

A FUTURES APPROACH TO WATER DISTRIBUTION AND SEWER NETWORK (RE)DESIGN



Submitted by

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ABSTRACT

When designing urban water systems (i.e. water distribution and sewer systems) it is imperative that uncertainty is taken into consideration. However, this is a challenging problem due to the inherent uncertainty associated with both system loading requirements and the potential for physical components failure. It is therefore desirable to improve the *reliability* of each system in order to account for these uncertainties.

Although it is possible to directly evaluate the reliability of a water distribution systems (WDS) (using *reliability measures*), the calculation processes involved are computationally intensive and therefore unsuitable for some state-of-the-art, iterative design approaches (such as optimisation). Consequently, interest has recently grown in the use of *reliability indicators*, which are simpler and faster to evaluate than conventional direct reliability methods.

In this thesis, a novel *measure* (the R_{UF}) is developed to quantify reliability in urban water systems with a view to enhance their robustness under a range of future scenarios (Policy Reform, Market Forces, Fortress World and New-Sustainability Paradigm). The considered four future scenarios were synthesized in the EPSRC supported multidisciplinary 4 year project: Urban Futures. Each investigated urban future scenario is characterised by a distinct household water demand and local demand distribution (emerging due to different urban forms evolving in future scenarios). In order to assess the impact of urban futures, R_{UF} has been incorporated into Urban Water System (UWS) dynamic simulations for both WDSs and Foul Sewer Systems (FSSs) using open source codes of EPANET and SWMM.

Additionally, in order to overcome extensive computational effort, resulting from the use of traditional *reliability measures*, a new holistic *reliability indicator*, the hydraulic power entropy (I_{HPE}) has been developed and compared to existing reliability indicators. Additionally, the relationship between the new reliability indicator and the above mentioned R_{UF} *reliability measure* is investigated. Results suggest that the magnitude of the I_{HPE} in network solutions provides a holistic indication of the hydraulic performance and reliability for a WDS. However, the performance of optimal solutions under some Urban Futures indicates that additional *design interventions* are required in order to achieve desired future operation.

This thesis also proposes a new holistic foul sewer system (FSS) reliability indicator (the I_{FSR}). The I_{FSR} represents sewer performance as a function of excess pipe capacity (in terms of available *increase* and also *decrease* in inflow). The indicator has been tested for two case studies (i.e. different sewer network layouts). Results suggest that the magnitude of I_{FSR} has positive correlations with a number of identified key performance indicators (i.e. relating to capacity, velocity, blockages).

Finally, an Integrated Design Approach (IDA) has been developed in order to assess the implications of applying *design interventions* on both a WDS *and* downstream FSS. The approach holistically considers present and future operation of each interconnected system. The approach was subsequently demonstrated using two proposed *design interventions*. Results suggest that, for the considered *design interventions*, there is trade-off between the simultaneous improvement of both WDS and FSS operation and reliability.

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LIST OF ABBREVIATIONS

| | |
|-----------------|--|
| B-P | Butler-Pinkerton (chart) |
| D-S | diameter-slope |
| DS | downstream |
| DWF | dry weather flow |
| FSS | foul sewer system |
| FSR | foul sewer reliability |
| FW | fortress world (Urban Future) |
| HC | hydraulic constraint |
| IDA | integrated design approach |
| HPE | hydraulic power entropy |
| HWI | household water interaction |
| KPI | key performance indicator |
| MF | market forces (Urban Future) |
| NPV | net present value |
| NSGAI | non dominated sorting genetic algorithm II |
| NSP | new sustainability paradigm (Urban Future) |
| PR | policy reform (Urban Future) |
| R _{UF} | reliability measure |
| SATT | sewerage available to transport |
| UF | Urban Future |
| US | upstream |
| UWS | urban water system |
| WDS | water distribution system |

