

High strain rate deformation of porous sandstone and the asymmetry of earthquake damage in shallow fault zones

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Abstract

Observations of coseismic pulverization in porous sedimentary rocks in fault damage zones are scarce, in contrast to coseismic pulverization of crystalline rocks. Also, juxtaposition of stiff crystalline rocks and compliant porous rocks across a fault often yields an asymmetric damage zone geometry, with less damage in the more compliant side. In this study, we argue that such asymmetry near the sub-surface occurs because of a different response of lithology to similar transient loading conditions. Uniaxial unconfined high strain rate loadings with a split Hopkinson pressure bar were performed on dry and water saturated Rothbach sandstone core samples. Bedding anisotropy was taken into account by coring the samples parallel and perpendicular to the bedding. The results show that pervasive pulverization below the grain scale, such as observed in crystalline rock, does not occur in the sandstone samples for the explored strain rate range (60-150 s⁻¹). Damage is mainly restricted to the scale of the grains, with intragranular deformation occurring only in weaker regions where compaction bands are formed. The presence of water and the bedding anisotropy mitigate the formation of compaction bands and motivates intergranular dilatation. The competition between inter- and intragranular damage during dynamic loading is explained with the geometric parameters of the rock in combination

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with two classic micromechanical models: the Hertzian contact model and the pore-emanated crack model. In conclusion, the observed microstructures can form in both quasi-static and dynamic loading regimes. Therefore caution is advised when interpreting the mechanism responsible for near-fault damage in sedimentary rock near the surface. Moreover, the results suggest that different responses of lithology to transient loading are responsible for sub-surface damage zone asymmetry.

Keywords: Fault zone damage, Coseismic damage, Rock pulverization, High strain rate experiments, Sandstone compaction bands, Earthquake rupture mechanisms

1. Introduction

Intensively fractured rock or pulverized rock observed in fault damage zones are thought to be the product of transient coseismic loading and therefore have the potential to reveal past earthquake rupture conditions [1, 2, 3, 4, 5, 6]. These rocks are pervasively fractured down to the micron scale, but lack any shear or rotation of fragments [1, 7, 8]. The vast majority of pulverized rocks are observed in crystalline lithologies, often igneous, along crustal scale faults [2, 8, 6] and show intense pervasive damage, both inter- and intra-granular.

In contrast, there are very few observations on pulverization in sedimentary rock. Dolomites and limestones, sedimentary rocks that are crystalline in nature, were labeled pulverized in some studies [9, 10, 11]. For more porous sedimentary rocks, only a few observations exist [12, 13], of which the latter is related to a meteorite impact structure. Moreover, the interpretation as product of coseismic damage of these pulverized sedimentary rocks is ambiguous [6].

On a larger scale, pulverized crystalline rocks are often associated with an asymmetric distribution of sub-surface damage across a fault, where most damage is observed on the stiffer side consisting of crystalline rock [2, 8]. The juxtaposed more compliant side of the fault often consists of a less damaged sedimentary rock [2, 14]. Such asymmetric damage zone geometries have been

20 linked to a bimaterial contrast at depth [2, 14, 8] and subsequently to a preferred
21 rupture direction related to so-called Weertman pulses [15, 16, 17]. It is argued
22 that such ruptures systematically produce more damage on the stiffer side of
23 the fault, thereby explaining the lack of damage in the compliant lithology.

24 In this study, we propose another explanation of the discrepancy in damage
25 between porous sedimentary rocks and crystalline rocks: the former respond
26 differently to similar transient coseismic loadings than the latter. This response
27 involves the deformation mechanisms that accommodate strain during transient
28 loading conditions and the resulting microstructures. In the case of crystalline
29 rocks, several laboratory studies have described the response to high strain rate
30 loadings [18, 3, 19, 4, 20, 21]. However, to our knowledge, the high strain rate
31 response of porous sedimentary rocks has not been studied yet in the Earth's
32 science community, especially in terms of post-loading microstructures and de-
33 formation mechanisms.

34 Here, the mechanical and microstructural results are presented of uniaxial
35 high strain rate loadings (strain rates between 60 and 150 s⁻¹) performed on
36 Rothbach sandstone samples with a Split Hopkinson Pressure Bar (SHPB) ap-
37 paratus. The sedimentary bedding anisotropy in the rock allows for a study
38 on the effect of pore and grain geometry on the mechanical behavior and mi-
39 crostructures, by loading bedding-parallel and bedding-perpendicular samples.
40 Also, water saturated samples in these two orientations have been tested to
41 identify the influence of fluids during coseismic loading in porous rocks.

42 Next, the observed differences between the four series of samples are dis-
43 cussed by using the geometric parameters of the rock in combination with clas-
44 sic micromechanical models for deformation in porous rocks. The most note-
45 worthy microstructures induced by transient dynamic loading are compaction
46 bands, which usually form at higher confining pressures and at low strain rate
47 [22, 23]. The presented results suggest that the same microstructures form in
48 both quasi-static and dynamic loading regimes. Therefore caution is advised
49 when interpreting the mechanism responsible for near-fault damage in porous
50 sedimentary rocks near the surface. Moreover, the results suggest that different

51 responses of lithology to transient loading can explain sub-surface damage zone
52 asymmetry.

53 **2. Material and methods**

54 *2.1. Sandstone samples*

55 Rothbach sandstone has a Lower Triassic age and is found in the Vosges
56 region, France. The rock has been formed in a fluvial environment and con-
57 tains cross-bedded laminations, causing a scatter in bedding orientations of
58 $\sim 30^\circ$ (Figure 1a). X-ray diffraction analyses from the sandstone block that
59 was used for coring shows a mineralogical composition of 76.5% quartz, 13.4%
60 feldspar (microcline), 3.4% mica, 5.9% clay minerals, of which 4.9% smectite,
61 and less than 1% of various oxides. There is slightly less feldspar (-3%) and
62 more quartz (8%) in the block compared to the mineral content of Rothbach
63 sandstone in literature [24]. The porosity, obtained on five samples by water
64 imbibition, is 20.3 ± 2.8 %, similar to the value of 19.9 % porosity reported by
65 [24].

66 Some alternation between coarser and finer grained beds is observed in thin
67 sections of undeformed Rothbach sandstone, similar to the structure reported
68 by [25]. There is a higher abundance of feldspar in the finer grained beds. Image
69 analysis reveals that the pores and grains are elongated, with the longest axis
70 aligned along the bedding (Figure 1a, b). Pore diameters range between 25-270
71 μm with a mean of 90 μm for the short axis and between 30-500 μm with a
72 mean of 173 μm for the long axis (Figure 1c). The grain diameters are between
73 50 to 600 μm with a mean of 240 μm for the short axis and between 70 and
74 800 μm with a mean of 330 μm for the long axis (Figure 1c). These grain size
75 values are similar to reported mean grain radii that vary between 110 μm [26]
76 to 140 μm [27] and 152 μm [24]. P-wave measurements by [28] reveal a bedding-
77 related anisotropy as well, with the faster velocities in the bedding-perpendicular
78 direction. This effect increased when the pores were fluid-saturated [28]. The

79 sedimentary anisotropy causes mechanical anisotropy as well, as shown by quasi-
80 static deformation experiments by [29, 25].

