a range of
428 results between 12 and 19 years), and a service life of 23 years (with a range of results
429 between 19 and 27 years) for a lower level of demand; iii) Mayer and Bourke (2005)
430 obtained an estimated service life of 18 years for current renderings; iv) Gaspar and de Brito
431 (2008) estimated a service life of cement-rendered façades of 22 years; v) Silva et al.
432 (2013), using an artificial neural network model, obtained an estimated service life of 22
433 years with a 16-28 years range, and using a multiple linear regression model, obtained an
434 average estimated service life of 15 years with a range between 12 and 17 years; vi) a
435 comparative analysis of service life prediction methods applied to rendered façades (Silva et
436 al. 2016), led to an average value of the estimated service life of rendered façades ranging

437 between 16 and 22 years.

438 **4.2.2 Three-parameter distributions**

439 The results of the previous section show that the probability distributions with two
440 parameters show a better fit to the historical data when compared with the exponential
441 distribution. In an attempt to examine whether a probability distribution with three
442 parameters is an option to better model the degradation of façades over time, the Weibull 3-
443 parameters distribution was used. The probability density function of this distribution is
444 given by:

$$445 \quad f(t|\alpha, \beta, \gamma) = \frac{\beta}{\alpha} \left(\frac{t-\gamma}{\alpha}\right)^{\beta-1} e^{-\left(\frac{t-\gamma}{\alpha}\right)^\beta} \quad (5)$$

446 where α, β has the same definition given in Table 4 and $\gamma \in \mathbb{R}$ is the location parameter
447 of the distribution.

448 The optimal parameters obtained for Weibull 3-parameters distribution and the likelihood
449 obtained for this set of parameters are shown in Table 7. Table 8 shows the number of
450 observed and predicted façades in each condition level.

451 From the analysis of the results obtained in the two-parameters distribution (Tables 4
452 and 5) and the three-parameters distribution (Tables 7 and 8), it is found that the
453 Weibull 3-parameters shows a better fit than the two-parameters distribution, both in
454 terms of likelihood and mean relative error. However, the Weibull 3-parameters
455 distribution increases the level of complexity of the analysis (the number of parameters
456 to be optimized increase from 8 to 12).

457 5. CONCLUSIONS

458 In this paper, a model to assess and predict the life-cycle performance of building
459 façades based on stochastic Petri nets is proposed. The application of Petri nets to
460 degradation models is a recent research field, but this modelling technique has shown
461 several advantages relative to the more traditional Markov chains. The graphical
462 representation can be used to describe the problem in an intuitive way; PN are more
463 flexible than the Markov chains, allowing the incorporation of a multitude of rules in
464 the model to accurately simulate complex situations and keeping the model size within
465 manageable limits. Moreover, with this modelling technique, transition times are not
466 required to be exponential distributed.

467 The sojourn time is defined as a random variable for each condition level. The
468 deterioration rates were estimated from available historical data, based on the analysis of
469 the degradation condition of 99 rendered façades, located in Portugal. The Petri net model
470 with transition times exponentially distributed was used to validate the methodology
471 proposed by comparison with a benchmark model based on Markov chains. In order to
472 investigate whether other probability distributions would fit the historical data better than
473 the exponential distribution, five probability distributions were analysed using Petri net
474 models (Weibull 2-parameters, Weibull 3-parameters, Lognormal, Gumbel, and Normal).

475 From the results of the probabilistic analysis performed with Petri nets model, it was
476 found that the use of distributions with two parameters greatly improves the model's
477 goodness of fit. The likelihood values of the four distributions (Weibull 2-parameters,
478 Lognormal, Gumbel, and Normal) are quite similar and all significantly better than the
479 exponential distribution. Some improvement is obtained when a Weibull 3-parameters
480 distribution is considered, but this is obtained at the expense of a significant increase in

481 the complexity of the model.

482 In this study, the degradation condition of rendered façades is described by five condition
483 states, ranging between A (most favourable, without visible degradation) and E (most
484 serious, with generalised degradation). For the analysis of the service life and durability of
485 rendered façades, it is assumed that level D corresponds to the end of the service life of
486 rendered façades, beyond which a maintenance action must be performed. Based on the
487 Petri net model proposed, a rendered façade presents the higher probability of reaching the
488 end of its service (corresponding to level D) between 18 and 19 years. The results obtained
489 are consistent with physical reality and in agreement with the results present in the
490 literature. This study evaluates the loss of performance of rendered façades over time,
491 modelling the probability of transition between degradation conditions through Petri net
492 models. This study demonstrates the validity of this approach to model the degradation of
493 external claddings and, therefore, in future studies, the authors intend to apply a similar
494 methodology to predict the service life of other cladding systems, encompassing the effects
495 of their characteristics in their degradation process (e.g. environmental exposure
496 conditions).