LIST OF NOTATIONS

| Symbol | Description | Units |
|---------------------------|---|-------------------|
| b_i | trench width | m |
| C_o | average per capita consumption | m ³ /s |
| c_{mh} | individual manhole cost | £ |
| c_e | cost per unit of electricity | £/kWh |
| C_E | unit excavation cost | £/m ³ |
| C_F | estimated future average per capita consumption | m ³ /s |
| C_H | maximum global surplus FSS capacity | - |
| C_L | minimum global surplus FSS capacity | - |
| C_o | existing average per capita consumption | m ³ /s |
| C_{Hi} | proportional foul sewer pipe highest surplus capacity | - |
| C_{Li} | proportional foul sewer pipe lowest surplus capacity | - |
| $C_{FSS} (\dot{C}_{FSS})$ | (normalised) capital cost for WDS (re)design solution | £ |
| $C_{HWI} (\dot{C}_{HWI})$ | (normalised) capital cost to change HWI | £ |
| $C_{WDS} (\dot{C}_{WDS})$ | (normalised) capital cost for WDS (re)design solution | £ |
| $C_p(D)$ | pipe cost as a function of diameter | £ |
| $C_b(D)$ | trench width as a function of pipe diameter | m |
| C_{MH} | capital cost to construct manholes | £ |
| C_{PIPE} | capital cost to construct piping infrastructure | £ |

| | | |
|-------------|---|-------------------|
| C_{PMP} | pumping installation capital cost | £ |
| C_{TANK} | construction cost of WDS tanks | £ |
| C_{TOTAL} | (re)design solution capital cost | £ |
| C_{OP} | pumping NPV operational cost over system lifetime | £ |
| C_{OP}^* | annual operational cost of pumping | £ |
| d | foul sewer pipe depth of flow | m |
| d^* | randomly generated decimal variable | - |
| d_o | individual household water consumption | m ³ /s |
| d_{WDS} | individual household WDS water demand | m ³ /s |
| d_{FSS} | individual household FSS water demand | m ³ /s |
| D_{des} | WDS or FSS design demand | m ³ /s |
| D_E | future demand from existing population | m ³ /s |
| D_f | sewer demand multiplier for Urban Future f | - |
| D_F | estimated future global demand | m ³ /s |
| D_o | existing global demand | m ³ /s |
| D_N | future demand from additional (new) population | m ³ /s |
| $D_{N,i}$ | new future demand assigned to node i | m ³ /s |
| $D_{E,i}$ | existing future demand assigned to node i | m ³ /s |
| $D_{o,i}$ | demand originally assigned to node i | m ³ /s |
| $D_{T,i}$ | total future demand assigned to node i | m ³ /s |
| D_{FSS} | global FSS demand (inflow) | m ³ /s |

| | | |
|-------------|---|---------|
| D_{WDS} | global WDS demand | m^3/s |
| $D, [D_i]$ | diameter [of pipe i] | m |
| Dwf_i | average dry weather flow in foul sewer pipe i | m^3/s |
| E_p | total pump energy used over 24h | kWh |
| F_F | future fulfilment function | - |
| F_E | future exceeded function | - |
| G_{max} | total number of simulations performed | - |
| h | demand <i>increase</i> component of future <i>exceeded</i> function | - |
| $h_{ava,i}$ | available pressure head at node i | m |
| $h_{req,i}$ | required pressure head at node i | m |
| $H, (H_i)$ | pressure head (at node i) | m |
| H | demand <i>increase</i> component of future <i>fulfilment</i> function | - |
| h_i | depth of manhole i | m |
| I_{HPE} | Hydraulic power entropy indicator for WDS | - |
| I_{FSR} | Foul sewer reliability indicator | - |
| I_{RI} | Todini's resilience index for WDS | - |
| I_{NR} | Raad's network resilience for WDS | - |
| I_{CRE} | combined resilience-entropy index for WDS | - |
| I_{ENT} | informational entropy for WDS | - |
| IN | set of input nodes for WDS | - |
| l | demand <i>reduction</i> component of future <i>exceeded</i> function | - |