81 All samples were cored from the same block and are 1.5 cm in diameter
82 and length. The length/diameter ratio is close to 1 to reduce inertia effects
83 during SHPB-loading tests [30]. Due to the laminations in the rock, the samples
84 are technically oriented sub-perpendicular or sub-parallel to the bedding. In
85 total, 16 perpendicular and 13 parallel samples were produced, labeled VS# and
86 VSX#, respectively. Rectification ensured that the samples' top- and bottom
87 surfaces were parallel within 80 μm or less. Next, they were dried in an oven
88 at 60°C for at least 48 hours. Two sets of samples (9 perpendicular, 7 parallel)
89 were placed in a vacuum after which distilled water was added. They were kept
90 submerged for more than 24 hours. The initial porosity reported earlier was
91 calculated from the dry mass before imbibition, the mass after imbibition and
92 the Archimedes mass (mass of the sample while suspended in water).

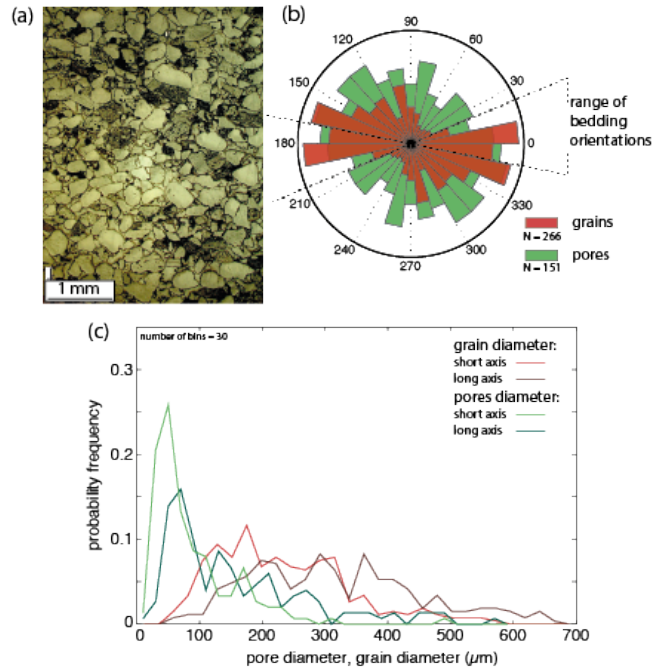


Figure 1: (a): Optical microscope image (incident light) of undeformed Rothbach sandstone. (b): The approximate range of orientations of the bedding planes in Rothbach sandstone. The orientation of the longest axis of the grains (red) show a clear anisotropic fabric related to the sedimentary bedding. The orientation of the longest axis of the pores (green) indicates heterogeneity in pore orientation related to the sedimentary bedding. Grain and pore orientations have been obtained by semi-automatic image analysis on thin sections. (c): The grain size (red curves) and pore size (green curves) distribution for the shortest and longest axes. For (b) and (c), $N = 266$ and $N = 151$ for grains and pores respectively.

93 *2.2. Experimental setup*

94 Uniaxial high strain rate loading experiments were performed on a custom
95 built "mini-Split Hopkinson Pressure Bar" (mini-SHPB) apparatus at the IS-
96 Terre laboratory in Grenoble described in [21]. It consists of steel in- and
97 output- bars between which the sample is located and a steel striker (all are
98 cylinders 2 cm in diameter). The striker is launched by a spring gun or com-
99 pressed air gun towards the input bar end. The resulting planar stress wave
100 subsequently travels the length of the input bar and loads the sample. During
101 loading, part of the wave is reflected back and part is transmitted through the
102 sample into the output bar. The stress waves are monitored by four strain gages,
103 two on each bar. A 1D-wave analysis applied on the strain data results in the
104 full stress-strain history of the sample. Further details on the SHPB apparatus
105 and the data analysis are found in [31, 32, 21].

106 The samples were mounted between the in- and output bars and attached to
107 the bars by high vacuum grease. A minimum time interval between mounting
108 and loading (< 60 s) was pursued to prevent drainage of the water-saturated
109 samples. Qualitatively, the amount of fluid that escaped from the samples was
110 negligible.

111 It is essential to ensure a smooth rise of the stress wave in the earlier stages of
112 loading to prevent stress disequilibrium issues. To do so, several types of pulse
113 shapers have been used, depending on the strength of the sample. Nonetheless,
114 it remained challenging to overcome stress disequilibrium problems at the onset
115 of loading and a final conservative selection resulted in 13 loadings that were
116 labeled reliable. The details on these loadings (3 dry and perpendicular, 3
117 saturated and perpendicular, 2 dry and parallel, 5 saturated and parallel) are
118 listed in Table 1.

119 After loading, some of the (partly) intact samples were impregnated with
120 epoxy resin and processed for thin sections, either parallel or perpendicular to
121 the loading direction. Microstructural observations were performed with an
122 optical microscope equipped with a digital camera and by SEM. Several digital
123 images were processed using the MatLab Image Analysis Toolbox.

Sample nr		Diameter (mm)	Length (mm)	Striker length (cm)	Pulse shaper	Striker velocity (m/s)	Peak strength (MPa)	Max. strain rate (s^{-1})
VS10	saturated	14.72	14.86	15	lead	4.57	33.0	119.5
VS11	saturated	14.70	14.92	15	lead	3.47	32.7	65.5
VS14	saturated	14.71	14.92	15	composite lead, tin foil, cardboard	6.70	38.2	95.6
VS15	dry	14.70	14.86	15	lead	4.62	45.9	110.2
VS16	dry	14.71	15.00	15	lead	3.27	39.3	71.9
VS6	dry	14.82	14.44	20	cardboard	2.87	49.6	152.1
VSX4	dry	14.45	15.06	10	composite lead, tin foil, cardboard	6.52	27.4	77.8
VSX7	dry	14.40	15.07	10	composite lead, tin foil, cardboard	6.75	28.7	78.5
VSX9	saturated	14.08	15.13	10	composite lead, tin foil, cardboard	4.96	23.9	59.1
VSX10	saturated	14.30	15.01	10	composite lead, tin foil, cardboard	4.07	26.2	75.2
VSX11	saturated	14.40	14.97	10	composite lead, tin foil, cardboard	6.55	22.8	80.9
VSX12	saturated	14.45	15.30	10	composite lead, tin foil, cardboard	8.55	30.4	131.6
VSX13	saturated	14.40	14.73	10	composite lead, tin foil, cardboard	5.99	26.5	76.8

Table 1: Sample characteristics, experimental settings, peak strength, and maximum strain rate.

124 **3. Results**

125 *3.1. Macroscopic damage*

126 The end-state of the samples is different for each set of loading conditions.
127 For bedding-perpendicular loaded samples in dry conditions, the damage varied
128 from intact to spalling-like surfaces parallel to the bedding (Figure 2a, orange
129 frame). Also, there are suggestions of finer grained bands perpendicular to the
130 loading direction. Their saturated counterparts remained intact or were reduced
131 to a large amount of fragments (Figure 2a, pale blue frame). On those samples
132 that remained intact, the fine-grained bands were not as pronounced as in the
133 dry case.

134 The fine-grained bands and the spalling surfaces are not observed in the
135 end-states of the bedding parallel loaded samples. Instead, both dry and satu-
136 rated samples broke into several fragments with the fractures oriented parallel
137 or at a low angle ($\sim 30^\circ$) to the direction of loading (Figure 2a, red and dark
138 blue frames). The size of the fragments decreased towards the grain size with
139 increasing loading conditions, especially for saturated conditions (Figure 2a,
140 sample VSX12).

141 The spalling-like fracture surfaces mentioned above are mostly observed near
142 the interface between the output bar and sample, and are typically created by
143 tensile loading. The microstructures indicate that compression occurred before
144 the formation of these tensile spalling surfaces; therefore it is suspected that
145 this damage type is a post-loading artifact. They were formed as follows: when
146 the transmitted stress pulse has completed one cycle along the length of the
147 output bar, the bar moves away from the sample. Often, this movement results
148 in direct detachment of the sample from the in- and output bars when loading
149 crystalline rocks. However, in the case of sandstone it seems that penetration of
150 the high vacuum grease into the abundantly available pores results in sufficient
151 capillary force to prevent immediate detachment. This results in the buildup of
152 tensile stress and failure along a weaker pre-damaged plane close to the output
153 bar side. This is supported by the fact that only the most damaged samples

154 show such features. As a consequence, the spalling microstructure is considered
155 to be an artefact of the experimental procedure and will not be discussed later.