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FIGURE CAPTIONS

673 Figure 1 - Illustrative example of the degradation conditions of rendered façades (photographs by Gaspar,
674 2009)

675 Figure 2 - Example of a Petri net including three places, and one transition

676 Figure 3 - An example of the Petri net scheme of the deterioration model

677 Figure 4 - Procedure for computing the probability of occurrence of the observed transition

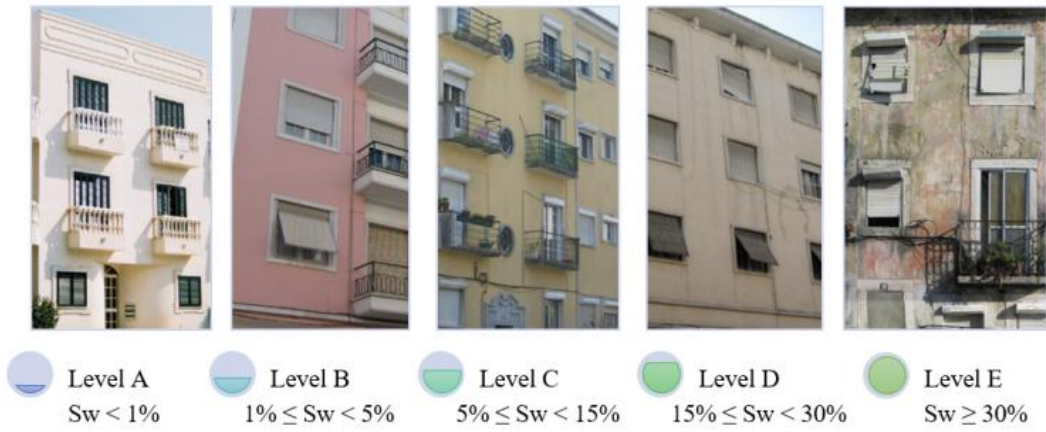
678 Figure 5 - Procedure for optimization of the parameters of the probability distributions (adapted from
679 Morcous and Lounis, 2005)

680 Figure 6 - Comparison of the predicted future condition profile over time for both deterioration models:
681 (a) average condition; (b) standard deviation of condition

682 Figure 7 - Comparison of the predicted future condition profile over time for all probability distribution
683 analysed: (a) mean condition; (b) standard deviation of condition

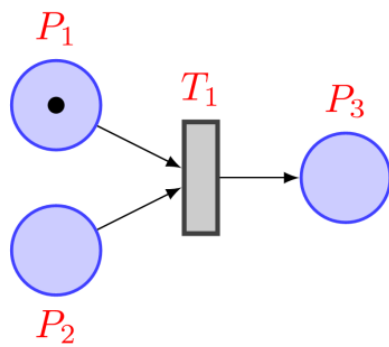
684 Figure 8 - Probabilistic distribution of all degradation condition levels over time: (a) Level A (black) and
685 B (grey); (b) Level C (black) and D (grey); (c) Level E

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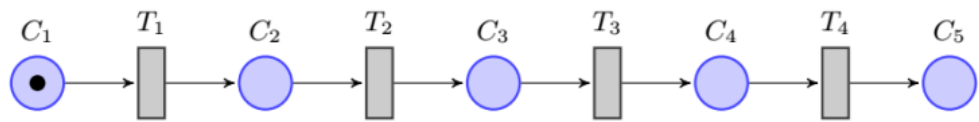
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Figure 1



