STATIC STRENGTH OF TUBULAR DT JOINTS USING LUSAS FINITE ELEMENT SOFTWARE

NORASHDAH BINTI ABD BAHHAN

UNIVERSITI TEKNOLOGI MALAYSIA

CH/ 69581



-

PSZ 19:16 (Pind. 1/97)

- 17 - 70	UNIVERS	ITI TEKNOL	OGI MALA	AYSIA	
BORA	BORANG PENGESAHAN STATUS TESIS				
JUDUL:	STATIC STRENG			USING LUSAS FINITE	
		<u>ELEMENT</u>	<u>SOFTWARE</u>		
	SESI I	PENGAJIAN: <u>2</u>	04/2005	-	
Saya	NO	ORASHIDAH BIN (HURUF BES		<u>MAN</u>	
	embenarkan tesis (P knologi Malaysia den			disimpan di Perpustakaan berikut:	
 Perpusta pengajia Perpusta institusi 	 Perpustakaan Universiti Teknologi Malaysia dibenarkan membuat salinan untuk tujuan pengajian sahaja. Perpustakaan dibenarkan membuat salinan tesis ini sebagai bahan pertukaran antara institusi pengajian tinggi. 				
	SULIT		aysia seperti ya	berdarjah keselamatan atau ng termaktub di dalam	
	TERHAD	(Mengandungi oleh organisasi/ł	naklumat TER badan di mana p	HAD yang telah ditentukan enyelidikan dijalankan)	
V	TIDAK TERHAD)	Di	isahkan oleh	
-	HP.		AM	<u>s</u> i/	
(TAN	DATANGAN PENU	ILIS)	(TANDATA	ANGAN PENYELIA)	
<u>PETAI, 217</u>	o: <u>Ah Kos Rendai</u> <u>00 Kuala Bera</u> Anu. Terenggan	NG, HULU SAA	D	R.SARIFFUDDIN BIN	
Tarikh:	15 APRIL 20	005 Taril	ch:	15 APRIL 2005	

CATATAN: *

- Potong yang tidak berkenaan. Jika tesis ini SULIT atau TERHAD, sila lampirkan surat daripada pihak berkuasa/organisasi berkenaan dengan menyatakan sekali sebab dan tempoh tesis ini perlu **
- dikelaskan sebagai SULIT atau TERHAD. Tesis dimaksudkan sebagai tesis bagi Ijazah Doktor Falsafah dan Sarjana secara penyelidikan, atau disertasi bagi pengajian secara kerja kursus dan penyelidikan, atau Laporan Projek Sarjana Muda (PSM). ٠

STATIC STRENGTH OF TUBULAR DT JOINTS USING LUSAS FINITE ELEMENT SOFTWARE

NORASHIDAH BINTI ABD RAHMAN

A project report submitted in partial fulfillment of requirement for the award of the degree of Master of Engineering (Civil – Structure)

Fakulti Kejuruteraan Awam Universiti Teknologi Malaysia

APRIL 2005

"I declare that this project report entitled " Static Strength of Tubular DT Joints Using LUSAS Finite Element Software" is the result of my own research except as cited in the references. The report has not been accepted for any degree and is not concurrently submitted in candidature of any other degree"

Singnature	: Jof.
Name	NORASHIDAH BINTI ABD RAHMAI
Date	: <u>15 APRIL 2005</u>
	15 APRIL 2005

•.

"I declare that I have read through this project report and to my opinion this project report is adequate in term of scope and quality for the purpose of awarding the degree of Master of Engineering (Civil – Structure)".

> Signature : Name of Supervisor : Date :

ASSOC. PROF. DR. SARIFFUDDIN BIN SAAD 15 APRIL 2005

DEDICATION

To father and mother Thank you for your support L To sisters and brothers thank you for eveything

ACKNOWLEDGEMENT

Alhmadulilah, Praise to Almighty Allah for the blessing and His permission, I am able to complete my master project.

I wish to extent my greatest thank you and gratefulness to my supervisor, Assoc. Prof. Dr. Sariffuddin Bin Saad for his valuable guidance, advice and suggestions throughout this project. With his effort and concern, I am able to complete my project. Thank you also to Mr. Koh Heng Boon of the Department of Civil Engineering at KUITTHO, for his help and advise.

I am also grateful to my most beloved parents for their love and kindness towards me and for their strong support during my study period.

I wish to thank KUITTHO and the Public Service Department for the financial support during my stay at UTM.