156 *3.2. Mechanical data*

157 The stress-strain curves of bedding-perpendicular loadings show a different
158 mechanical behavior between dry and saturated loading conditions (Figure 2b).
159 The dry samples achieve higher peak strength than saturated samples (Table
160 1). Also, these peak stresses are higher than the uniaxial compressive strength
161 obtained at low strain rate, which is around 35 MPa [33]. Saturated samples
162 accumulate more axial strain than dry samples and reach their peak strength
163 at a higher axial strain. The elastic deformation interval yields a slightly lower
164 slope for the saturated samples. For the dry samples, the deviation from the
165 linear elastic part of the stress-strain curve is between 35 and 38 MPa. The
166 saturated samples yield at lower stress, between 23 and 25 MPa.

167 All samples that are loaded bedding-parallel reach lower peak strengths re-
168 lative to the bedding-perpendicular ones. The effect of fluid saturation is much
169 less apparent on the bedding-parallel loaded samples. The peak strength of the
170 dry samples is slightly higher than the saturated ones (Figure 2b and Table 1),
171 and the saturated samples reach their peak strength at lower strain (Figure 2b).
172 The linear elastic interval is similar for dry and saturated samples and indicates
173 a more compliant rock compared to dry bedding-perpendicular loaded samples.
174 Yielding commences at low stresses (15-20 MPa).

175 The loading histories are slightly different from the classical loading curves
176 obtained at high strain rate loading on crystalline rocks [18, 21]. High strain rate
177 loadings on crystalline rock typically yield a strain rate history with one or two
178 distinct peaks and a distinct ‘hinge’-point after the first peak, where the first
179 peak is related to elastic deformation, the ‘hinge’-point to the start of dynamic
180 fracturing and the second peak to catastrophic collapse of the sample [18, 21].
181 Here, such a clear distinction is harder to make since the elastic and inelastic
182 contributions to the strain rate overlap. Using the approximate yielding point
183 from the stress-strain curves (Figure 2b), the first strain rate peak related to

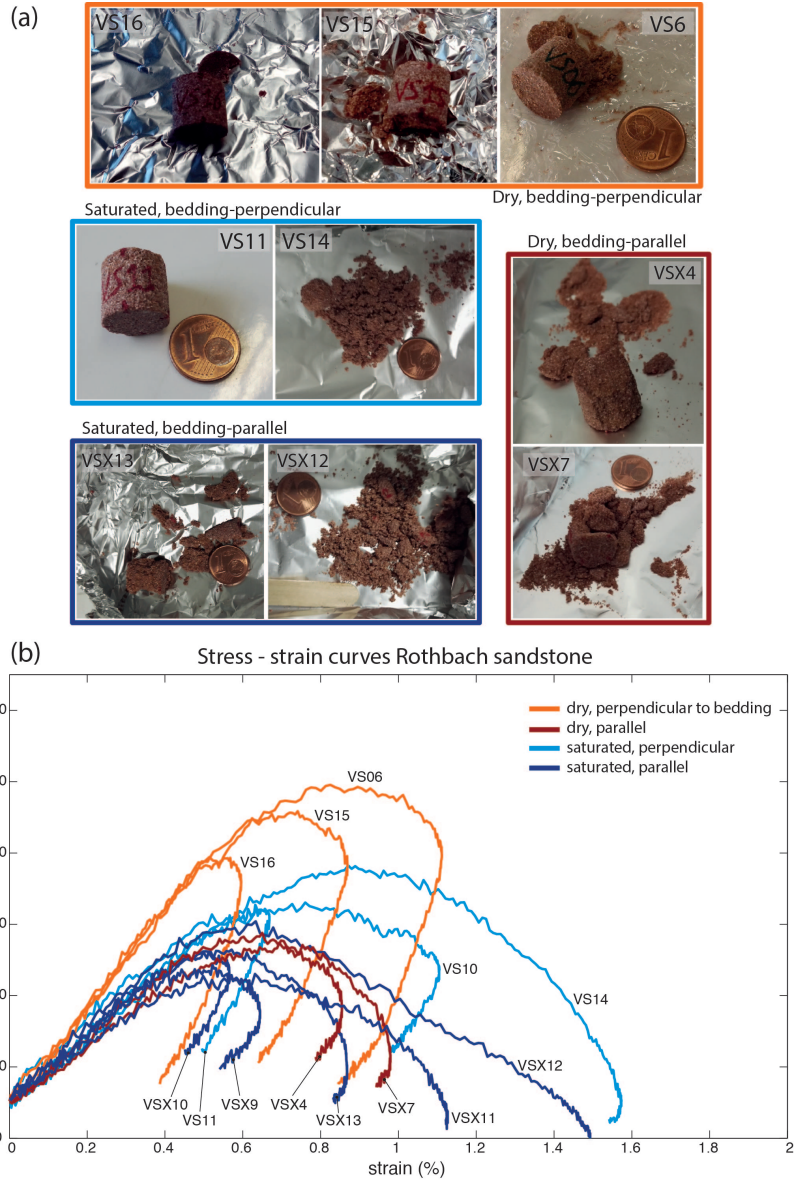


Figure 2: (a) Photographs of the post-mortem samples showing end-states that vary from intact to individual grains. The colored frames correspond to the colors used in (b) for the sample conditions. (b) The stress-strain curves for dynamic loading and damage of the 13 reliable samples. Color-coding indicates the sample conditions. The stress-strain curves have been cut off at 5 MPa to facilitate direct comparison.

184 elastic deformation can be recognized (Figure 3a). After, rather than a yield-
185 point there is a yield-interval up to a poorly defined second strain rate peak
186 (Figure 3a). This second strain rate peak is only recognized in the heavily
187 fragmented samples (indicated in Figure 3b).

188 A weak dependence of the peak strength as function of peak strain rate
189 is only recognized for the bedding-perpendicular dry loadings (Figure 3b). A
190 clear strain rate threshold that separates the loadings with a second strain rate
191 peak that result in heavily fragmented samples from the other samples in their
192 respective series is not visible.

193 The full deformation history of the samples can be represented as the dissi-
194 pated energy (the area bound by the stress-strain curve). The dissipated energy
195 density is revealed by plotting the dissipated energy versus the residual axial
196 strain (Figure 3c). The residual axial strain is the permanent strain after load-
197 ing. Three linear trends become apparent: one for the dry and one for the
198 saturated bedding-perpendicular series (57 and 30 MJ/m³, respectively), and
199 one for the saturated bedding-perpendicular series (19 MJ/m³). The bedding-
200 parallel series contain too few samples over a too small range of dissipated energy
201 and strain to define a clear trend. A lower slope means that less energy is dis-
202 sipated for a similar amount of axial deformation. Thus, the presence of water
203 makes the rock easier to deform and the rock is weaker when loaded parallel to
204 the bedding instead of perpendicular to the bedding.

205 The approximate contours of the maximum strain rate are horizontal, ex-
206 cept for sample VS14 that falls out of the general trend (Figure 3c). This,
207 in combination with the dissipated energy density, indicates that deformation
208 in dry conditions depends more on strain rate than deformation in saturated
209 conditions.