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Figure 2



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Figure 3

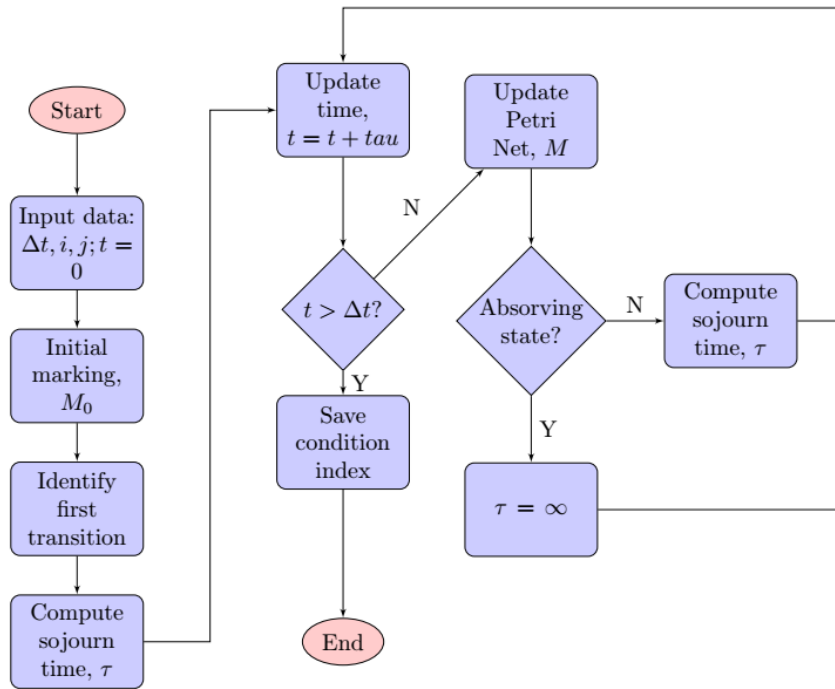


Figure 4

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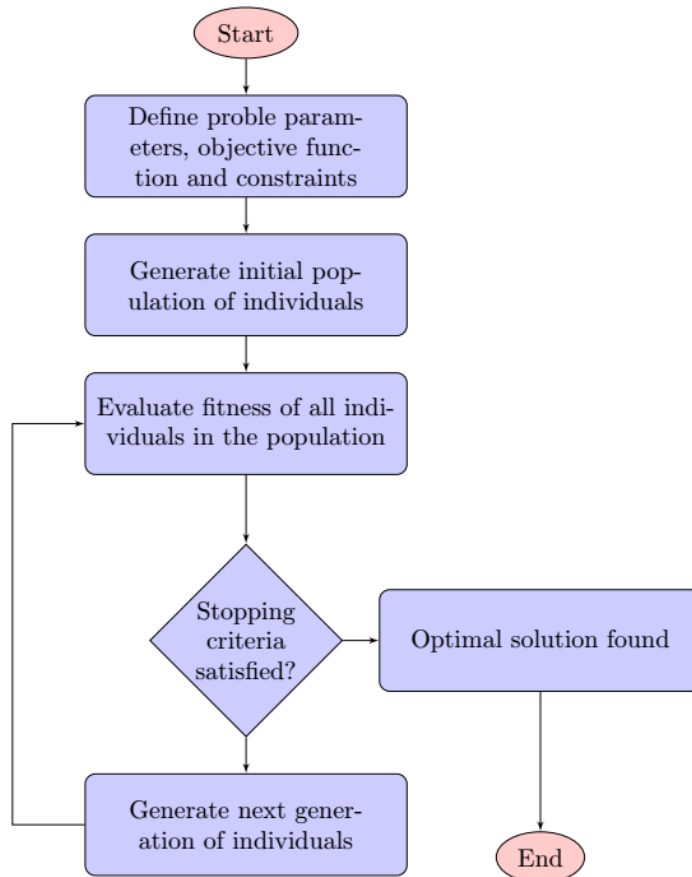
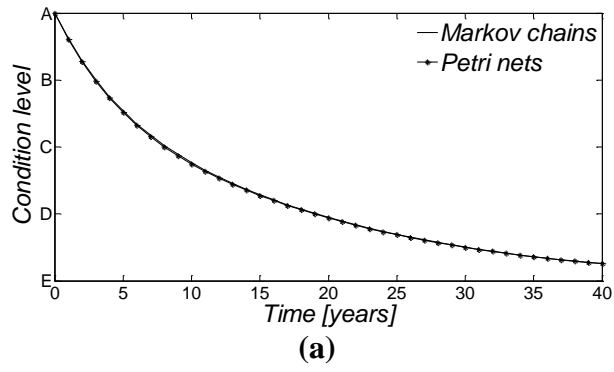