Finally, a lot of thank you to all staff of Faculty of Civil Engineering, University Teknologi Malaysia, Skudai, Johor and also for all my friends, student of postgraduate of Structural and Material Department for their support and cooperation throughtout my study.

Thank you very much.

ABSTRACT

Structural tubular are widely used in the construction of offshore structures. As these structures are located in hostile environment, these joints represent structural weak spots and so it is desirable to develop reliable methods of determining their static collapse loads. This studies focus on the analysis of static strength of tubular DT joints under brace compression loading by using LUSAS finite element software. The numerical static strength result is compared with an experimental test result obtained from the literature. The value of the static strength obtained in this work is 56% lower than that of the experimental test. A parameter study was performed to study the effect of the geometric parameters α , β , γ and τ as well as the effect of the yield strength σ_y on the static strength of DT joint model. Finally, a simple equation relating the static strength to the above parameters is proposed.

ABSTRAK

Struktur sambungan tubular lazimnya digunakan untuk pembinaan struktur lepas pantai. Oleh kerana struktur ini terletak di persekitaran yang agresif, ia akan menyebabkan sambungan tubular struktur tersebut menjadi lemah. Oleh itu, satu kaedah yang baik adalah perlu untuk menentukan beban kegagalan statik bagi sambungan tersebut. Oleh itu kajian ini tertumpu kepada analisis kekuatan statik bagi sambungan DT bila brace dikenakan beban mampatan dengan menggunakan perisian LUSAS. Nilai kekuatan statik ini kemudiannya telah dibandingkan dengan keputusan ujian makmal yang diperolehi daripada literatur. Dalam kajian ini, nilai kekuatan statik yang diperolehi adalah 56% lebih rendah daripada nilai ujian makmal. Kajian parameter telah dijalankan untuk mengkaji kesan parameter geometri α , β , γ dan τ serta juga kesan kekuatan alah σ_y kepada kekuatan statik sambungan DT dengan parameter-parameter di atas telah dicadangkan.

CONTENTS

CHAPTER			PAGE
	TITT	LE	i
	DEC	LARATION	ii
	DED	ICATION	iii
	ACK	NOWLEDGEMENT	iv
	ABS	ГКАСТ	v
	ABST	ГКАК	vi
	CON	TENTS	vii
	LIST	OF TABLE	х
	LIST	OF FIGURE	xi
	TATT	ODUCTION	1
1	1.1	RODUCTION Introduction to the tubular structure	1
	1.1	Problem of study	2
	1.2	Objective of study	2
	1.5	Scope of study	4
2	LITH	ERATURE REVIEW	5
	2.1	Introduction	5
	2.2	Tubular Structure	5
	2.3	Tubular Joint	6
	2.4	Development of Static Strength Design Guidance	7
	2.5	Previous Study of DT Tubular Joint	8
	2.6	Finite Element Method (FEM)	12
		2.6.1 Advantages and Disadvantages of FEM	14
	2.7	BackGround of LUSAS	15

		2.7.1	LUSAS Software Characteristic	15
		2.7.2	Procedure Analysis According to LUSAS	16
			software	
3	MO	DELLIN	G DT TUBULAR JOINT USING THE	17
	LUS	AS SOF	TWARE	
	3.1	Introduc	otion	1 7
	3.2	Dimensi	ions and Geometric Parameter	17
	3.3	Modelli	ng Step for One eighth Model	19
		3.3.1	New file	19
		3.3.2	To Generate One eighth of The DT Joint	20
			Model	
	3.4	Nonline	ar Model	35
		3.4.1	Meshing	35
		3.4.2	Geometric Definition	37
		3.4.3	Material Definition	38
		3.4.4	Loading Definition	39
		3.4.5	Support Definition	41
		3.4.6	Nonlinear Control	43
		AT VOIC	RESULTS AND DISCUSSION	46
4	4.1	Introdu		49
	4.1		hth Model Vs. Full Model	52
	4.2		supportt	67
	4.5		nonlinearity	68
	4.5	~ 1	Convergence studies	69
	4.5		tric study	55
	4.0		elationship between failure load F and α	55
				56
			Relationship between failure load F and β	57
			Relationship between failure load F and γ	
			Relationship between failure load F and τ)	58
		4.6.5	Relationship between failure load F and σ_y	59

viii

CONCLUSION	63
5.1 Conclusion	63
5.2 Suggestion	64
REFERENCES	65

APPENDIX A (The loading calculation)

5

LIST OF TABLES

TABLE NO.	TITLE	
3.1	Dimensions (in mm) of the DT joint	18
3.2	Support condition for one eight model	42
4.1	Summary of Maximum Load with a Difference Type of Nonlinearities	51
4.2	Result of Mesh Convergence Study	53