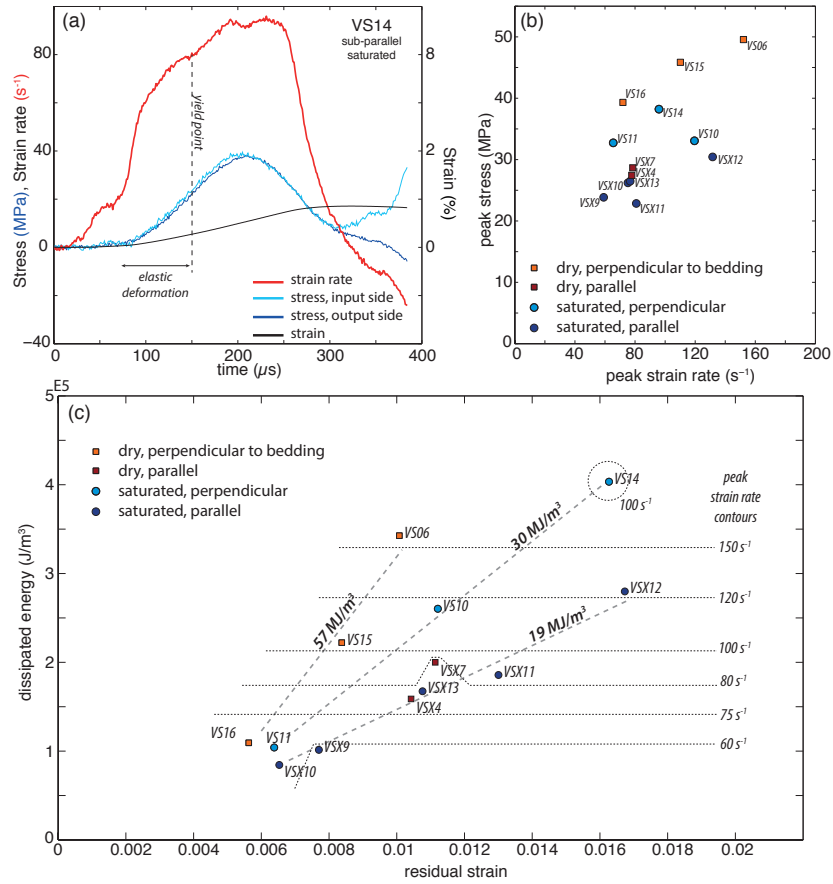


Figure 3: (a) Strain rate (red), stress (pale blue and dark blue) and strain (black) versus time for sample VS14. The pale blue stress curve is the stress at the input bar - sample interface, the blue for the output bar - sample interface. Both curves overlap, indicating good stress equilibrium. After 300 μs , incident and reflected waves overlap, resulting in artificial data. (b) The peak stress plotted versus the peak strain rate for all loadings. (c) Dissipated energy plotted versus the residual strain for all loadings. Strain rate contours indicate the approximate peak strain rates that were experienced by the sample. Slopes have been determined for three of the sample sets.

210 *3.3. Microstructures*

211 For bedding-perpendicular loadings, darker colored bands are visible at the
212 scale of the sample, which are not as evident in undamaged rock (Figure 4a-
213 c). These bands are more pronounced in samples loaded under dry conditions
214 than samples loaded under saturated conditions (Figure 4b). The bands consist
215 of finer grained material and are oriented sub-parallel with the sedimentary
216 bedding. They do not always run over the whole diameter of the sample and
217 their boundaries are somewhat diffuse (Figure 4b). A measure of the porosity
218 for these bands is obtained by mapping the pore space on optical microscopy
219 images taken parallel to the loading direction (for details, see Supplementary
220 Material A). The surface occupied by pores in undeformed sandstone is 17.1%.
221 For the bedding-perpendicular dry loadings, the mean surface is reduced to
222 11.8%, for the saturated samples it is slightly higher at 13.8%. The pore space
223 in these same samples determined from individual images outside the dark bands
224 is higher, around 15-16%. This reveals a strong reduction in pore space within
225 these bands relative to the regions outside these bands and therefore they can
226 be interpreted as compaction bands.

227 At higher magnification, the compaction bands contain a larger amount of
228 feldspars, micas, and clays relative to the non-compacted regions (Figure 4c).
229 Grain crushing, especially of feldspar grains, has caused local pore collapse (Fig-
230 ure 4c and 5a). Also, grains of micas show sign of intense deformation by folding
231 (Figure 5a). This crushing of the grains and subsequent collapse of the pores
232 and rearrangement of the grains causes the local compaction. The localization
233 of compaction into bands is caused by the higher amounts of feldspar and micas
234 in some sedimentary layers.

235 The grain size outside the compaction bands is generally larger and the
236 matrix contains a higher amount of quartz grains. Some of these larger grains
237 contain plenty of intra-granular fractures, while a large amount of the grains
238 remain undamaged (Figure 5a). For dry loading conditions, these fractures are
239 not often trans-granular, whereas for saturated conditions they are more often
240 trans-granular and have been opened up.

241 Inter-granular fracturing is difficult to distinguish in the thin sections, since
242 rupturing in the cement is obscured by the chaotic structure of the cement itself.
243 However, at some locations in bedding-parallel loaded samples, clear evidence of
244 intergranular fracturing can be observed (Figure 5b). Also, image analysis shows
245 that the total pore space is 20.9%, which is higher than the undamaged rock
246 (see Supplementary Material A). Interestingly, some intragranular deformation
247 is observed close to dilated pores (Figure 5b). These structures suggest that
248 incidental intragranular fracturing has occurred before intergranular fracturing.

249 Dilatational intergranular deformation has not been observed in the bedding-
250 perpendicular loaded samples under dry conditions. Here, intergranular defor-
251 mation caused by pore collapse can be inferred, but this is different in nature
252 compared to the dilatational fracturing in the bedding-parallel loaded samples.
253 For the saturated bedding-perpendicular loadings, intergranular dilatational de-
254 formation outside the compaction bands, in zones with larger grain sizes, did
255 seem to have occurred, although in lesser degree compared to the bedding-
256 parallel loaded samples.