Figure 5

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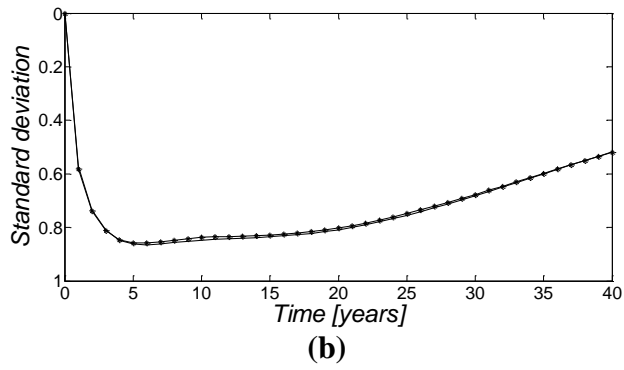
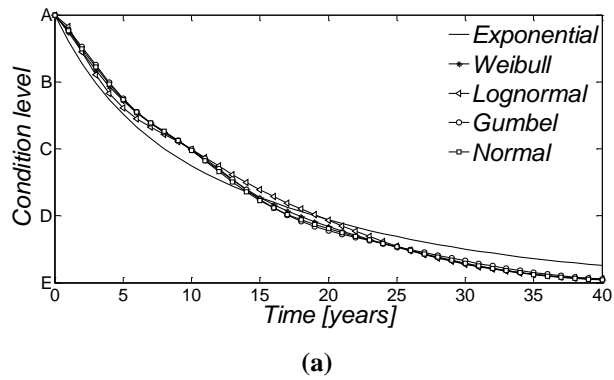


Figure 6

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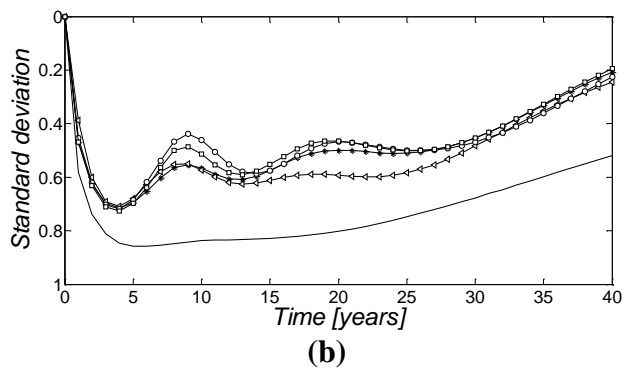
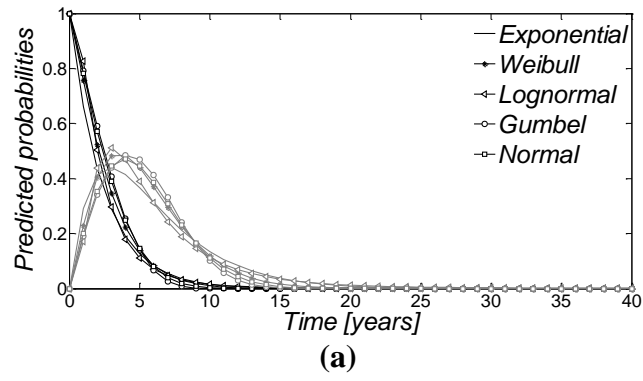
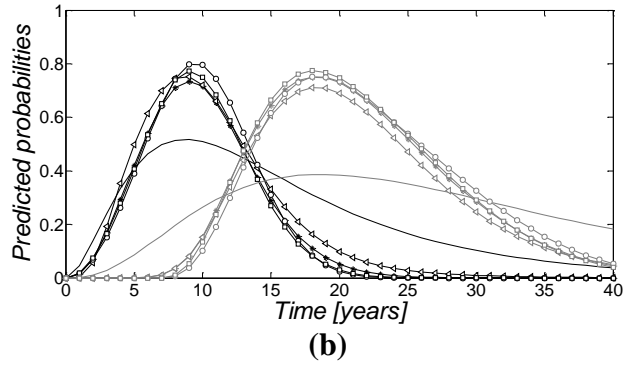


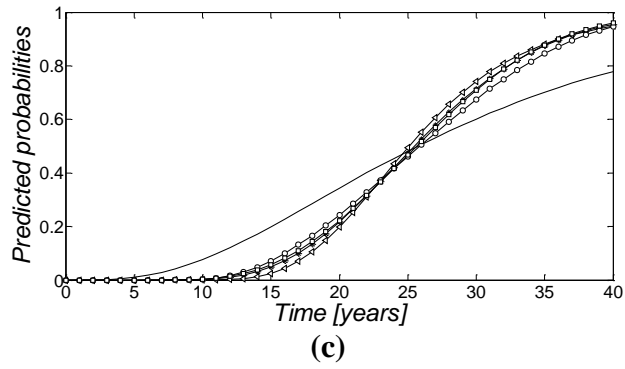
Figure 7



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Figure 8

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TABLE CAPTIONS

- 719 Table 1 Description of the degradation conditions of rendered façades
- 720 Table 2 - Comparison of the optimal parameters of the Markov chains and Petri net models
- 721 Table 3 - Number of observed and predicted coating on each degradation condition for both models
- 722 Table 4 - Optimal parameters obtained in all probability distribution analysed
- 723 Table 5 - Number of observed and predicted coating in each condition level for each probability distribution
- 724 Table 6 - Mean error [%] obtained for each probability distribution
- 725 Table 7 - Optimal parameters obtained for Weibull 3-parameters distribution
- 726 Table 8 - Number of observed and predicted coating for Weibull 3-parameters distribution
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Table 1