| | | |
|-----------|--|-----|
| L | demand <i>reduction</i> component of future <i>fulfilment</i> function | - |
| L_i | length of pipe i | m |
| n | foul sewer pipe Manning roughness coefficient | - |
| n^* | time of cash flow for calculating net present value | yrs |
| n_E | number of “edge” nodes | - |
| n_h | number of households served by WDS or FSS | - |
| N_{PC} | number of nodes selected for polycentric growth | - |
| N_{ED} | number of nodes selected for edge growth | - |
| N_C | number of conduits in FSS | - |
| N_n | number of nodes in WDS or FSS | - |
| N_p | total number of pipes in WDS or FSS | - |
| N_R | number of WDS supply nodes (reservoirs/tanks) | - |
| $N_{P,i}$ | number of pipes entering node i | - |
| N_{PMP} | number of pumps | - |
| N_S | number of simulation time steps | - |
| P_o | population served by WDS or FSS | - |
| P_F | estimated future population | - |
| P_N | additional (new) future population | - |
| p_{ij} | hydraulic power entering WDS node i from pipe j | kW |
| P_k | rated pump power | kW |
| P_T | total WDS hydraulic power supplied by all input sources | kW |

| | | |
|-------------|---|-------------------|
| $P_{in,i}$ | WDS hydraulic power from input source i | kW |
| $P_{ava,i}$ | available hydraulic power at demand node i | kW |
| $P_{req,i}$ | required hydraulic power at demand node i | kW |
| q_i | demand at WDS node i | m ³ /s |
| q_{ij} | flow into WDS node i from pipe j | m ³ /s |
| $Q, [Q_i]$ | WDS or FSS pipe or node flow [in pipe i] | m ³ /s |
| $Q_{min,i}$ | minimum feasible flow in foul sewer pipe i | m ³ /s |
| $Q_{max,i}$ | maximum feasible flow in foul sewer pipe i | m ³ /s |
| $Q_{L,i}$ | lowest constrained flow in foul sewer pipe i | m ³ /s |
| $Q_{H,i}$ | highest constrained flow in foul sewer pipe i | m ³ /s |
| r | discount rate for calculating net present value | % |
| R^2 | coefficient of determination | - |
| R_{MF} | reliability measure (Market Forces) | - |
| R_{FW} | reliability measure (Fortress World) | - |
| R_{NSP} | reliability measure (New Sustainability Paradigm) | - |
| R_{PR} | reliability measure (Policy Reform) | - |
| S_i | slope of foul sewer pipe i | - |
| S | set of nodal demands describing all network in/outflows | m ³ /s |
| S_s | number of successful (feasible) simulations | - |
| T | total flow entering WDS network from all sources | m ³ /s |
| T_i | total flow passing through WDS node i | m ³ /s |

| | | |
|----------------|---|---------|
| V | foul sewer pipe flow velocity | m/s |
| $V_{e,i}$ | excavation volume for construction of pipe i | m^3 |
| V_{min} | foul sewer minimum (sedimentation) velocity | m/s |
| W_i | demand assignment weighting of node i | - |
| W_T | cumulative demand assignment weighting of all nodes | - |
| X_i | stochastic normal future demand for node i | m^3/s |
| X, \hat{X} | logarithmic proportion in the form $X \ln X$ | - |
| α | WDS demand coefficient | - |
| $\dot{\alpha}$ | population weighted WDS demand coefficient | - |
| β | FSS demand coefficient | - |
| $\dot{\beta}$ | population weighted FSS demand coefficient | - |
| δ | design demand factor | - |
| γ | proportion of households with altered HWI | - |
| γ^* | specific weight of water | N/m^3 |
| γ_P | proportional change in population | - |
| γ_C | proportional change in average per capita consumption | - |
| γ_D | proportional change in global demand | - |
| η | pump efficiency | - |
| φ | average household occupancy | - |