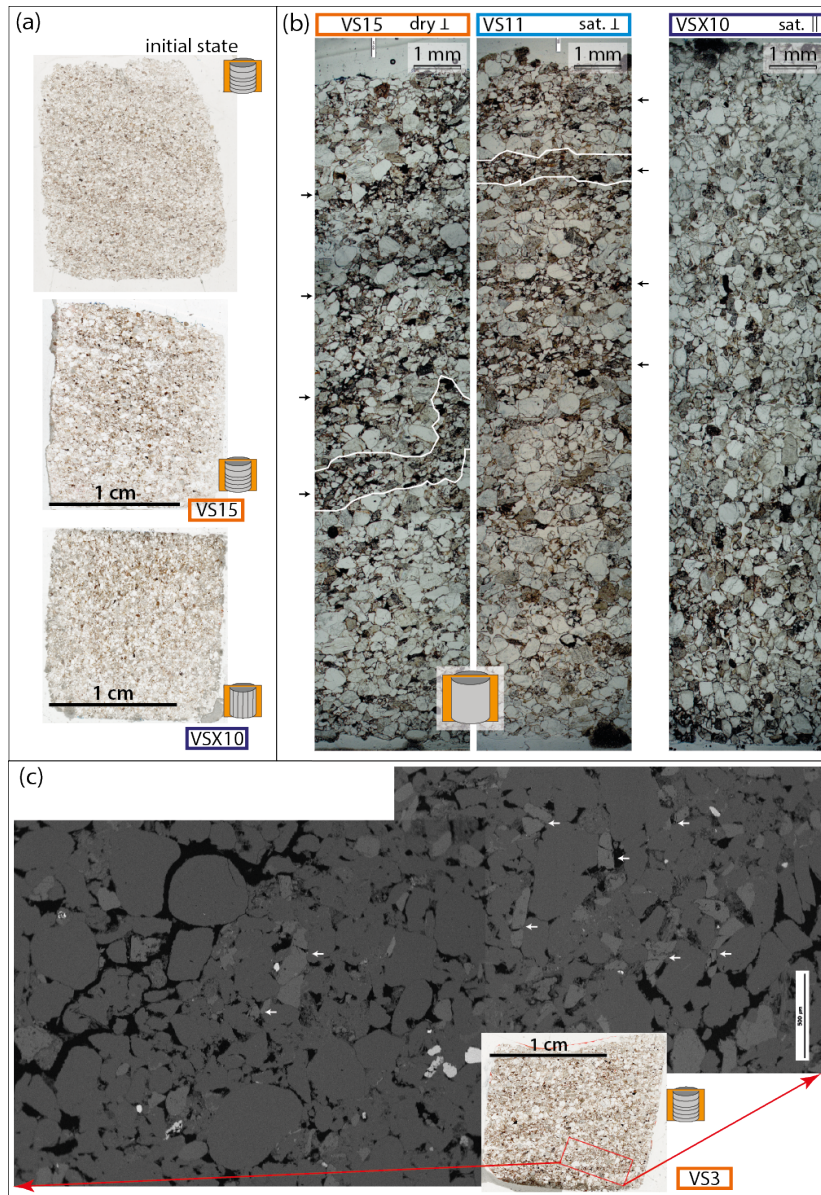


Figure 4: (a) Whole thin sections photographs of an undeformed Rothback sandstone sample (top), a dry bedding perpendicular loaded sample (bottom left) and a saturated bedding parallel loaded sample (bottom right). The bedding parallel loading sample shows dark compaction bands aligned with the sedimentary bedding. Note that there are no compaction bands (darker horizontal bands) visible in the initial sample and in the bedding-parallel loaded sample, only some alternation between finer and coarser grained material. (b) Photomicrograph compositions along the length of three samples that were loaded bedding perpendicular (left: dry conditions, middle: saturated conditions) and bedding parallel (right, saturated conditions). The bedding-perpendicular loaded samples show compaction bands (indicated by the black arrows. In each sample, one has been highlighted by a white rim). In contrast, the bedding-parallel loaded sample does not show compaction bands and contains more pores. (c) Overview image of compaction bands in sample VS3, where feldspar grains are often fractured (light gray grains, indicated by white arrows). Dark gray grains are mainly quartz grains, black areas is pore space. (Mechanical data of sample VS3 not included because of stress equilibrium issues).

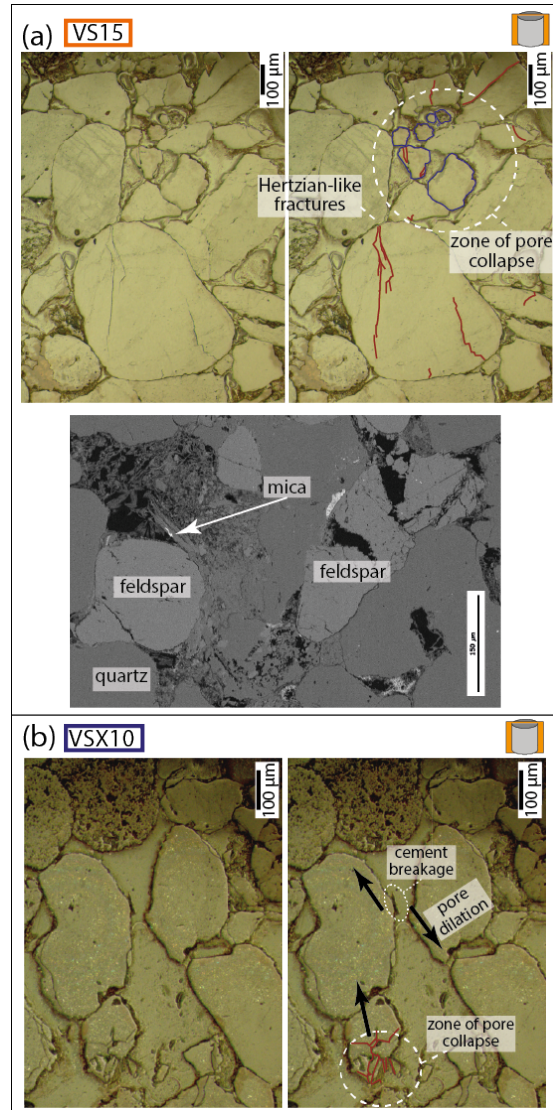


Figure 5: (a) Examples of intergranular deformation. Top: pristine and interpreted microphotograph of Hertzian-like fractures in large quartz grains and pore collapse by grain shattering. Bottom: SEM image taken in a compaction band showing a collapsed feldspar grain and deformed mica grain. (b) Intragranular deformation evidenced by fragments of cement on one of the grains and Hertzian fracturing of a grain. The bottom picture shows the interpretation, including the direction of movement of the grains. Note that Hertzian fracturing occurred before intragranular fracturing.

257 4. Discussion

258 4.1. Micromechanical damage models

259 Three damage types are observed: 1) compaction bands that are formed due
260 to grain breakage and pore collapse, mostly in feldspar and mica grains when the
261 samples are loaded bedding perpendicular; 2) intra- or trans-granular fracturing
262 in clusters with the largest quartz grains; 3) intergranular dilatational fractur-
263 ing, that is hardly seen in dry deformed samples but that is more abundant
264 in water-saturated samples. The presence of compaction bands is remarkable
265 since these typically do not form in quasi-static experiments at low confining
266 stress in sandstone [23]. Quasi-static loading data on Rothbach sandstone in
267 P-Q space (mean stress-differential stress) defines a 'cap'-region where yielding
268 results in the formation of shear- and compaction bands (Figure 6) [25]. It be-
269 comes apparent that the dynamic loading conditions do not reach the yield cap
270 for compaction (Figure 6, red loading path).

271 Two micromechanical models are considered to explain the observed mi-
272 crostructures, especially the compaction bands. The Hertzian contact model
273 [34] describes intra- and trans-granular fracturing in quartz and feldspar. Intra-
274 granular fracturing is assumed to be the cause for pore collapse and subsequently
275 the formation of compaction bands. The pore-emanated crack model [35] has
276 been developed for intergranular fracturing. First, both models are introduced
277 and applied for quasi-static loading conditions. After, the models are adapted
278 for dynamic loading conditions. We argue that competition between the mi-
279 cromechanal models is influenced by loading rate, by the presence of pore fluids,
280 and by microstructural properties of the rock.