Condition level	Description
Condition A	Most favourable condition. Complete mortar surface with no visible degradation, with uniform colour, showing no dirt or detachment
Condition B	Mortar with a non-uniform surface with likelihood of localized voids determined by percussion, but no signs of detachment. Small cracking (0.25 mm to 1.0 mm) in localized areas and changes in the general colour of the surface might exist. Eventual presence of microorganisms.
Condition C	Mortar with localized detachments or perforations, revealing a hollow sound when tapped and detachments only in the socle, with easily visible cracking (1.0 mm to 2.0 mm) and showing dark patches of damp and dirt, often with microorganisms and algae.
Condition D	Mortar with an incomplete surface due to detachments and falling of mortar patches, showing wide or extensive cracking (≥ 2 mm) and very dark patches with probable presence of algae.
Condition E	Most serious condition, requiring an immediate corrective action, associated with incomplete mortar surface due to detachments and falling of mortar patches. Also revealing a wide or extensive cracking (≥ 2 mm), with very dark patches and probable presence of algae.

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Table 2

Model	Optimal parameters				Likelihood
	α_1	α_2	α_3	α_4	
Markov chains ¹	0.4016	0.2819	0.0994	0.0761	82.4245
Petri net (Exponential)	0.4201	0.2743	0.0966	0.0804	82.2582

¹ Data adapted from Silva et al. (2015)

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Table3

Degradation condition	Observed	Predicted		Error [%]	
		Markov chains ¹	Petri net	Markov chains	Petri net
Level A	13	12.57	12.17	3.3	6.4
Level B	18	17.77	17.63	1.3	2.1
Level C	31	28.64	28.96	7.6	6.6
Level D	15	17.29	17.47	15.3	16.5
Level E	22	22.74	22.77	3.3	3.5

¹ Data sourced from Silva et al. (2015)

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Table 4

Parameters	Exponential	Weibull	Lognormal	Gumbel	Normal	
Parameter 1	α_1	0.4201	2.8616	0.7001	0.6112	0.4811
	α_2	0.2743	3.8199	0.8702	1.5270	1.3919
	α_3	0.0966	7.9483	2.0754	7.8258	7.2940
	α_4	0.0804	14.1976	2.3615	11.4260	11.6725
Parameter 2	β_1	-	1.2149	0.7435	4.2326	3.2519
	β_2	-	1.4040	0.8572	4.4394	3.4303
	β_3	-	6.0816	0.3077	0.4219	0.1330
	β_4	-	2.0100	0.4612	11.7718	7.6393
Likelihood	82.2582	70.4602	70.2610	70.4666	70.1237	

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Table 5

		Level A	Level B	Level C	Level D	Level E
Observed		13	18	31	15	22
Predicted	Exponential	12.17	17.63	28.96	17.47	22.77
	Weibull	13.58	18.62	29.30	15.49	22.01
	Lognormal	13.09	17.58	32.07	14.67	21.60
	Gumbel	14.12	18.34	29.95	14.53	22.06
	Normal	14.11	18.06	29.38	15.27	22.18

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Table 6

	Level A	Level B	Level C	Level D	Level E
Exponential	6.4	2.1	6.6	16.5	3.5
Weibull	4.4	3.5	5.5	3.3	0.0
Lognormal	0.7	2.3	3.4	2.2	1.8
Gumbel	8.6	1.9	3.4	3.2	0.3
Normal	8.5	0.4	5.2	1.8	0.8

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Table 7

Parameters	$i = 1$	$i = 2$	$i = 3$	$i = 4$
α_i	1.3998	2.5269	4.5874	1.4221
β_i	0.7026	0.8977	1.8966	0.4718
γ_i	0.8803	0.7551	4.1532	7.7902
Likelihood	69.0345			

742

743

Table 8

	Level A	Level B	Level C	Level D	Level E
Observed	13	18	31	15	22
Weibull 3-parameters	12.96	17.68	32.04	14.31	22.01
Mean error [%]	0.3	1.8	3.3	4.6	0.0

744