LIST OF FIGURES

FIGURE. NO.		TITLE	PAGE
1.1	A typical jacket structure		2

1.1	Tr (jpiour juonot bu uoturo	
1.2	An elastic plastic response of the joint	4
2.1	Various type of tubular joint	6
2.2	A typical DT joint	7
2.3	DT joint test setup	11
2.4	Experimental load-displacement curves for DT joints under brace/chord compression	11
2.5	A finite element model representing a real engineering problem	12
2.6	A tubular joint finite element model	13
3.1	Model geometric	18
3.2	One eighth of the DT joint model	19
3.3	New Model start up	20
3.4	The dialogue box "Enter coordinates"	21
3.5	L1 Line	21
3.6	The sweep (rotate) dialogue box	22
3.7	Surface S1, line L2 and L3	22
3.8	Curve L3	23
3.9	Sweeping (translate) dialogue box	23
3.10	The overall chord view	24

xi

3.11	Dialogue box to divide the chord section	25
3.12	The two new chord surface (S2 and S3)	25
3.13	Line L12	26
3.14	The dialogue box used to rotate line L12	27
3.15	New surface, S4, for the top brace section	27
3.16	The sweep dialogue box	28
3.17	Surface S4 was created	28
3.18	The surface Splitting In Equal Divisions dialogue box	29
3.19	Two new surfaces of the brace member after the splitting process	29
3.20	Model after intersection process	30
3.21	The new group command	31
3.22	The DT joint model after the deletion process	31
3.23	Line L34 was create after manifolding process	32
3.24	Line L4 and L10 must be separated at points P24 and P27 respectively	33
3.25	New surfaces S12 and S13	34
3.26	One-eighth of DT model joint	34
3.27	The dialogue box to define surface meshing	36
3.28	Model with the surface meshing definition	36
3.29	Surface Geometry dialogue box	37
3.30	Isotropic material dialogue box	38
3.31	Elastic plastic dialogue box	39
3.32	The structural loading Dataset dialogue box	40
3.33	Model with the loading at the brace end	40
3.34	Structural support dialogue box	41

3.35	Model with full support for the linear model	42
3.36	Model with the full support for non-linear model	43
3.37	Load Case properties dialogue box	44
3.38	The Nonlinear & Transient dialogue box	44
3.39	The advance nonlinear incrementation parameter dialogue box	45
4.1	One eighth of DT joint Model A	47
4.2	An elastic perfectly plastic material model	48
4.3	Load-displacement graphs for Model A &	48
4.4	Full DT joint Model B	49
4.5	Load-displacement graph for both types of support	50
4.6	Load-displacement graph response for both nonlinearity types	52
4.7	Load-displacement graphs of DT joint with various number of element division	53
4.8	Failure mode of FE model	54
4.9	Failure mode of model tested by Kang (1998)	54
4.10	Relationship between F and α	56
4.11	Relationship between F and β	57
4.12	Relationship between F and γ	58
4.13	Relationship between F and τ	59
4.14	Relationship between F and σ_y	60

NOTATION LIST

D	=	Chord outer diameter
d	=	Brace outer diameter
Т	=	Chord thickness
Т	=	Brace thickness
L	=	Total chord length
1	=	Total brace length
σ_y	=	Yield strength
N	=	Newton
α	-	Length parameter (2L/D)
β	=	Diameter ratio (d/D)
γ	=	Chord radius to thickness ratio (D/2T)
τ	=	Wall thickness ratio (t/T)

CHAPTER I

INTRODUCTION

1.1. Introducing to The Tubular Structure

Tubular members are widely used in both onshore and offshore structures. Their attributes such as the high strength-to-weight ratio, low drag coefficient, and the ability to use their internal space have made them particularly useful in the offshore industry over many years. A typical example of the use of tubular members in an offshore situation is the fixed offshore platform (see Figure 1.1), where tubular members form a space frame to support the topside structure. Tubular connection design is a major factor in the design of a structure and can even be the limiting factor in terms of the strength of the structure.

Circular hollow sections are widely used in the construction of offshore structures in Malaysia. These sections offer many advantages over other sections. The sections have the ability to distribute load consistently. From the architect point of view, it has a minimum amount of surface area to unclean matter effect, rust and other spoil. With a circular form, it has an advantage in reducing the effect of wind, wave and blast loadings.