The pore-emanated crack model considers circular pores in a 2D elastic medium. Under axial stress, mode I wing cracks emanate from the pores parallel with the loading direction. The mode I stress intensity factor (K_I) for a single fracture evolves as follows with increasing stress [35]:

$$K_I = \sqrt{L}\sigma_1\sqrt{\pi a} \left[\frac{1.1(1 - 2.1\lambda)}{(1 + L)^{3.3}} - \lambda \right] \quad (1)$$

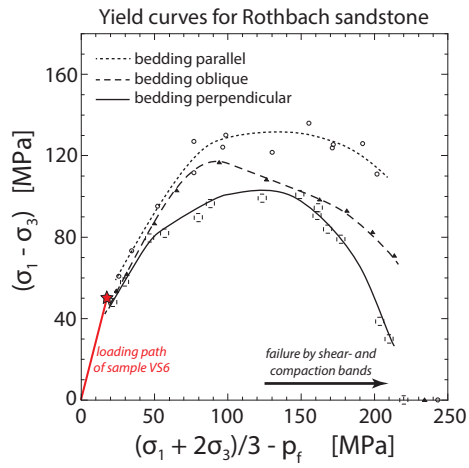


Figure 6: Mean stress-differential stress diagram for Rothbach sandstone, tested at quasi-static loading conditions for different bedding orientations by [25]. At higher mean stress the samples yield by shear bands (higher differential stress) and compaction bands (lower differential stress). At lower mean stress samples fail by classic brittle failure. The loading path for sample VS6 (red line, red star) illustrates that the dynamically loaded samples in this study do not reach conditions for shear- and compaction band formation. Nonetheless, compaction bands did form. Figure adapted from [25].

where L is the normalized crack length (l/a , l is the crack length, a is the pore radius), σ_1 is the largest principal stress, and $\lambda = \sigma_3/\sigma_1$ (σ_3 is the smallest principal stress). Compression is taken as positive. The elastic interactions between pores adds another component to the stress intensity factor [35]. Assuming $\lambda \ll 1$ for the experiments (no confining pressure), equation 1 then becomes:

$$K_I = \sigma_1 \sqrt{\pi a} \left[\frac{1.1\sqrt{L}}{(1+L)^{3.3}} + \frac{\sqrt{2}}{\pi} \sqrt{\Phi(L+1)} \right] \quad (2)$$

where Φ is the porosity. When K_I is equal to the fracture toughness (K_{IC}), stable crack growth commences. Instable crack growth occurs when the cracks have reached a critical length L_{cr} , which normally results in macroscopic failure of the sample by the coalescence of cracks. The peak stress at this critical crack length is the macroscopic uniaxial compressive strength ($\sigma_1 = \sigma_{UCS}$). Here, the analytical estimate of the formula above is adopted from [36], which involves known bulk quantities (pore size and porosity):

$$\sigma_{UCS} = \frac{1.325}{\Phi^{0.414}} \frac{K_{IC}}{\sqrt{\pi a}} \quad (3)$$

The Hertzian fracture model involves the stress concentration at a Hertzian contact point between two spherical grains. This produces tensile stress on micro flaws within the grains, leading to intragranular fracturing. The far field stress (σ_1) necessary for the failure of a grain can be predicted by [34]:

$$\sigma_1 = \frac{2.2 (1 - \mu^2)^2}{E^2 (1 - 2\mu)^3} \frac{(K_{IC}^g)^3}{(\alpha\Phi d)^{3/2}} \quad (4)$$

281 where E is Young's modulus, μ is the Poisson's ratio, K_{IC}^g is the fracture tough-
 282 ness of the grain material, d is the grain radius, α is the ratio between the initial
 283 micro flaw size, and d is the grain radius. Note that the stress needed to break
 284 a single grain is not necessarily similar to the peak strength of the rock, but
 285 indicates the onset of pore collapse.

286 To obtain approximations for the uniaxial peak strength of Rothbach sand-
 287 stone at quasi-static loading conditions, the following values were adopted:
 288 $\Phi = 0.20$ and intergranular $K_{IC} = 27 \text{ MPa m}^{1/2}$. Since feldspar is elastically
 289 highly anisotropic [37] and much harder to describe by a single parameter, only

290 quartz is considered here. For quartz, $E = 95.4$ GPa [38], $\mu = 0.077$ [34], and
 291 $K_{IC}^g = 31$ MPa m^{1/2}. The ratio α varies between 1.7×10^{-4} and 3.6×10^{-5} , where
 292 the largest value indicates larger flaws (e.g. to represent cleavage of feldspars)
 293 and the smallest one indicates very small flaws (e.g. pristine quartz) [34]. This
 294 leaves the pore and grain radii (a and d , respectively) as variables.

295 The uniaxial compressive strength (UCS) of bedding-parallel loaded sand-
 296 stone predicted by the pore-emanated crack model is in good agreement with
 297 reported UCS-values [33](Figure 7a): the rock is expected to develop inter-
 298 granular fractures from the largest pore sizes that were constrained from the
 299 undamaged rock (Figure 1b, c). Also, intra-granular fracturing is only expected
 300 to occur in the largest grains, assuming a value of α of 1×10^{-4} or higher (Figure
 301 7a). The value of α of 1.7×10^{-4} , representative for weaker feldspar grains, is not
 302 considered for larger grain sizes because feldspar grains in the rock are generally
 303 small.

304 However, pore collapse and intra-granular fracturing during dynamic load-
 305 ing has been observed but not explained by the quasi-static micromechanical
 306 models. Also, higher peak stresses than expected for quasi-static loading were
 307 reached during dynamic loading on dry samples. Therefore, it is necessary to
 308 incorporate the dynamic effect into the micro mechanical models, originally de-
 309 veloped for quasi-static loading conditions.

For high strain rates it has been shown that the fracture initiation tough-
 ness exceeds the quasi-static initiation toughness of the material due to time-
 dependent processes near the fracture tips [39, 40]. Thus, a material can experi-
 ence higher stresses before failure. To express this dynamic effect, an empirical
 function of the dynamic fracture initiation toughness (K_{IC}^D) has been obtained
 from experimental high strain rate loading data of several materials that col-
 lapsed onto a single curve by the following normalization [41]:

$$K_{IC}^D = K_{IC} + \dot{K} \times 2 \times 10^{-5} \quad (5)$$

310 where \dot{K} is the stress intensity factor rate and the constant with which it is
 311 multiplied is in seconds and qualitatively represent the competition between

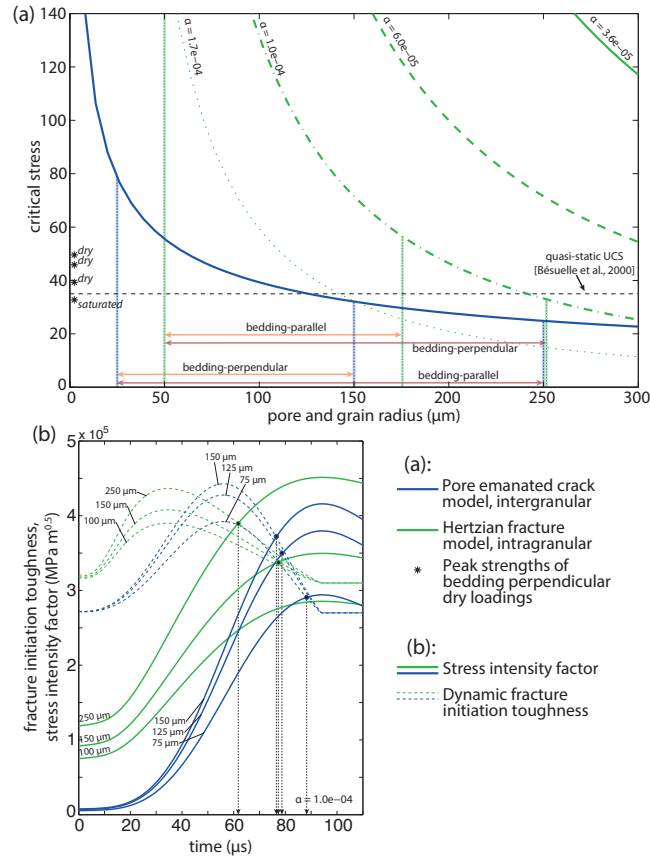


Figure 7: (a) Critical stress or peak stress for quasi-static loading of Rothbach sandstone predicted by the pore emanated crack model (blue curve) and the Hertzian fracture model (green curves) as function of pore size and grain size. For the Hertzian fracture model, four different values of α (ratio between the initial micro flaw size and grain radius d) are shown. The quasi-static UCS (uniaxial compressive strength) is indicated by the dashed line [33]. The bedding-parallel (orange) and bedding-perpendicular (red) pore and grain size ranges for Rothbach sandstones (Figure 1) are given as well. This shows that the pore-emanated crack model determines the quasi-static strength. The asterisks indicate the dynamic peak strengths achieved during loading of samples VS6, VS15 and VS16 (dry) and VS11 (saturated). (b) The evolution of the dynamic stress intensity factor (solid curves) and the dynamic fracture initiation toughness (dashed curves) with time for the pore emanated crack model (blue) and Hertzian fracture model (green). The stress history of loading VS6 has been used as input. Curves for pores radii of 75, 125 and 150 μm are shown for the pore emanated crack model. Grain radii for the Hertzian fracture model are 100, 150 and 250 μm . Black circles indicate the moment at which the fracture criteria are satisfied.

312 fracture activation time and loading rate. For low loading rates, K_{IC}^D is equal
313 to K_{IC} . Note that equation 5 does not describe the intrinsic micro strength,
314 but does predict the increase of strength well enough so that it matches with
315 experimental data.

316 For the pore-emanated crack model, the stress intensity factor is linearly
317 proportional to the applied axial stress (equations 1 - 3). Thus, the dynamic
318 fracture initiation toughness is proportional to the loading rate. In case of the
319 Hertzian fracture model, the stress intensity factor is proportional to the cubed
320 root of the applied stress (equation 4). Consequently, the dynamic fracture
321 initiation toughness is proportional to the cubed root of the loading rate.

322 This different dependence on the loading rate explains why intragranular
323 fracturing, and subsequently the formation of compaction bands, is observed in
324 dynamic bedding-perpendicular loading. When equations 3, 4 and 5 are applied
325 to the loading history of one of the experiments, it reveals the moment at which
326 the fracture criterion of each micromechanical model is satisfied (Figure 7b).
327 This shows that during dynamic loading, the fracture criterion for Hertzian
328 fracturing in larger grains is satisfied early on in the loading history. When
329 considering that feldspar grains, when oriented well, are even weaker than the
330 parameters used here, so they break early as well (despite the smaller grain
331 size of feldspar). This matches well with the observation of compaction band
332 formation in relatively feldspar rich layers. Pore emanated fracturing sets in at
333 a later moment than intragranular fracturing.

334 Thus, the presence of the two different deformation mechanisms reflected
335 in the microstructural observations is due to differences in response time of
336 these mechanisms, so that they compete at high strain rate while they would
337 not at quasi-static strain rates. Differences in response times for deformation
338 mechanisms in dynamically loaded rocks were suggested by [42], although mi-
339 cromechanical and microstructural arguments were not provided. It should also
340 be noted that other deformation mechanisms, such as grain sliding and rotation
341 and deformation of mica and other weak minerals, play a role as well but are
342 not considered in the mechanical models.

343 *4.2. Effect of pore fluids on dynamic damage*

344 The macroscopic failure mode is similar in both dry and wet cases of bedding-
345 perpendicular loading: from intact samples to fragmented samples containing
346 compaction bands. However, in the saturated samples the deformation is more
347 efficient (Figure 2c), compaction band formation is less pronounced and there
348 is some intergranular dilation.

349 Pore fluids in quasi-static rock deformation experiments have a weakening
350 effect that is attributed to a drop in the specific surface energy compared to
351 vacuum [43, 44]. Water lowers the energy needed to create new fracture surface.
352 This effect is only valid when the fracture tip speed, the speed at which fractures
353 grow, allows for fluids to flow into the newly created fracture volume. However,
354 much higher fracture tip speeds are expected during dynamic loading: a trans-
355 granular fracture in larger grain of 300 μm is formed during a loading lasting
356 $\sim 100 \mu\text{s}$, resulting in an average velocity of 3 m/s. This number is a lower
357 bound, but still too high to expect fluids to flow into the fracture tip zone.

Instead, we propose that the mechanical effect of the pore fluid is responsible
for weakening of the rock. At the onset of loading, the fluid pressure is equal to
the atmospheric pressure. It can be argued that the sample is in an undrained
state during loading, meaning that the fluid pressure (P_f) cannot be dissipated
fast enough and rises with increasing stress [45]:

$$\Delta P_f = B \Delta \sigma \left(\frac{1}{3} \right) \quad (6)$$

358 where B is Skempton's coefficient, which varies between 0 and 1. This poroelas-
359 tic constant can be determined for quasi-static loading by measuring axial and
360 radial strain of the rock in both dry and saturated conditions. With our current
361 experimental setup these were not measured. Besides, such measurement could
362 be complicated for high strain rate loading due to radial inertia effects. An
363 alternative could be to develop a system to measure pore fluid pressure directly
364 during loading.

365 Nonetheless, this additional fluid pressure contributes to the stress intensity
366 factor of pore-emanated cracks (equation 3), increasing the effective stress and

367 lowering the critical applied stress needed to commence intergranular fractur-
368 ing. This explains the suppression of compaction band formation in saturated
369 samples and larger intergranular dilation.

370 *4.3. Anisotropy effect on damage mechanism*

371 The role of bedding orientation on the strength of the rock is large (Figure 2),
372 as the effect of pore fluids is suppressed for bedding parallel loaded samples com-
373 pared to bedding perpendicular ones. This indicates that the anisotropy, mainly
374 caused by grain orientation, dictates the strength of the rock. This anisotropy
375 has an effect on both micromechanical models. In case of the Hertzian fracture
376 model, the range of grain size radii is smaller in this orientation. Hence the
377 critical stress for intragranular fractures increases (Figure 7a). The range of
378 pore sizes is larger and therefore the critical stress for pore emanated cracks is
379 reduced (Figure 7a). Therefore, in contrast to bedding perpendicular loading,
380 there is no competition between the two deformation mechanisms and inter-
381 granular fracturing is the dominant mechanism for bedding parallel loading.

382 *4.4. Dissipated energy and deformation mechanism*

383 The dissipated energy density shows that less energy is needed for the de-
384 formation of samples that are dominated by pore-emanated crack formation,
385 such as the bedding parallel loaded samples and the fluid saturated samples
386 (Figure 2c). This is because of two reasons: 1) it costs less energy to produce
387 new fractures between the grains than in the grains, since the grain boundaries
388 are already weak interfaces; 2) for the pore-emanated crack model, the small-
389 est length scale at which energy is dissipated is the grain scale. Intragranular
390 fracturing, however, dissipates energy at a length scale below the grain scale.
391 Therefore, energy can be dissipated at a much smaller scale, resulting in a higher
392 dissipated energy density. In order to produce the same amount of deformation
393 at the sample scale a much larger amount of energy is therefore needed relative
394 to intergranular fracturing.

395 *4.5. Dynamic damage across bimaterial fault zones*

396 The presented experiments are similar in terms of strain rate (up to 150 s^{-1})
397 and loading duration to experiments performed on crystalline rocks [3, 4, 20, 21].
398 Only peak strengths have been lower, which is a consequence of the difference
399 in strength between sandstones and granites. For crystalline rock, these loading
400 conditions are near or at the pulverization strain rate threshold for unconfined
401 single loading experiments. These pulverized crystalline rocks are pervasively
402 fragmented down to micron scale or even lower [46, 21].

403 By contrast, sandstone at the same loading conditions remains either intact
404 or is reduced to its individual grains. For the latter case, the smallest scale of
405 pervasive damage is the grain scale because of intergranular dominated deforma-
406 tion. Therefore, we conclude that these samples cannot be labeled as pulverized
407 since most of the smallest fragment sizes are equal to the grain size. To actually
408 pulverize sandstones below the scale of individual grains, intragranular fractur-
409 ing needs to dominate all over the sample. For the samples with compaction
410 band formation, intragranular fracturing was only restricted to local zones with
411 more weaker feldspar grains. We propose that higher strain rates are necessary
412 for pervasive intragranular fracturing, and sandstone pulverization, than the
413 ones applied in this study.