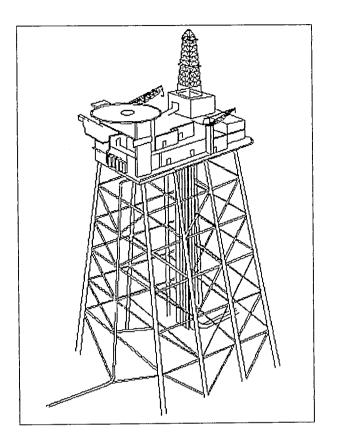


Figure 1.1: A typical jacket structure [1]

1.2. Problem of Study

Structural tubular joints are widely used in the construction of offshore structures. As these structures are located in hostile environment, these joints represent structural weak spots and so it is desirable to develop reliable methods of determining their static collapse loads.

It is impractical to test actual joints due to their massive sizes and also in view of the associated testing costs. Testing small-scale steel joint models of various shapes was widely carried out in the past. However, the manufacture of these joint models needs highly skilled welders and the exact shape of fillet welds is not repeatable.

An attractive alternative is to carry out finite element analysis using a suitable software to obtain the static strength results. If the results are good, this method can be used to performed a parameter study to investigate the effect various geometric parameters on a joint static strength.

1.3. Objectives of Study

The static strength of tubular DT joints will be studied using LUSAS finite element software [2]. Objectives of the study are:

- a) to create a good finite element DT joint model.
- b) to carry out a mesh convergence study.
- c) to define the maximum load attained during the elastic plastic response of the joint.
- to compare the static strength results of tubular DT joint between finite element software and previous experimental test.
- e) to performed parameter study to investigate the effect of the various geometric parameters on the static strength of DT joints.
- f) to develop a simple formula to calculate the static strength of DT joints.

1.4. Scope of Study

The LUSAS finite element software will be used to determine the static strength of tubular DT joint. In this study, the static strength of a tubular DT joint is defined as the maximum load attained during the elastic plastic response of the joint and this is shown in Figure 1.2. Data for the analysis are taken from a previous experimental testing on a similar DT joint performed by Kang et al.[3].

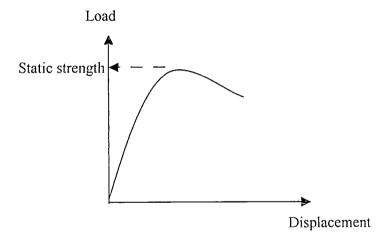


Figure 1.2: An elastic plastic response of the joint

To obtain the static strength by using LUSAS software, the dimensions of the DT joint finite element model had been based on the dimensions and data of the actual joint test performed by Kang et al.[3]. Before the comparison of the static strength result predicted by the LUSAS software and that of the actual test, a mesh refinement was conducted by performing non-linear analysis of the DT joint model under brace compression loading using different element density applied to the chord area near the brace wall.

CHAPTER II

REVIEW OF LITERATURE

2.1. Introduction

Tubular joint constitute one of the main problems and high cost areas in the design, construction and maintenance of steel structures and have been the subject of considerable research effort. The Department of Energy commissioned a study in 1980 of the various design documents, with particular emphasis on the static strength of tubular joints [4].

2.2. Tubular Structure

A tubular structure consists of a framework of hollow pipes made from steel. There are two types of hollow section used, circular and rectangular. However, circular hollow sections are more generally used in offshore structure construction. It is because these sections have a small surface area, can minimise the wind and wave load and also have a high ecstatic value.

2.3. Tubular Joint

A tubular joint is a joint between the brace and the chord. Figure 2.1 shows the various types of joint widely used for offshore construction work.

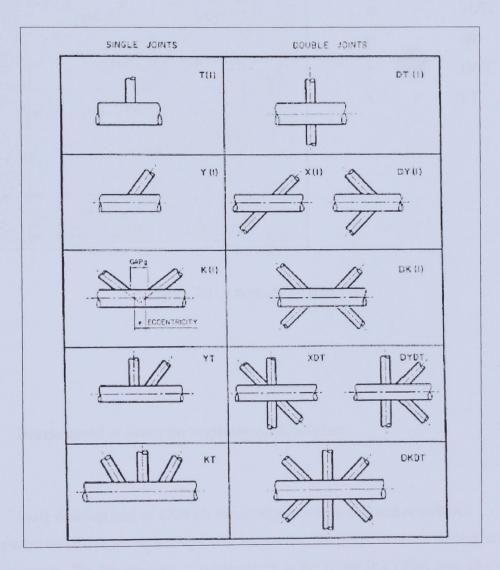


Figure 2.1: Various types of tubular joint [5]