414 For crystalline rock, successive loadings below the pulverization strain rate
415 threshold result in eventual pulverization of the rock [19, 21]. To pulverize sand-
416 stone by successive loadings, such loadings need to be sufficiently strong to cause
417 energy dissipation below the grain scale. Therefore, it can be argued that suc-
418 cessive coseismic loadings must be higher than for crystalline rock. Besides, the
419 formation of compaction bands cause a stronger anisotropy in the rock so that
420 progressive damage is concentrated within these compaction bands, possibly
421 shearing these bands.

422 The dynamically induced microstructures that are observed in the Rothbach
423 sandstone are not exclusive for high strain rate. Compaction bands, inter- and
424 intra-granular breakage are commonly observed in natural sandstone that is not
425 exposed to large earthquakes. Also, several experimental studies have been able

426 to create such microstructures in Rothbach sandstone during quasi-static exper-
427 iments [25]. However, such quasi-statically formed compaction bands do neces-
428 sitate high confining pressure. Thus, dynamically formed compaction bands can
429 easily be mistaken for quasi-static ones and vice versa without proper geological
430 constraints on the depth of formation.

431 In the setting of a seismically active fault zone, transient coseismic loading
432 is the result of the stress waves radiating from a passing earthquake rupture
433 tip. The loading conditions, mainly strain rate, can be calculated for simple
434 rupture mechanisms (e.g. sub-Rayleigh wave speed rupture at constant velocity,
435 supershear rupture). Here, we adapt a simple conceptual model where there is
436 no preferred earthquake rupture directionality along a bimaterial fault. This
437 causes systematic compressional loading with similar transient strain rates on
438 both sides of the fault (Figure 8a).

439 For the crystalline lithology, the transient coseismic loading results in a wide
440 damage zone that is clearly defined by an increasing density of fractures towards
441 the fault core (Figure 8b, [21]). Even more, a band of pulverized rock forms close
442 to the fault core when loading conditions are sufficiently high (Figure 8b, [21]).
443 On the other side of the fault with the porous lithology, pervasive pulverization
444 below the grain scale occurs at higher strain rates, resulting in an asymmetric
445 distribution of pulverized rocks (Figure 8b). Moreover, damage in the porous
446 rock farther from the fault core (where strain rates are lower) is harder to recog-
447 nize because energy is dissipated mostly at the grain scale, causing intergranular
448 deformation, and because the microstructures such as compaction bands are not
449 unique for dynamic loading.

450 The different mechanical response of porous rock and crystalline rock to sim-
451 ilar transient loading conditions can explain the subsurface damage asymmetry
452 across fault zones as observed by [2, 14] and [8]. However, it should be noted that
453 the rupture mechanism and preferred rupture directivity is governed at seismo-
454 genic depth. Thus, the assumption of perfectly bidirectional ruptures causing
455 similar transient loading conditions can differ from reality. Our findings there-
456 fore do not exclude the possibility of a preferred rupture direction. Nonetheless,

457 one should use caution when assessing fault damage zone asymmetry across a
458 bimaterial fault by taking the different dynamic mechanical response into ac-
459 count. Also, sub-surface damage asymmetry as a stand-alone observation might
460 not be sufficient to constrain a preferential rupture mode or direction. In such a
461 case, the support from other geophysical observations, such as an elastic velocity
462 contrast at depth, is vital.

463 Besides, the hypothesis of a preferred rupture direction as the cause for the
464 asymmetric damage distribution across a fault zone assumes that the stiffer
465 crystalline side of the fault is subject to systematic tensile loading [16]. This
466 implies that tensile loading causes pulverization rather than compressive load-
467 ing. The current state of experimental research on pulverized rocks has not yet
468 considered dynamic tensile loading, and therefore this remains an open question
469 for now.

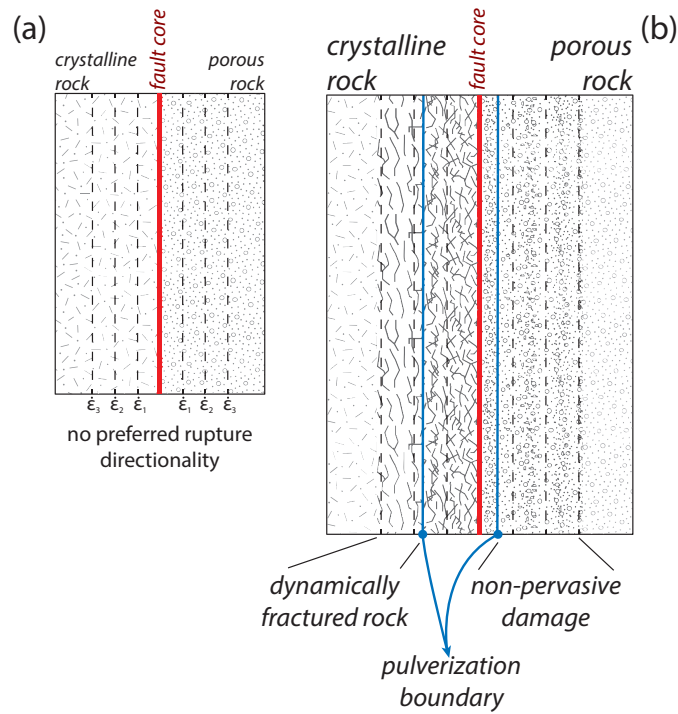


Figure 8: (a) Top view of a simple conceptual model of the fault damage zone geometry across a bimaterial fault consisting of crystalline rocks on the left side and porous rock on the right side. The strain rate contours are the representation of the same strain rate pattern on both sides of the fault caused by several identical earthquake ruptures travelling in opposite directions. The dimensions of the system depend on the size of the earthquakes and the rupture mode. (b) The different response of the lithologies causes an asymmetric distribution of pulverized rock across the fault. Also, damage in porous rock farther from the fault core is harder to recognize because it is intergranular and can be confused for quasi-statically induced damage.

470 **5. Conclusion**

471 High strain rate deformation experiments on Rothbach sandstone were per-
472 formed to study dynamic coseismic damage in porous rock. The results show
473 that for the investigated strain rate range (60-150 s⁻¹) the strength and the
474 damage microstructures depend on the loading direction with respect to the
475 sedimentary bedding and, in a lesser extent, on the presence of pore fluids.
476 Pervasive fracturing below the grain scale, what could be called pulverization,
477 has not been observed. Instead, energy was dissipated at the grain scale, i.e.
478 intergranular deformation, and locally below the grain scale in the form of in-
479 tragranular fractures and compaction bands. The competition between these
480 two mechanisms is explained by using the geometrical properties of the rock,
481 that differ with the orientation relative to the bedding, in combination with two
482 classic micromechanical models by [35, 34]. This analysis shows that the inertia-
483 related response time for intergranular breakage is shorter than for intragranular
484 breakage, suggesting that pervasive fracturing below the grain scale might occur
485 at even higher strain rates than the ones used here. Straightforward comparison
486 with crystalline rocks tested at similar high strain rate loading conditions reveals
487 that damage zone asymmetry across bimaterial fault zones can be caused by a
488 different response in terms of damage. Moreover, the microstructures observed
489 in the porous rock are not exclusive indicators for high strain rate loadings.
490 Therefore, caution should be used when assessing the origin of off-fault damage
491 in porous rock.

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